

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1057(24)	1979	68	146

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SYMBOLS

- SECTION LETTER OR DETAIL NUMBER
SHEET WHERE DETAIL OR SECTION IS SHOWN
16-1/2" OCTAGONAL PILES (TYPE I)

ABBREVIATIONS

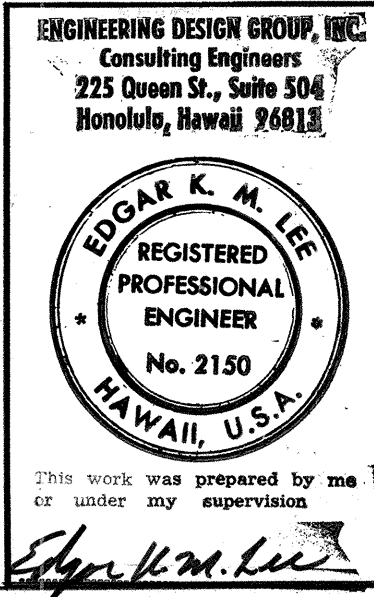
AASHTO - THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS	HECO - HAWAIIAN ELECTRIC COMPANY
ABUT. --- ABUTMENT	HWY --- HIGHWAY
ADD. --- ADDED	HORIZ. - HORIZONTAL
ADJ. --- ADJACENT	K ---- KIPS
ALT. --- ALTERNATE	LBS. - POUNDS
ANG. --- ANGLES	LT., L - LEFT
APPROX. - APPROXIMATE	L.F. - LINEAR FOOT
ARCH. --- ARCHITECTURAL	L6. -- LONG
ASTM --- THE AMERICAN SOCIETY FOR TESTING & MATERIALS	LONGIT. - LONGITUDINAL
AT ---- AT	L.S. - LUMP SUM
BAL. --- BALANCE	MIN. - MINIMUM
BASE LINE	MISC. - MISCELLANEOUS
BEAMS	MAX. - MAXIMUM
BEARING	NO. -- NUMBER
BETWEEN	N.T.S. - NOT TO SCALE
BLOCKS	O.C. - ON CENTER
BOTTOM	PL. -- PLATE
BOTH FACE	P.S.F. - POUND PER SQUARE FOOT
	P.S.I. - POUND PER SQUARE INCH
	P.T. - POST-TENSION
CENTER LINE	RAD., R. - RADIUS
CL, CLR - CLEAR	RAILG - RAILING
CONC. -- CONCRETE	REF. - REFERENCE
COL. --- COLUMN	REINF. - REINFORCEMENT
CONN. -- CONNECTION	REM. - REMAINDER
CONST. - CONSTRUCTION	RET. - RETAINING
C.J. --- CONSTRUCTION JOINT	R.W. - RETAINING WALL
CONT. -- CONTINUOUS	REQD. - REQUIRED
C.Y. --- CUBIC YARDS	RT., R. - RIGHT
	RD. -- ROAD
DEPTH	SCHED. - SCHEDULE
DETS. -- DETAILS	SECT. -- SECTION
DIA. --- DIAMETER	SHT. --- SHEET
DIAG. -- DIAGONAL	
DIM. --- DIMENSION	
DOUBLE	SIM. --- SIMILAR
DOWN	SPA., SPCS. - SPACES
DRAWINGS	SPECS. - SPECIFICATIONS
	STA. -- STATION
EACH	STD. -- STANDARD
EACH FACE	STRP. - STIRRUPS
EACH WAY	STL. -- STEEL
ELEV. --- ELEVATION	STRUCT. - STRUCTURAL
ENGR. -- ENGINEER	SYMM. - SYMMETRICAL
EQ. ---- EQUAL	
EXIST. - EXISTING	T.P. -- TEST PIT
EXP. --- EXPANSION	T&B --- TOP AND BOTTOM
EXT. --- EXTERIOR	T.F.E. - POLYTETRAFLUORETHYLENE
	TYP. -- TYPICAL
FIN. --- FINISH	VERT. - VERTICAL
FTGS. -- FOOTINGS	
FOUND. - FOUNDATION	
	WATERPRFG. - WATERPROOFING
GALV. -- GALVANIZED	W. ---- WIDTH
GA. ---- GAUGE	W/ ---- WITH
GRD. --- GRADE OR GROUND	W.P. -- WORK POINT

ESTIMATED QUANTITIES

ITEM NO.	ITEM	UNIT	QUANTITIES
202.0442	CONCRETE REMOVAL & PREPARATION OF EXISTING ABUTMENT	L.S.	1
206.5000	STRUCTURE EXCAVATION FOR RETAINING WALLS	C.Y.	520
206.6000	STRUCTURE EXCAVATION FOR BRIDGES	C.Y.	1,670
206.7000	STRUCTURE BACKFILL	C.Y.	990
206.7200	STRUCTURE BACKFILL FOR BRIDGE ABUTMENTS, WINGWALLS AND RETAINING WALLS	C.Y.	850
206.8000	FILTER MATERIAL	C.Y.	80
501.0110	STRUCTURAL STEEL FOR H1-K BRIDGE	LBS.	14,620
503.1050	CONCRETE IN RETAINING WALLS	L.S.	(241 C.Y.)
503.1053	CONCRETE IN RETAINING WALL FOOTINGS	L.S.	(216 C.Y.)
503.1090	CONCRETE IN H1-K BRIDGE	L.S.	(3,010 C.Y.)
503.1091	CONCRETE IN FOOTINGS FOR H1-K BRIDGE	L.S.	(681 C.Y.)
504.8100	POST-TENSIONING OPERATION FOR CAST-IN-PLACE STRUCTURES	L.S.	(207,000 LBS)
505.0001	PILE DRIVING EQUIPMENT EXPENSE	L.S. (NOT TO EXCEED \$8,000.00)	1
505.0010	PILE LOAD TESTS	EACH	1
505.0020	TEST PILES, DRIVEN	L.F.	1,034
505.0410	TYPE I PRESTRESSED CONCRETE PILES, FURNISHED	L.F.	20,741
505.4100	TYPE I PRESTRESSED CONCRETE PILES, DRIVEN	L.F.	19,707
505.9000	EXTENSIONS OR 'BUILD-UPS' OF PILES	F.A.	
506.0400	T.F.E. AND STAINLESS STEEL BEARING AND EXPANSION PLATES	S.F.	49
507.6000	CONCRETE RAILING AND END POST	L.F.	2,704
602.0051	REINFORCING STEEL IN RETAINING WALLS	L.S.	(56,200 LBS)
602.0053	REINFORCING STEEL FOR H1-K BRIDGE FOOTINGS	L.S.	(82,500 LBS)
602.0090	REINFORCING STEEL IN H1-K BRIDGE	L.S.	(968,100 LBS)

DATE	
SURVEY PLOTTED BY	
DRAWN BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
NO.	

6/6/80	Revised quantities for Item Nos. 206.6000 & 206.7000, per Addendum No. 1.
DATE	REVISION



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
INDEX TO DRAWINGS,
ESTIMATED QUANTITIES,
ABBREVIATIONS & SYMBOLS
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157):24
Scale: As Noted Date: 4-10-80
SHEET NO. 6-1 OF 5-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1057-24	1979	69	146

GENERAL NOTES

SPECIFICATIONS: AASHTO

STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES (12TH EDITION) WITH
SUBSEQUENT ADDITIONS AND MODIFICATIONS.

DESIGN LOADS: HS 20-44

DESIGN STRESSES	F'c	CLASS
CONCRETE: POST-TENSIONED SUPERSTRUCTURE (INCLUDING DECK SLAB)	5,500 PSI	
PIER COLUMNS AND PIER CAPS	5,500 PSI	
ABUTMENTS & WALLS & RAILINGS	3,000 PSI	A
PILE CAPS AT PIER COLUMNS	4,500 PSI	E
ABUTMENT & WALL FOOTINGS	3,000 PSI	A
REINFORCING STEEL: $F_s = 20$ KSI TENSION (24 KSI FOR PIER COLUMN VERTICAL BARS)		
PILES: 75 TON CAPACITY TYPICAL (50 TONS AT ABUTMENT #1 ONLY)		
SPREAD FOOTINGS: 4,000 PSF		

MATERIALS:

- ALL CONCRETE SHALL BE CLASS A EXCEPT AS NOTED ABOVE AND WHERE OTHER-
WISE NOTED ON THE PLANS.
- REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM A 615
AND SHALL BE OF THE FOLLOWING GRADES:

PIER COLUMN VERTICAL BARS	GRADE 60
ALL OTHER REINFORCING STEEL	GRADE 40
- ALL STRUCTURAL STEEL SHALL BE ASTM A 36 STEEL, GALVANIZED AFTER
FABRICATION.

CONSTRUCTION METHODS:

- SIDES OF FOOTINGS SHALL BE EXCAVATED AND POURED TO NEAT LINES WHEN
CALLED FOR ON THE PLANS. FOR EXCAVATION OF BOTTOM OF FOOTINGS, SEE
STANDARD SPECIFICATIONS. OVER EXCAVATION BEYOND NEAT LINES FOR
SIDES OF FOOTINGS AND THE ELEVATIONS INDICATED ON THE PLANS FOR THE
BOTTOM OF FOOTINGS SHALL BE BACKFILLED WITH A MINIMUM OF CLASS D
CONCRETE AT THE CONTRACTOR'S EXPENSE AND AS DIRECTED BY THE ENGINEER.
- ALL BACKFILL BEHIND ABUTMENTS SHALL BE COMPLETE IN PLACE BEFORE
PLACING OF SUPERSTRUCTURE, EXCEPT AS SHOWN IN THE DETAILS.
- VERTICAL COLUMN BARS SHALL BE ARRANGED IN SUCH A MANNER AS TO MISS
PIER CAP REINFORCING ABOVE.
- IN GENERAL, TOP OF CONCRETE DECK OF STRUCTURE SHALL BE CONSTRUCTED
TO FOLLOW THE FINISH ROADWAY VERTICAL AND HORIZONTAL CURVES AND GRADES.
- SHORING UNDER THE ENTIRE STRUCTURE SHALL REMAIN IN PLACE UNTIL THE
POST-TENSIONING OPERATIONS ARE COMPLETED, EXCEPT WHERE OTHERWISE NOTED
ON THE PLANS OR AS DIRECTED BY THE ENGINEER.

CAMBER:

OBTAIN CAMBER BY FORMING AND SHORING CONCRETE SURFACES ABOVE PROFILE
GRADES AMOUNT SHOWN IN CAMBER DIAGRAM AND AS INDICATED ON DETAILED DRAW-
INGS.

GENERAL:

- THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL EXISTING UTILITY LINES
AND NOTIFY THEIR RESPECTIVE OWNERS BEFORE COMMENCING WITH WORK.
- THE CONTRACTOR SHALL FURNISH SHOP PLACEMENT AND DETAIL DRAWINGS FOR
ALL STRUCTURAL STEEL WORK INCLUDING PROTECTIVE NOSING STEEL ANGLES,
STEEL SOLE PLATES AND LIGHT POLE BRACKETS.
- MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:

A. SLABS: TOP BARS - 1-1/2"	BOTTOM BARS - 1-1/4"
B. BOX GIRDERS: STIRRUPS, AT OUTSIDE FACES - 1-1/4"	STIRRUPS, AT INSIDE FACES - 1"
C. ABUTMENTS, WALLS: 2"	
D. PIERS: 2" TO TIES	
E. CONCRETE CAST AGAINST EARTH: 3"	
- ITEMS TO BE PAID FOR UNDER STRUCTURAL STEEL FOR HI-K SHALL INCLUDE:

A. STEEL ANGLE PROTECTIVE NOSINGS
B. STEEL SOLE PLATES
C. LIGHT POLE BRACKET ASSEMBLAGES & BRACKET ANCHOR PLATES
D. ANCHORS AND ANCHOR BOLTS

POST-TENSIONING NOTES

- SEE SPECIFICATIONS FOR OTHER POST-TENSIONED CONCRETE REQUIREMENTS.
- ALL DUCTS SHALL BE SECURELY ANCHORED AT ALL TIMES DURING CONSTRUCTION
TO PREVENT ANY DAMAGE. CONTRACTOR SHALL EXERCISE THE MAXIMUM AMOUNT
OF CARE DURING PLACEMENT OF DUCTS, REINFORCING AND CONCRETE IN ORDER
TO PROTECT THE DUCTS FROM DAMAGE AND TO PREVENT INTRUSION OF CONCRETE
INTO THE DUCTS.
- POST-TENSIONING SEQUENCE SHALL BE AS FOLLOWS:

A. STRESS NO MORE THAN 1/2 OF THE TENDONS IN ONE GIRDER BEFORE AN EQUAL NUMBER ARE STRESSED IN ADJACENT GIRDERS.
B. AT NO TIME DURING THE STRESSING OPERATIONS SHALL MORE THAN 1/6 OF THE TOTAL PRESTRESSING FORCE BE ECCENTRIC ABOUT THE CENTER LINE OF THE BRIDGE.
C. TENSION UPPER AND LOWER TENDONS ALTERNATELY TO MINIMIZE THE ECCENTRICITY OF THE PRESTRESS FORCE ABOUT THE CENTER OF GRAVITY OF THE BRIDGE SECTION.
- AFTER ALL POST-TENSIONING OPERATIONS (AND TESTING, IF REQUIRED) ARE
COMPLETED, ALL DUCTS SHALL BE GROUTED IN ACCORDANCE WITH THE SPECIFI-
CATIONS.
- AFTER GROUTING IS COMPLETED, THE ANCHORAGE ASSEMBLY SHALL BE REINFORCED
AND PROTECTED AS SHOWN ON THE DRAWINGS.
- THE MINIMUM VERTICAL CLEAR SPACING OF POST-TENSIONING DUCTS AT ANCHOR-
AGE SHALL BE 1-1/2" OR 1-1/2 TIMES THE MAXIMUM SIZE OF THE CONCRETE
AGGREGATE, WHICHEVER IS GREATER. MINIMUM HORIZONTAL CLEAR SPACING
BETWEEN DUCTS SHALL BE 2-1/2".
- WHEN POST-TENSIONING STEEL IS DRAPED OR DEFLECTED, POST-TENSIONING
DUCTS MAY BE BUNDLED VERTICALLY IN GROUPS OF THREE MAXIMUM, PROVIDED
THAT THE SPACING SPECIFIED IN NOTE 6 ABOVE IS MAINTAINED IN THE END
THREE FEET AT ANCHORAGE.
- ALL STRANDS SHALL BE PLACED IN RIGID METAL DUCTS AFTER BRIDGE CONCRETE
IS COMPLETED, UNLESS OTHERWISE NOTED ON DRAWINGS.
- DUCTS SHALL BE SECURELY TIED AT 4'-0" INTERVALS TO THE VERTICAL
STIRRUPS TO PREVENT DISPLACEMENT DURING CONCRETING. PROVIDE EXTRA TIES &
SUPPORTS WHERE REQUIRED TO MAINTAIN TENDON PROFILE.
- REINFORCING STEEL SHALL BE ADJUSTED OR RELOCATED DURING THE INSTAL-
LATION OF PRESTRESSING DUCTS AS REQUIRED TO PROVIDE PLANNED
CLEARANCES TO THE PRESTRESSING TENDONS, ANCHORAGES, JACKS AND
EQUIPMENT. GIRDER STEMS SHALL BE FLARED NEAR ANCHORAGES WHERE
REQUIRED TO PROVIDE A MINIMUM OF 1-1/2" CONCRETE COVER OVER THE
POST-TENSIONING DUCTS.
- POST-TENSIONING ANCHORAGES AND DUCTS ARE SHOWN ON DRAWINGS SCHEM-
ATICALLY ONLY. EXACT NUMBER, ARRANGEMENT AND SIZE OF POST-
TENSIONING COMPONENTS SHALL BE DETERMINED BY THE CONTRACTOR. THE
EFFECTIVE WORKING POST-TENSIONING FORCE REMAINING AFTER ALL LOSSES,
AND THE TENDON PROFILE, SHALL BE AS SHOWN ON THE DRAWINGS.
CONTRACTOR SHALL SUBMIT FOR REVIEW COMPLETE DETAILS OF THE METHOD,
MATERIALS AND EQUIPMENT TO BE USED IN THE PRESTRESSING OPERATION,
INCLUDING PRESTRESSING PLACEMENT DRAWINGS SHOWING ANY ADDITIONS
OR REARRANGEMENT OF REINFORCING STEEL OR REVISED GIRDER FLARES.
FRICTION, SEATING LOSS AND ELONGATION CALCULATIONS SHALL ALSO BE
SUBMITTED.
- DESIGN IS BASED ON THE FOLLOWING FRICTION COEFFICIENTS:

DUCT	K	U
LARGER THAN 3" DIAMETER	0.0002	0.25
3" DIAMETER OR LESS	0.0003	0.30
- THE MAXIMUM JACKING FORCE SHALL NOT EXCEED 75% OF THE ULTIMATE
TENSILE STRENGTH OF THE STRANDS UNLESS OTHERWISE DIRECTED BY THE
ENGINEER. HOWEVER, THE AMOUNT OF STRANDS TO BE FURNISHED SHALL BE
COMPUTED ASSUMING THAT THE JACKING FORCE WILL BE ONLY 70% OF THE
ULTIMATE TENSILE STRENGTH OF THE STRANDS.

- CONTRACTOR SHALL TAKE STEPS NECESSARY TO ALLOW SUPERSTRUCTURE TO
CONTRACT DURING TENSIONING OPERATIONS. DURING TENSIONING OPERATIONS,
THE BOX GIRDER SOFFIT FORMS WILL BE SUBJECT TO THE SAME COMPRESSIVE
FORCE AS THE CONCRETE BOX GIRDER UNLESS COMPRESSIBLE JOINTS AND/OR
BOND BREAKERS ARE PROVIDED TO RELIEVE THIS FORCE.
- NO TENSIONING OR GROUTING SHALL BE PERFORMED IN THE ABSENCE OF THE
ENGINEER.
- FRICTION TEST SHALL BE CONDUCTED PRIOR TO FINAL TENSIONING OF STRANDS
TO VERIFY ASSUMED FRICTION LOSS. PAYMENT TO BE INCIDENTAL TO POST-
TENSIONING OPERATION FOR CAST-IN-PLACE STRUCTURES. NO FRICTION TESTS
ARE REQUIRED AT TENDONS WITH DEAD END ANCHORAGES.
- TWO (2) TESTS WILL BE CONDUCTED FOR EACH DUCT. LENGTH AND PROFILE EXCEPT
WHERE ANCHORAGES ARE NOT ACCESSIBLE AT BOTH ENDS OF DUCTS.
- CONTRACTOR SHALL CHECK ALL POST-TENSIONING DUCTS FOR BLOCKAGES PRIOR
TO PLACING THE TOP SLAB OF THE BOX GIRDER STRUCTURE.
- MAXIMUM ALLOWABLE ANCHOR SET SHALL BE 5/8".
- ASSUMED PRESTRESS LOSSES OTHER THAN FRICTION AND ANCHOR SET = 33,000
P.S.I.
- PRESTRESSING TENDONS SHALL BE COMPOSED OF 1/2 INCH 270 K.S.I.
STRANDS CONFORMING TO ASTM A-416.
- BEARING PLATES SHALL BE PLACED TIGHT AGAINST FORMS. CENTERLINE
OF TENDONS ARE TO BE NORMAL TO PLATES.
- DURING THE POST-TENSIONING OPERATION, THE WEIGHT OF THE
STRUCTURE WILL BE REDISTRIBUTED AND EXTRA LOADS WILL
BE ADDED TO THE STRUCTURE AND THE SUPPORTING
FALSEWORK AT THE FOLLOWING LOCATIONS:

a. STA. 3+25 (ABUTMENT NO. 1): ADD A DOWNWARD LOAD OF 280 KIPS.
b. STA. 4+36 (EXPANSION JOINT): ADD A DOWNWARD LOAD OF 830 KIPS.
c. STA. 4+71 TO STA. 6+18: ADD A UNIFORM DOWNWARD LOAD OF 10 KIPS PER FOOT OF SPAN.
d. STA. 8+34 (EXPANSION JOINT): ADD A DOWNWARD LOAD OF 150 KIPS.
e. STA. 11+40.25 (ABUTMENT NO. 2): ADD A DOWNWARD LOAD OF 250 KIPS.

AT CONTRACTOR'S OPTION, THE ADDITIONAL LOADS DUE
TO THE POST-TENSIONING MAY BE REDUCED BY DISTRI-
BUTING THE LOADS TO THE STRUCTURE AND THE FALSE-
WORK IN PROPORTION TO THEIR RELATIVE STIFFNESSES.

ANCHORAGE ASSEMBLY PROTECTION NOTES

- AFTER ALL POST-TENSIONING WORK IS COMPLETED, CLEAN POST-
TENSIONING HARDWARE AS REQUIRED IN ORDER TO PROVIDE GOOD
BONDING SURFACE.
- PROVIDE ADDED REINFORCING AS SHOWN ON THE DRAWINGS.
- PAINT ENTIRE ANCHORAGE ASSEMBLY (INCLUDING POST-TEN-
SIONING HARDWARE) WITH TWO COMPONENT EPOXY (COST INCIDENT-
AL TO CONCRETE).
- JUST BEFORE NEW CONCRETE IS PLACED AGAINST POST-TEN-
SIONING ANCHOR PLATES, APPLY A NEW COAT OF EPOXY FOR
BOND WITH CONCRETE.

FOUNDATION NOTES

- A STRUCTURE FOUNDATION REPORT, DATED MARCH 1979, HAS BEEN PREPARED
BY THE DEPARTMENT OF TRANSPORTATION, LAND TRANSPORTATION FACILITIES
DIVISION, MATERIALS TESTING AND RESEARCH BRANCH. A COPY OF THIS
REPORT WILL BE AVAILABLE FOR REVIEW AT THE DEPARTMENT OF TRANSPORT-
ATION.
- INDEX TEST PILES: TWO INDEX TEST PILES SHALL BE DRIVEN AT EACH
PIER FOOTING AND AT ABUTMENT #1. (TOTAL OF 12 INDEX TEST PILES
WILL BE REQUIRED.) LOCATION OF TEST PILES IS SHOWN ON SHEET S-5
AND S-15. TEST PILES SHALL BE SUBJECT TO RELOCATION BY THE ENGINEER.
- LOAD TEST PILE: ONE LOAD TEST PILE SHALL BE DRIVEN AT PIER NO. 4
OR PIER NO. 5. LOAD TEST PILE LOCATION TO BE DETERMINED BY THE
ENGINEER AFTER THE INDEX TEST PILES HAVE BEEN DRIVEN. LOAD TEST
PILE SHALL HAVE A 5" DIAMETER CENTRAL HOLE AND 3 ROD EXTENSOMETERS.
- SPLICES IN PILES SHALL BE IN ACCORDANCE WITH THE TYPICAL PILE SPLICE
DETAILS. ONLY ONE SPLICE SHALL BE USED IN ANY PILE. LENGTH OF EACH
SECTION OF PILE SHALL BE SET SO THAT THE SPLICE IS LOCATED WITHIN
THE MIDDLE THIRD OF THE TOTAL PILE LENGTH.
- CENTRAL HOLE FOR INSPECTION IN LOAD TEST PILES SHALL
BE FORMED BY A 5" DIA. CORRUGATED PLASTIC PIPE CAST
IN THE CENTER OF THE PILE. THE PIPE SHALL HAVE A
RADIAL TOLERANCE OF 1/8" OFF-SET FROM CENTER OF PILE
EXCEPT THAT WITHIN 3 FEET OF THE ENDS OF PILE
SEGMENTS THE RADIAL TOLERANCE SHALL BE 1/16" OFF-SET
FROM THE CENTER OF THE PILE. THE ALLOWABLE DEVIATION
FROM STRAIGHT FOR THE PIPE IS 1/8" IN ANY 10 FEET OF
LENGTH. STOP AND CAP PLASTIC PIPE 2'-0" FROM PILE TIP.
- THE 5" DIA. CORRUGATED PLASTIC PIPE SHALL BE
SIMILAR OR EQUAL, AS DETERMINED BY THE ENGINEER,
TO THE CORRUGATED PLASTIC DRAINAGE TUBING
MANUFACTURED BY ADVANCE DRAINAGE SYSTEMS AND
SHALL HAVE A WALL THICKNESS OF 0.05 INCHES, 5.73
INCHES O.D. ON OUTSIDE OF CORRUGATION OF 5.07
INCHES O.D. ON INSIDE OF CORRUGATION, AND A PITCH
OF 0.7 INCHES. THE PIPE SHALL BE MADE IN STRAIGHT
LENGTHS, NOT COILED, OF 10 FEET EACH AND BE NON-
PERFORATED.
- AT PILE SPLICES IN LOAD TEST PILES, PROVIDE 7-1/2"
DIA. CENTER HOLE IN 1/2" PLYWOOD THAT OCCURS
BETWEEN ENDS OF PILE SECTIONS.
- PILE SPLICES AND 5" DIA. CORRUGATED PLASTIC PIPE
FOR CENTRAL INSPECTION HOLES SHALL BE INCIDENTAL
TO "TYPE I PRESTRESSED CONCRETE PILES, FURNISHED",
AND WILL NOT BE PAID FOR SEPARATELY.

6/6/80 Added note 23 under "POST-TENSION-
ING NOTES" and notes 5, 6, 7 & 8 under
"FOUNDATION NOTES", per Addendum No. 1.

DATE REVISION



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

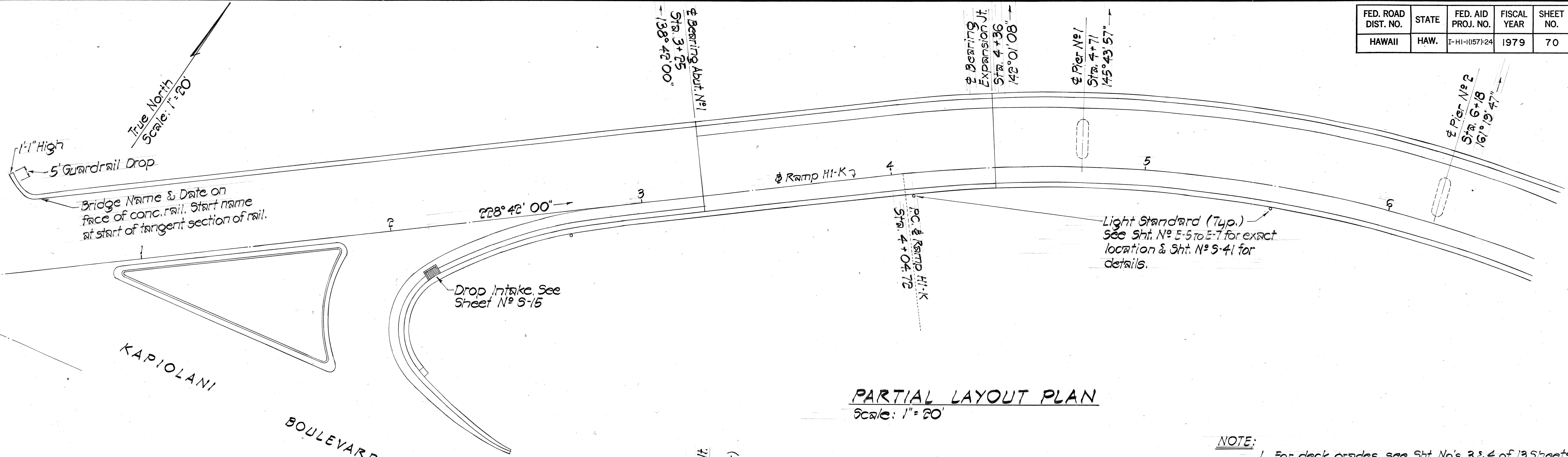
GENERAL NOTES, POST-TENSIONING
NOTES & FOUNDATION NOTES

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(197)-24

Scale: As Noted Date: 4-10-80

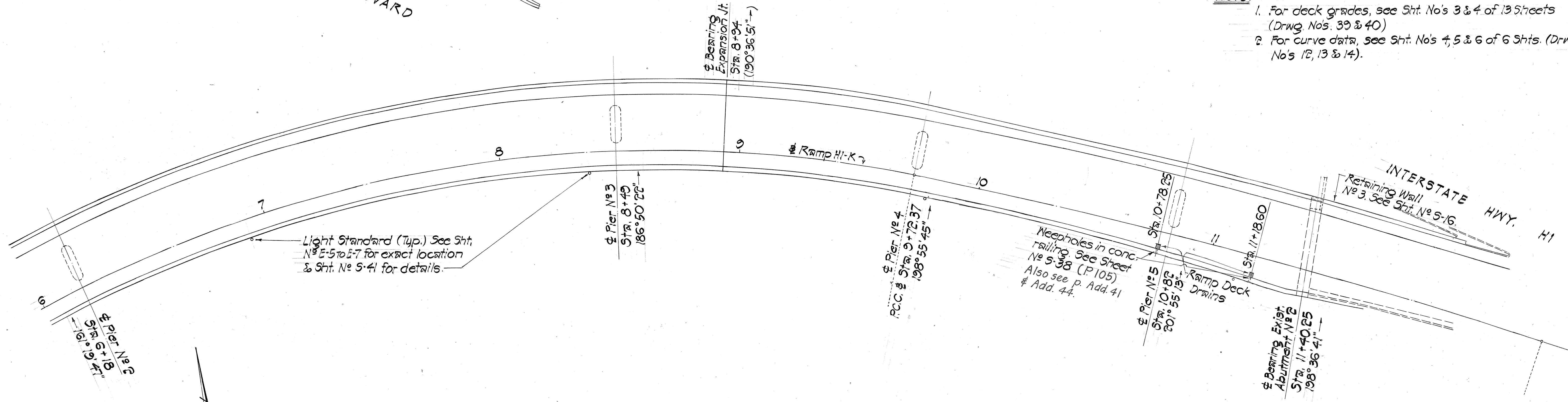
SHEET NO. 5-2 OF 5-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	70	146



PARTIAL LAYOUT PLAN
Scale: 1" = 20'

- NOTE:**
1. For deck grades, see Sht. No's 3 & 4 of 13 Sheets (Drawg. Nos. 39 & 40)
 2. For curve data, see Sht. No's 4, 5 & 6 of 6 Shts. (Drawg. Nos. 12, 13 & 14).



PARTIAL LAYOUT PLAN
Scale: 1" = 20'

ORIGINAL PLAN	DATE
DESIGNED BY	
TRACED BY	
NOTED BY	
QUANTITIES BY	
CHECKED BY	
No.	

True North
Scale: 1" = 20'

ENGINEERING DESIGN GROUP, INC.
Consulting Engineers
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EDGAR K. M. LEE
REGISTERED PROFESSIONAL ENGINEER
No. 2150
HAWAII, U.S.A.

This work was prepared by me or under my supervision

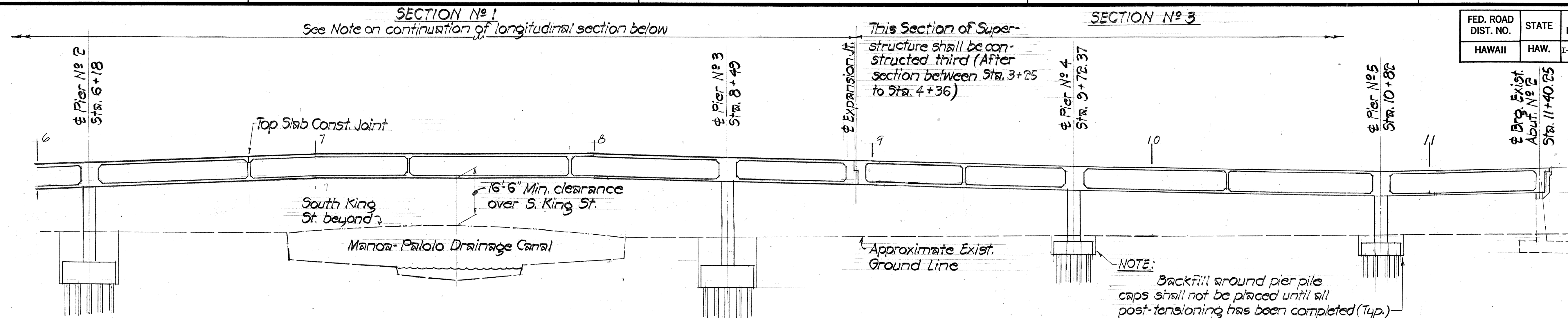
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

LAYOUT PLAN

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24
Scale: As Noted Date: 4-10-80

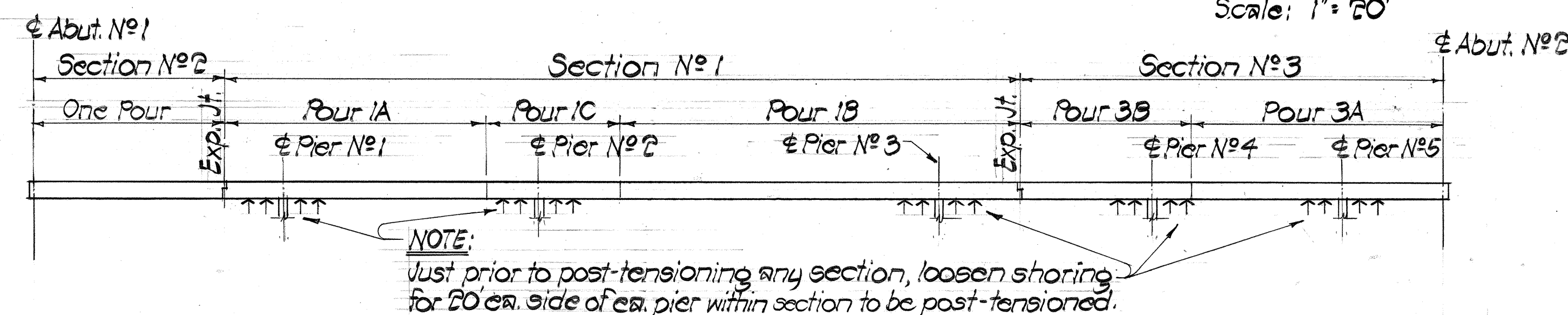
SHEET No. 5-30 OF 5-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)24	1979	71	146



LONGITUDINAL SECTION ALONG RAMP HI-K

Scale: 1" = 20'

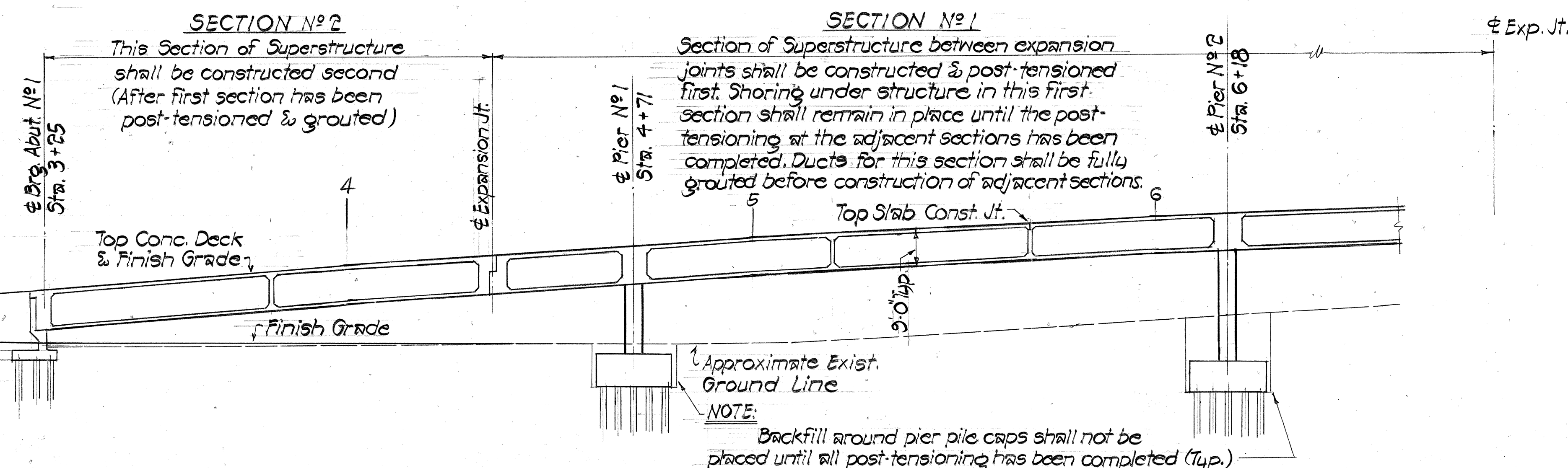


SCHEMATIC GIRDER CONSTRUCTION SEQUENCE DIAGRAM

No Scale

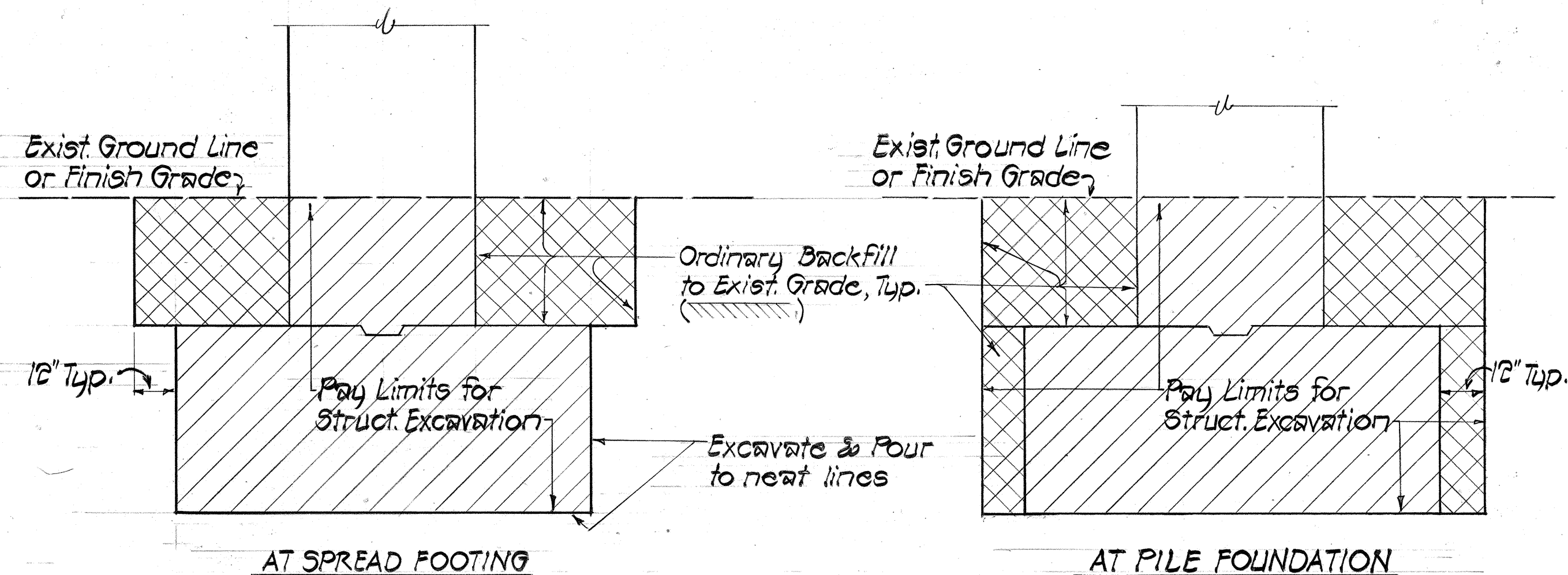
Construction Sequence shall be as follows:

1. Pour Section 1A
2. Pour Section 1B } These pours can be done simultaneously
3. Wait 28 days, then pour Section 1C
4. Post-tension & grout tendons for Section N° 1, pour post-tensioning blockouts
5. Pour Section N° 2
6. Pour Section 3A } These pours can be made simultaneously
7. Wait 14 days, then pour Section 3B
8. Complete post-tensioning, grouting of tendons, etc.



LONGITUDINAL SECTION ALONG RAMP HI-K

Scale: 1" = 20'

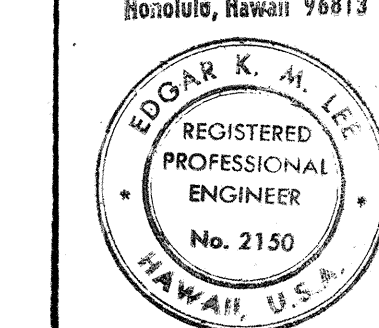


TYPICAL LIMITS OF STRUCTURAL EXCAVATION PAYLINE & BACKFILL

NOTE:

For Limits of structural excavation at retaining wall footings, see Sht. S-35

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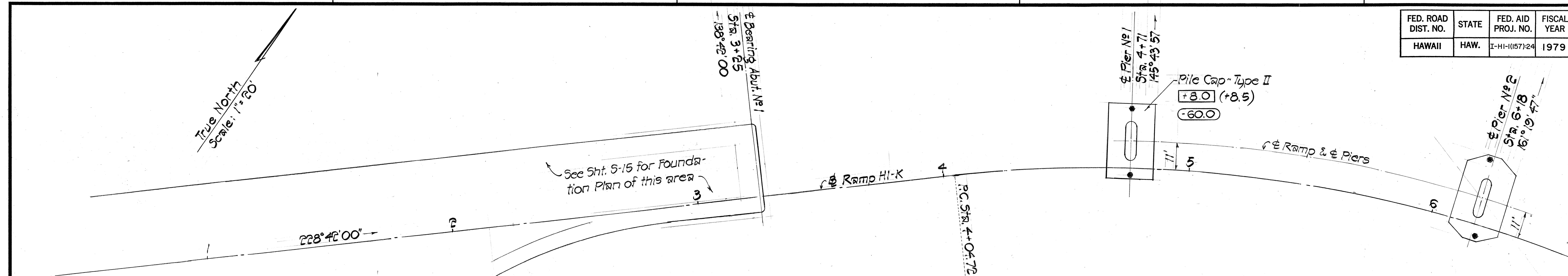
LONGITUDINAL SECTION

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
P.A.T. PROJ. NO. I-HI-1(157)24

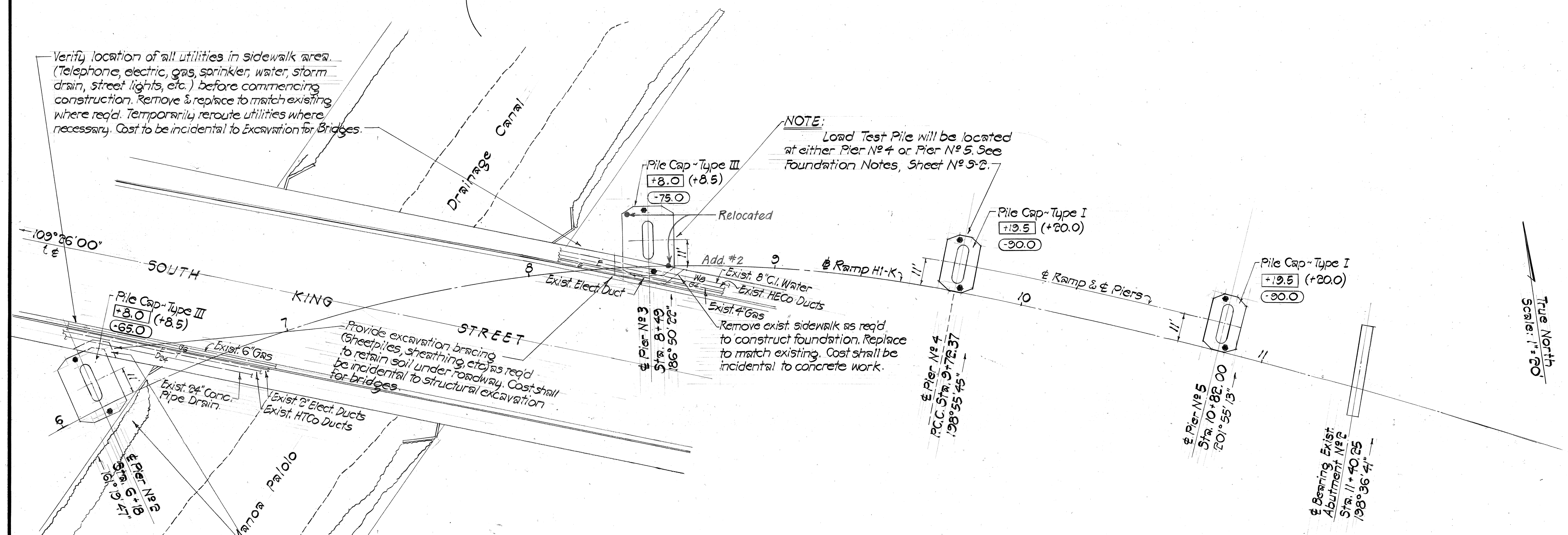
Scale: As Noted Date: 4-10-80

SHEET No. 5-4 OF 5-4 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	72	146



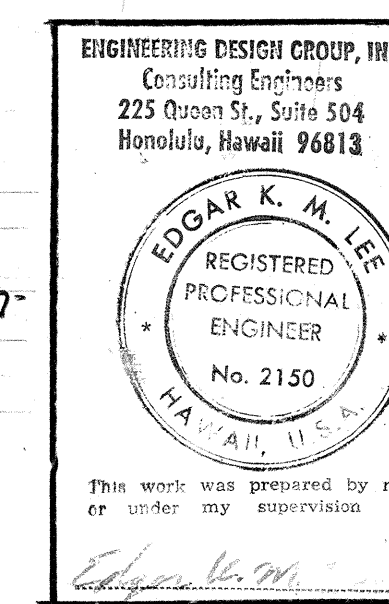
PARTIAL FOUNDATION PLAN
Scale: 1" = 20'



PARTIAL FOUNDATION PLAN
Scale: 1" = 20'

NOTES:

- See Sht. 5-21 for pile cap details.
 - See Sht. 5-22 for pier column schedule & details.
 - Long dimension of piers shall be oriented @ right angles to Ramp HI-K.
- +19.5 Bot. of Pile Cap Elevation
(+20.0) Pile Cut-off Elevation
(-90.0) Pile Tip Elevation



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

FOUNDATION PLAN

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24

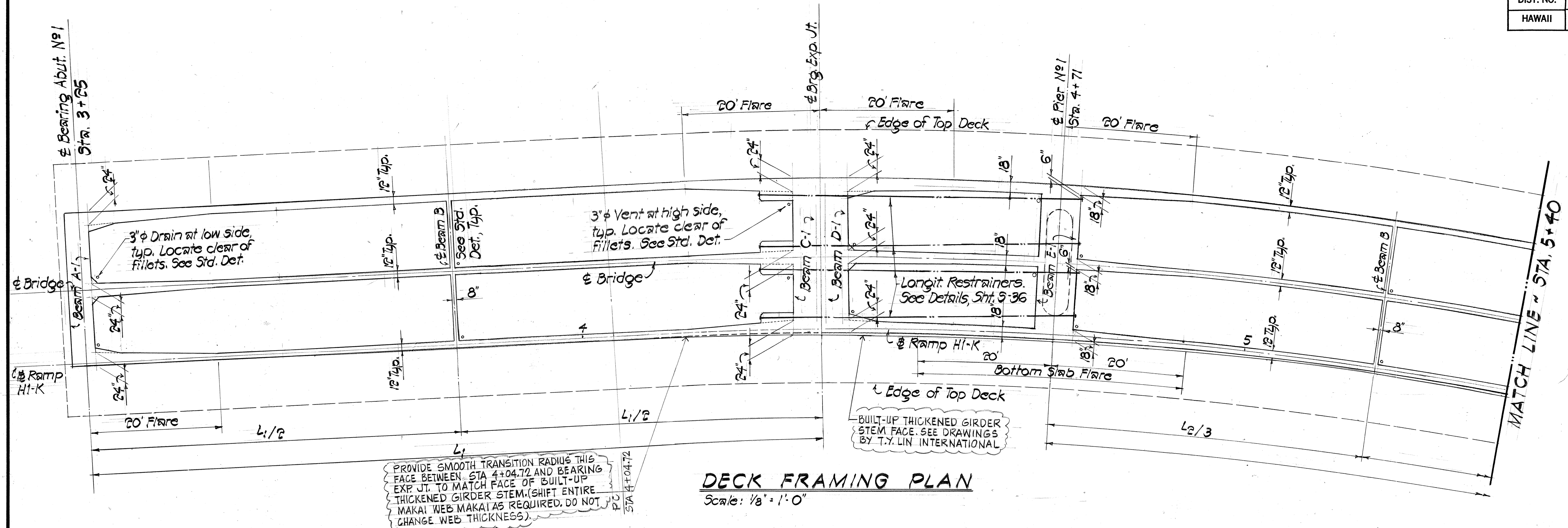
Scale: As Noted Date: 4-10-80
SHEET No. 6-5 OF 6-4 SHEETS

ORIGINAL PLAN	DATE
DESIGNED BY	
TRACED BY	
NOTE BOOK	
QUANTITIES BY	
CHECKED BY	

6/16/80	Revised existing 12" Water to 8" Water near Pier No. 3, per Addendum No. 2.
DATE	REVISION

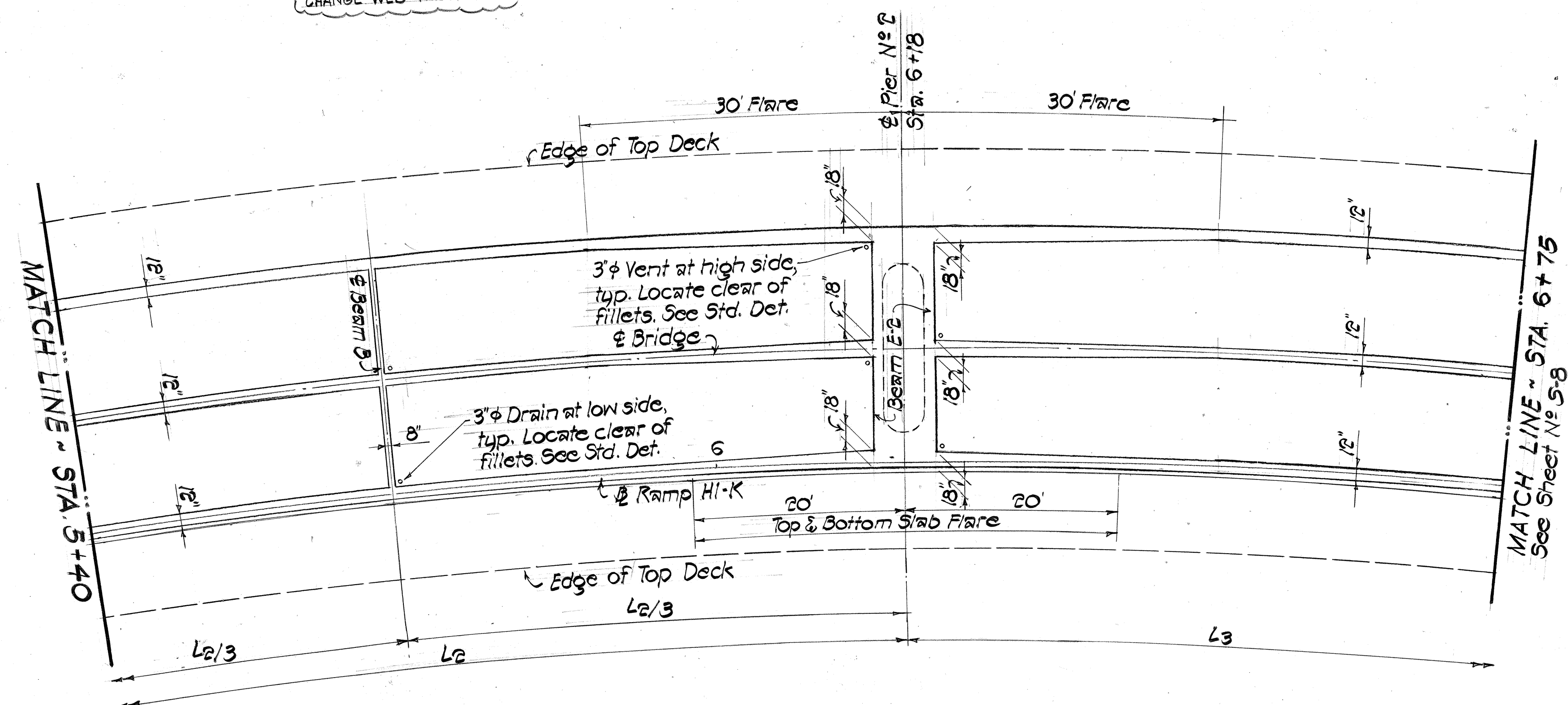
● Index Test Pile. See Foundation Notes, Sheet No. 5-2.

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157):24	1979	74	146



DECK FRAMING PLAN

Scale: 1/8" = 1'-0"

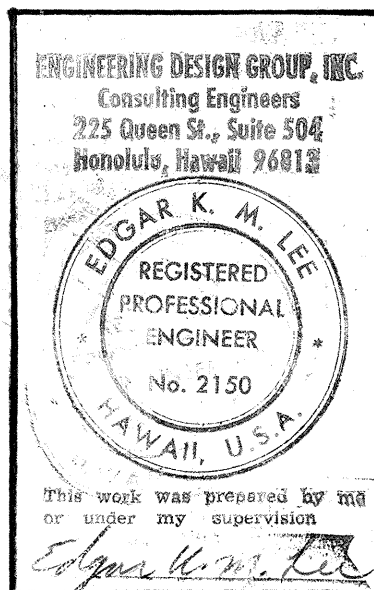


DECK FRAMING PLAN

Scale: 1/8" = 1'-0"

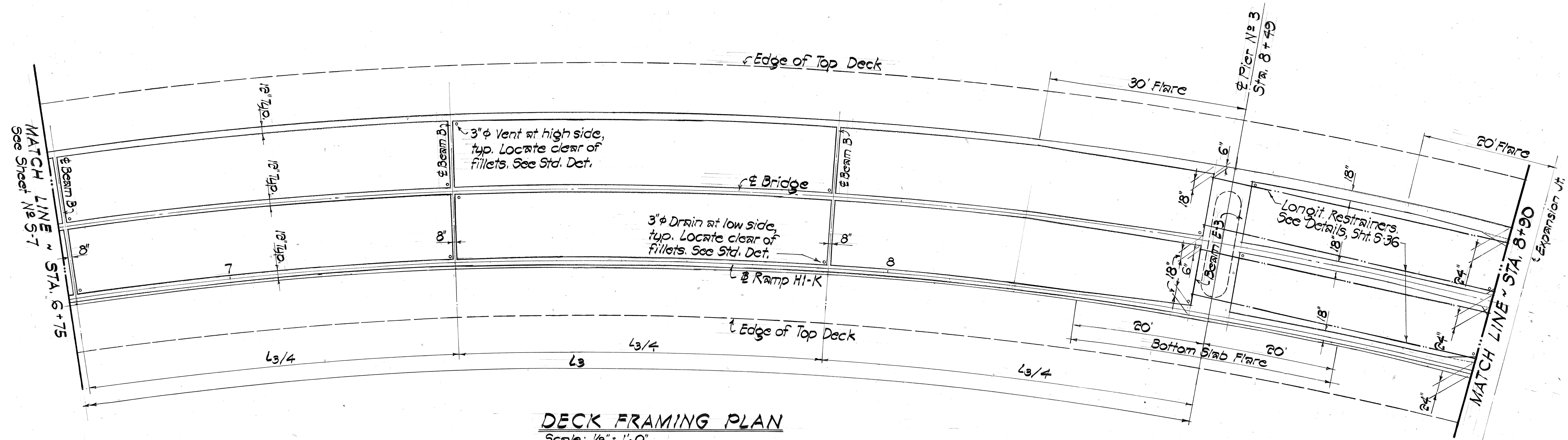
ORIGINAL PLAN	SURVEY PLOTTED BY	DATE
NOTE BOOK	DRAWN BY	
No.	DESIGNED BY	
	QUANTITIES BY	
	CHECKED BY	

3-25-83	Shifted Makai Web @ Exp Jt
DATE	REVISION

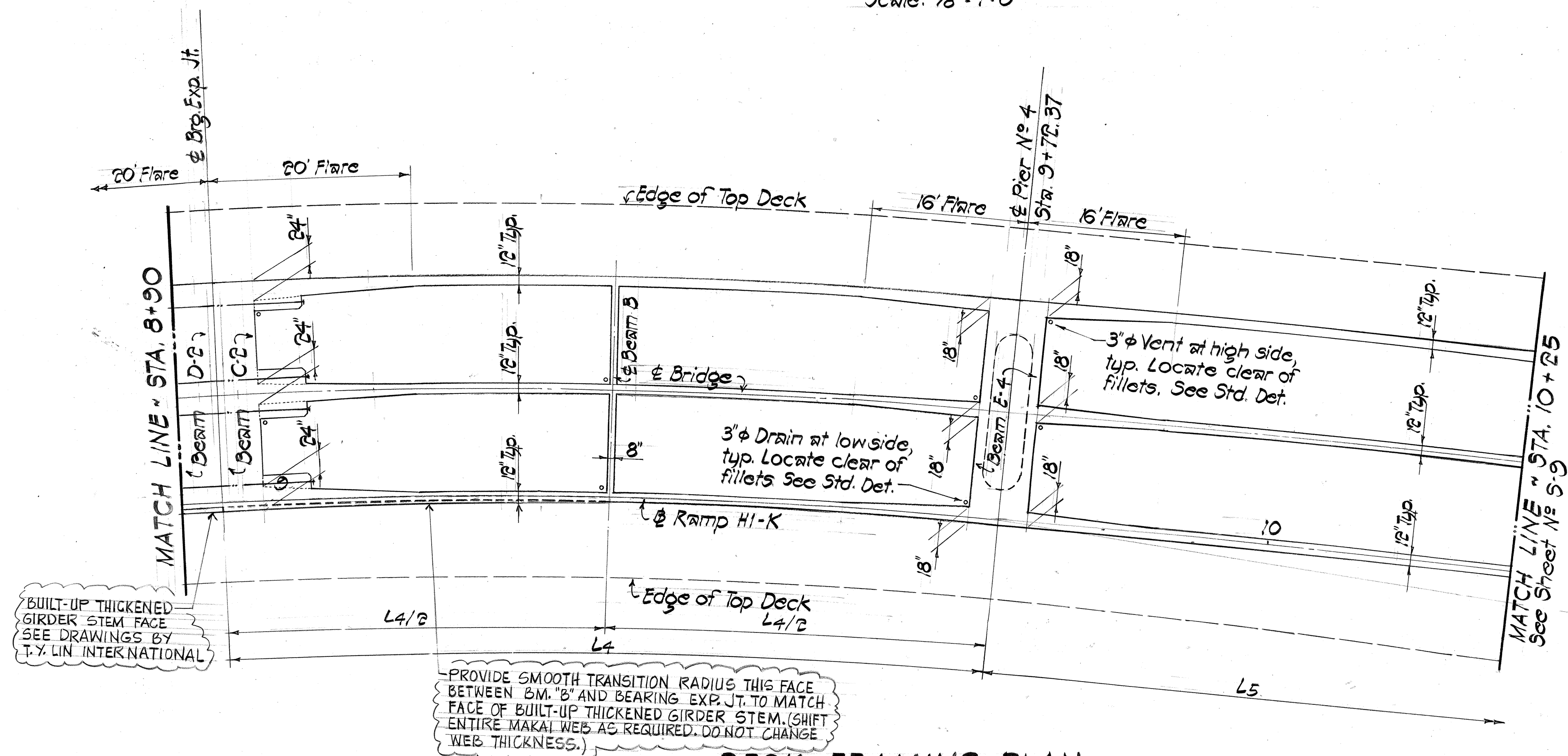


STATE OF HAWAII DEPARTMENT OF TRANSPORTATION LAND TRANSPORTATION FACILITIES DIVISION	
DECK FRAMING PLAN STA. 3+25 TO STA. 6+75	
INTERSTATE ROUTE H-1 KAPIOLANI INTERCHANGE ON-RAMP F.A.I. PROJ. NO. I-HI-1(157):24	
Scale: As Noted	Date: 4-10-80
SHEET No. 5-7 OF 5-41 SHEETS	

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)24	1979	75	146



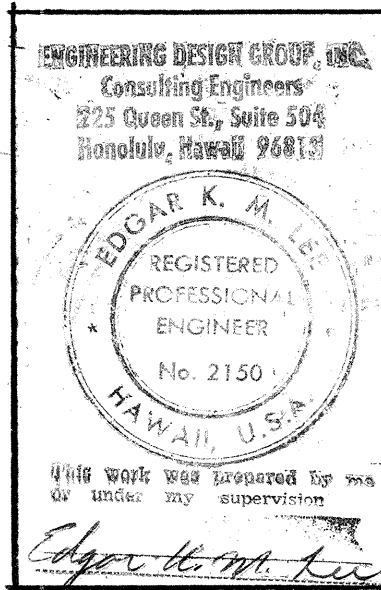
DECK FRAMING PLAN
Scale: 1/8" = 1'-0"



PROVIDE SMOOTH TRANSITION RADIUS THIS FACE BETWEEN 8M. 'B' AND BEARING EXP. JT. TO MATCH FACE OF BUILT-UP THICKENED GIRDER STEM. (SHIFT ENTIRE MAKAI WEB AS REQUIRED. DO NOT CHANGE WEB THICKNESS.)

DECK FRAMING PLAN
Scale: 1/8" = 1'-0"

3-25-83 Shifted Makai Web @ Exp. Jt.
DATE REVISION



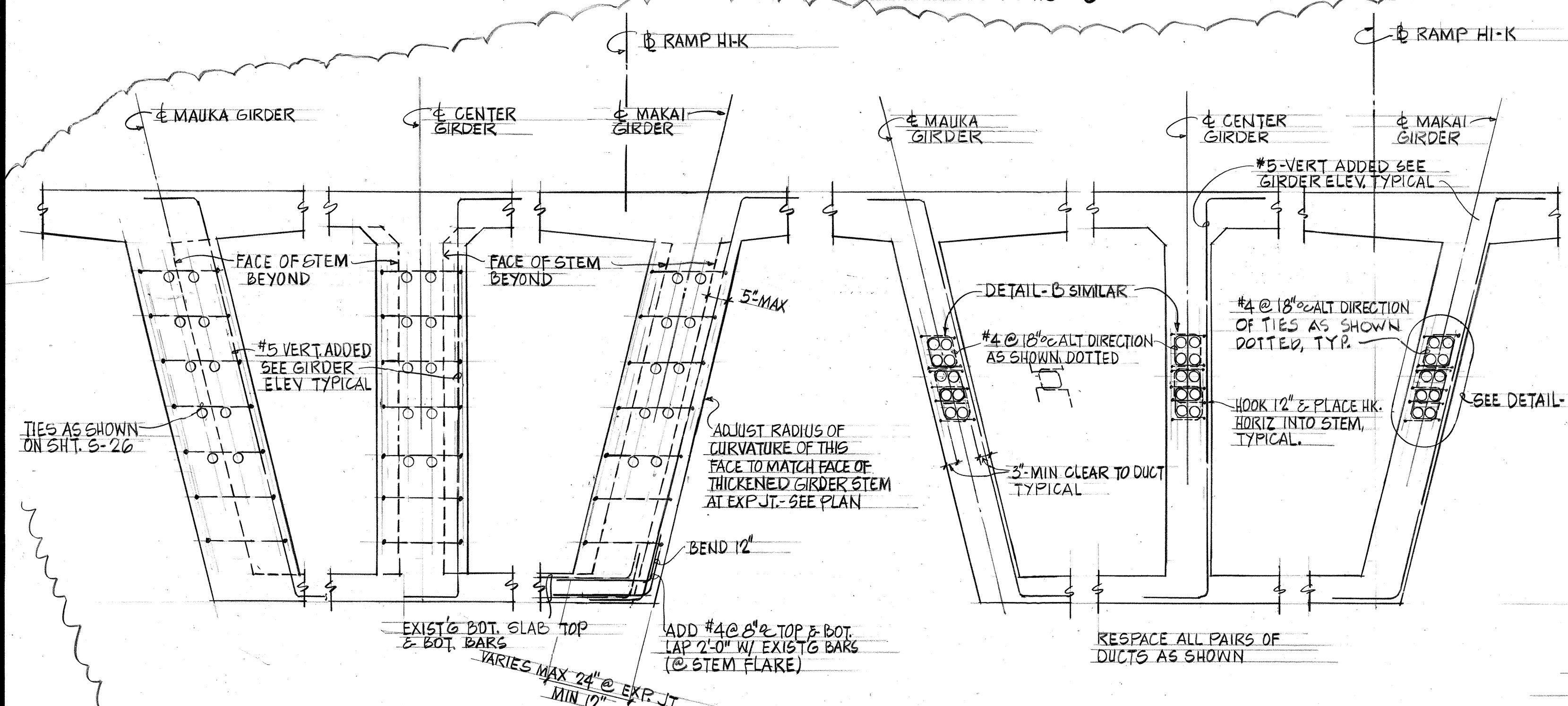
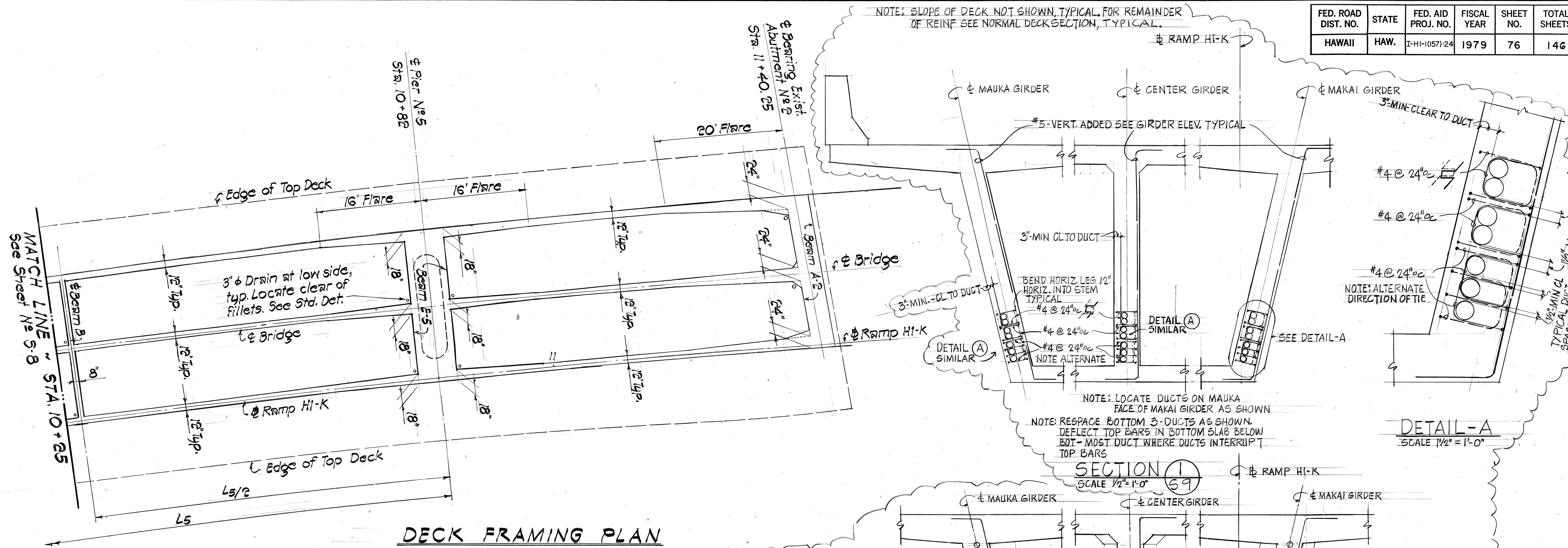
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

DECK FRAMING PLAN
STA. 6+75 TO STA. 10+25

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)24

Scale: As Noted Date: 4-10-80
SHEET No. 5-8 OF 5-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-H-1(157):24	1979	76	146

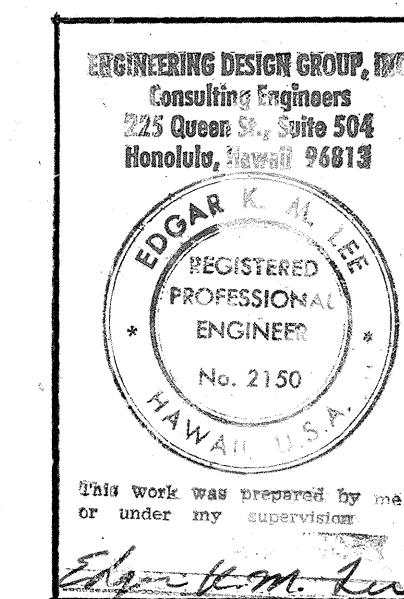


DETAIL - B
SCALE 1/2" = 1'-0"

SECTION 3
SCALE 1/2" = 1'-0"

SECTION 4
SCALE 1/2" = 1'-0"

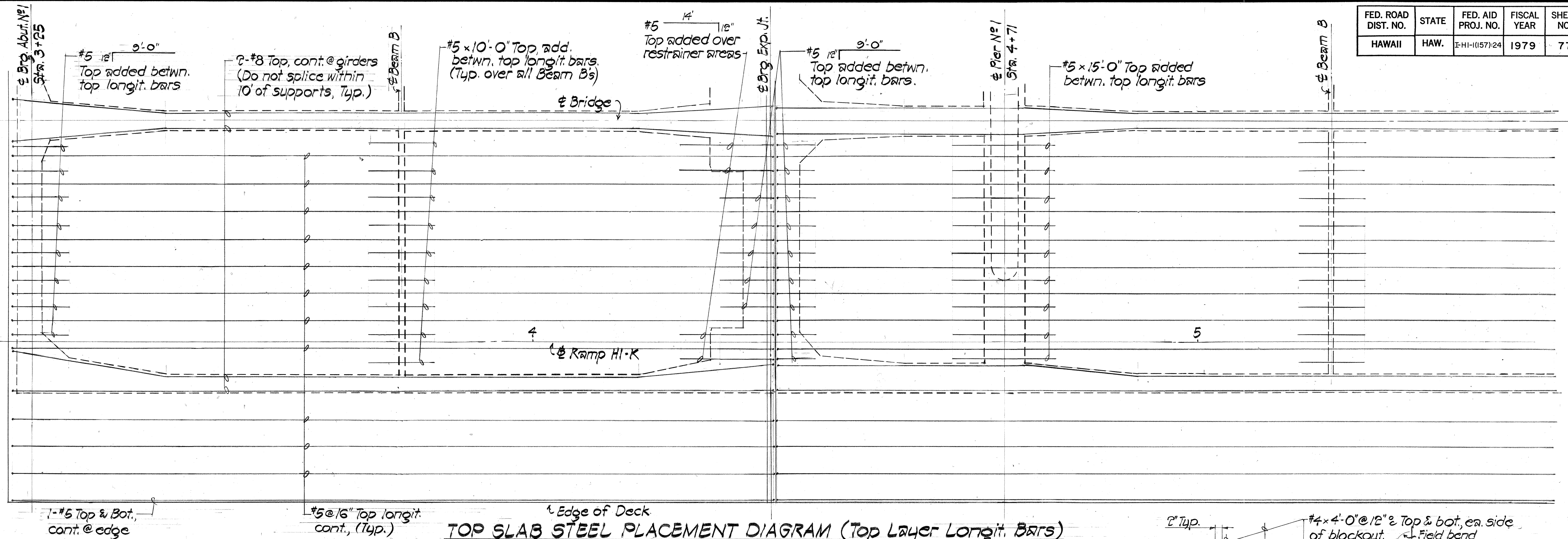
3-25-83 Added Sect. 1, 2, 3, 4 and Det A & B
DATE REVISION



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
DECK FRAMING PLAN
STA. 10+25 TO STA. 11+40.25
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-H-1(157):24
Scale: As Noted Date: 4-10-80
SHEET NO. 5-9 OF 5-41 SHEETS

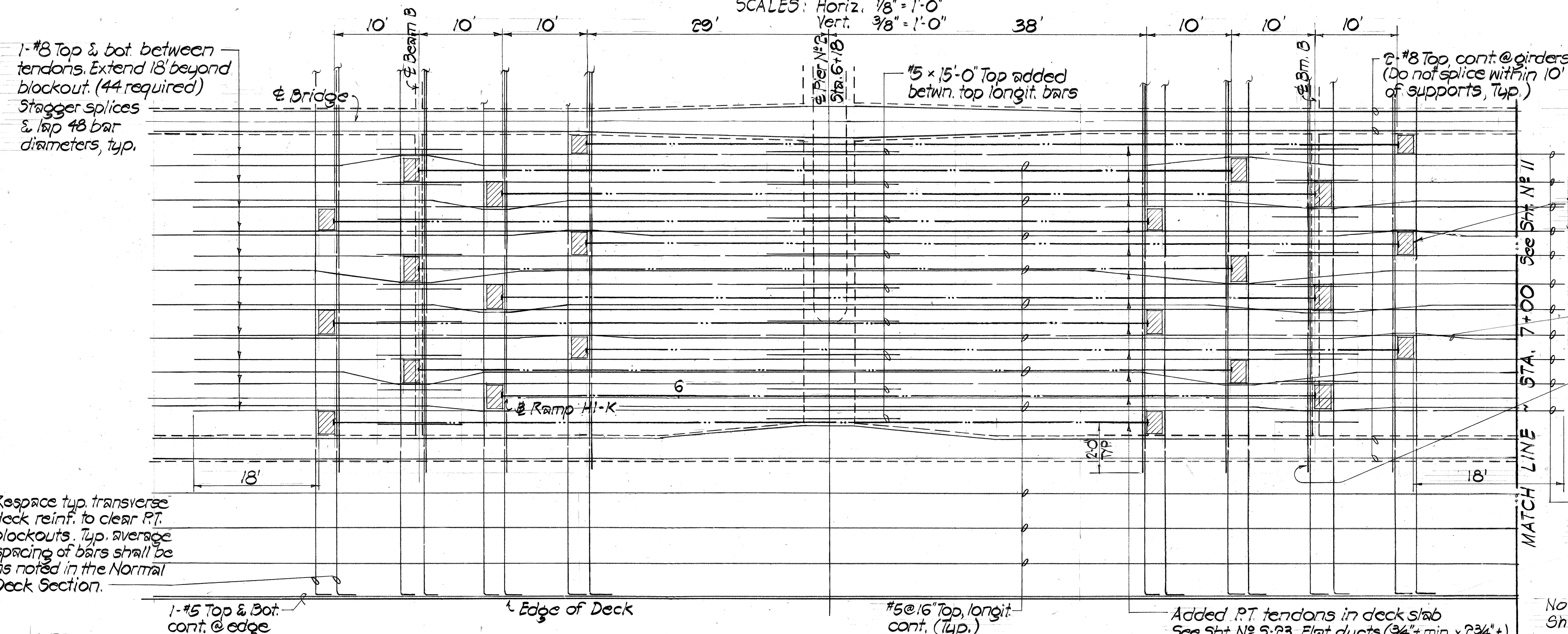
DATE	5-2-83
SURVEY PLOTTED BY	
DRAWN BY	
DESIGNED BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
No.	

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	77	146



TOP SLAB STEEL PLACEMENT DIAGRAM (Top Layer Longit. Bars)

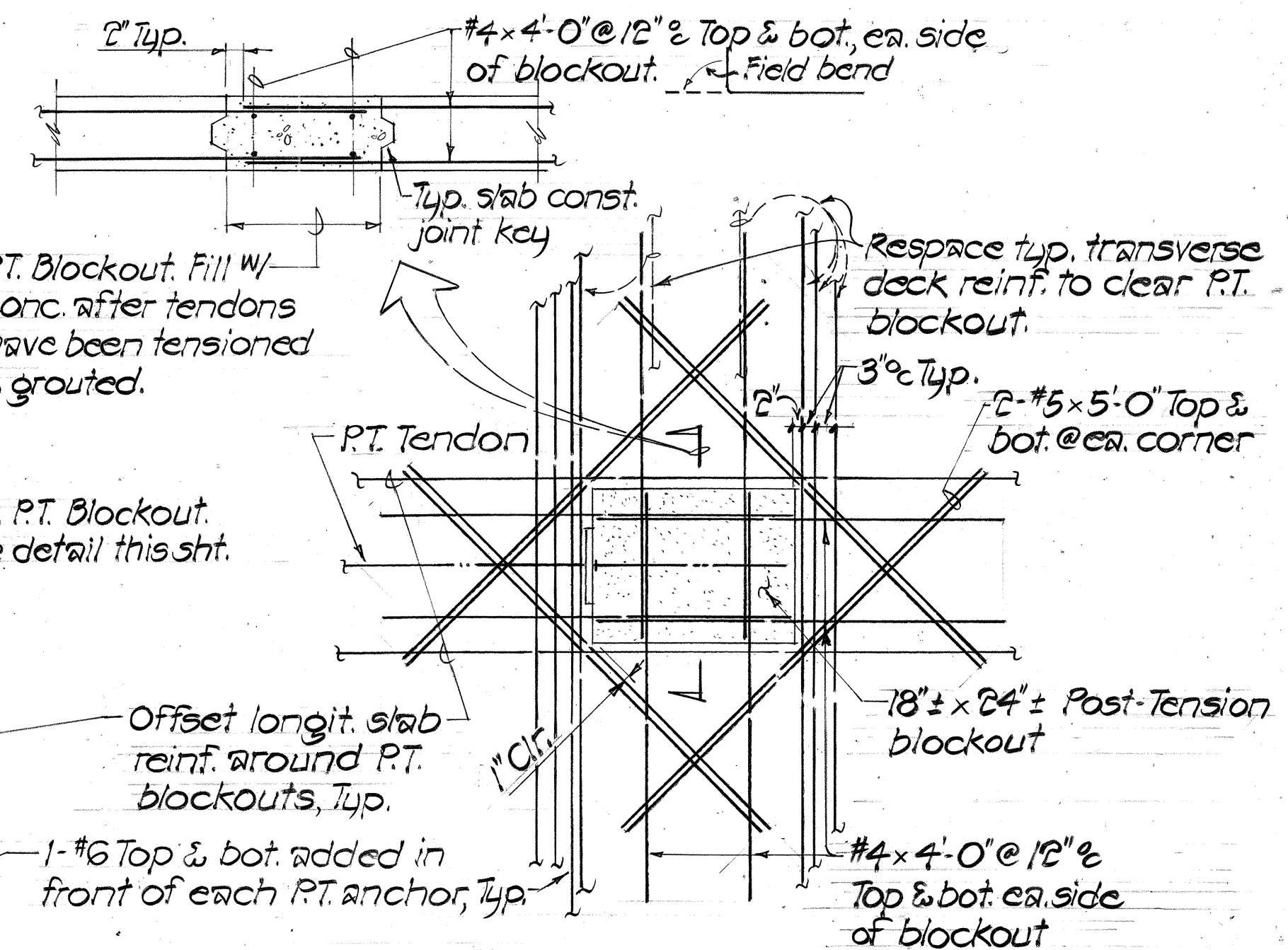
SCALES: Horiz. $\frac{1}{8}" = 1'-0"$
Vert. $\frac{3}{8}" = 1'-0"$



TOP SLAB STEEL PLACEMENT DIAGRAM (Top Layer Longit. Bars)

SCALES: Horiz. $\frac{1}{8}" = 1'-0"$
Vert. $\frac{3}{8}" = 1'-0"$

Added P.T. tendons in deck slab See Sht. No. 5-23. Flat ducts ($\frac{3}{4}" \pm$ min. x $2\frac{3}{4}" \pm$) & rectangular bearing plates ($4" \pm$ x $9\frac{3}{4}" \pm$) shall be used. Strands shall be protected with a corrosion inhibitor if they are placed in ducts before concrete is placed.



TYP. P.T. BLOCKOUT IN DECK SLAB DETAIL

Scale: $\frac{3}{4}" = 1'-0"$

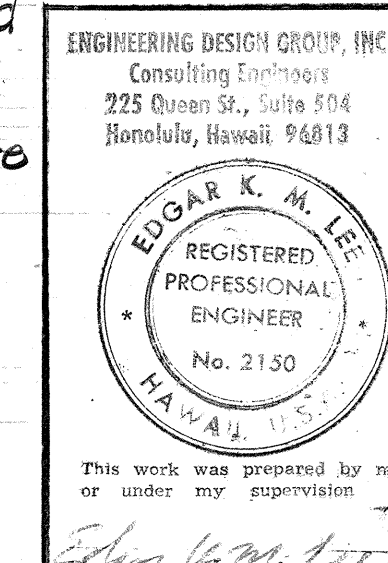
Top & bot. added bars betwn tendons - See note at opposite end.

Note: See notes, Sht. No. 5-11

Respace typ. transverse deck reinf. to clear P.T. blockouts. Typ. average spacing of bars shall be as noted in the Normal Deck Section.

NOTE:

All bars to follow curvature of girders. See Deck Framing Plan & Layout Plans for true curvature of deck.



STATE OF HAWAII
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LAND TRANSPORTATION FACILITIES DIVISION

TOP SLAB STEEL PLACEMENT DIAGRAM - STA. 3+25 TO STA. 7+00

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24

Scale: As Noted Date: 4-10-80

SHEET No. 5-10 OF 5-41 SHEETS

MATCH LINE ~ STA. 7+00 See Sht. No 5.10

#8 Top, cont. @ girders
(Do not splice within 10' of supports, Typ.)

Beam B

Beam B

#5 x 10'-0" Top, add.
betwn. top longit. bars.
(Typ. over all Beam B's)

10' 20'

Bridge

Pier No 3
Sta. 8+49

#5 x 15'-0" Top added
betwn. top longit. bars

9'-0"

#5 1/2" Top added betwn.
top longit. bars

Ramp H-1-K

Exp. Jt.

1/5 Top & Bot. cont. @ edge

#5 @ 16" Top Longit. cont., (Typ.)

Edge of Deck

TOP SLAB STEEL PLACEMENT DIAGRAM (Top Layer Longit. Bars)

SCALES: Horiz. 1/8" = 1'-0"
Vert. 3/8" = 1'-0"

#8 x 60'-0" Top, add. betwn. top longit. bars. Stagger as shown (29 Total)

Hand-drawn structural drawing of a bridge deck cross-section. The drawing shows a wide rectangular deck with a central vertical line labeled "Center Line". On the left side, there is a vertical line labeled "Beam B". On the right side, there is a vertical line labeled "Beam C". The top of the deck is labeled "Top added between top longit. bars". The bottom of the deck is labeled "Bottom added between bottom longit. bars". The drawing includes various dimensions and notes: "9'-0" 3'-0"" at the top left, "9'-0" 3'-0"" at the top right, "9'-0" 3'-0"" at the bottom left, and "9'-0" 3'-0"" at the bottom right. A note on the right side reads "MATCH LINE ~ STA. 10+00 See Sheet No. 512". A note on the left side reads "Top added between top longit. bars". A note on the right side reads "Bottom added between bottom longit. bars". A note on the right side reads "9'-0" 3'-0"".

1. All bars to follow curvature of girders. See Deck Framing Plan & Layout Plan for true curvature of deck.
2. See Normal Deck Section for remainder of slab reinf.
3. Reinforcing is symmetrical about ϕ bridge.

#5 1/4" 1/2" 1-#5 Top & bot. cont. @ edge #5 @ 16" Top longit. cont. Typical Edge of Deck

TOP SLAB STEEL PLACEMENT DIAGRAM (Top Layer Longit. B)

SCALES: Horiz. 1/8" = 1'-0"
Vert. 3/8" = 1'-0"

2-#8 Top, cont. @ girders
(Do not splice within 10'
of supports, Typ.)

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or under my supervision

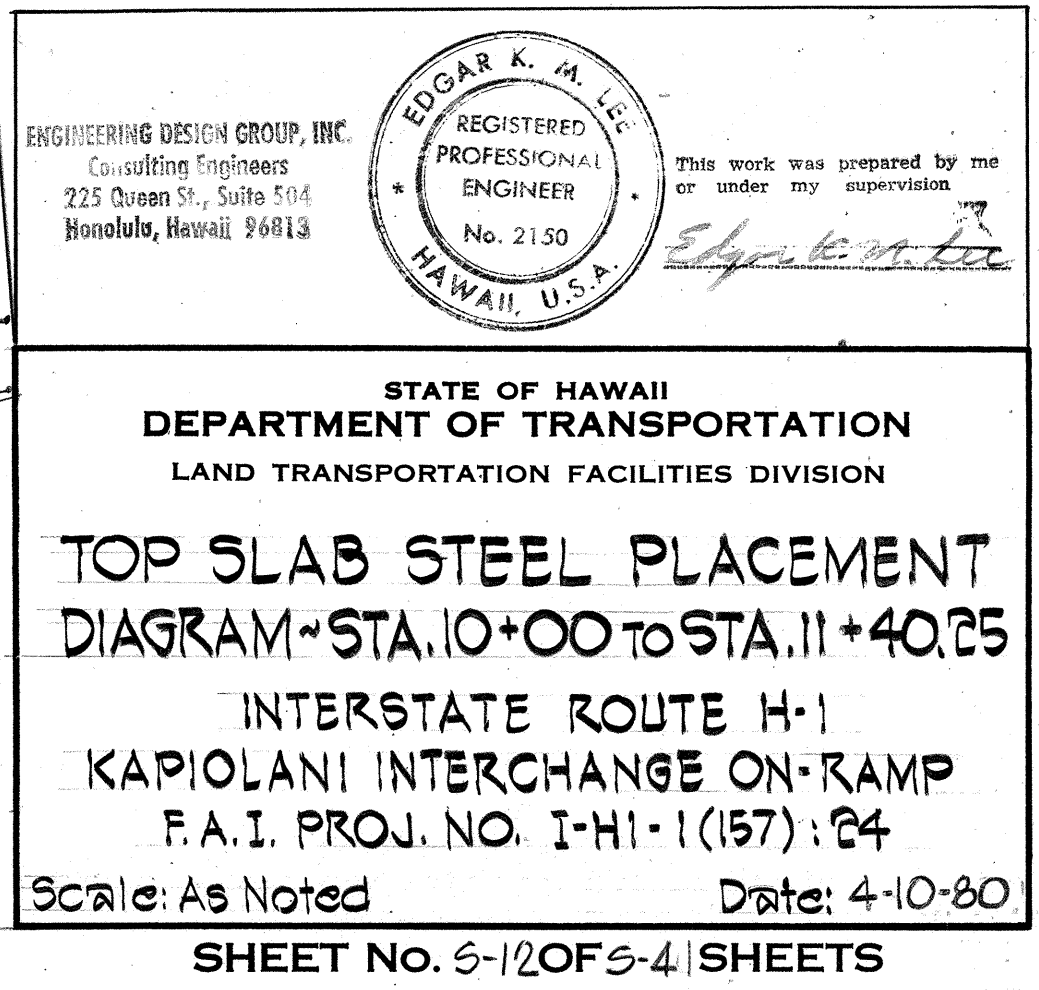
Edgar K. M. Lee

TOP SLAB STEEL PLACEMENT
DIAGRAM ~ STA. 7+00 TO STA. 10+00

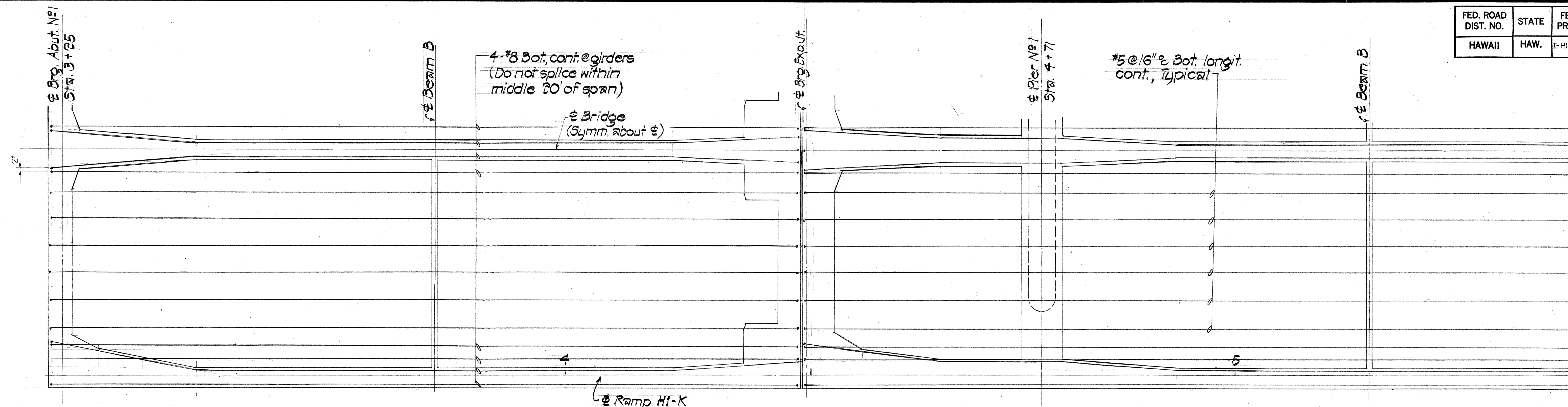
Scale: As Noted Date: 4-10-80

78

ORIGINAL PLAN	SURVEY PLOTTED BY _____	DATE _____
	DRAWN BY _____	" _____
	TRACED BY _____	" _____
NOTE BOOK	DESIGNED BY _____	" _____
	QUANTITIES BY _____	" _____
No. _____	CHECKED BY _____	" _____

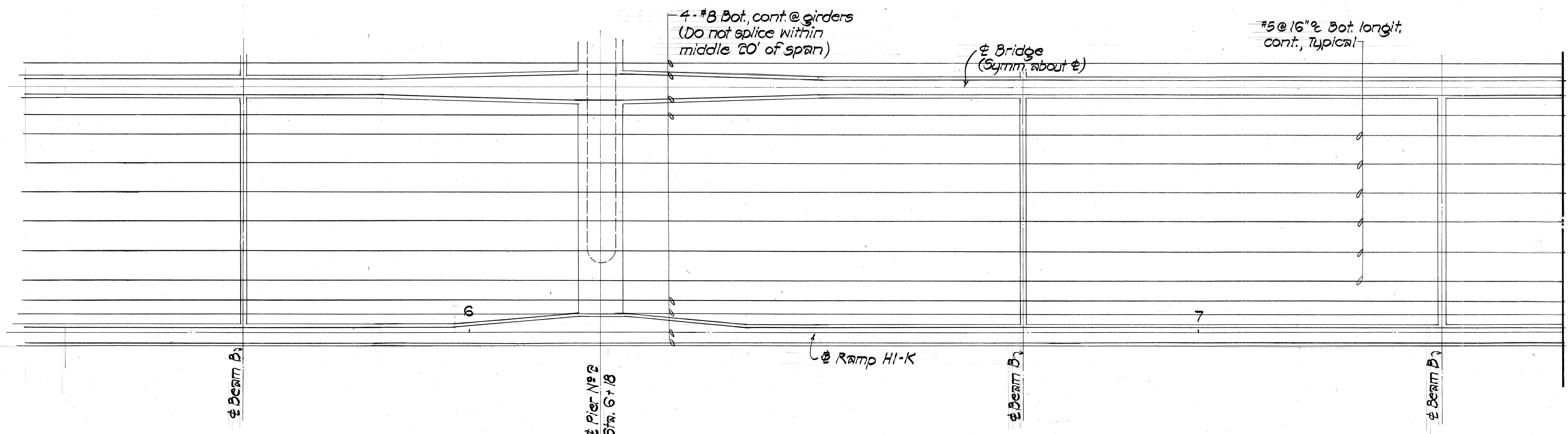


FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157):24	1979	80	146



BOTTOM SLAB STEEL PLACEMENT DIAGRAM (Bot. Layer Longit. Bars)

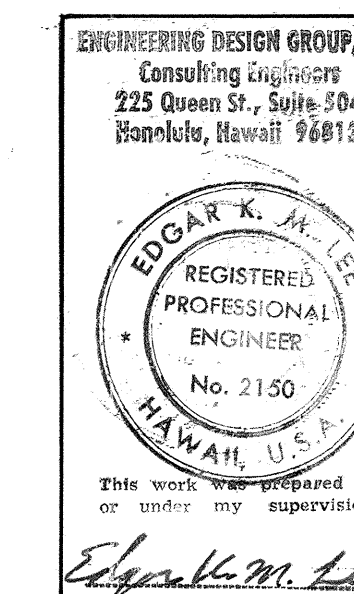
SCALES: Horiz. $\frac{1}{8}" = 1'-0"$
Vert. $\frac{3}{8}" = 1'-0"$



BOTTOM SLAB STEEL PLACEMENT DIAGRAM (Bot. Layer Longit. Bars)

SCALES: Horiz. $\frac{1}{8}" = 1'-0"$
Vert. $\frac{3}{8}" = 1'-0"$

NOTE:
See Notes on Sheet S-11.

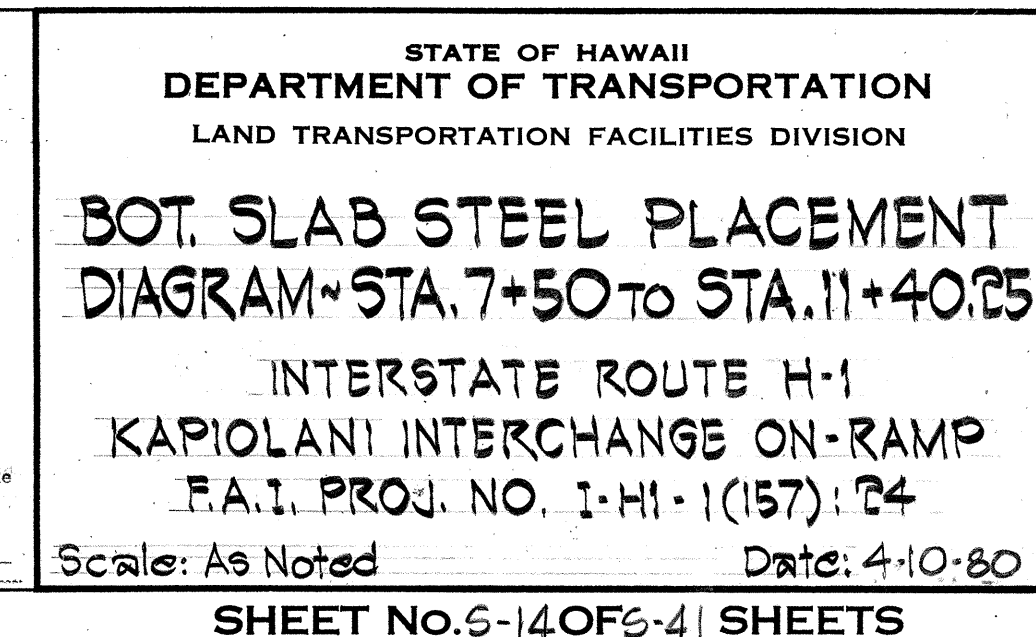


STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
**BOT. SLAB STEEL PLACEMENT
DIAGRAM ~ STA. 3+25 TO STA. 7+50**
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157):24
Scale: As Noted Date: 4-10-80
SHEET No. 6-13 OF 6-4 SHEETS

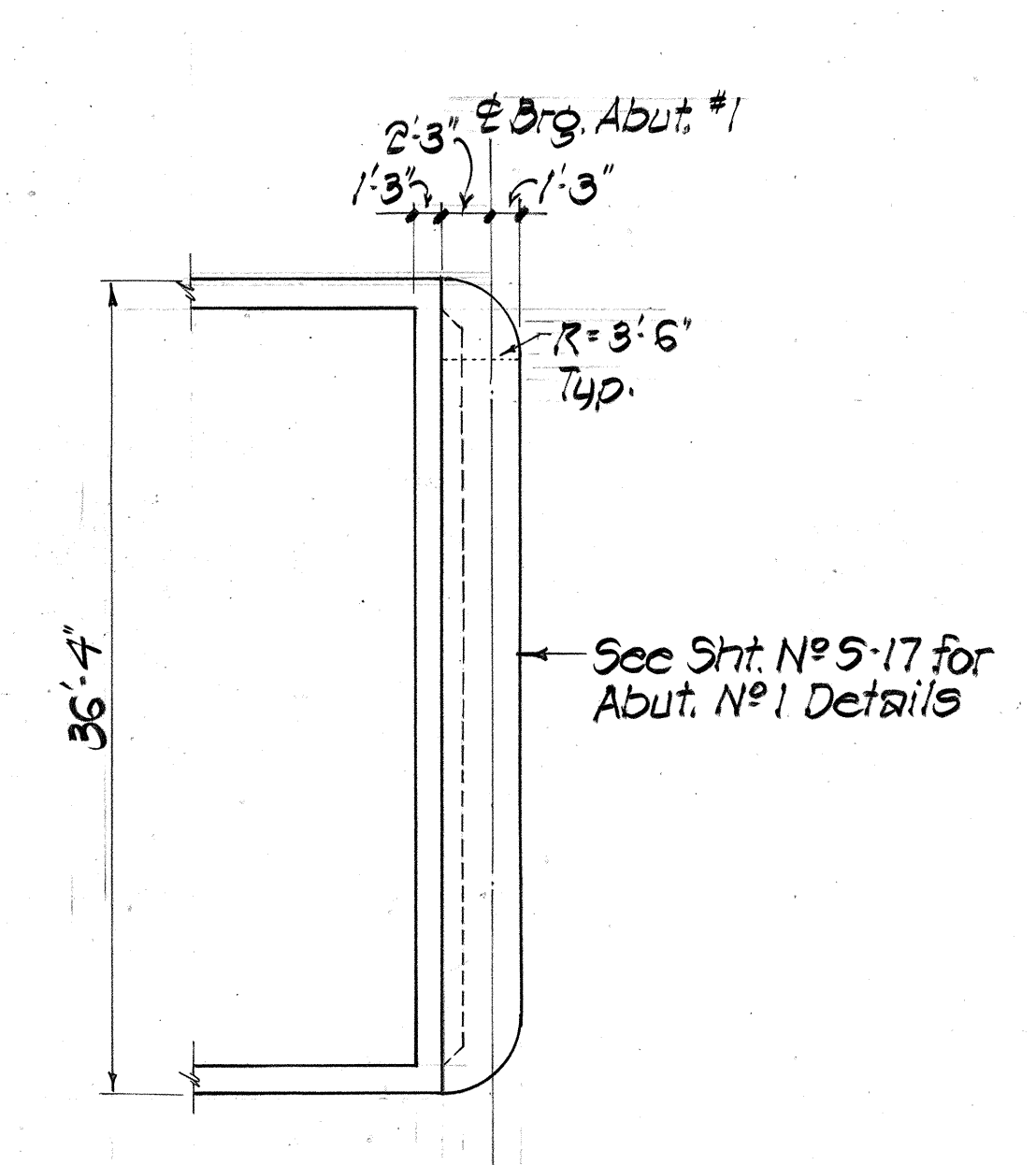
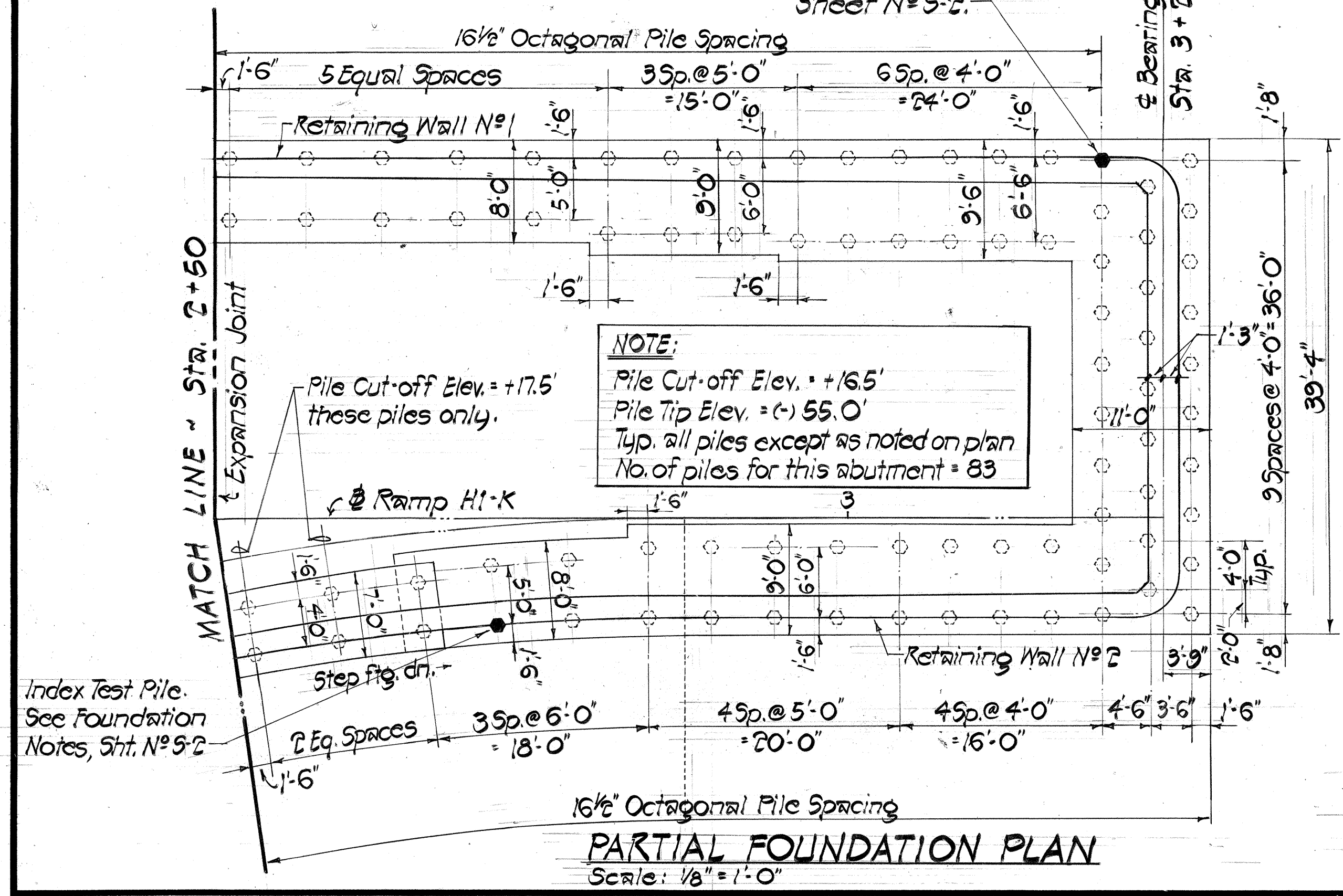
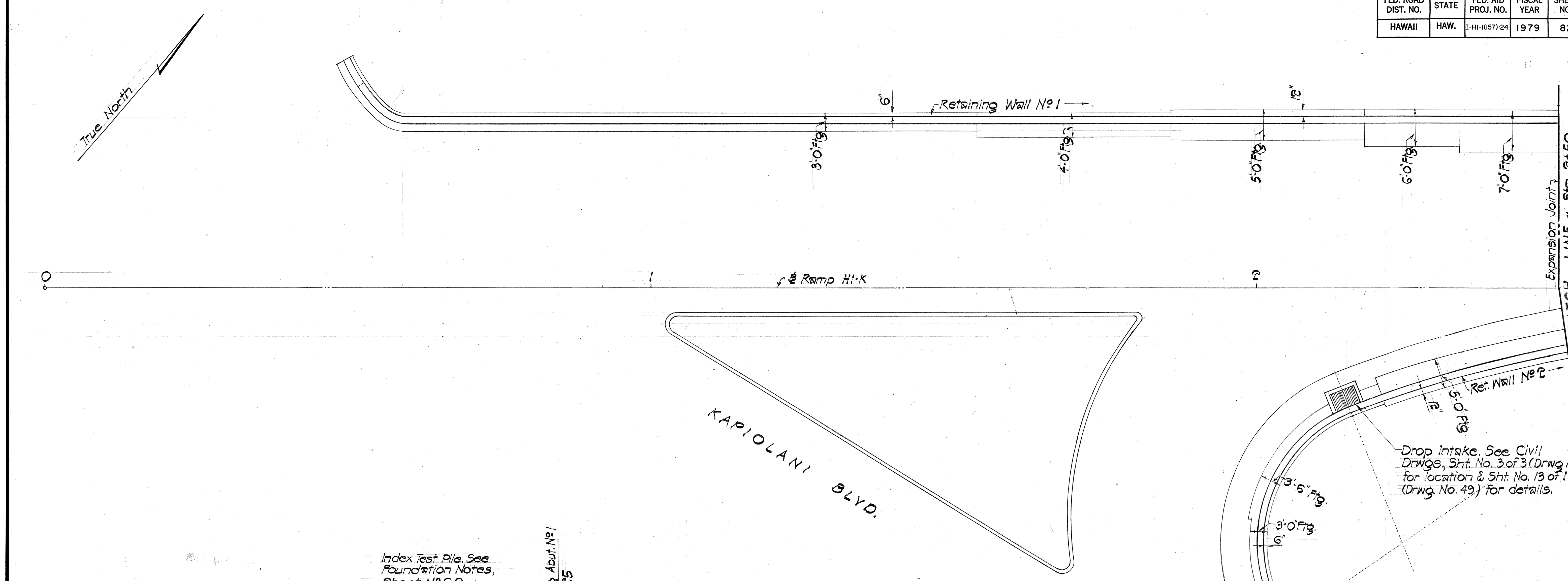
MATCH LINE ~ STA. 7+50 see Sht. No S-13



4. Designing EX/5
Abutment + No. 2
Sta. 11 + 40.25



FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	82	146



PARTIAL FOUNDATION PLAN
 Scale: 1/8" = 1'-0"

NOTE:
 1. See Sht. No. 5-16 for Retaining Wall Elevations.
 2. See Sht. No. 5-34 for Retaining Wall Details

PARTIAL PLAN-SECTION ABOVE BRIDGE BEARINGS
 Scale: 1/8" = 1'-0"

ORIGINAL PLAN	SURVEY PLOTTED BY	DATE
NO.	DRAWN BY	
	TRACED BY	
	DESIGNED BY	
	QUANTITIES BY	
	CHECKED BY	

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 LAND TRANSPORTATION FACILITIES DIVISION

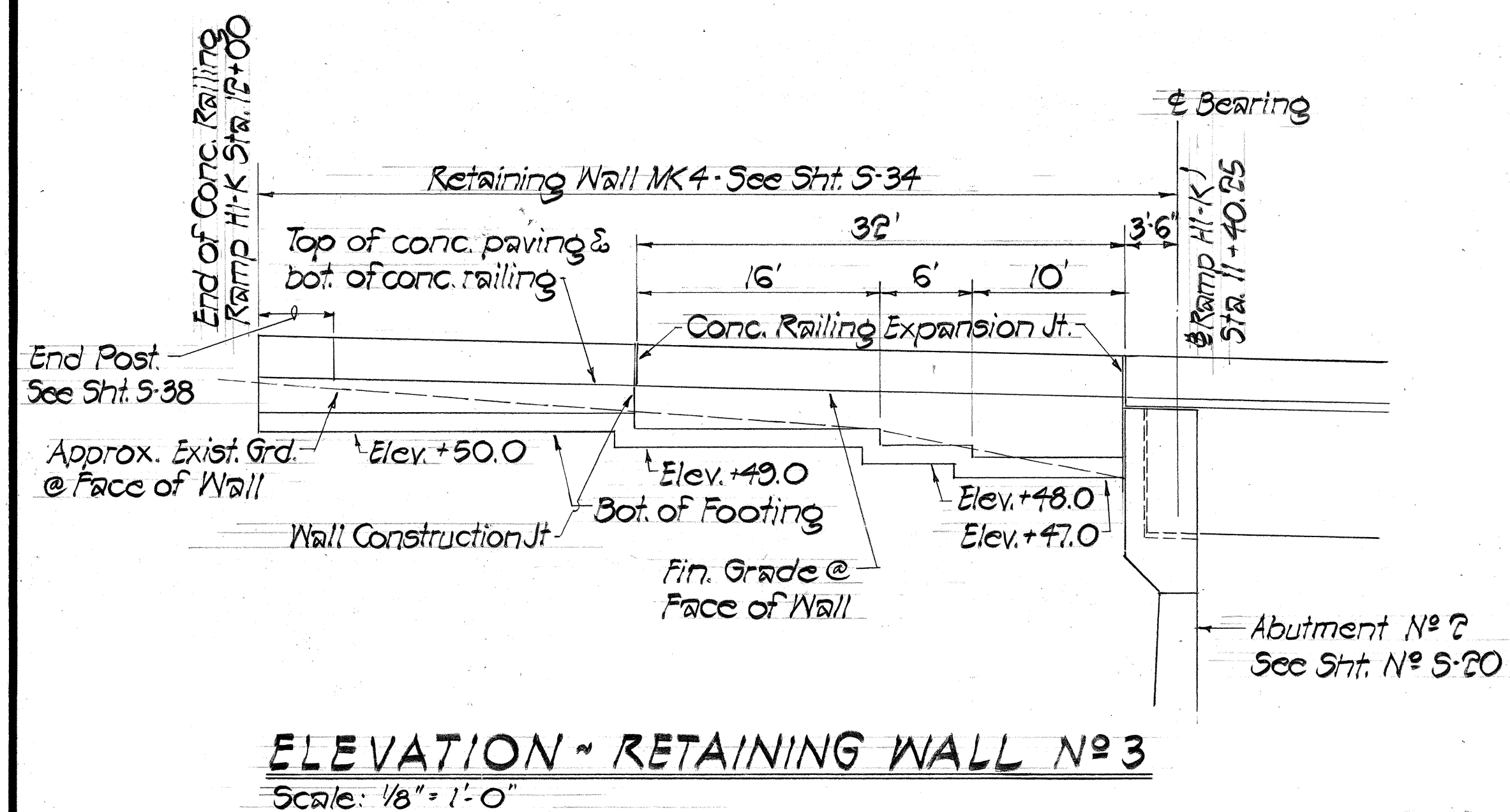
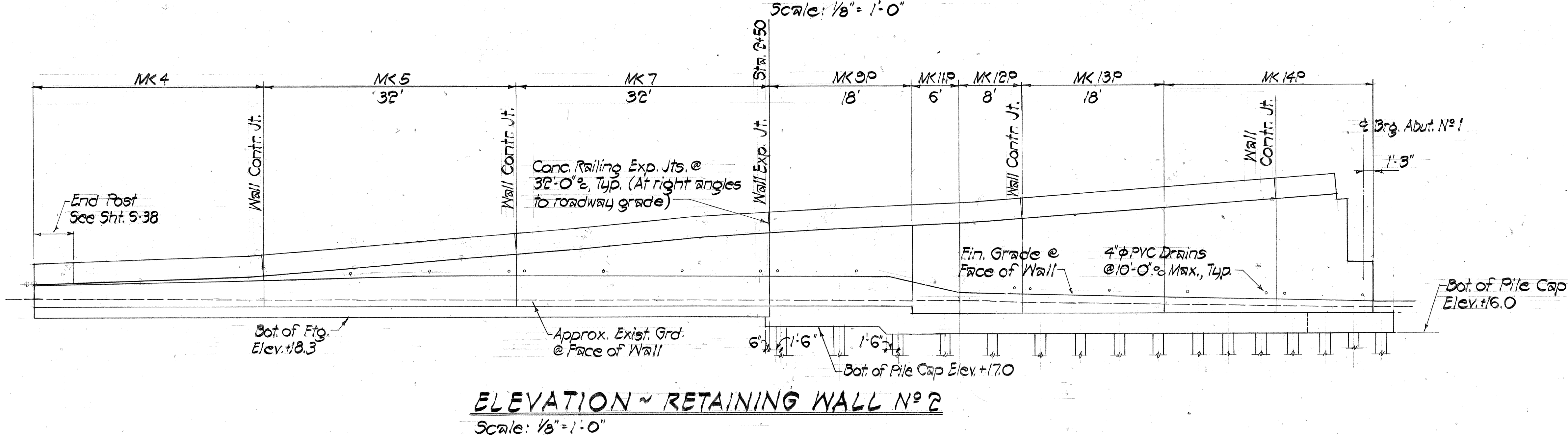
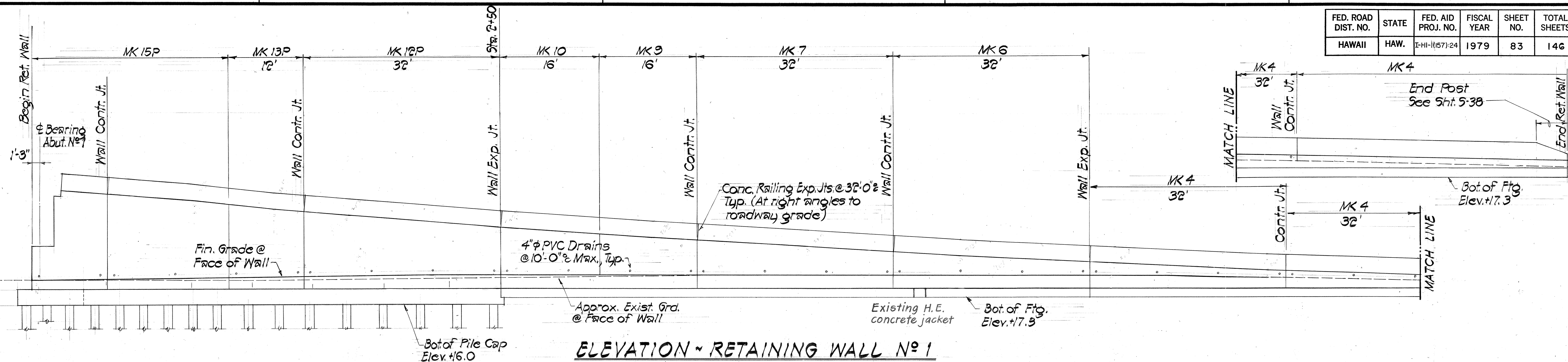
**ABUTMENT No. 1
 FOUNDATION PLAN**

INTERSTATE ROUTE H-1
 KAPIOLANI INTERCHANGE ON-RAMP
 F.A.I. PROJ. NO. I-HI-1(157)-24

Scale: As Noted Date: 4-10-80

SHEET No. 5-15 OF 5-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-H1-1(157)-24	1979	83	146



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LAND TRANSPORTATION FACILITIES DIVISION

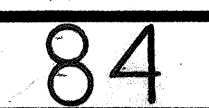
**ABUTMENT N° 1 & 2
RETAINING WALL ELEVATIONS**

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. 1-H1-1(157)-24

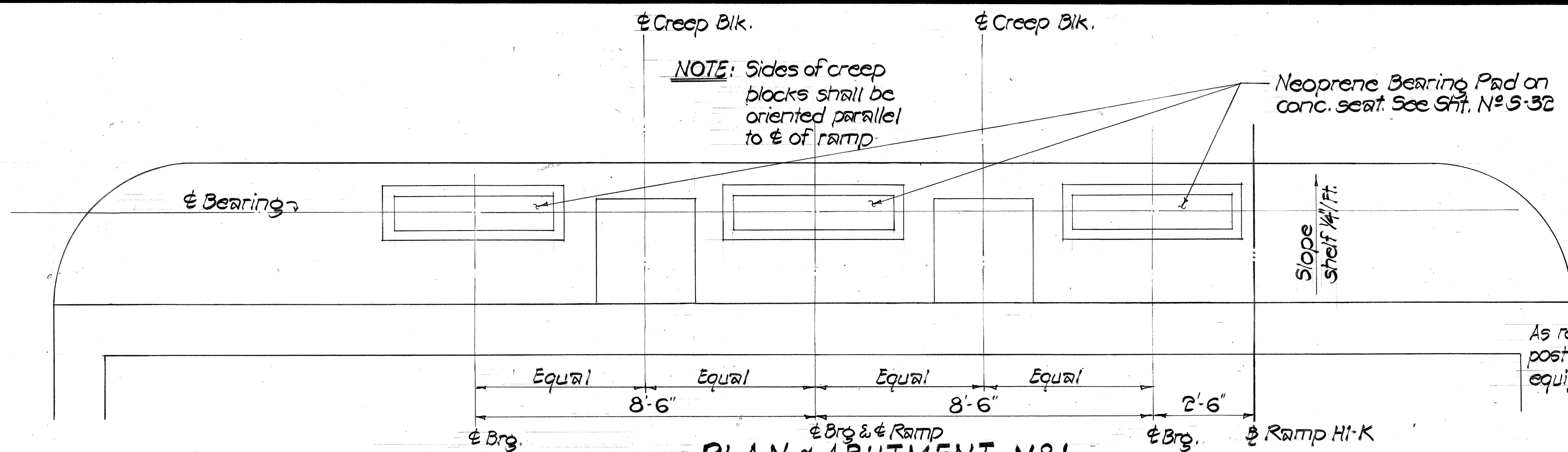
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SHEET No. 6-16 OF 3-41 SHEETS

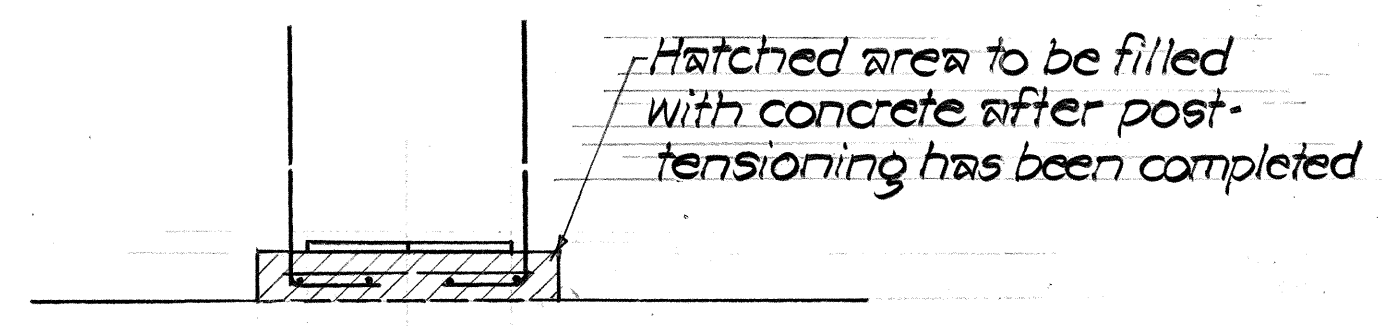
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NOTE BOOK	DRAWN BY _____	_____
	TRACED BY _____	_____
	DESIGNED BY _____	_____
	QUANTITIES BY _____	_____
No. _____	CHECKED BY _____	_____



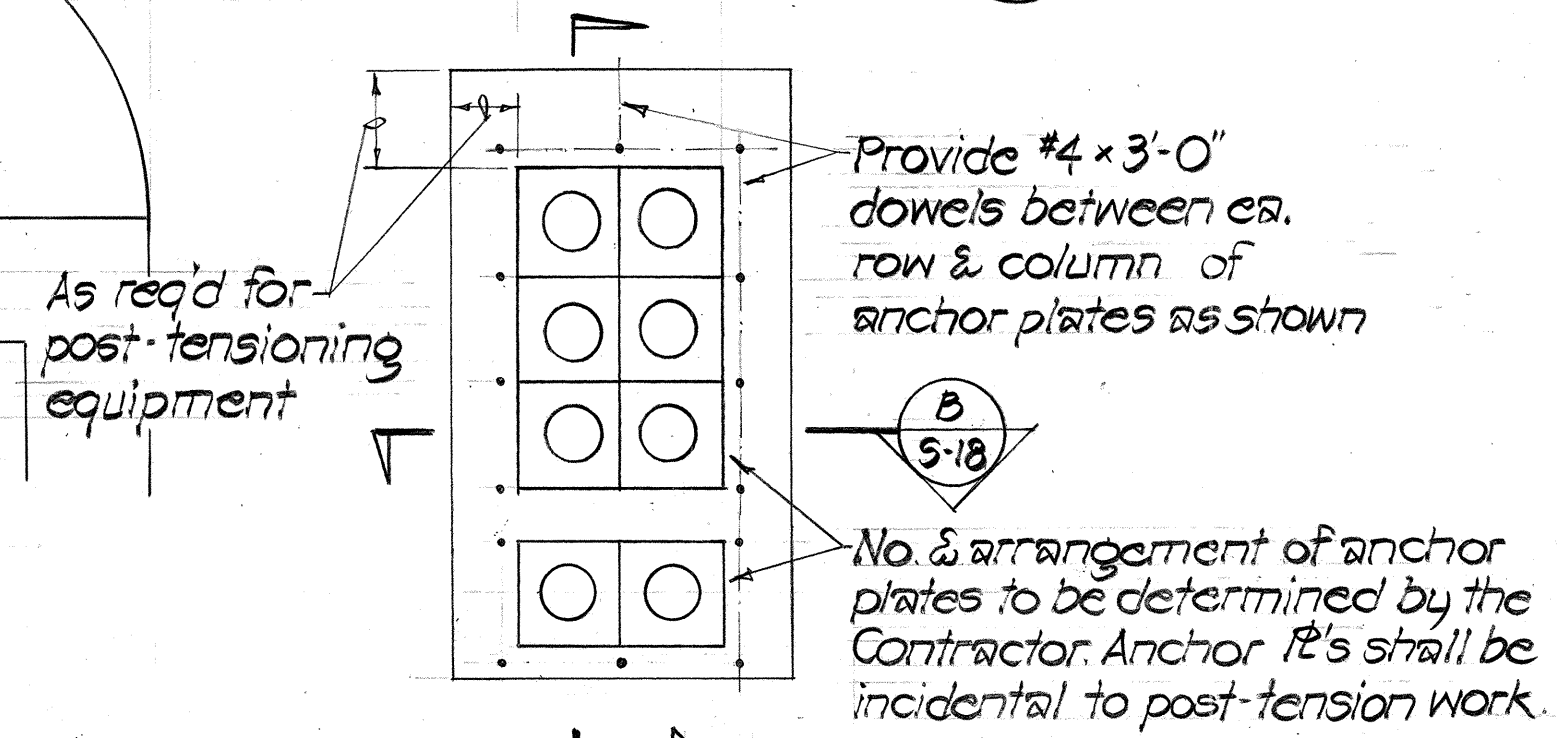
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	85	146



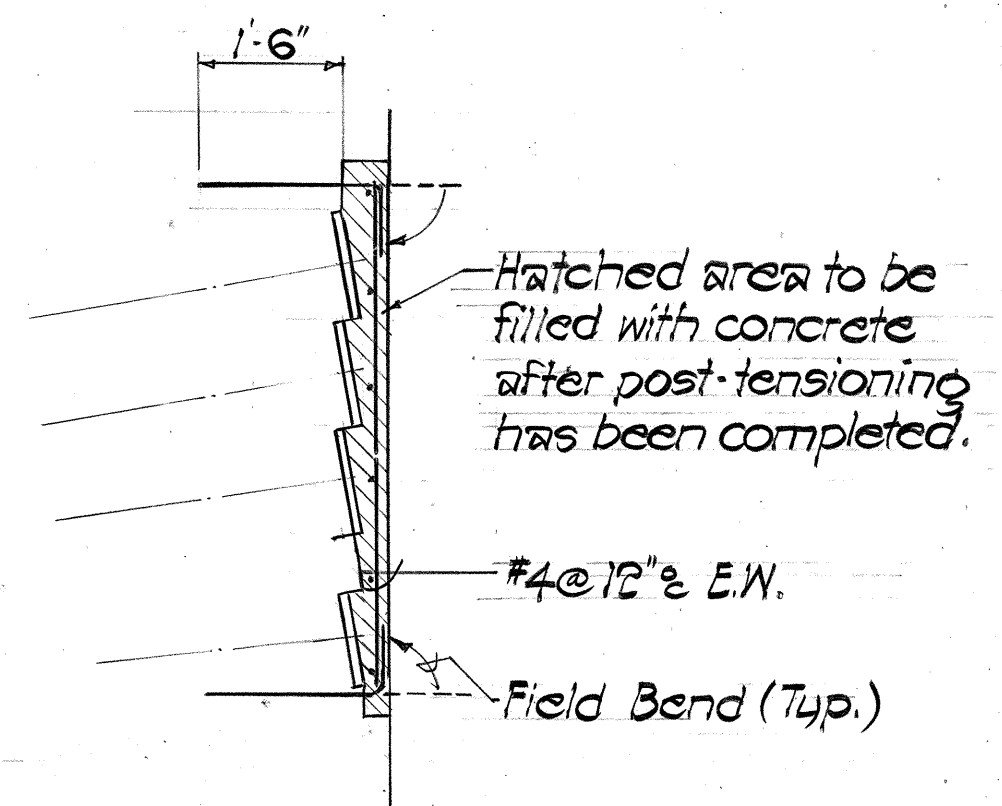
PLAN - ABUTMENT NO. 1
Scale: 1/2" = 1'-0"



PLAN - SECTION B
5-18



ELEVATION
5-18

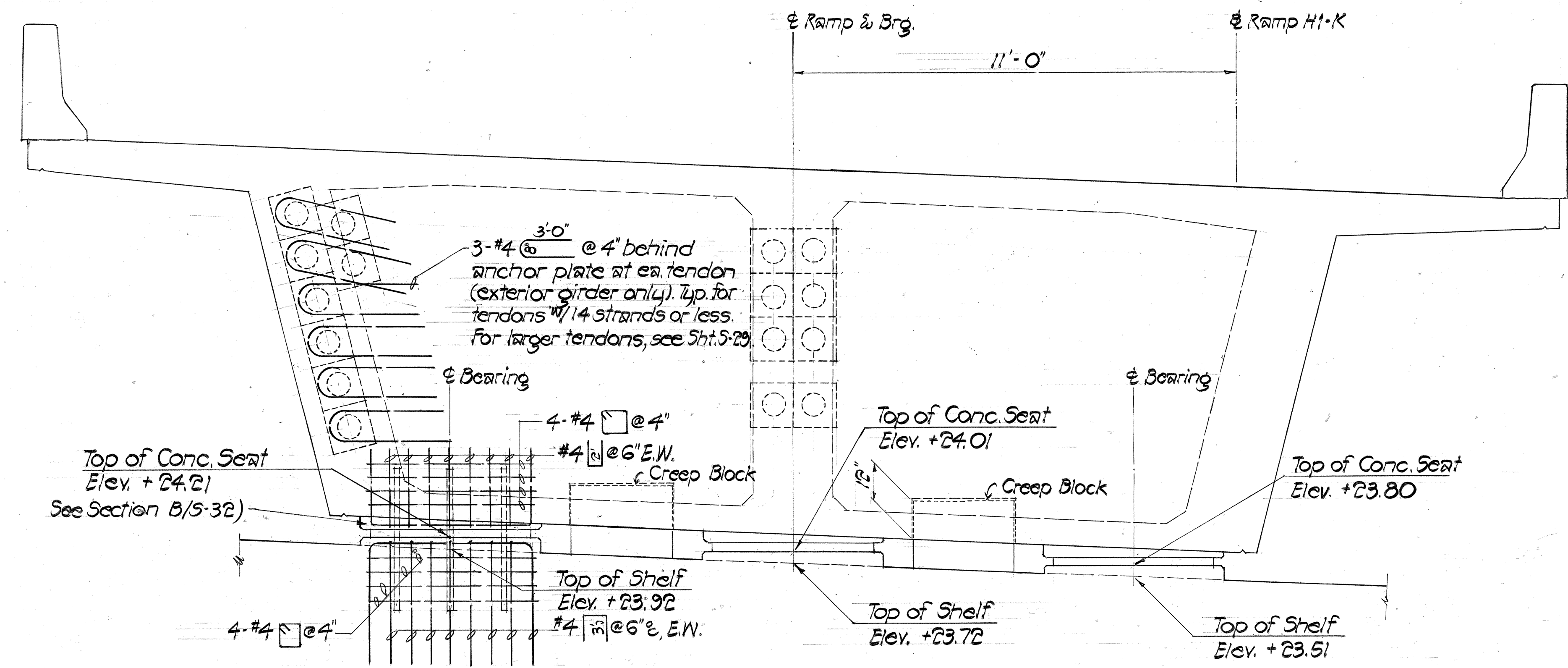


SECTION C
5-18

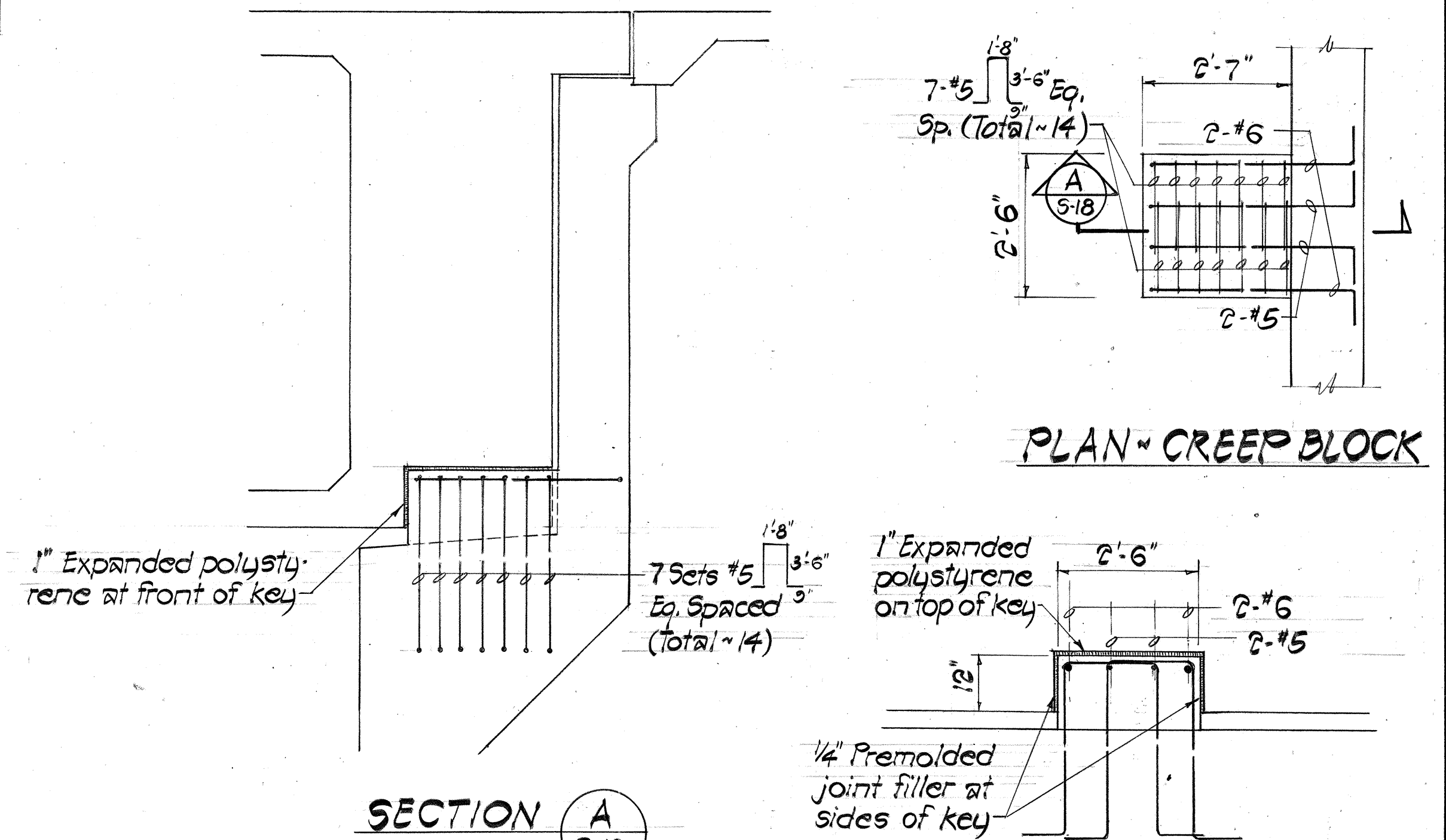
NOTE:
Cost of conc. patch in back of anchor plates shall be incidental to concrete work.

TYP. BLOCKOUT DETAIL @ POST-TENSIONING ANCHORS

Scale: 1/2" = 1'-0"



ELEVATION - ABUTMENT NO. 1
Scale: 1/2" = 1'-0"



PLAN - CREEP BLOCK

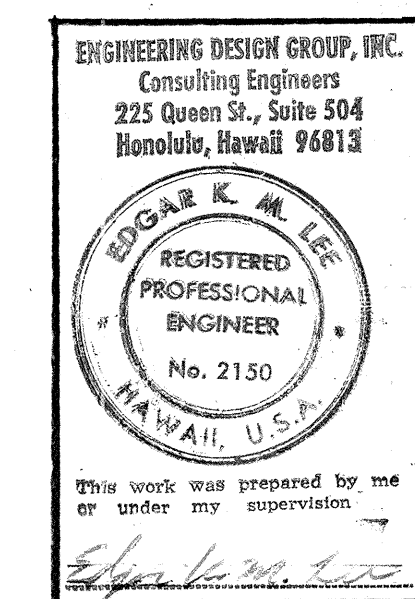
SECTION A
5-18

FRONT ELEVATION

TYPICAL CREEP BLOCK DETAIL

Scale: 1/2" = 1'-0"

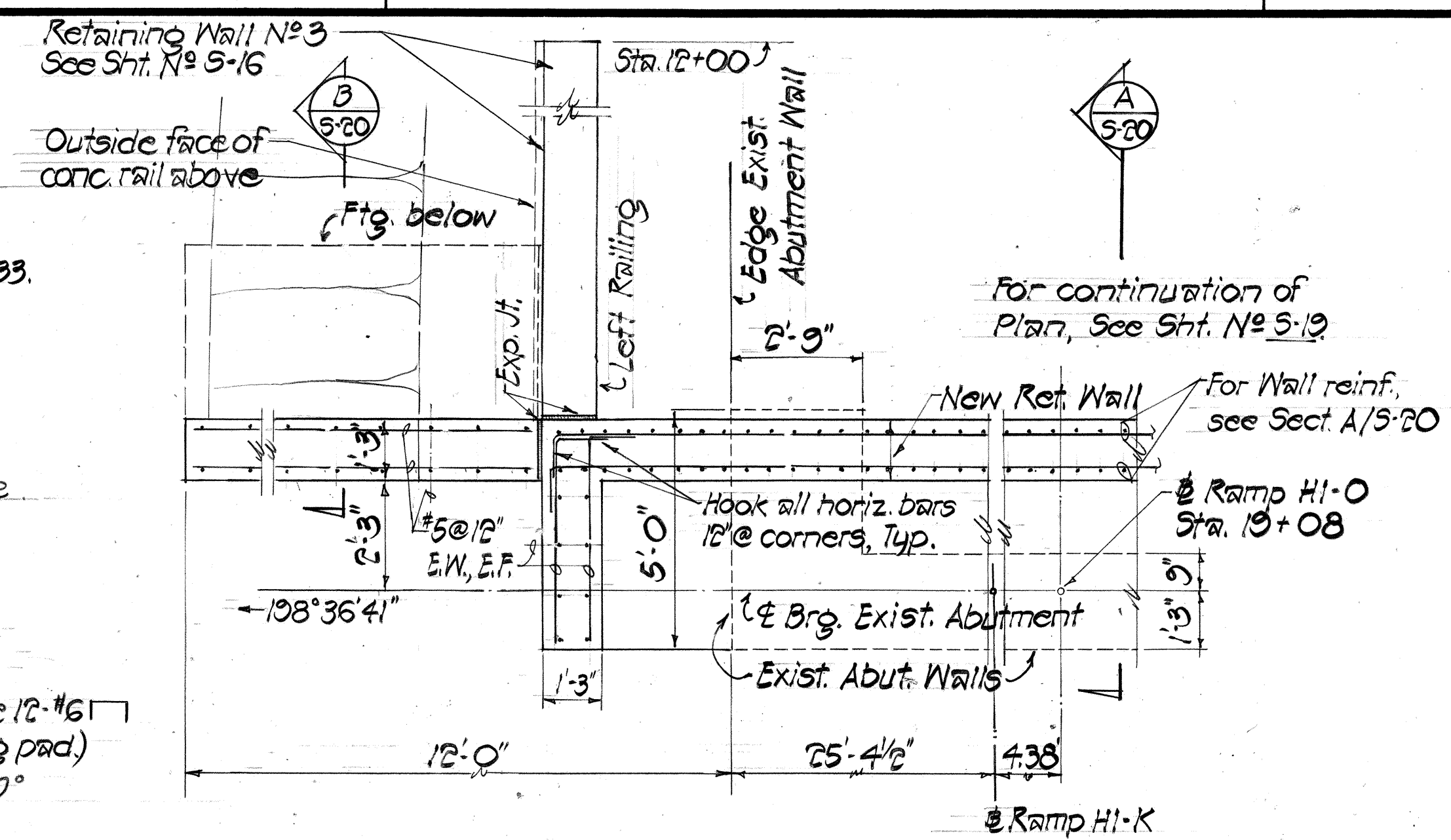
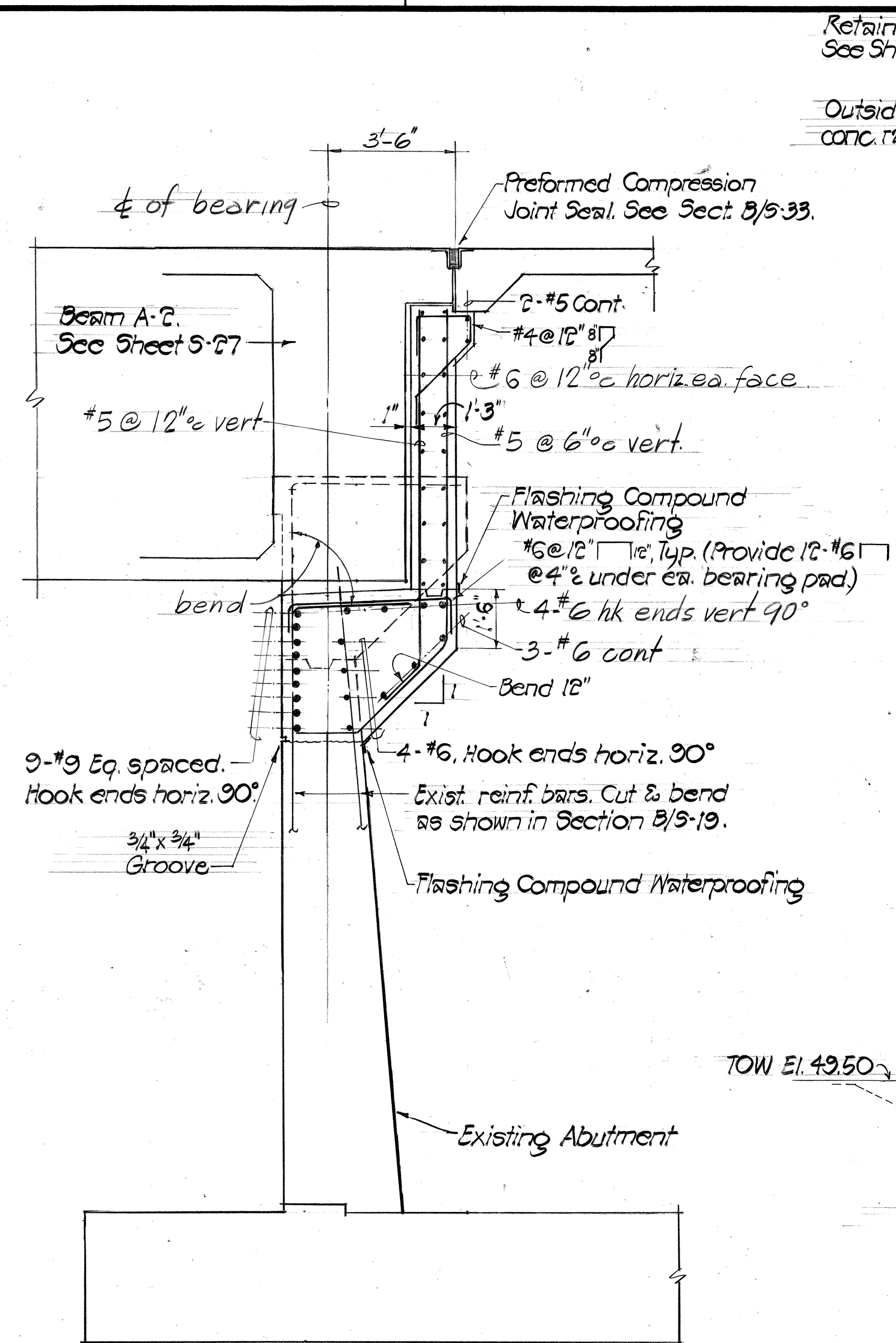
DATE	_____
SURVEY PLOTTED BY	_____
DRAWN BY	_____
TRACED BY	_____
DESIGNED BY	_____
QUANTITIES BY	_____
CHECKED BY	_____
ORIGINAL PLAN	_____
NOTE BOOK	_____
No.	_____



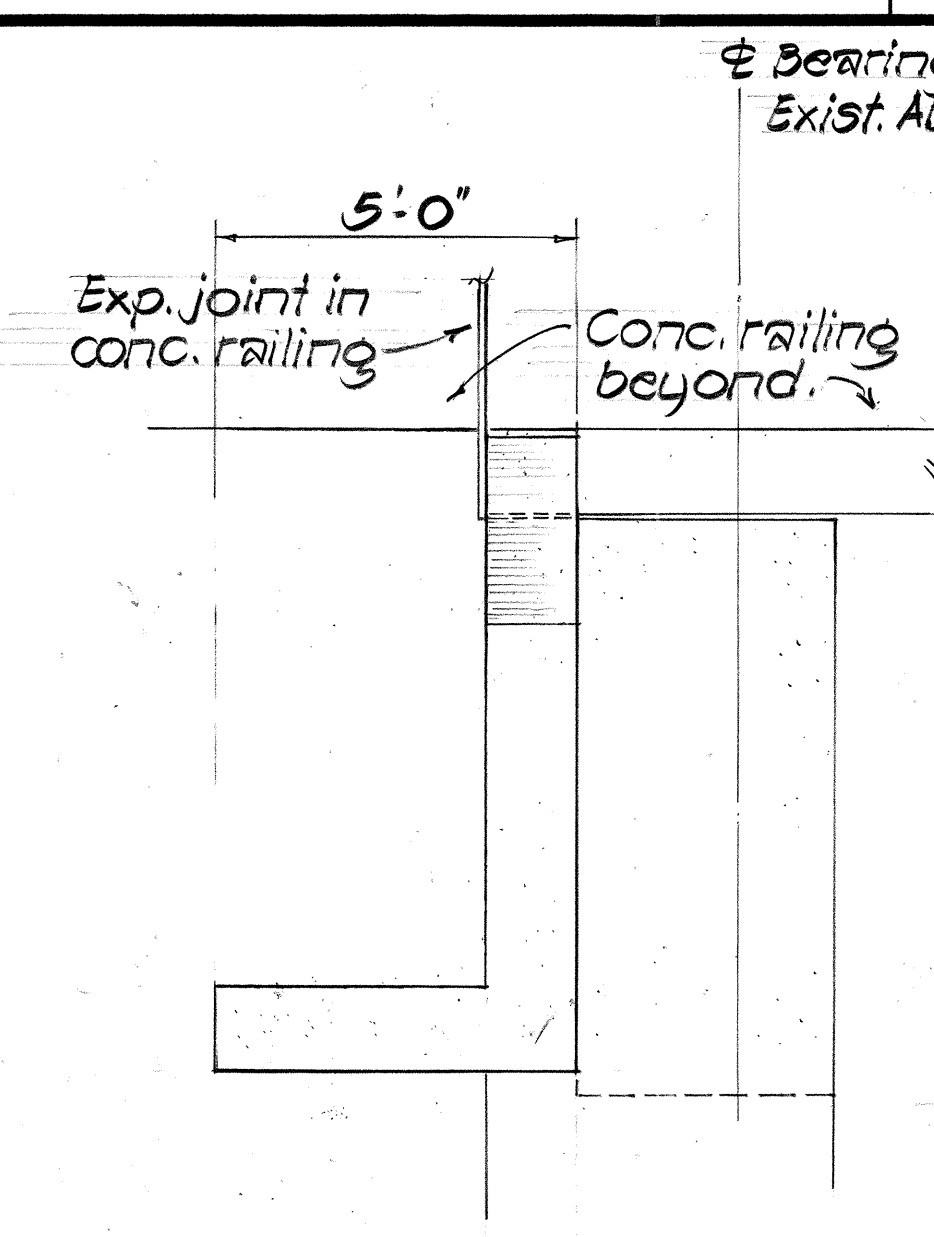
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
TYPICAL ABUTMENT PLAN & ELEVATION
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24
Scale: As Noted Date: 4-10-80
SHEET NO. 5-18 OF 6-41 SHEETS

86

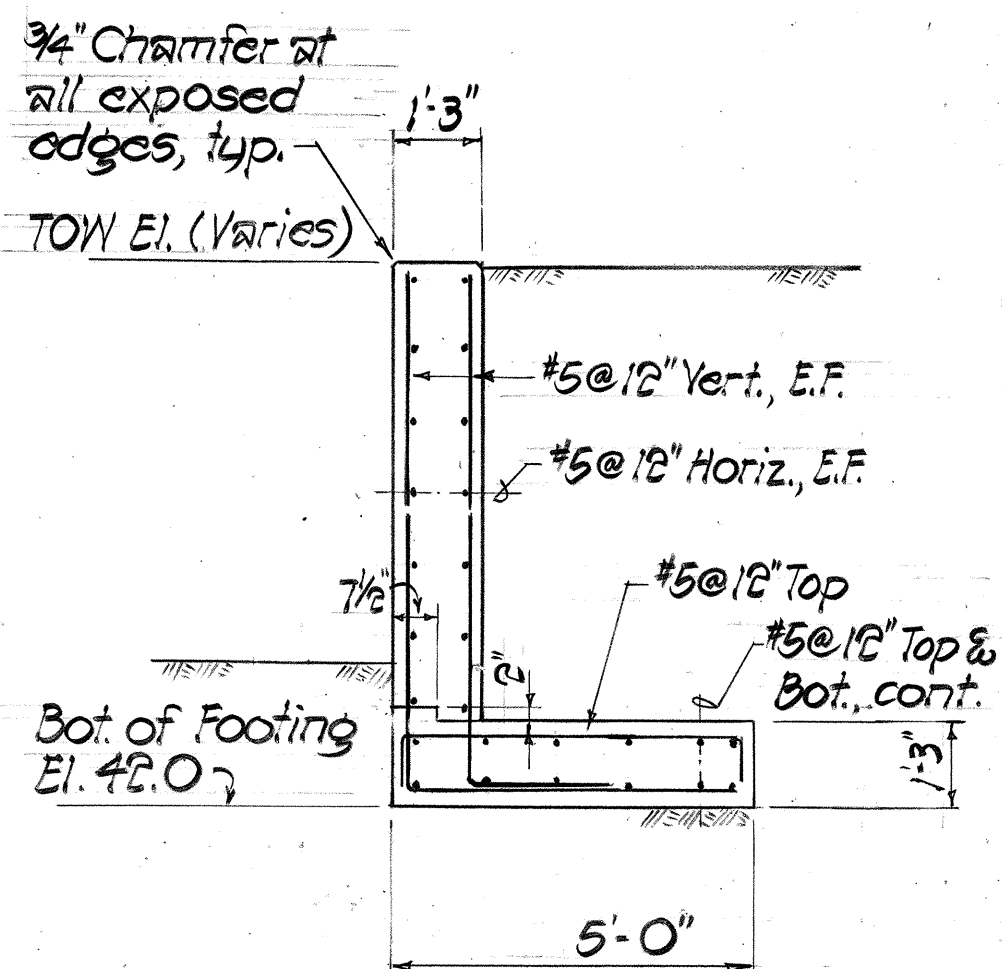
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157):24	1979	87	146



PLAN AT EXIST. ABUTMENT
Scale: 3/8" = 1'-0"



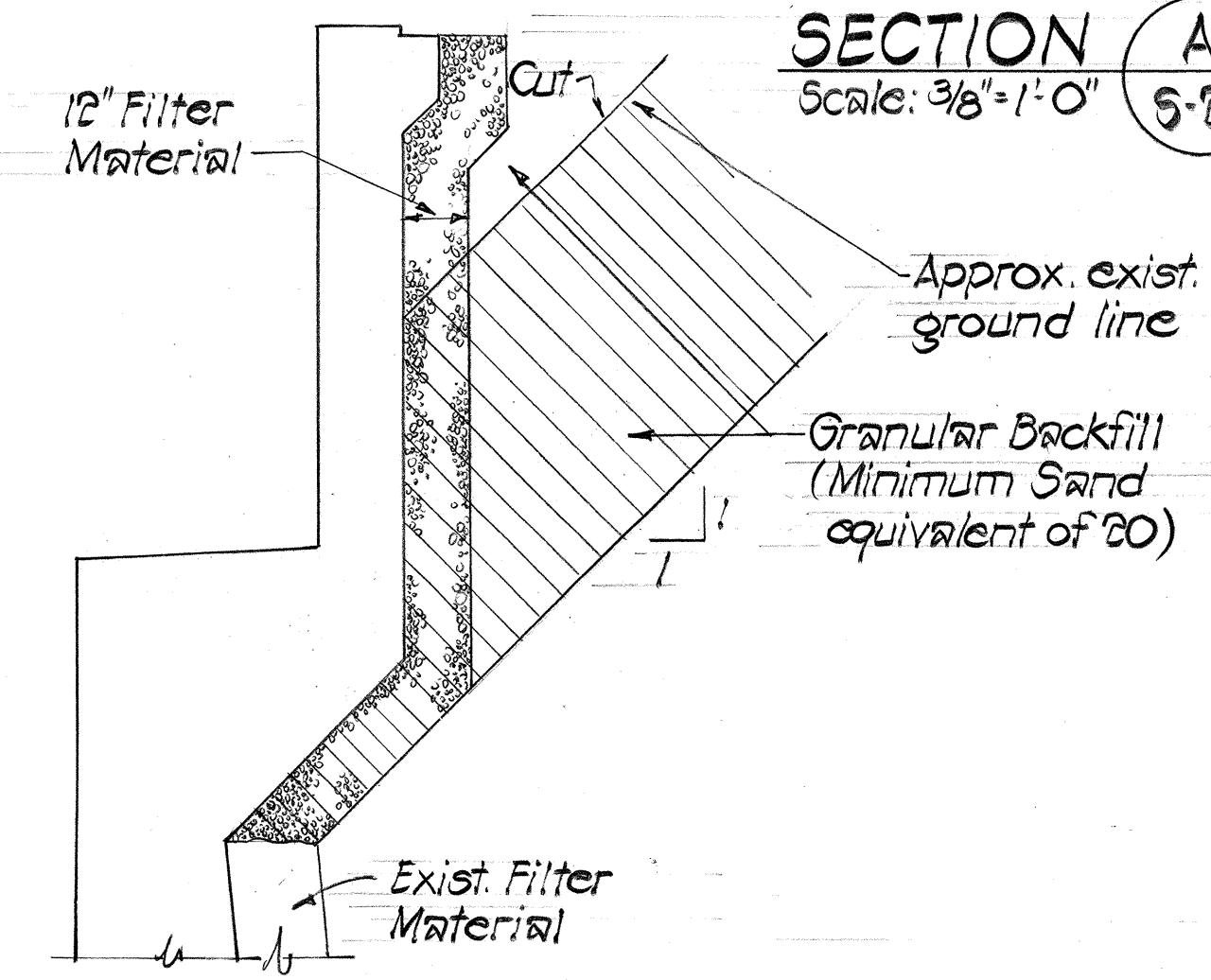
WINGWALL ELEVATION
Scale: 3/8" = 1'-0"



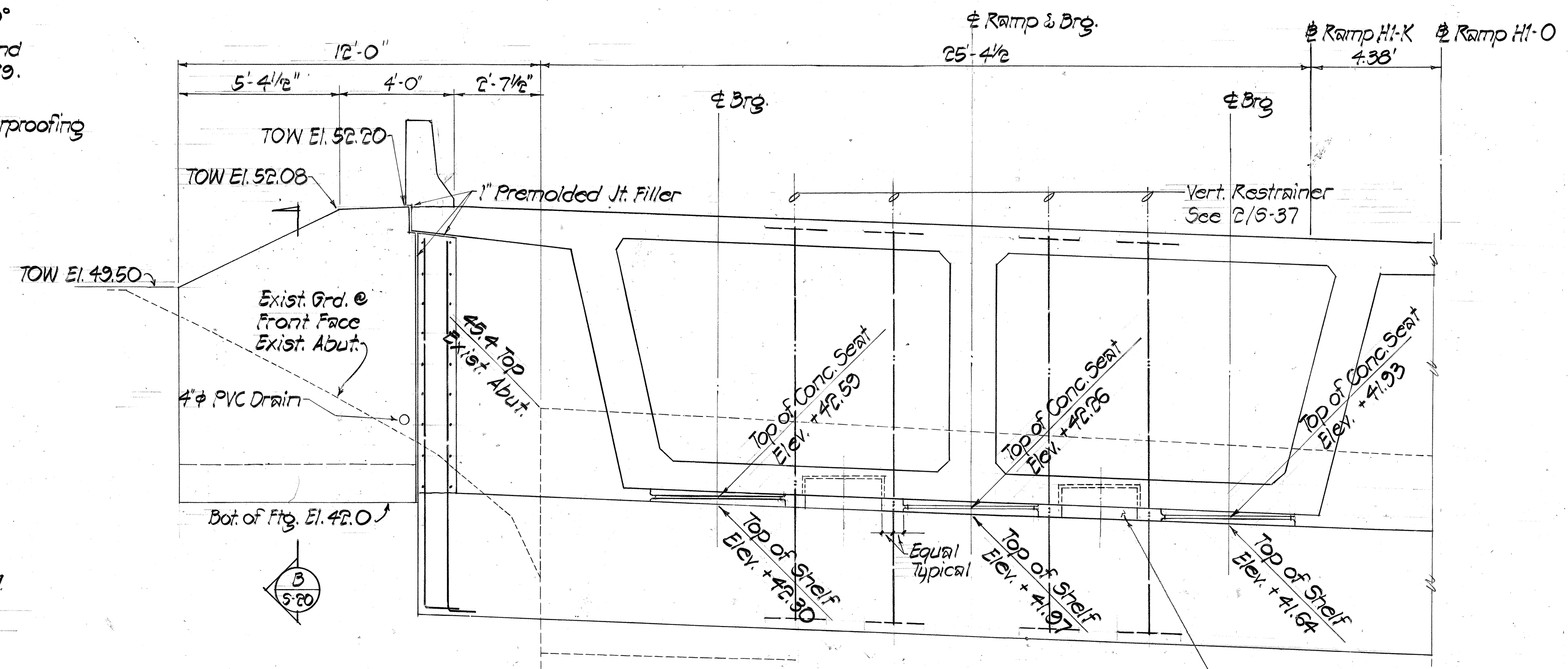
SECTION B
Scale: 3/8" = 1'-0"

NOTE:
For dimensions & details not noted, see S-27

SECTION A
Scale: 3/8" = 1'-0"



LIMITS OF STRUCTURAL EXCAVATION & BACKFILL



ELEVATION AT EXIST. ABUTMENT
Scale: 3/8" = 1'-0"

DATE	_____
SURVEY PLOTTED BY	_____
DRAWN BY	_____
CHECKED BY	_____
NOTE BOOK	_____
NO.	_____

ENGINEERING DESIGN GROUP, INC.
Consulting Engineers
225 Queen St., Suite 504
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EDGAR K. M. LEE
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No. 2150
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This work was prepared by me or under my supervision.

Edgar K. M. Lee

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

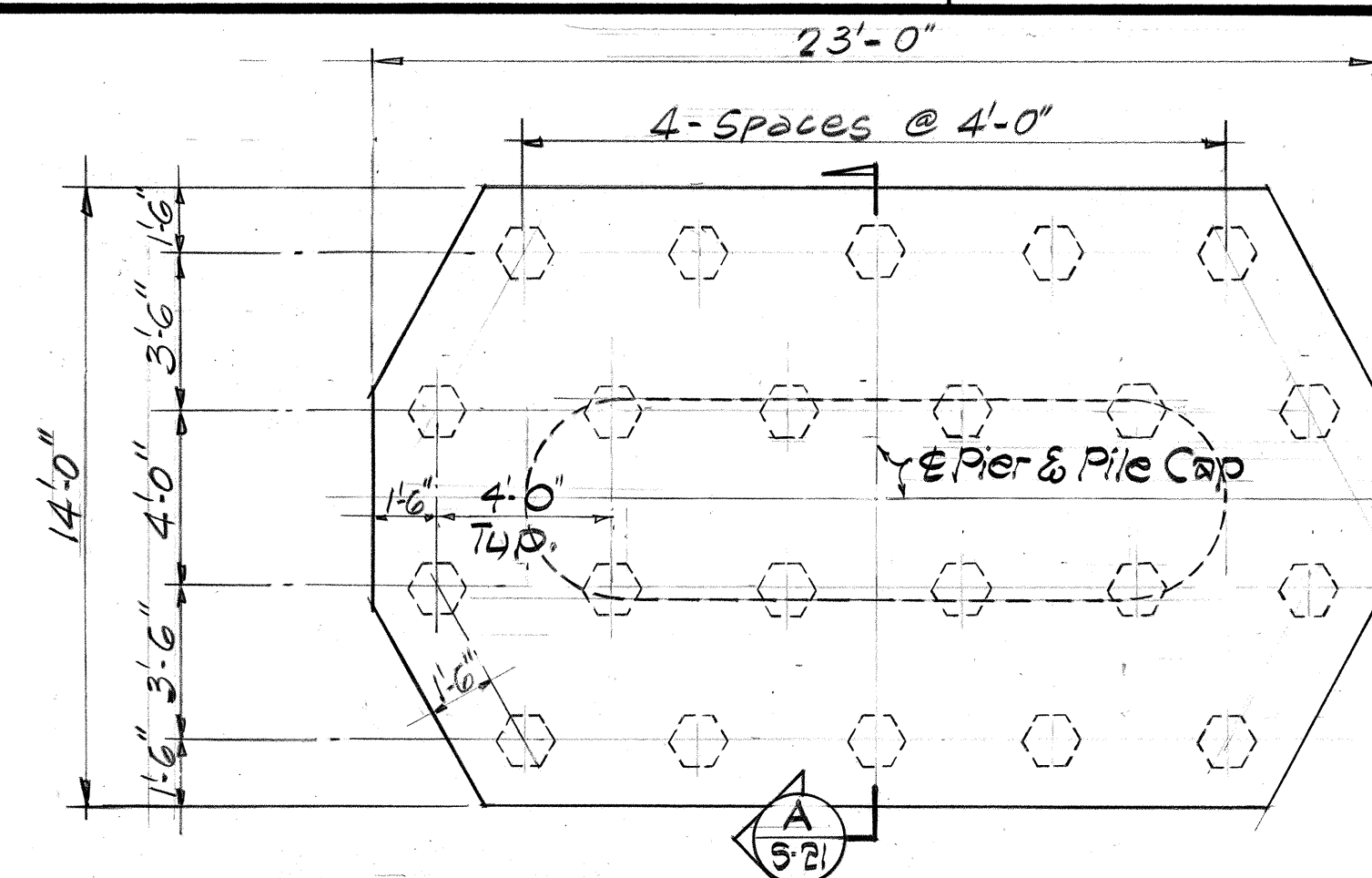
ABUTMENT No. 2
SECTIONS & DETAILS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157):24

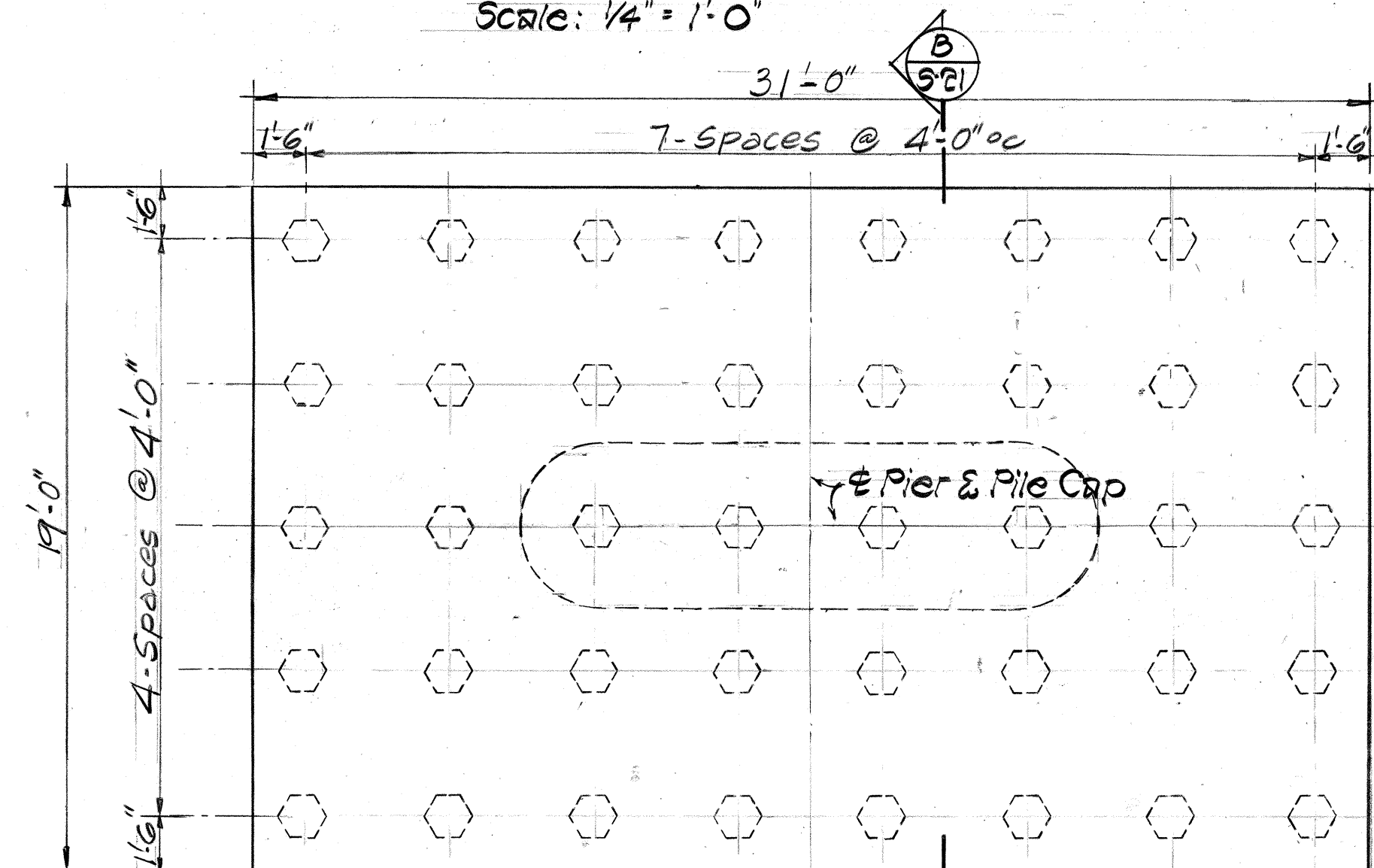
Scale: As Noted Date: 4-10-80

SHEET No. 52 OF 541 SHEETS

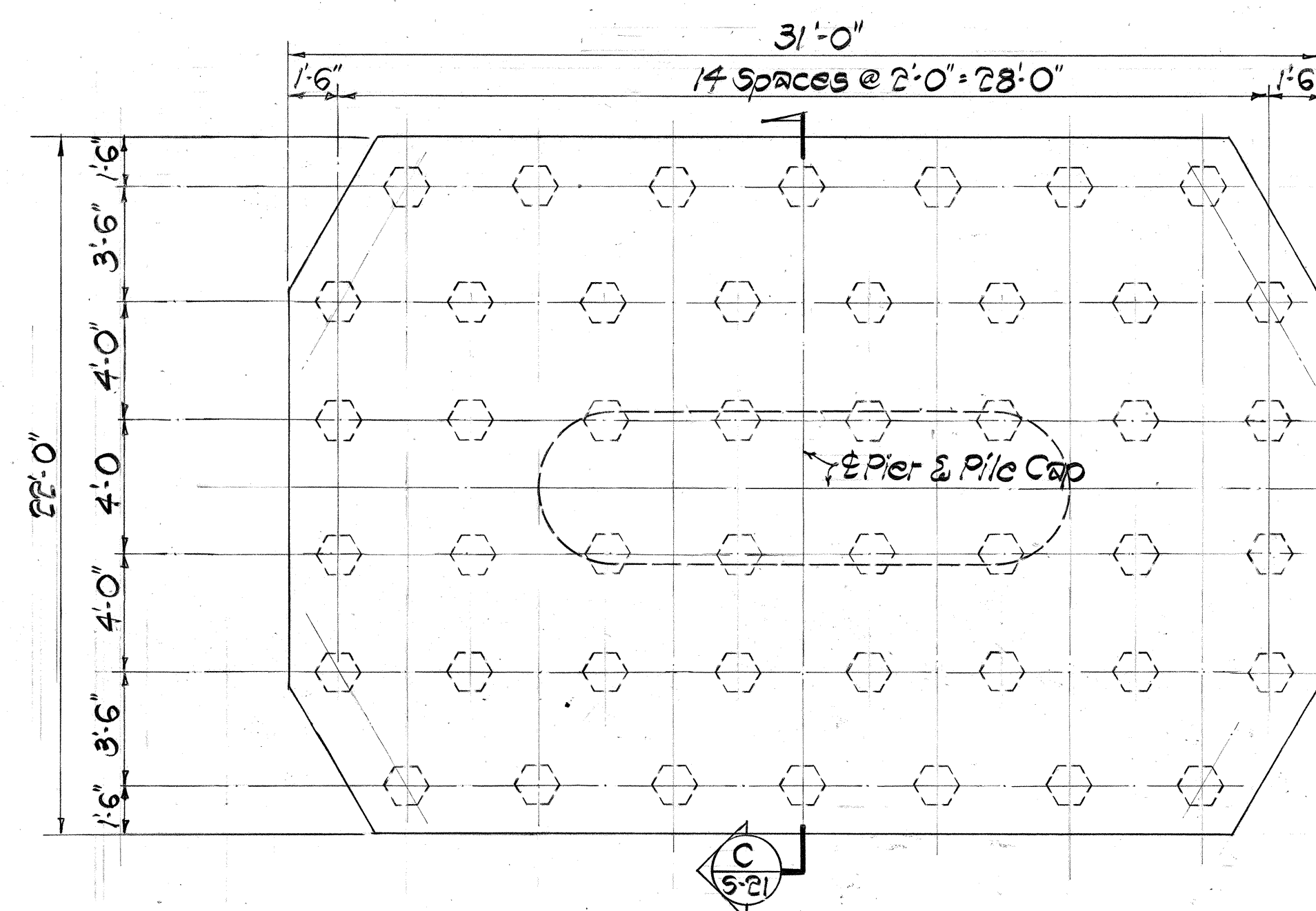
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)24	1979	88	146



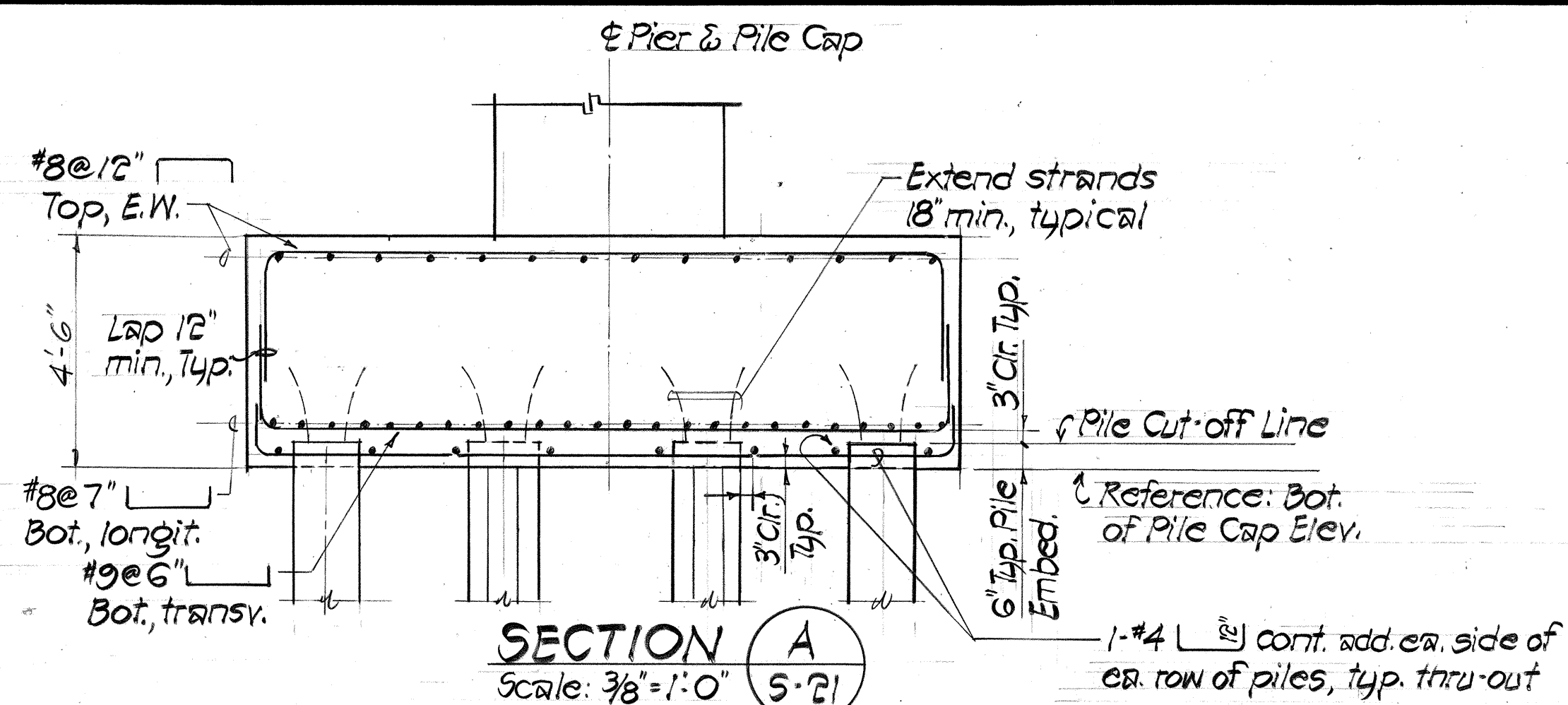
PLAN - TYPE I PILE CAP
Scale: 1/4" = 1'-0"



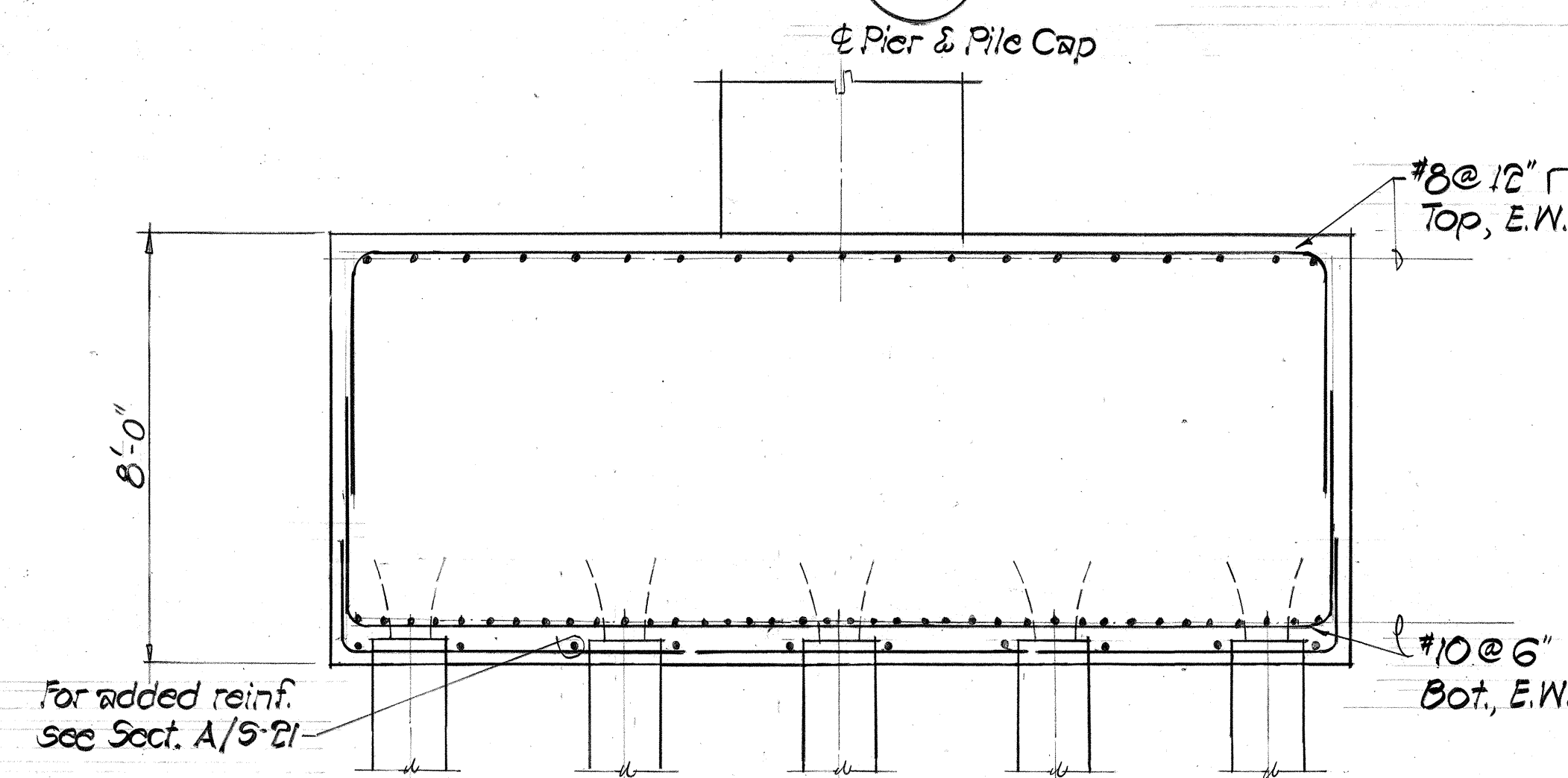
PLAN - TYPE II PILE CAP
Scale: 1/4" = 1'-0"



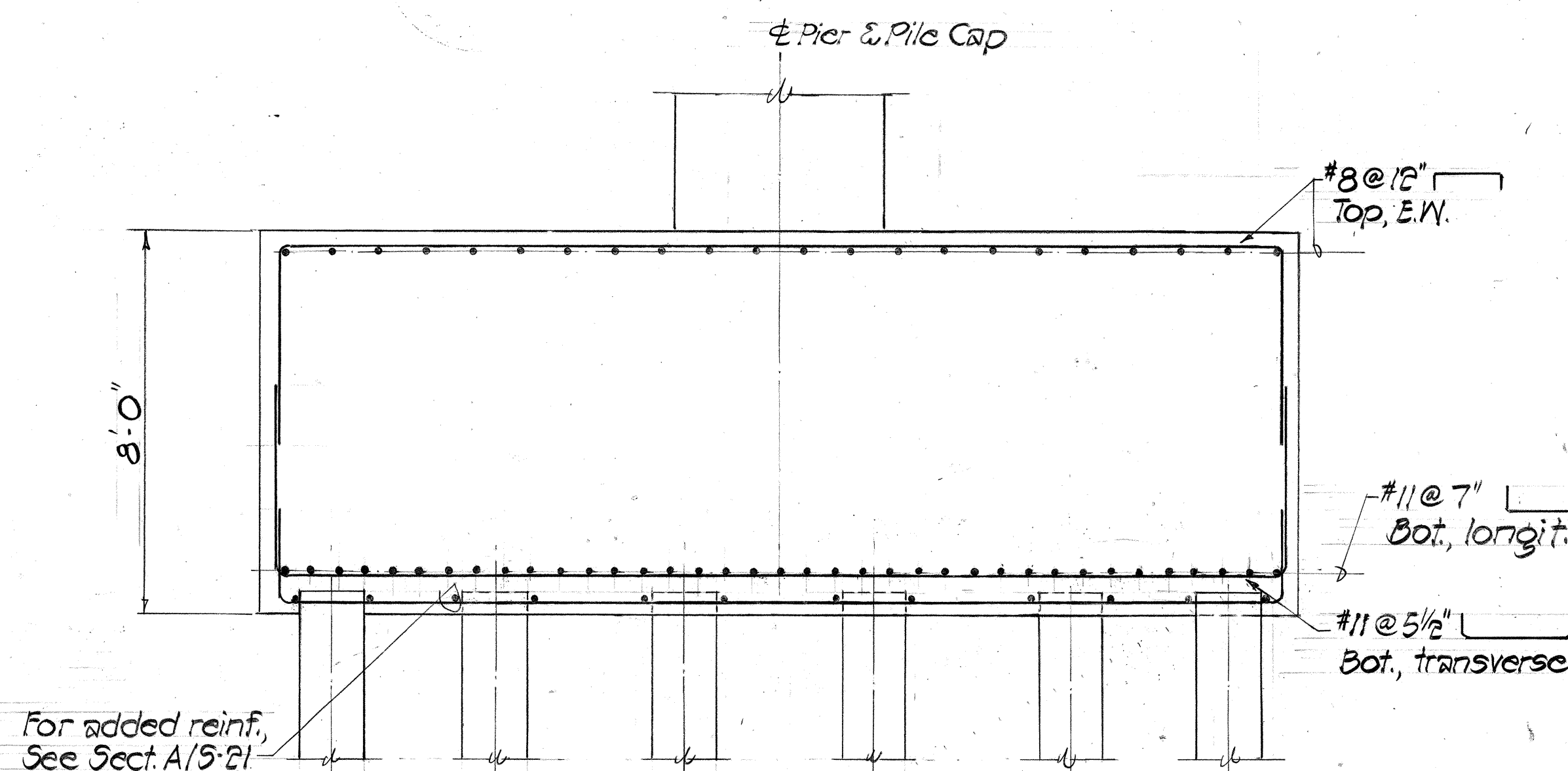
PLAN - TYPE III PILE CAP
Scale: 1/4" = 1'-0"



SECTION A
Scale: 3/8" = 1'-0"



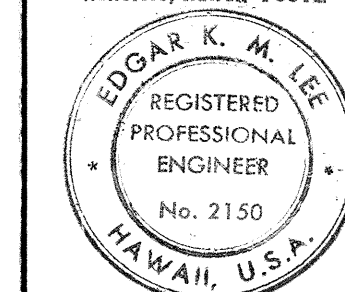
SECTION B
Scale: 3/8" = 1'-0"



SECTION C
Scale: 3/8" = 1'-0"

- NOTE:**
1. For Pier Details, see Sht. No. 5-22
 2. For Pile Details, see Std. Det. Sht. DB-300.

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Honolulu, Hawaii 96813



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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

PILE CAP PLANS & SECTIONS

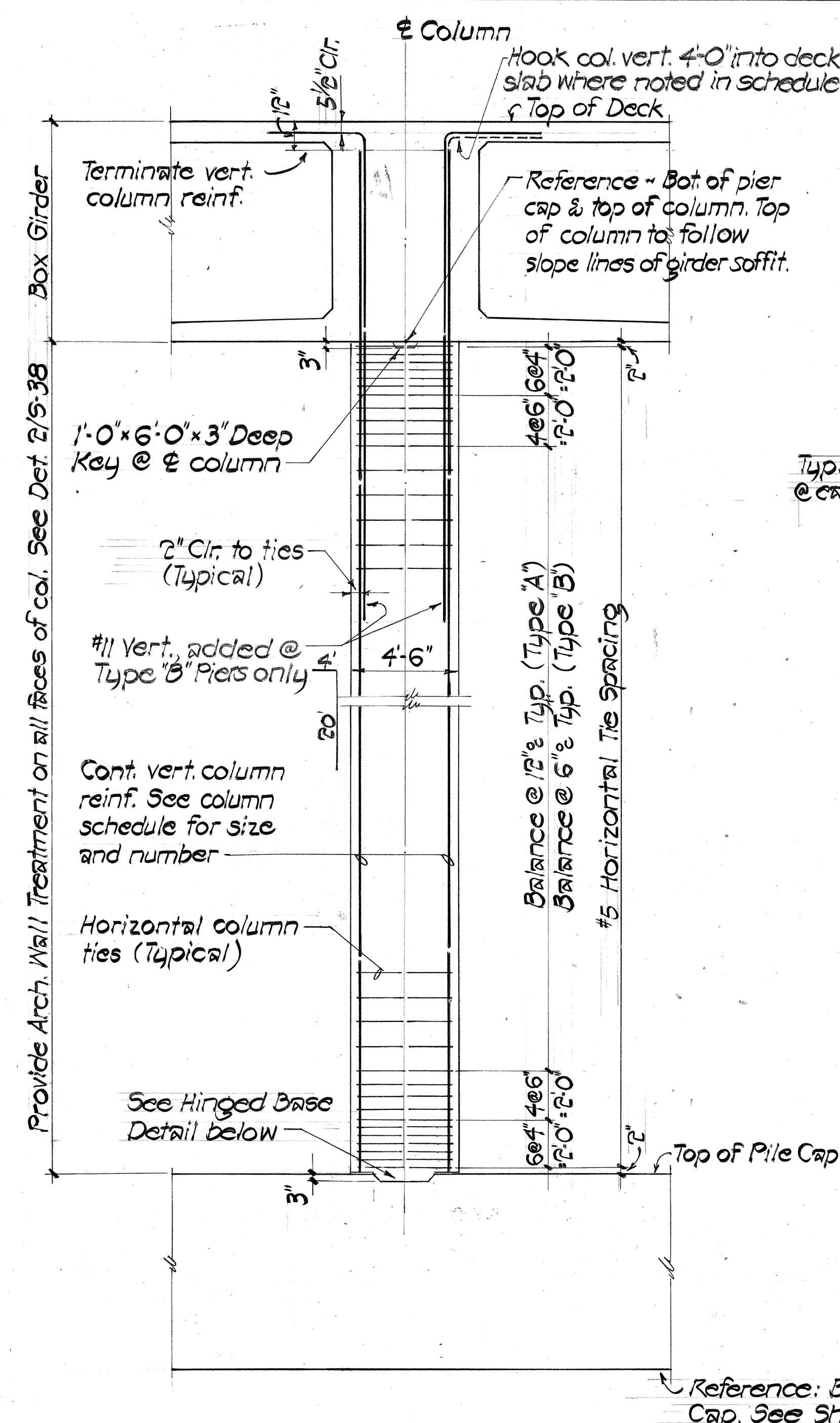
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)24

Scale: As Noted Date: 4-10-80

SHEET No. 3-2 OF 3-4 SHEETS

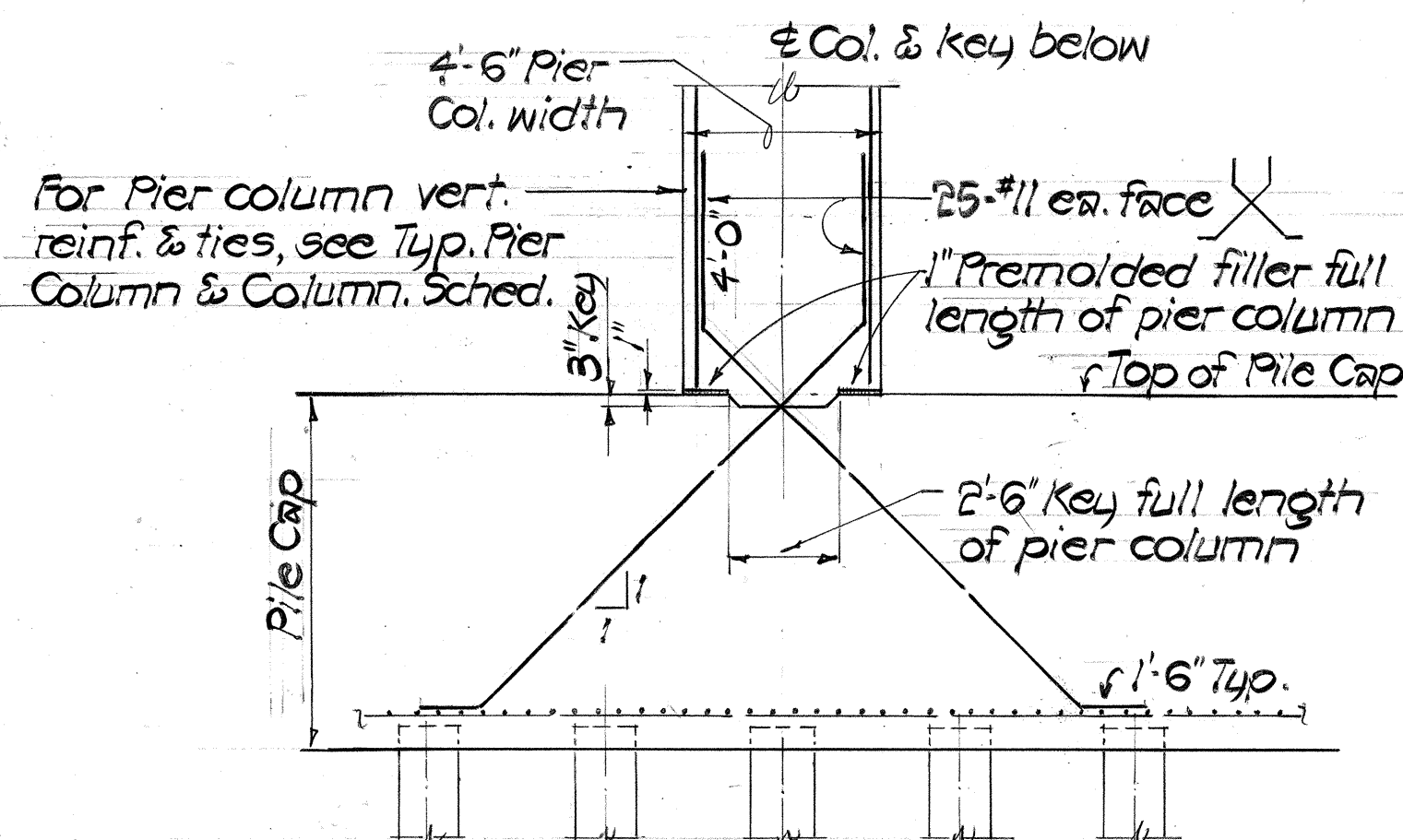
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	89	146

* Dimensions do not include thickness of arch. wall treatment



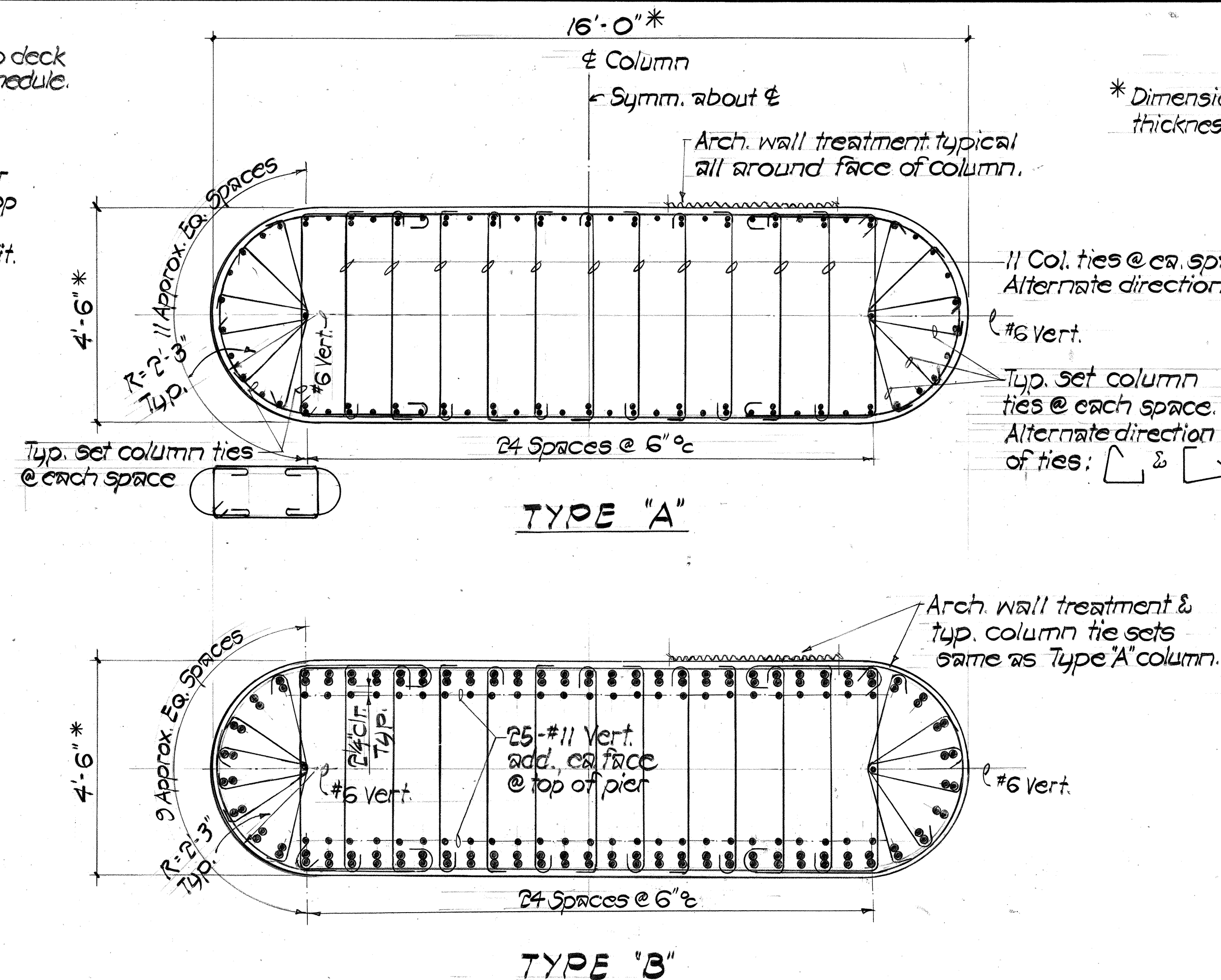
TYPICAL PIER COLUMN

Scale: 1/4" = 1'-0"



TYPICAL PIER COLUMN HINGED BASE DETAIL

Scale: 1/4" = 1'-0"



TYPICAL PIER COLUMN SECTIONS

Scale: 1/2" = 1'-0"

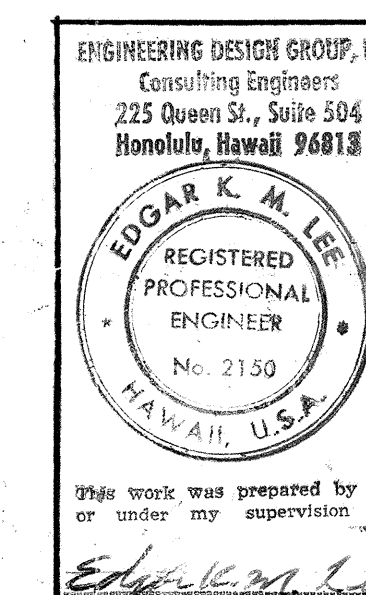
NOTE:

1. Bundle vert. bars as shown.
2. Locate vert. bars to clear pier cap reinf. See Sect. A/S-25.
3. All column ties to be #5.
4. Cost of arch wall treatment shall be incidental to concrete work.

~PIER COLUMN SCHEDULE~					
PIER Nos ~	Pier No1	Pier No2	Pier No3	Pier No4	Pier No5
Reference Top of Deck & Pier Cap					
Bottom of Pier Cap					
Top of Col. Elev.	E1. + 35.91	E1. + 43.94	E1. + 44.79	E1. + 42.95	E1. + 42.45
Column Type & Vert. Reinforcement	Type "B" Section 132-#18 Vertical Add 50-#11 Vertical	Type "B" Section 132-#18 Vertical Add 50-#11 Vertical	Type "B" Section 132-#18 Vertical Add 50-#11 Vertical	Type "A" Section 96-#11 Vertical	Type "A" Section 96-#11 Vertical
Top of Pile Cap					
Reference Bottom of Pile Cap					
PILE CAP TYPE~	Type II	Type III	Type III	Type I	Type I

Hook 25 bars ea. face, ea. pier column (Pier 4 & 5 only)

NOTE: For Pile Cap Details, see Sheet S-21.
For Bot. of Pile Cap Elev., see Sheet S-5.



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

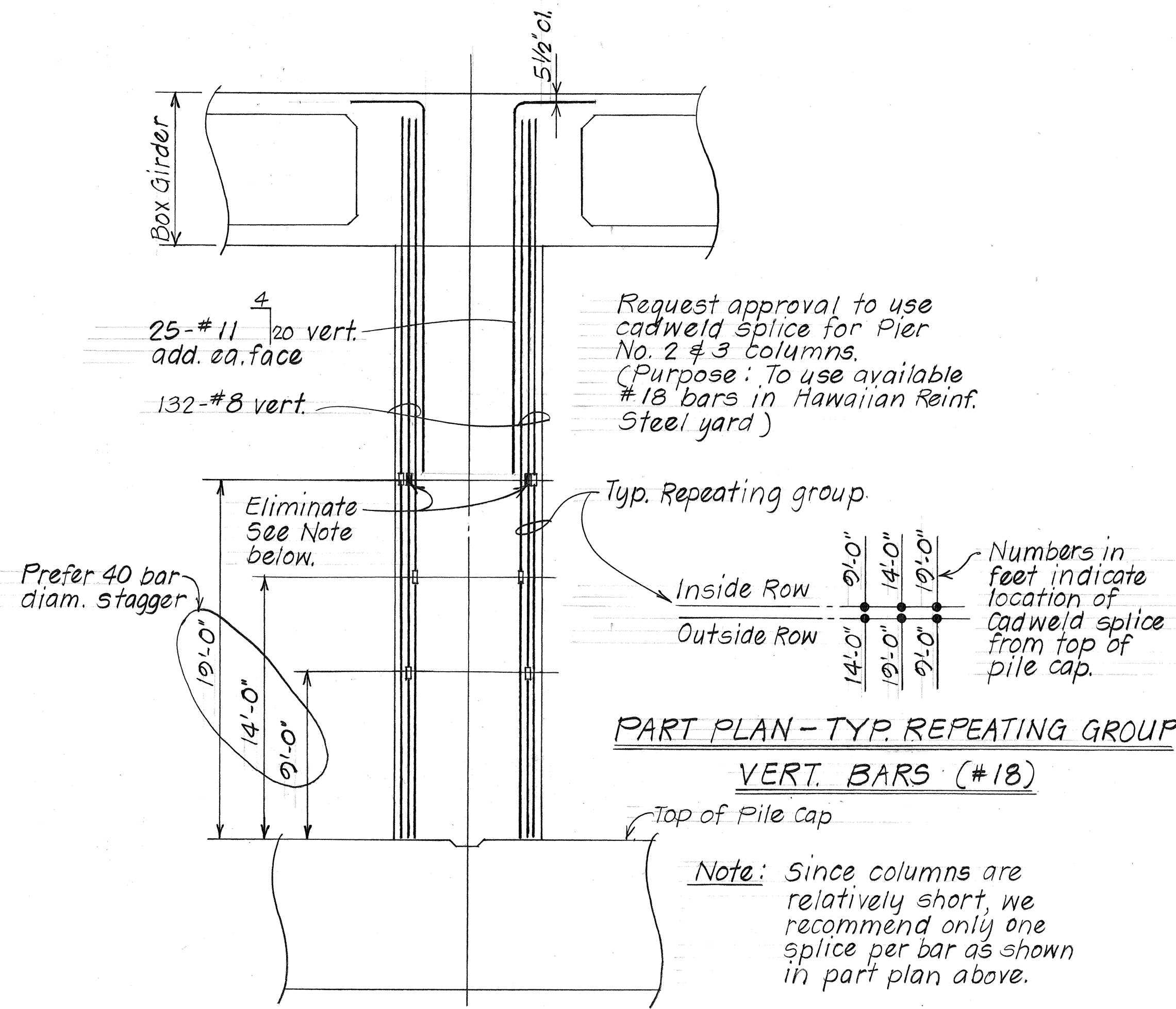
**TYPICAL PIER COLUMN,
SECTIONS & SCHEDULE**

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24

Scale: As Noted Date: 4-10-80

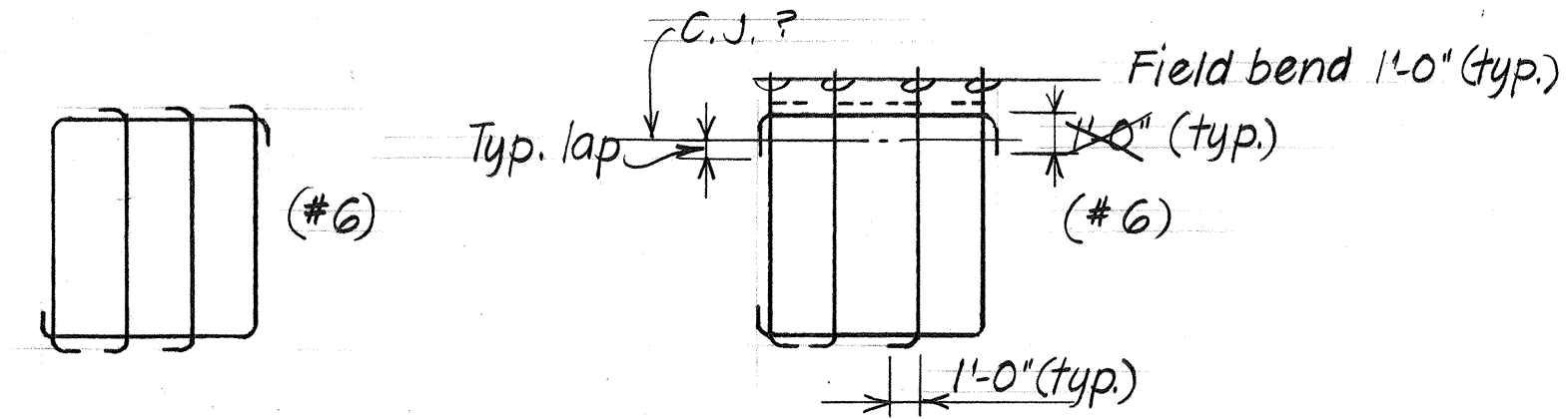
SHEET No. 6-22 OF 6-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157):24	1977	895-1	146



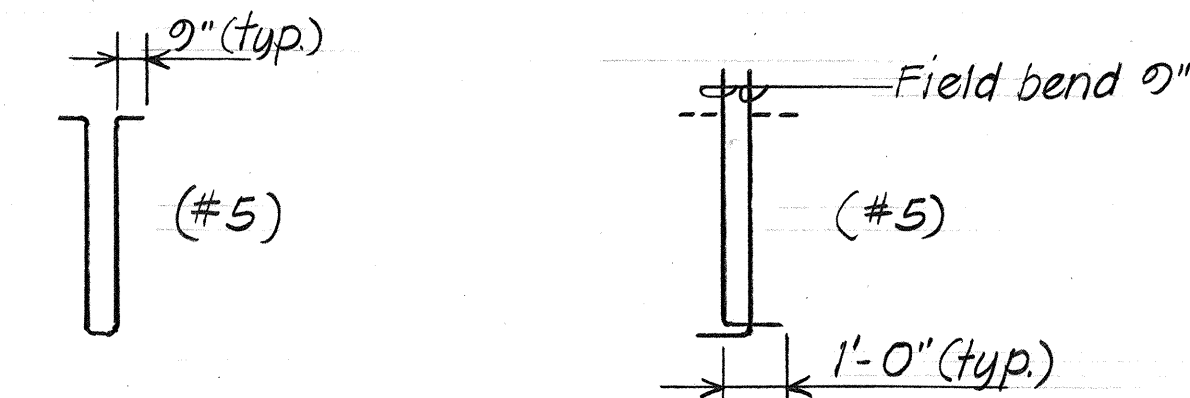
Note: Refer to sht. 89 for bal. of reinf.

PIER NOS. 2 & 3



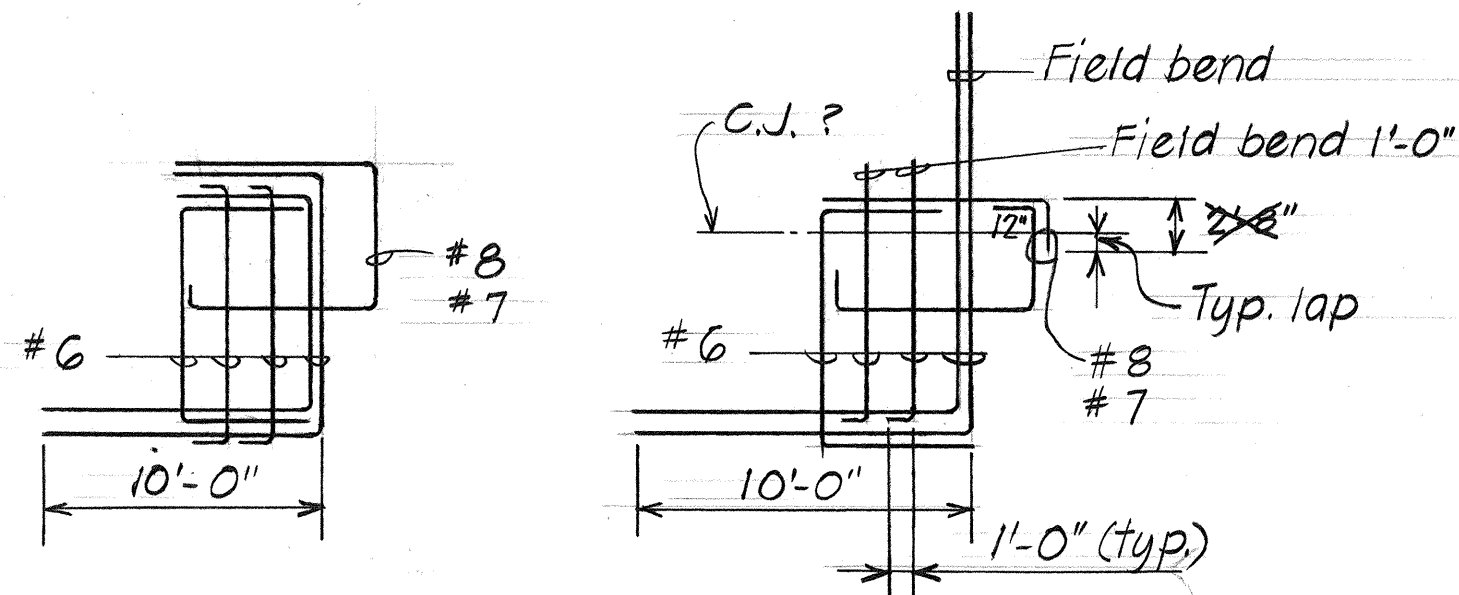
BEAM A-1 & A-2 STIRRUPS

Refer to shts. 93 & 94 for bal. of reinf.



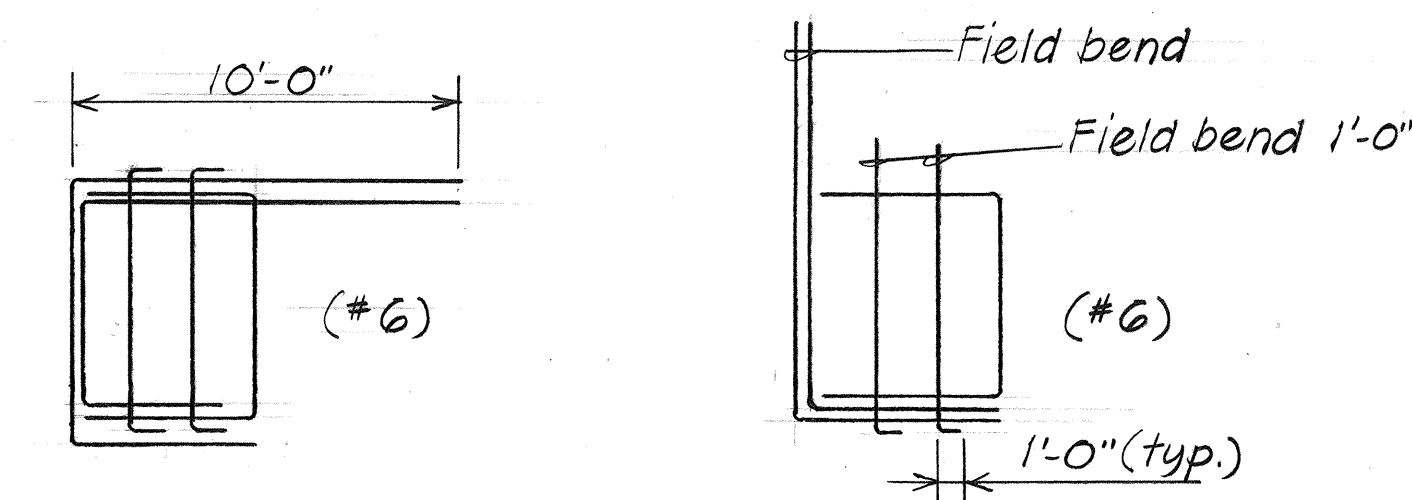
BEAM B STIRRUPS

Refer to sht. 110 for bal. of reinf.



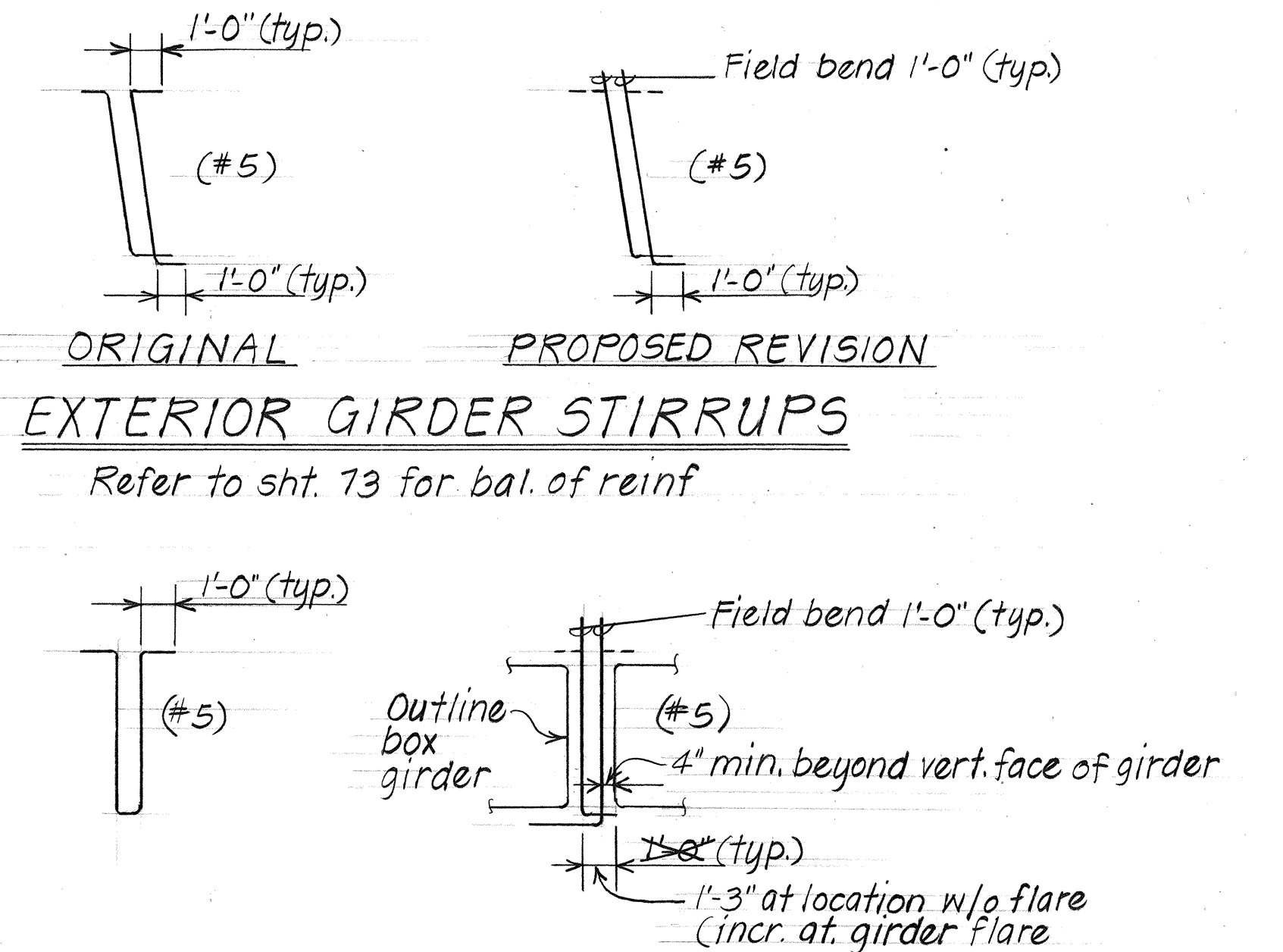
BEAM C1 & C2

Refer to sht. 98 for bal. of reinf.



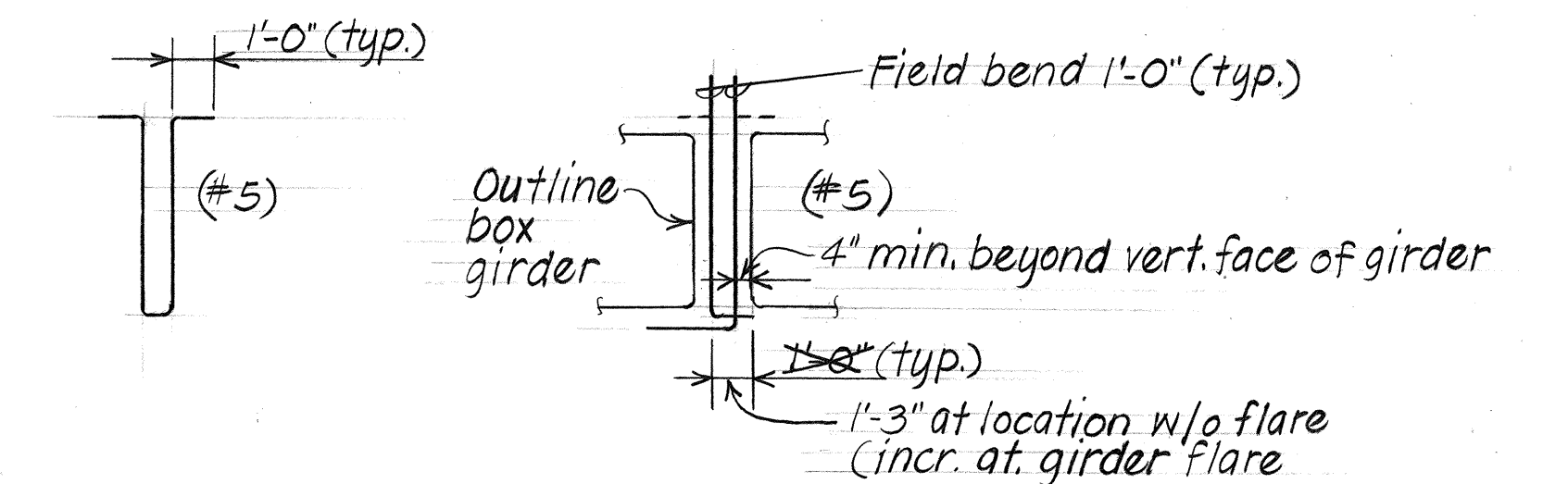
BEAM D-1 & D-2 STIRRUPS

Refer to sht. 98 for bal. of reinf.



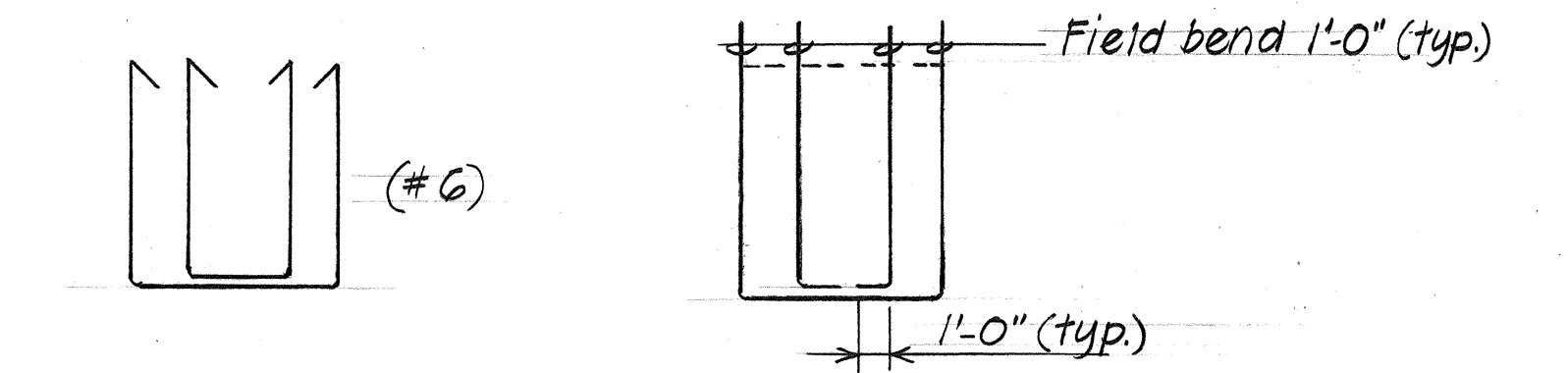
EXTERIOR GIRDER STIRRUPS

Refer to sht. 73 for bal. of reinf.



INTERIOR GIRDER STIRRUPS

Refer to sht. 73 for bal. of reinf.



PIER CAP STIRRUPS

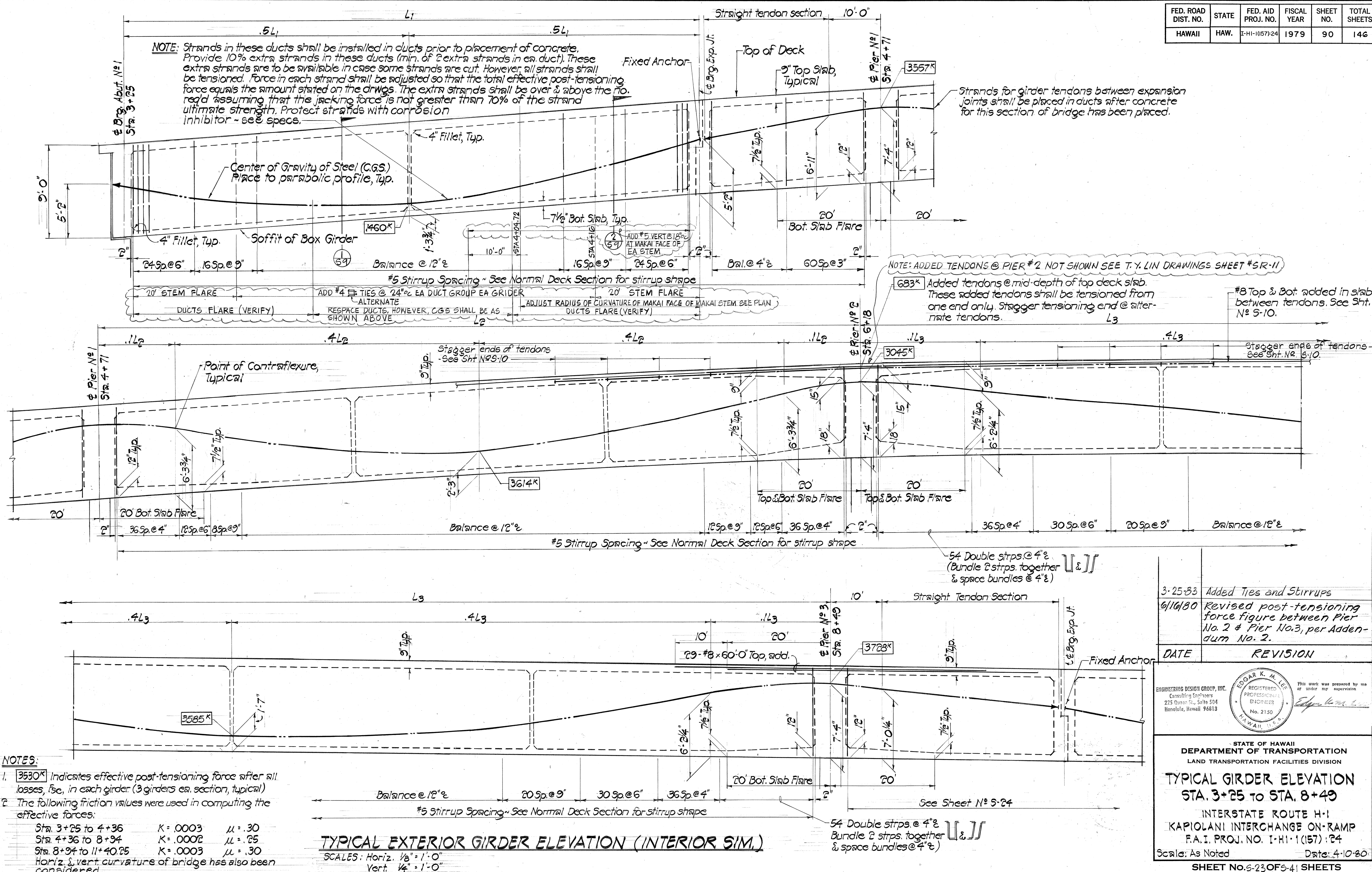
Refer to sht. 92 for bal. of reinf.

This tracing prepared during "As Built" postings.

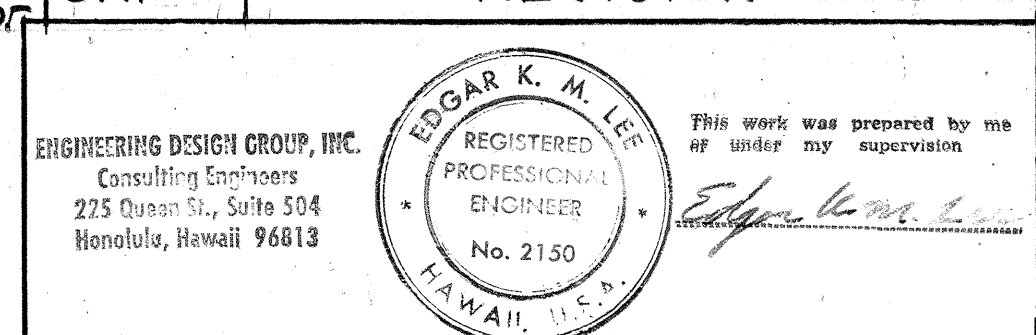
ORIGINAL PLAN	DATE
DESIGNED BY	
TRACED BY	
NOTE BOOK	
QUANTITIES BY	
CHECKED BY	
NO.	

STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION
SPLICE DET. - PIER NOS. 2 & 3 BEAM, GIRDER & PIER CAP STIRRUP DETS.
INTERSTATE ROUTE H-1 KAPIOLANI INTERCHANGE ON-RAMP F.A.I PROJ. NO. I-HI-1(157):24
Scale: N.T.S. Date: 7-13-88
SHEET No. 522A OF 6-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)24	1979	90	146



DATE	REVISION
3-25-83	Added Ties and Stirrups
6/16/80	Revised post-tensioning force figure between Pier No. 2 & Pier No. 3, per Addendum No. 2.



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

TYPICAL GIRDER ELEVATION
STA. 3+25 TO STA. 8+49

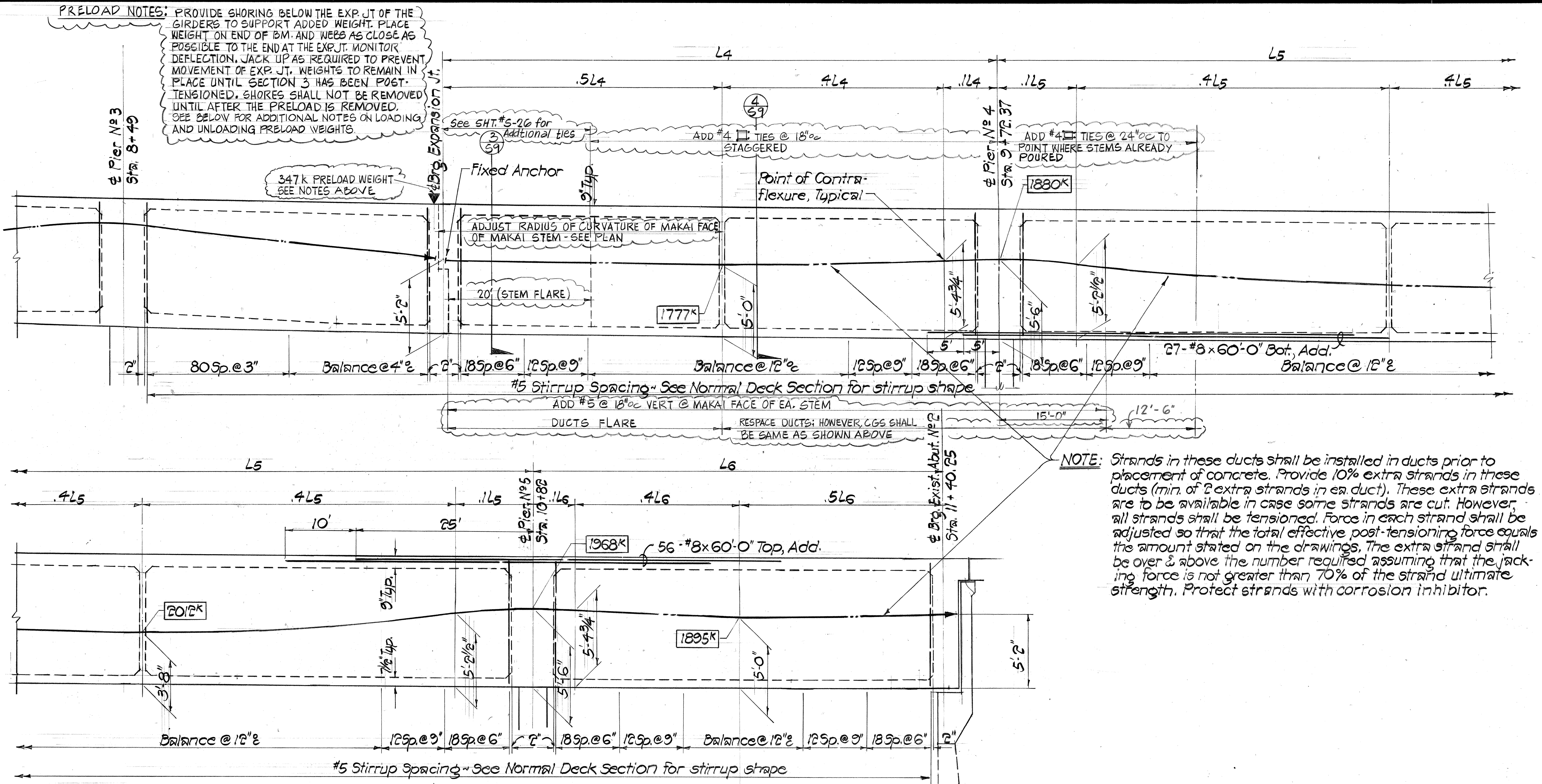
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24

Scale: As Noted Date: 4-10-80

SHEET NO. 5-23 OF 5-41 SHEETS

C.O. 90R

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-(157)-24	1979	91	146



TYPICAL EXTERIOR GIRDER ELEVATION (INTERIOR SIM.)

SCALES: Horiz. 1/8" = 1'-0"
Vert. 1/4" = 1'-0"

PRELOAD LOADING SEQUENCE:

1. INSTALL SHORING UNDER AREA TO BE PRELOADED. (SEE SPECIFICATIONS AND ADDENDUM NO. 1)
2. TENSION FIRST THREE (3) TENDONS.
3. PLACE APPROXIMATELY 40 KIPS OF PRELOAD WEIGHT.
4. PLACE REMAINDER OF PRELOAD WEIGHTS IN INCREMENTS AS THE REMAINDER OF THE TENSIONING IS DONE. (PLACE APPROXIMATELY 40 KIPS OF PRELOAD WEIGHT FOR EACH ADDITIONAL TWO (2) TENDONS TENSIONED)
5. PRELOADING MAY BE DONE CONCURRENTLY WITH THE TENSIONING SO LONG AS THE PRELOADING PERCENTAGE IS APPROXIMATELY EQUAL TO THE TENSIONING PERCENTAGE.

PRELOAD UNLOADING SEQUENCE:

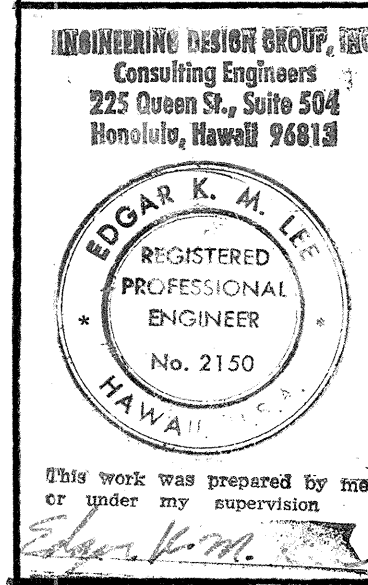
1. AFTER SECTION 3 IS POST-TENSIONED, LOOSEN SHORES BETWEEN PIERS 4 & 5 AND BETWEEN PIERS 2 & 3. (SEE SPECIFICATIONS FOR DETAILED REQUIREMENTS FOR REMOVAL OF FALSEWORK.)
2. REMOVE PRELOAD WEIGHTS.
3. REMOVE SHORES BETWEEN PIERS 3 & 4.

NOTE:

See Notes, Sht. No. S-23.

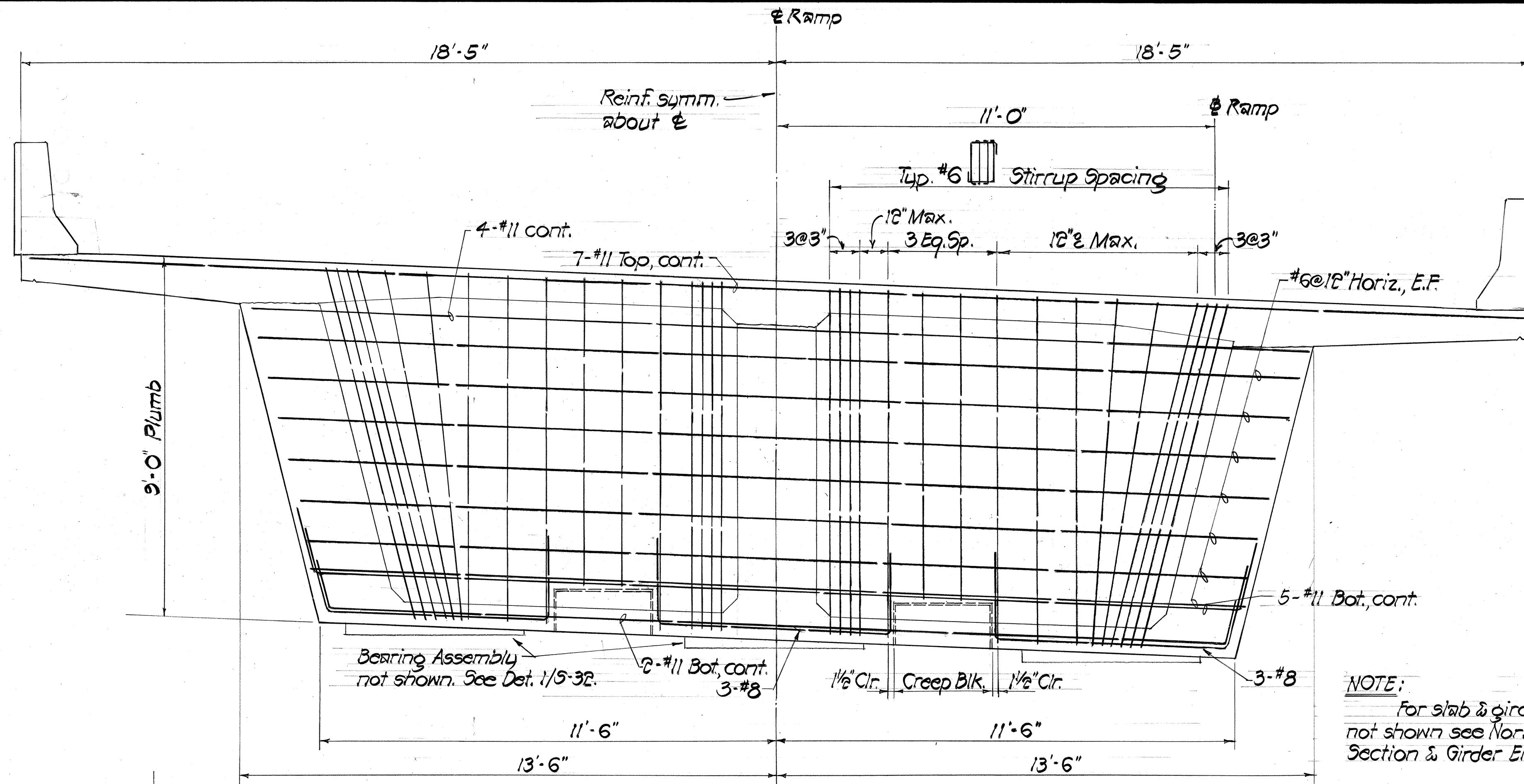
3-25-83 Added Ties, Stirrups, Preload at Exp. Jt. and Preload loading & unloading sequence

DATE REVISION



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
TYPICAL GIRDER ELEVATION
STA. 8+49 TO STA. 11+40.25
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1 (157) 24
Scale: As Noted Date: 4-10-80
SHEET No. 5-24 OF 5-41 SHEETS

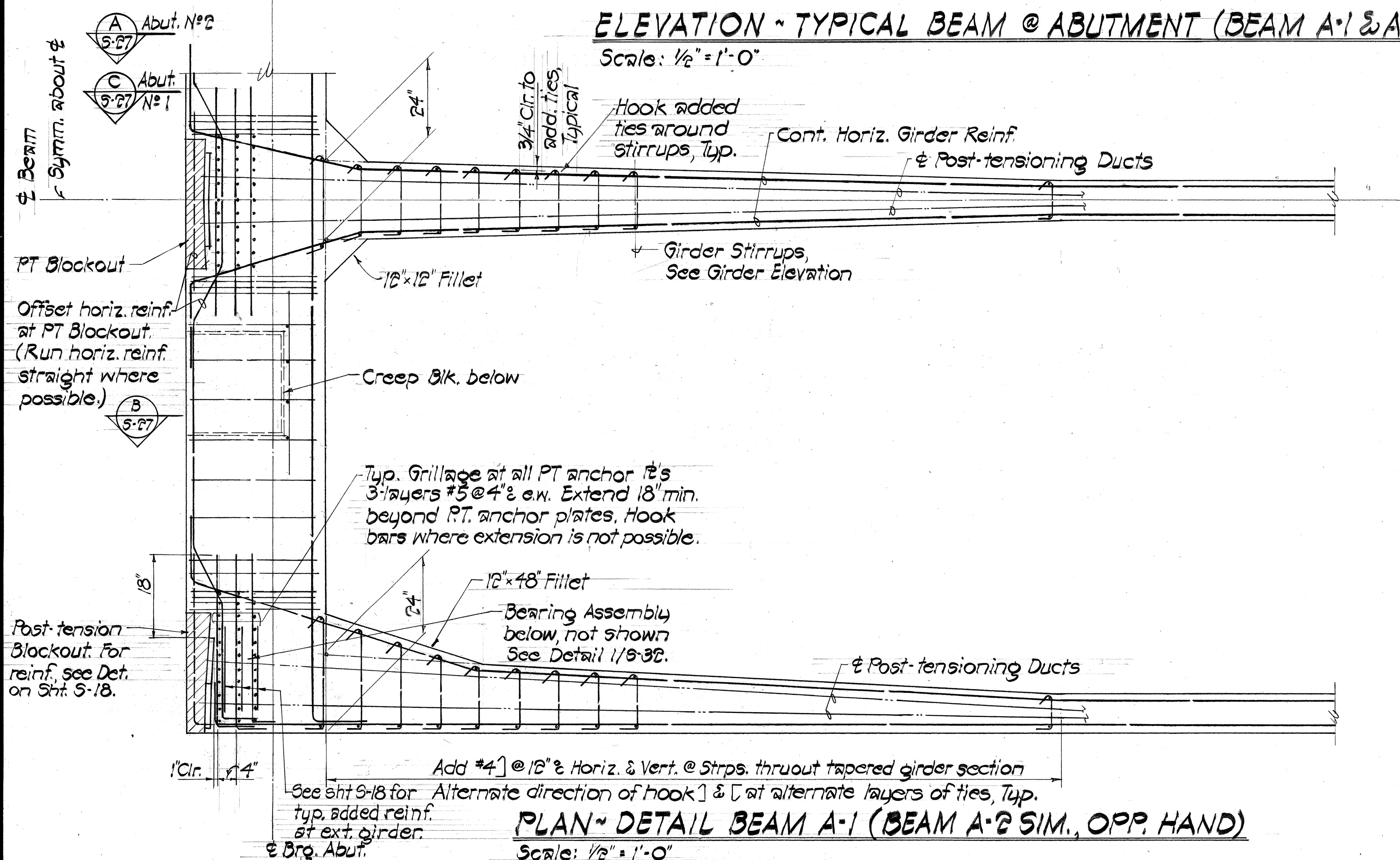
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	93	146



ELEVATION - TYPICAL BEAM @ ABUTMENT (BEAM A-1 & A-2)

Scale: 1/2" = 1'-0"

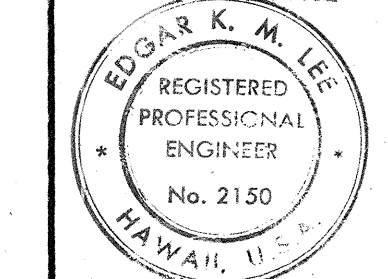
NOTE:
For slab & girder reinf.
not shown see Normal Deck
Section & Girder Elevation.



PLAN - DETAIL BEAM A-1 (BEAM A-2 SIM., OPP. HAND)

Scale: 1/2" = 1'-0"

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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

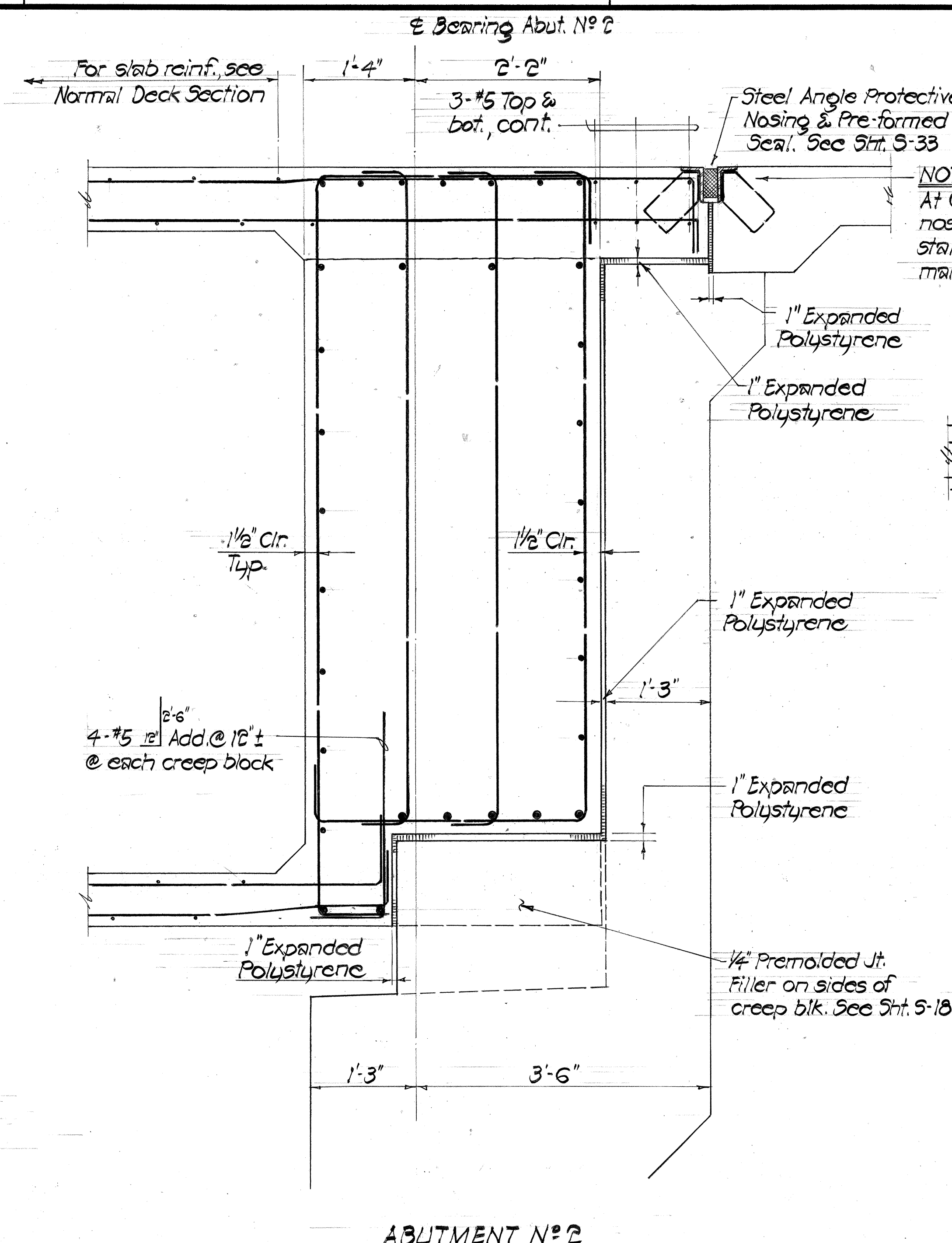
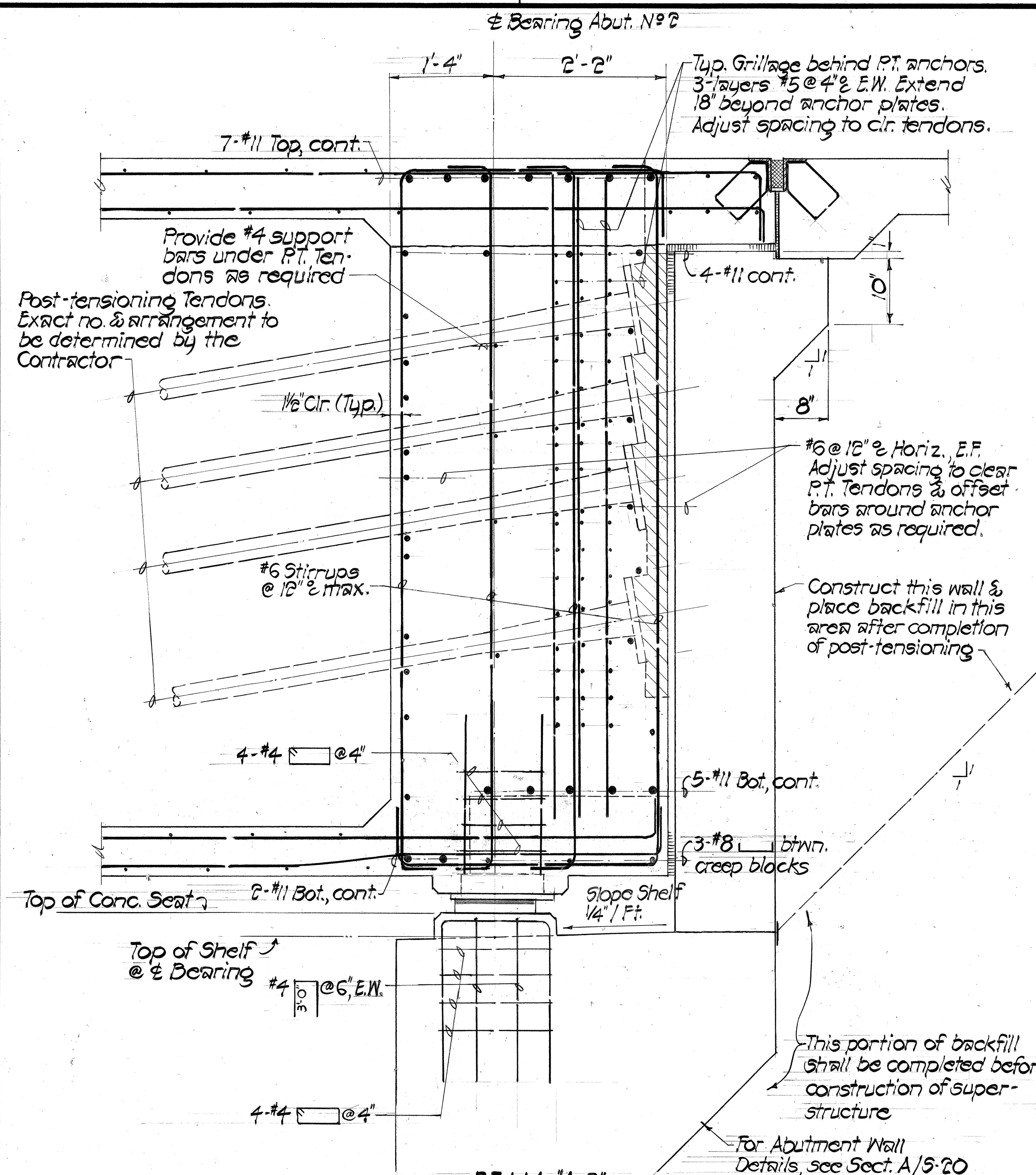
TYP. BEAM @ ABUTMENT (A-1 & A-2)
PLAN & ELEVATION

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24

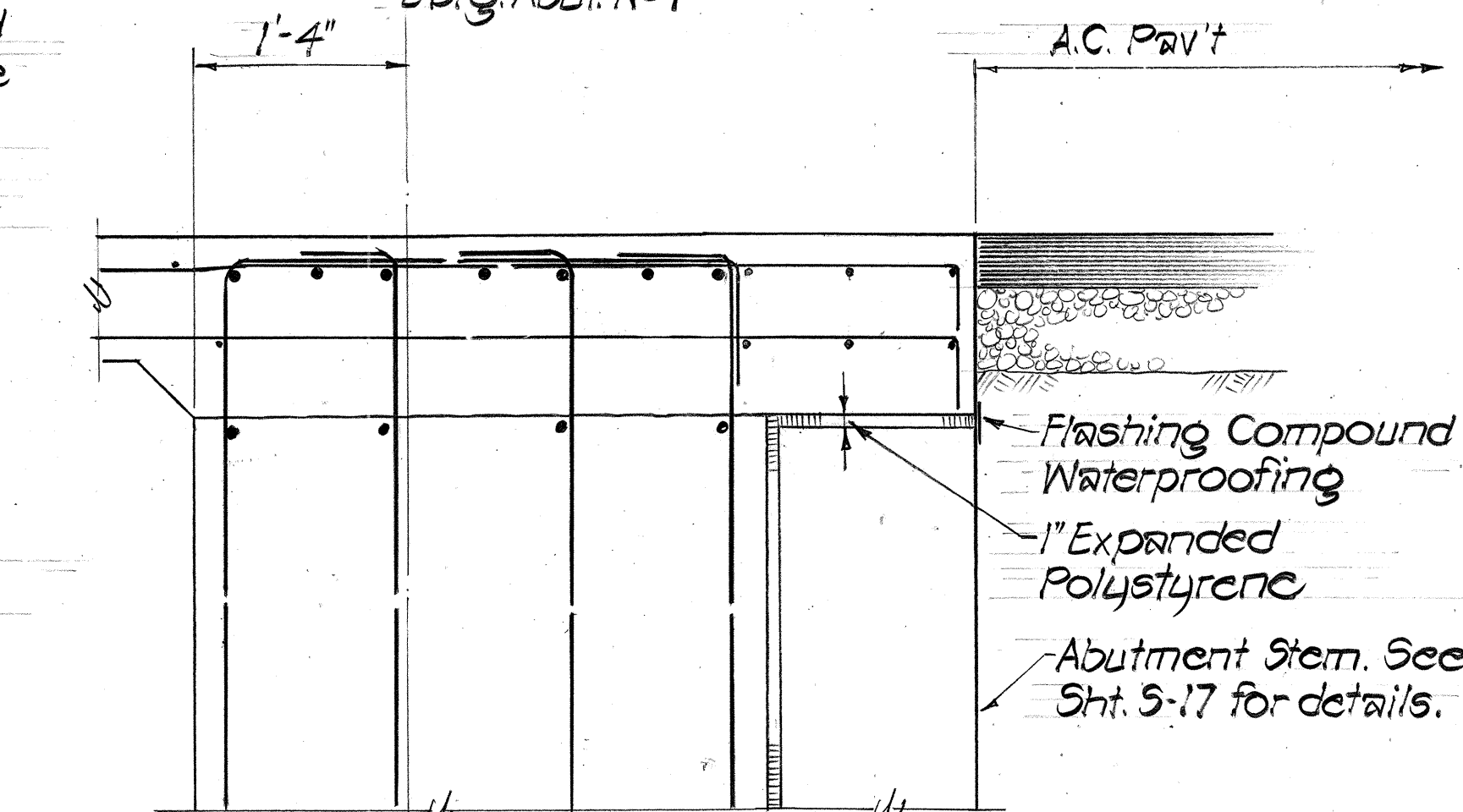
Scale: As Noted Date: 4-10-80

SHEET NO. 93 OF 94 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	94	146



NOTE:
At Contractor's option, the angle nosing for this joint only can be installed without providing blockouts & making a separate closure pour. & Brg. Abut. No. 1



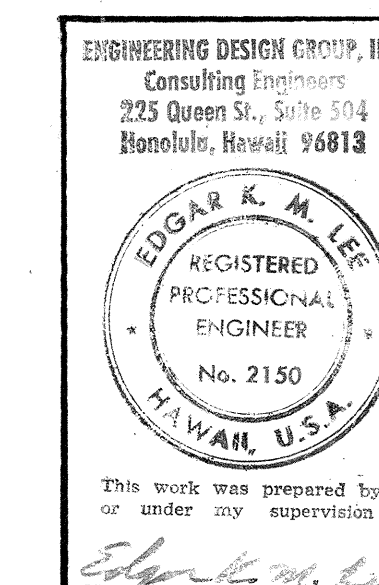
NOTE:
Reinf. & balance of detail similar to Section A/S-27.

BEAM "A-1"

TYP. ABUTMENT NO. 1 SECTION C

Scale: 1" = 1'-0"

ORIGINAL PLAN	DATE
SURVEY PLOTTED BY	
DESIGNED BY	
TRACED BY	
NOTE BOOK	
QUANTITIES BY	
CHECKED BY	
No.	



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

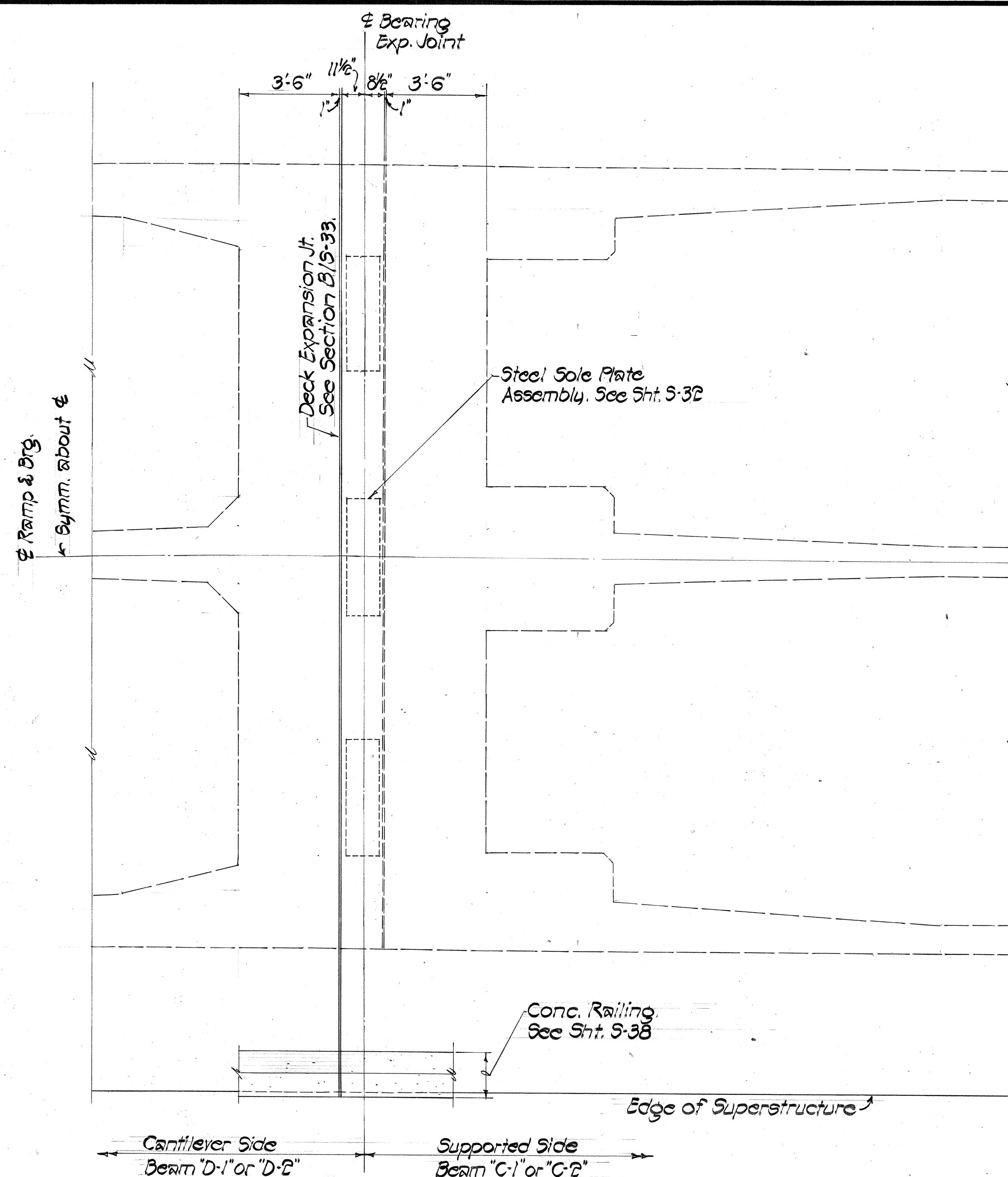
ABUTMENT SECTIONS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.T. PROJ. NO. I-HI-1 (157)-24

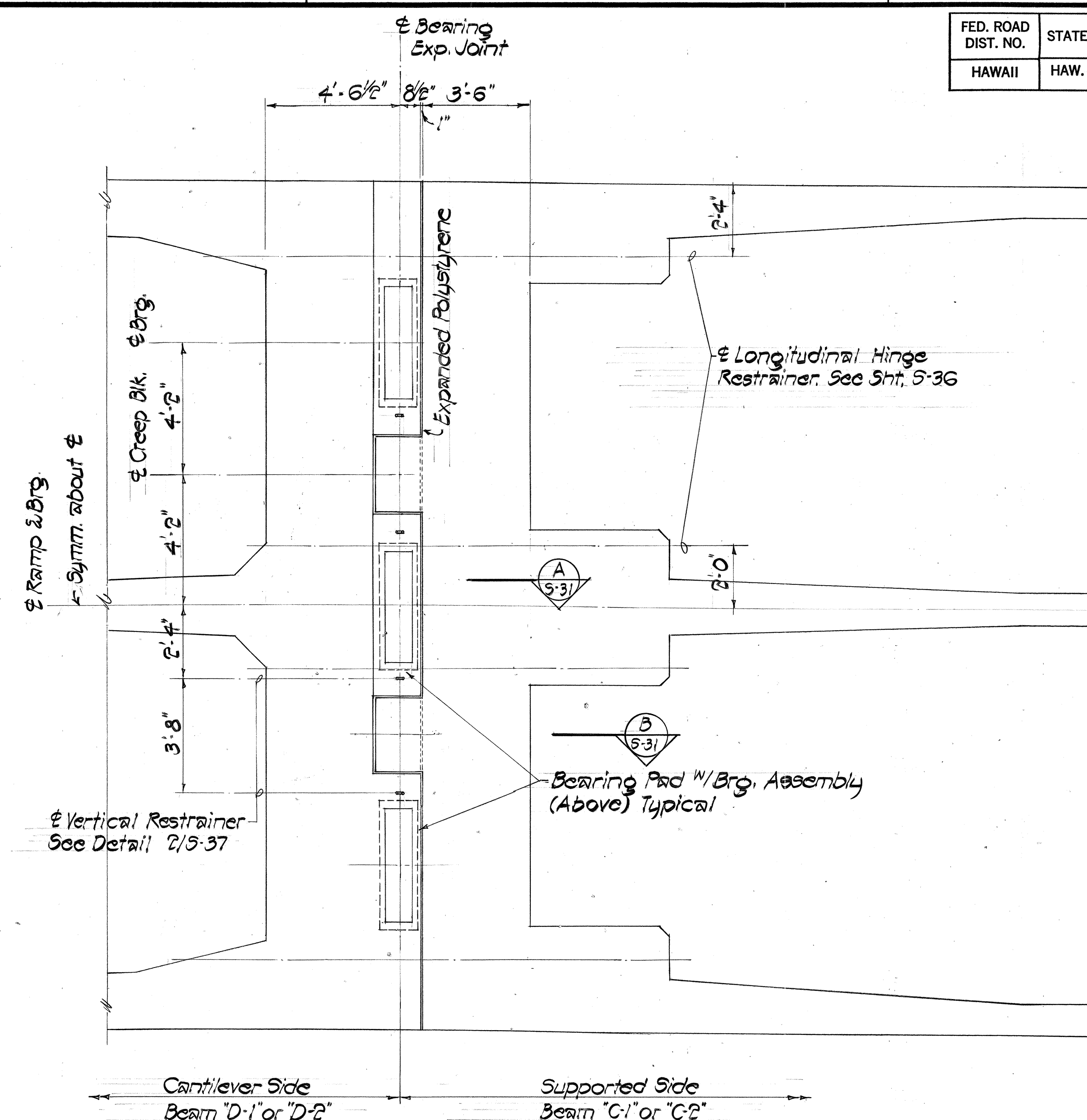
Scale: As Noted Date: 4-10-80

SHEET No. 5-27 OF 5-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	95	146



TYP EXPANSION JOINT PLAN AT DECK LEVEL
Scale: 3/8" = 1'-0"

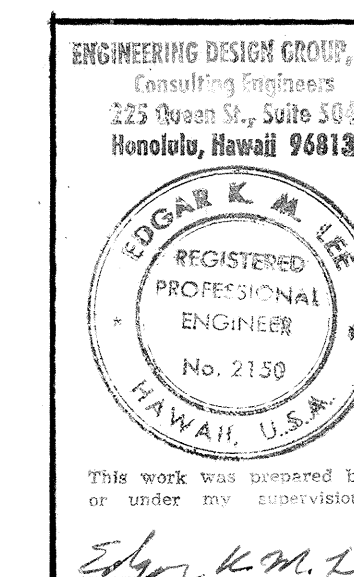


TYP EXPANSION JOINT PLAN AT BEARING LEVEL
Scale: 3/8" = 1'-0"

NOTE:

Expansion joint shown is at Sta. 8+94.
Expansion joint at Sta. 4+36 similar but opp. hand.

ORIGINAL PLAN	SURVEY PLOTTED BY	DATE
NOTE BOOK	DRAWN BY	
	DESIGNED BY	
	CHECKED BY	



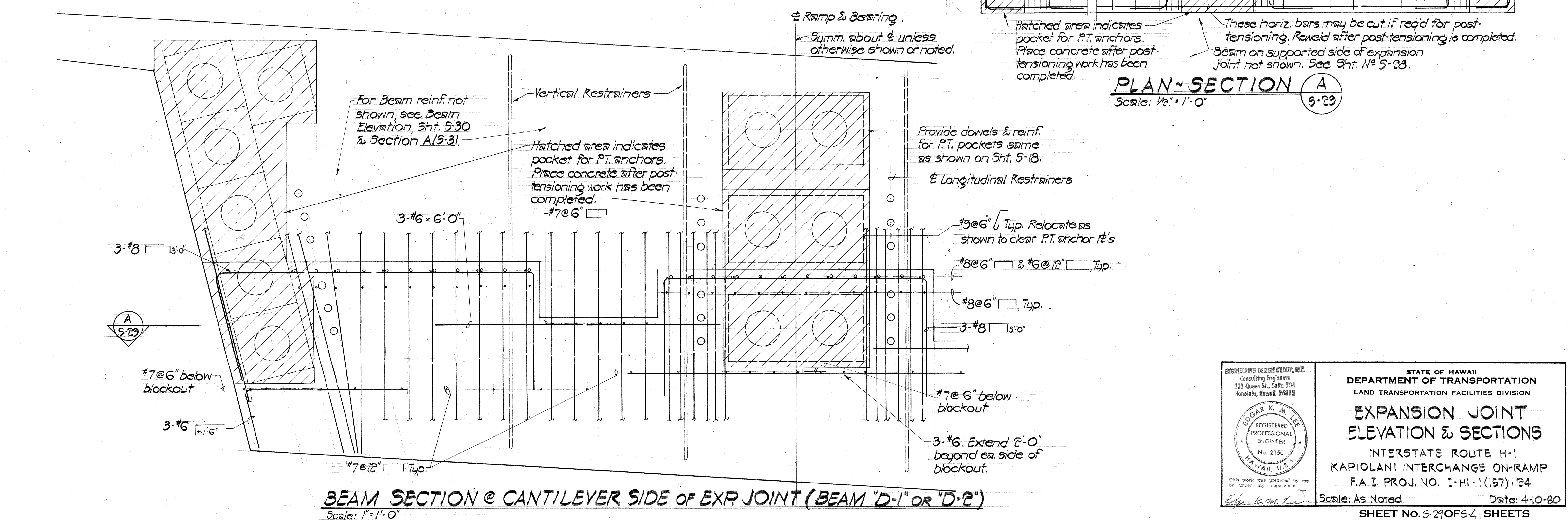
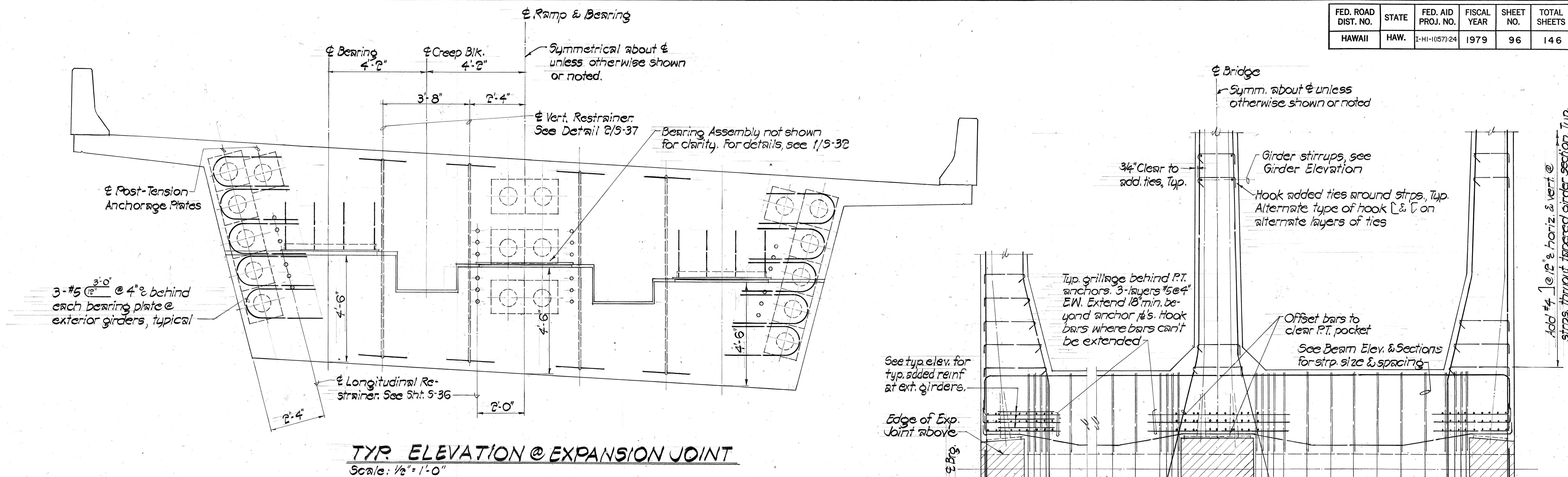
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

EXPANSION JOINT PLANS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24
Scale: As Noted Date: 4-10-80

SHEET No. 5-28 OF 5-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)24	1979	96	146



PLAN SECTION A
Scale: 1/2" = 1'-0"
5-29

ORIGINAL PLAN	DATE
DRAWN BY	
CHECKED BY	
NOTED BY	
QUANTITIES BY	
NO.	

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HAWAII, U.S.A.

This work was prepared by me or under my supervision

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

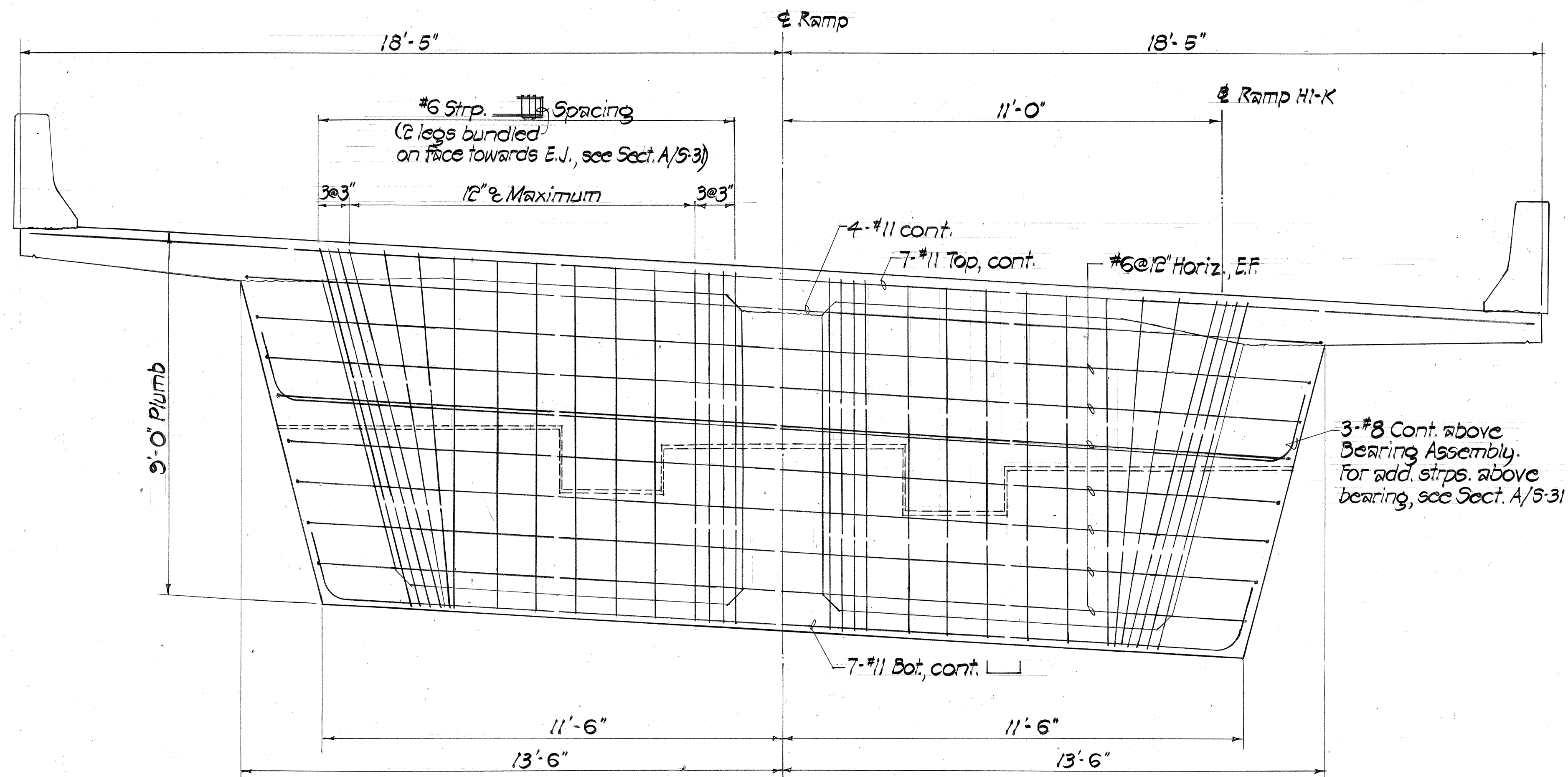
EXPANSION JOINT ELEVATION & SECTIONS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)24

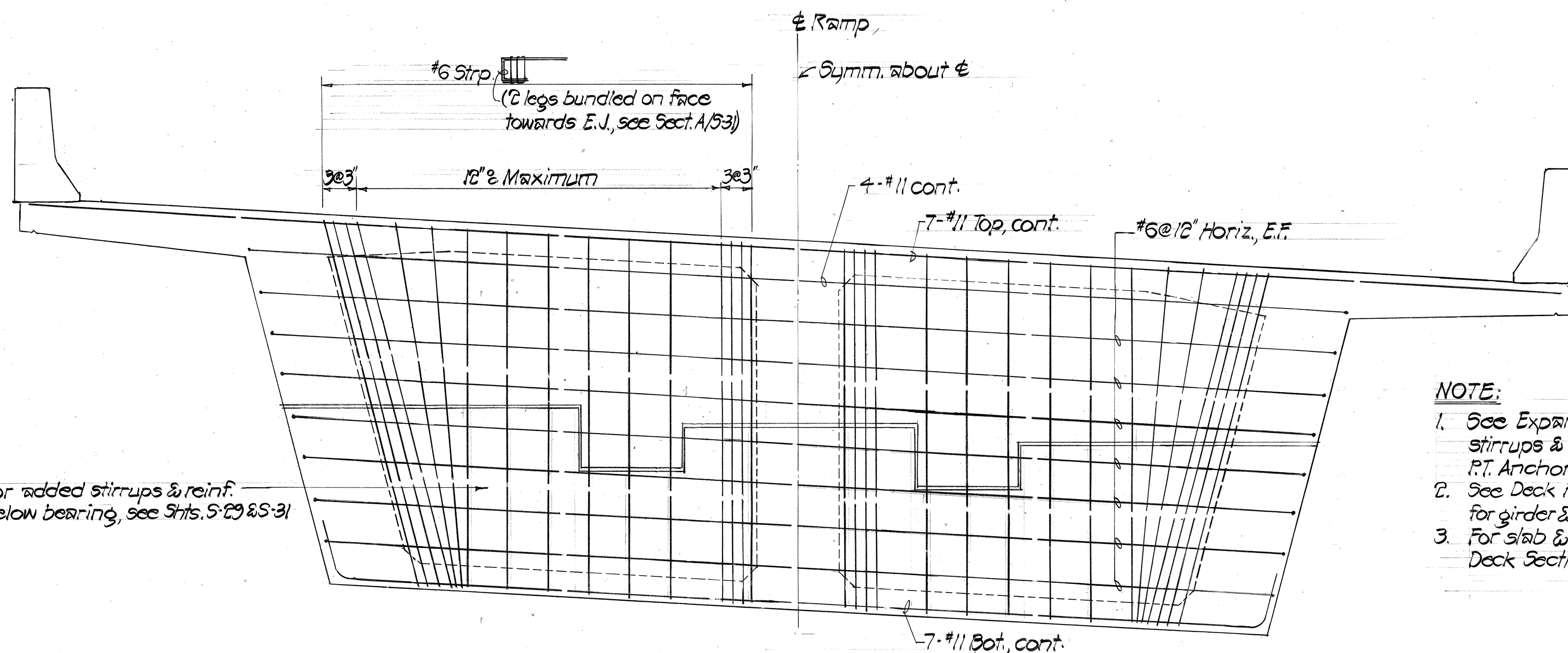
Scale: As Noted Date: 4-10-80

SHEET NO. 5-29 OF 5-4 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157):24	1979	97	146



ELEVATION ~ BEAM C-1 @ EXPANSION JOINT (SUPPORTED SIDE ~ BEAM C-2 SIM., OPP. HAND) (BM. C-1 @ STA. 4+36, BM. C-2 @ STA. 8+94)
Scale: 1/8" = 1'-0"

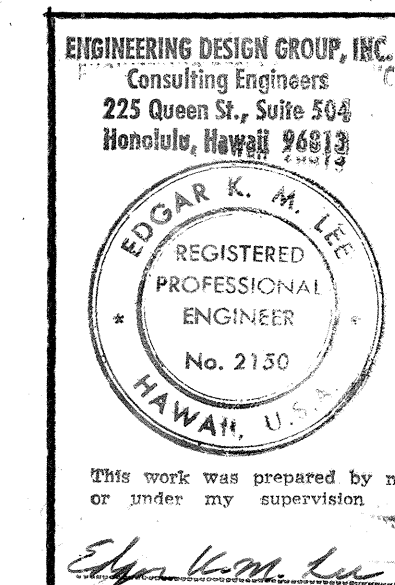


NOTE:

1. See Expansion Joint Sections for added stirrups & reinf. at bearings, creep blocks & P.T. Anchor plates, etc.
2. See Deck Framing Plans & Girder Elevations for girder & slab thicknesses and flares.
3. For slab & girder reinf. not shown see Normal Deck Section and Girder Elevation.

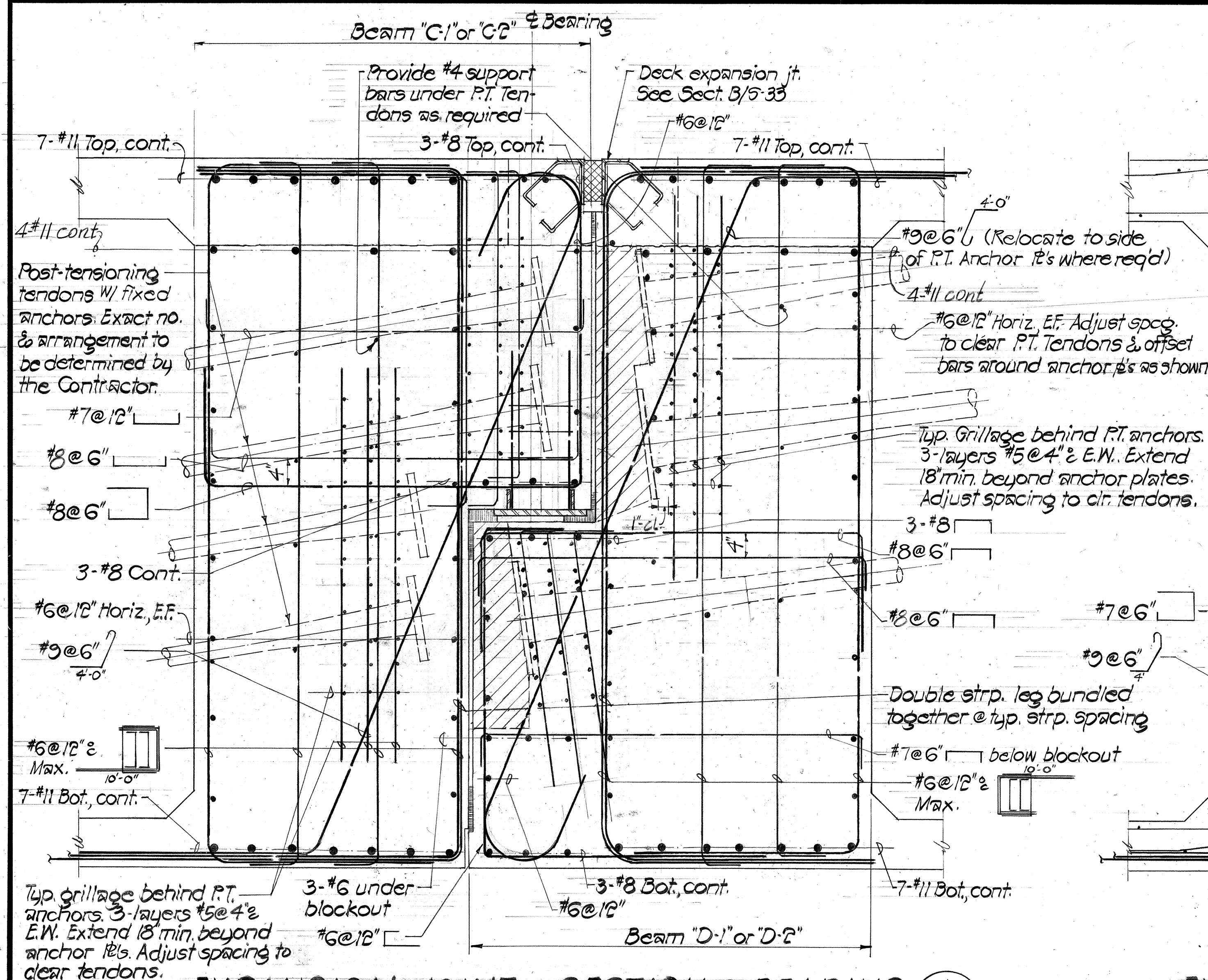
ELEVATION ~ BEAM D-1 @ EXPANSION JOINT (CANTILEVER SIDE ~ BM. D-2 SIM., OPP. HAND)
Scale: 1/8" = 1'-0" (BEAM D-1 @ STA. 4+36, BEAM D-2 @ STA. 8+94)

SURVEY PLOTTED BY	DATE
DRAWN BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
NO.	



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
EXPANSION JOINT BEAMS
(BEAMS C-1, C-2, D-1 & D-2)
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157):24
Scale: As Noted Date: 4-10-80
SHEET No. 6-30 OF 6-41 SHEETS

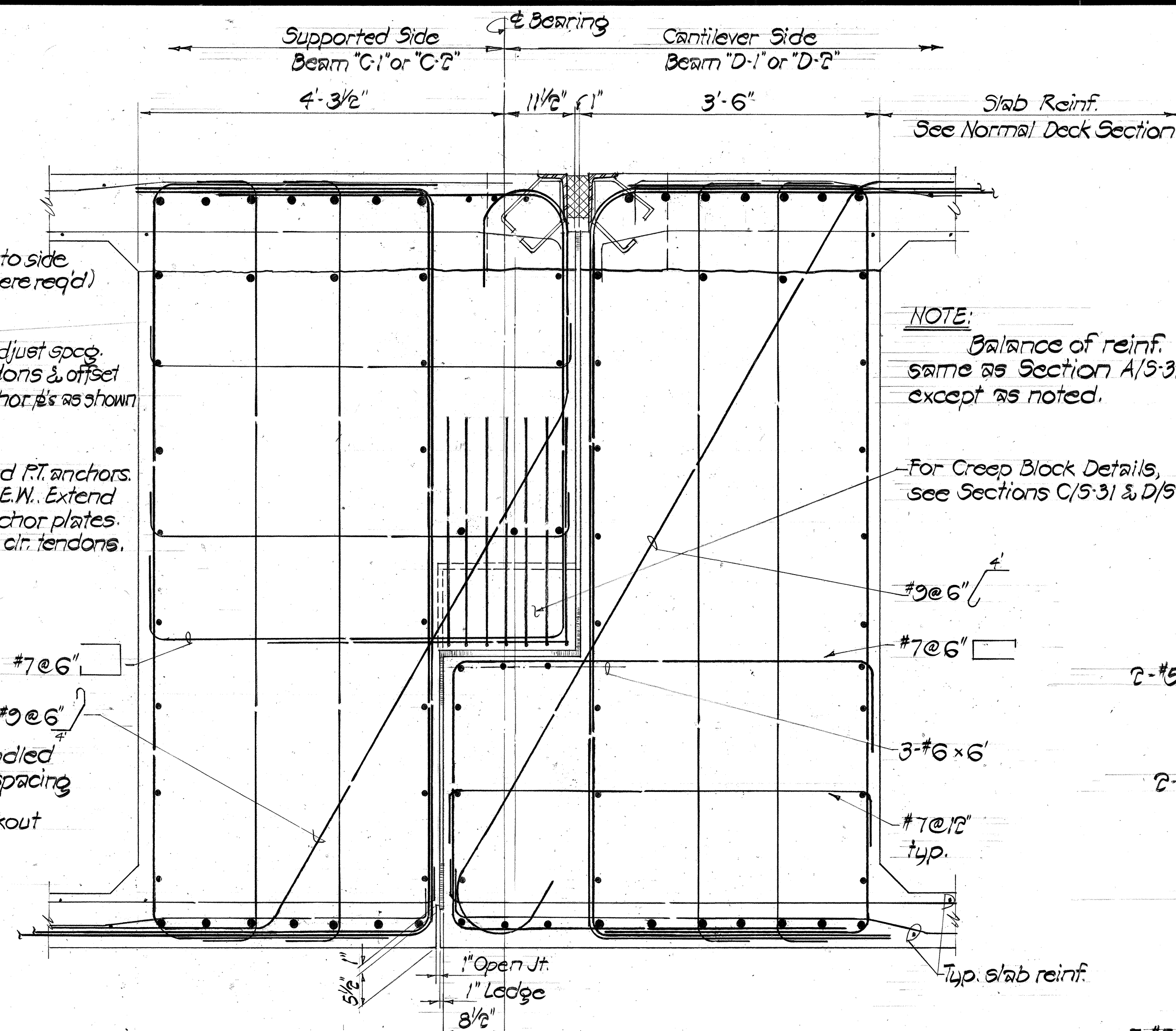
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	98	146



EXPANSION JOINT ~ SECTION @ BEARING (A)
Scale: 1" = 1'-0"

NOTES:

- For details of bearing assembly, see Sht. 5-32.
- For slab & girder reinf. not shown, see Normal Deck Section & Girder Elevations.

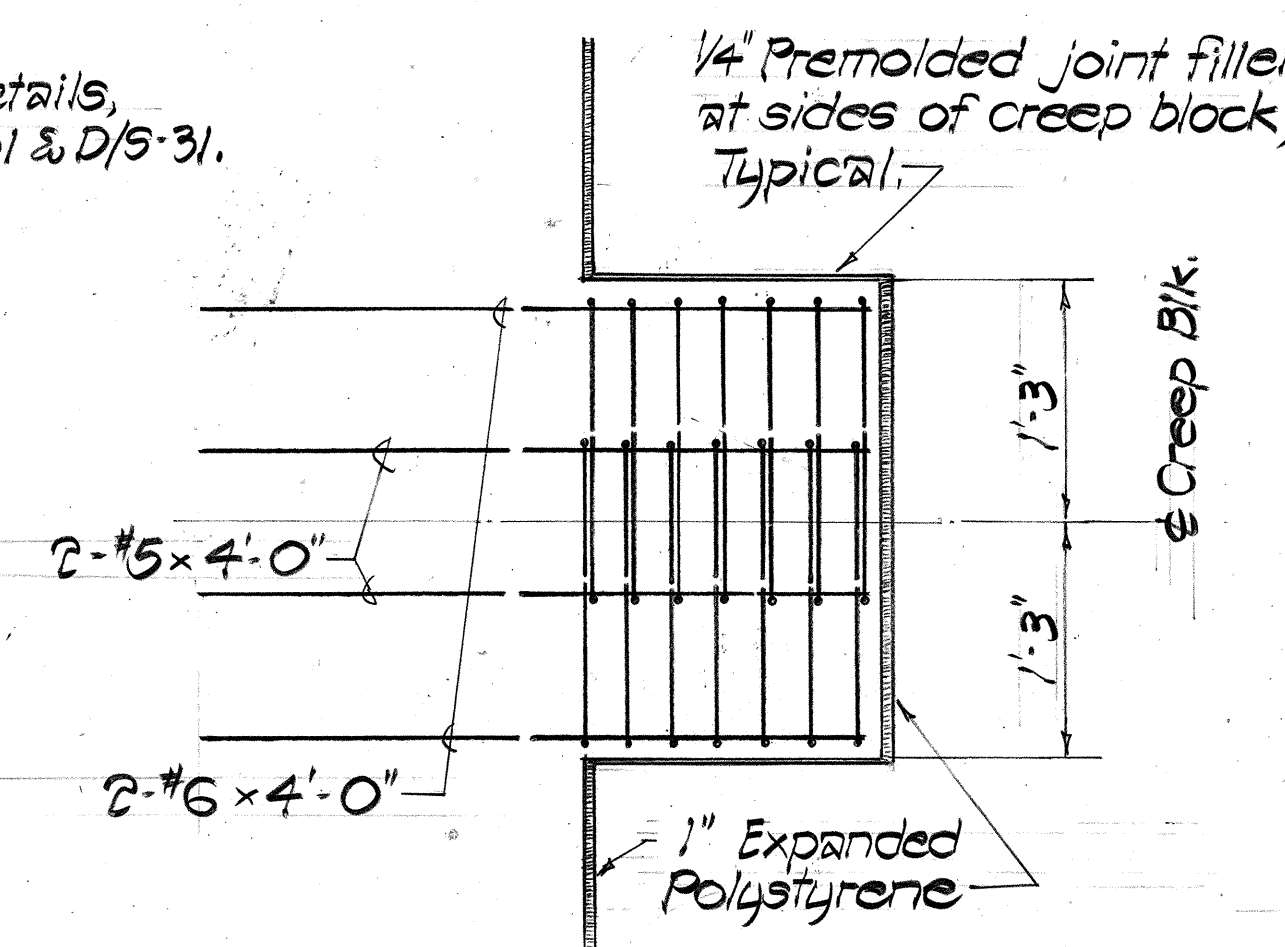


EXPANSION JOINT ~ SECTION @ CREEP BLOCK (B)
Scale: 1" = 1'-0"

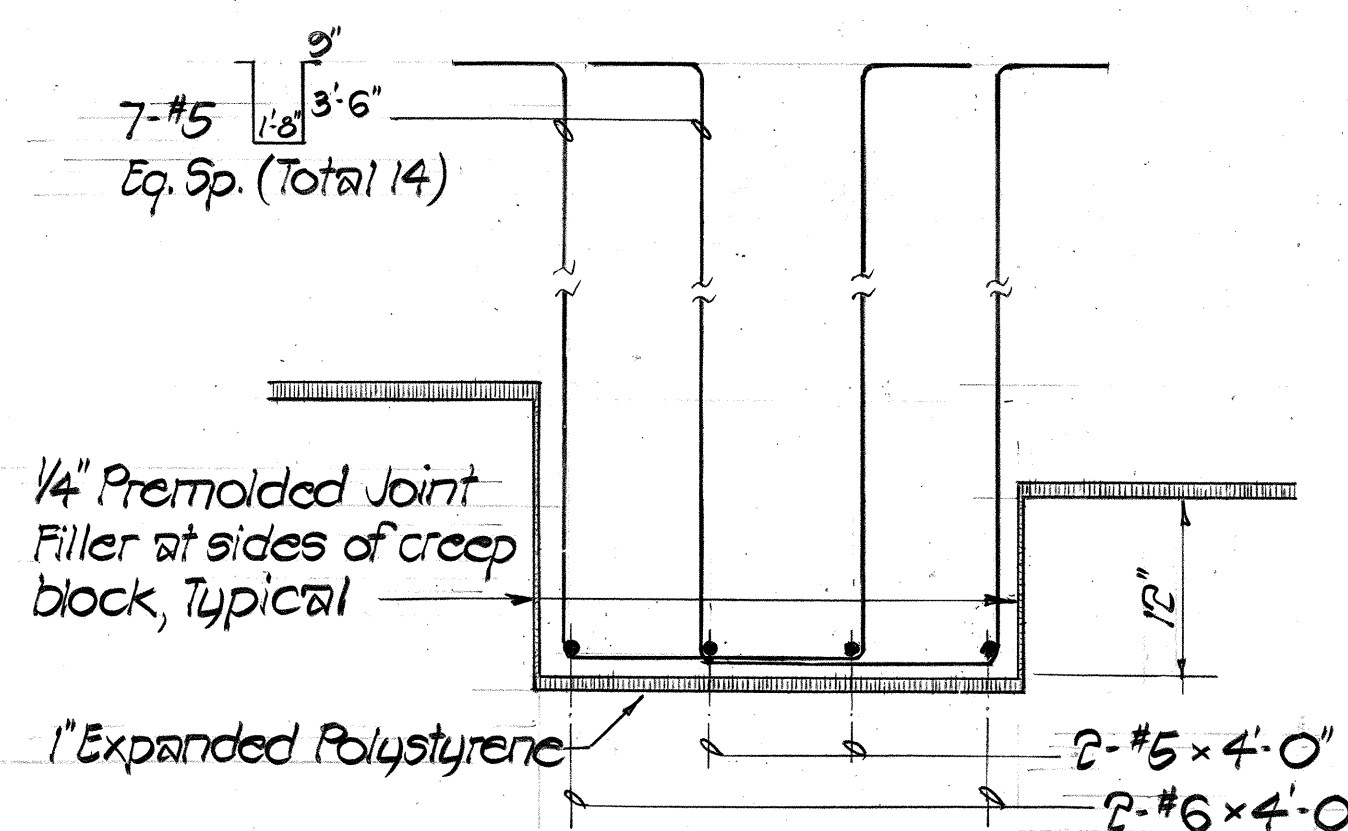
NOTE:

Balance of reinf. same as Section A/5-31 except as noted.

For Creep Block Details, see Sections C/5-31 & D/5-31.



PLAN ~ SECTION (C)
Scale: 1" = 1'-0"



FRONT ELEVATION (D)
Scale: 1" = 1'-0"

DATE	_____
SURVEY PLOTTED BY	_____
DESIGNED BY	_____
TRACED BY	_____
NOTE BOOK	_____
QUANTITIES BY	_____
CHECKED BY	_____
No.	_____

ENGINEERING DESIGN GROUP, INC.
Consulting Engineers
225 Queen St., Suite 504
Honolulu, Hawaii 96813

REGISTERED PROFESSIONAL ENGINEERS
No. 2159
HAWAII, U.S.A.

This work was prepared by me or under my supervision.

Edgar K. M. Lee

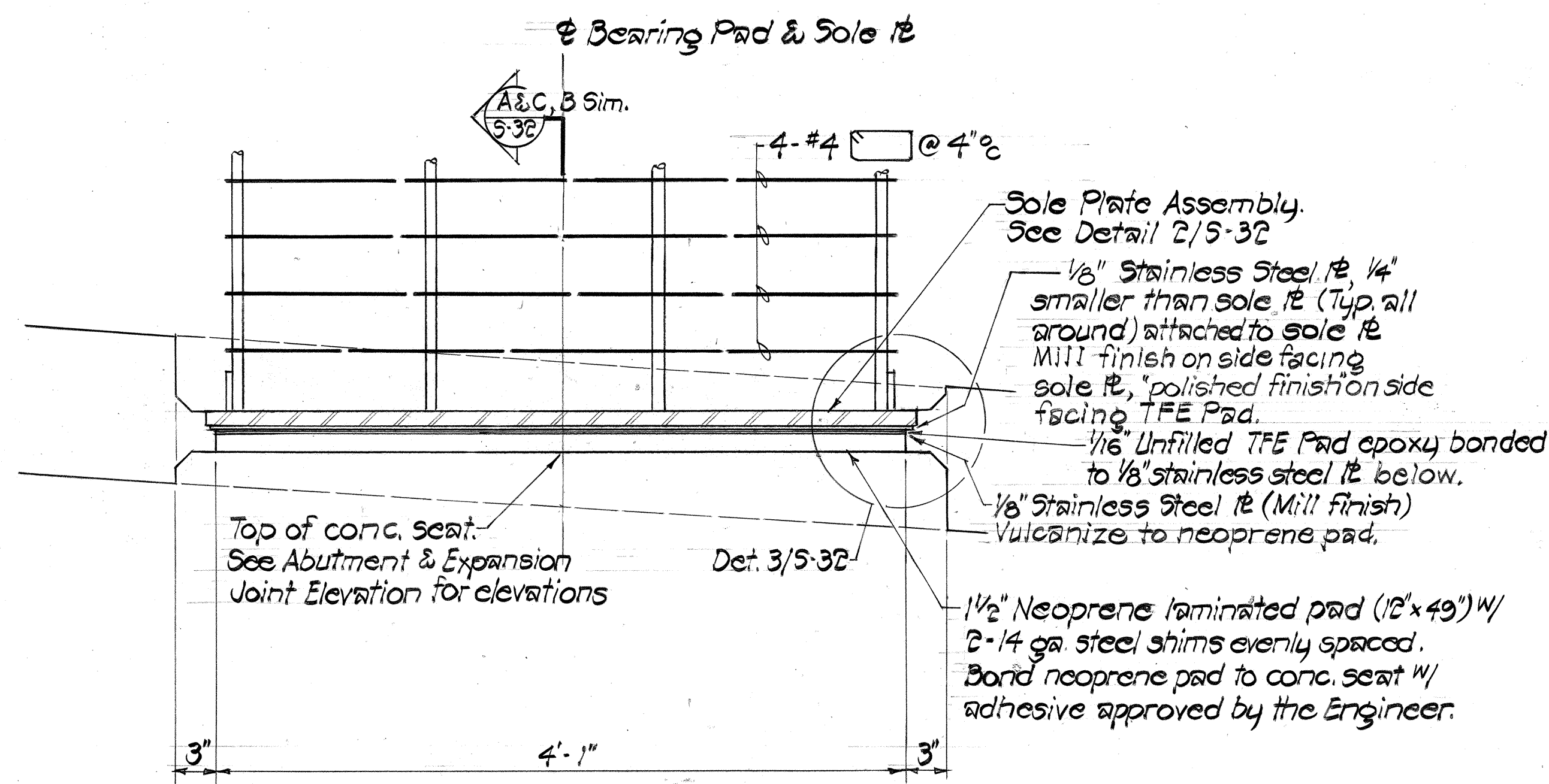
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

EXPANSION JOINT SECTIONS

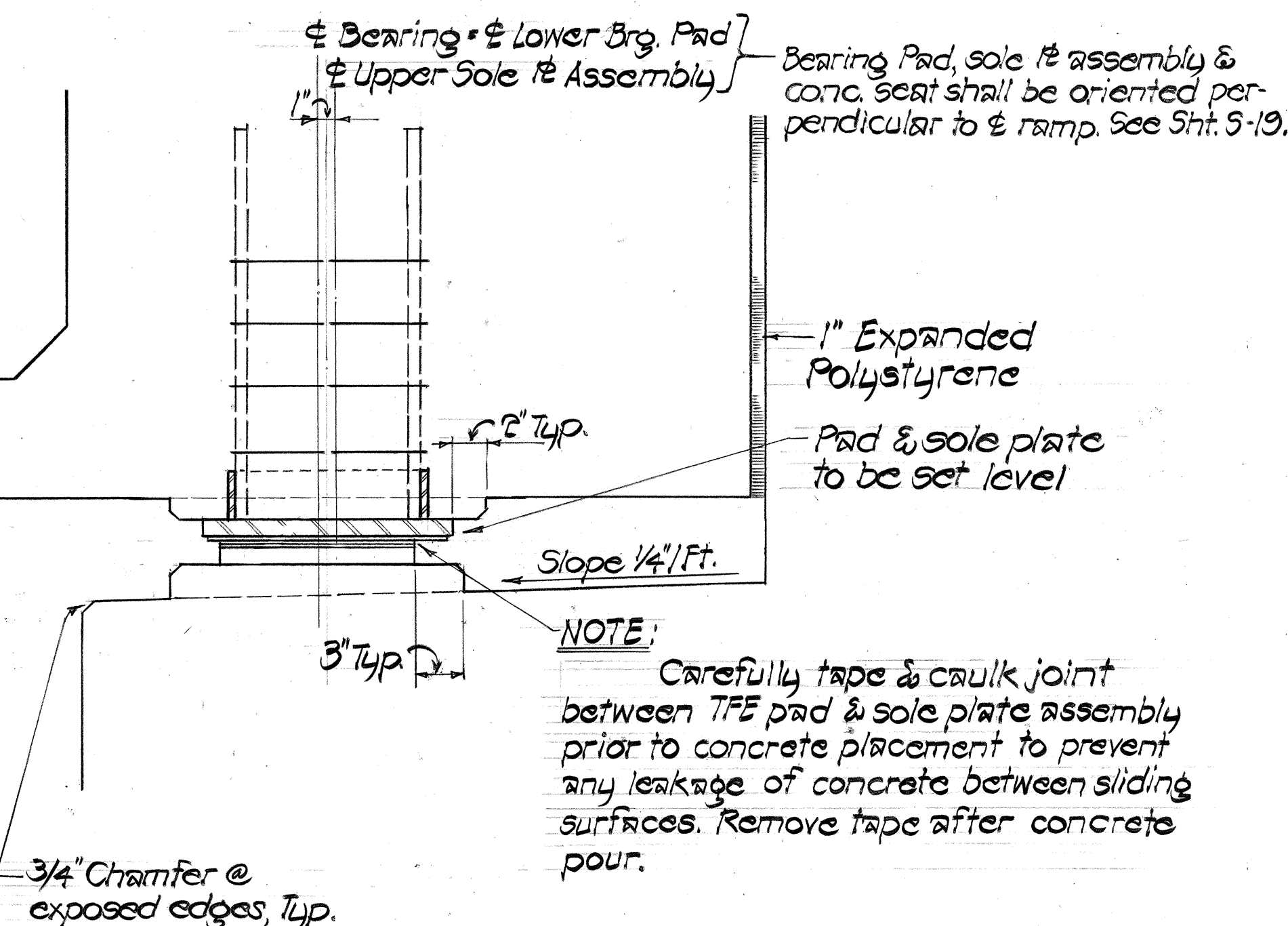
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24
Scale: As Noted Date: 4-10-80

SHEET No. 5-31 OF 5-41 SHEETS

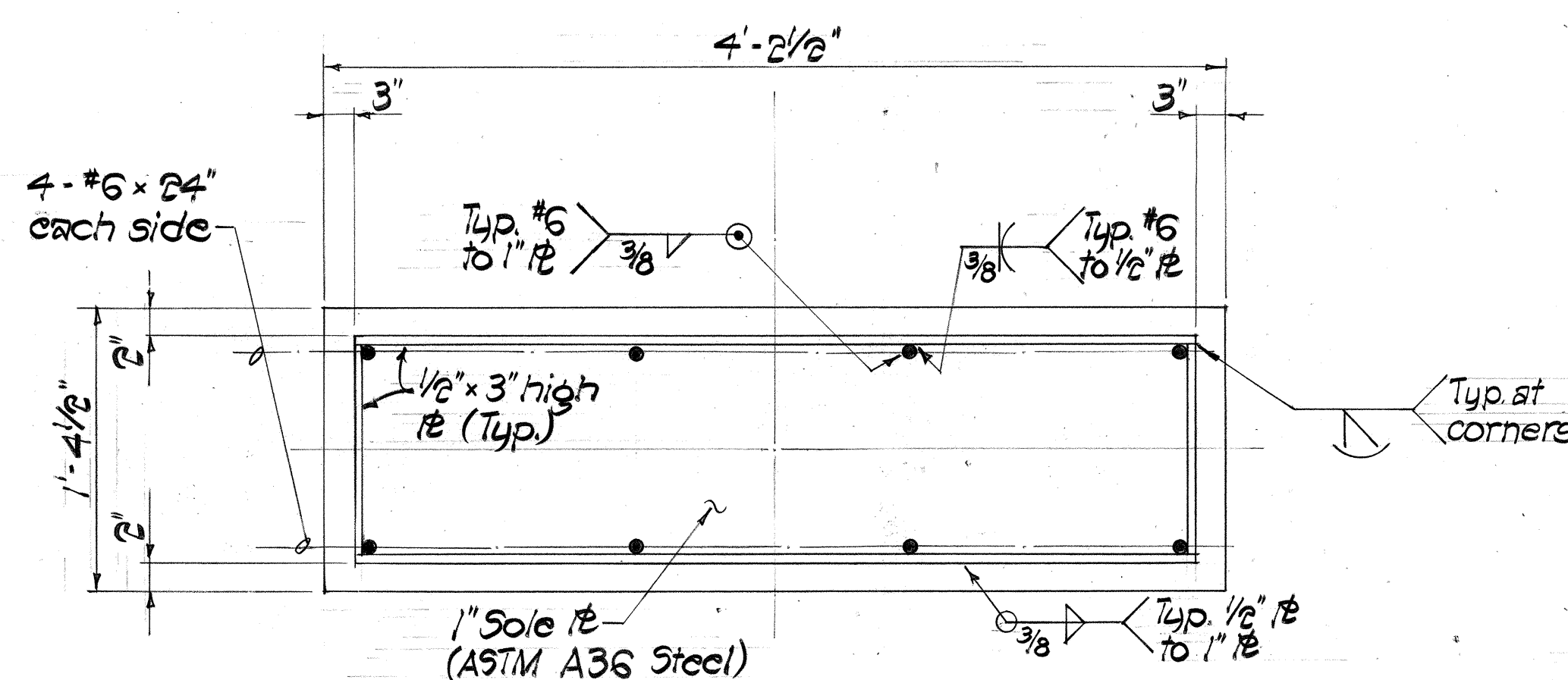
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	99	146



EXPANSION JOINT BEARING ASSEMBLY DETAIL 1
Scale: 1 1/2" = 1'-0"
S-32

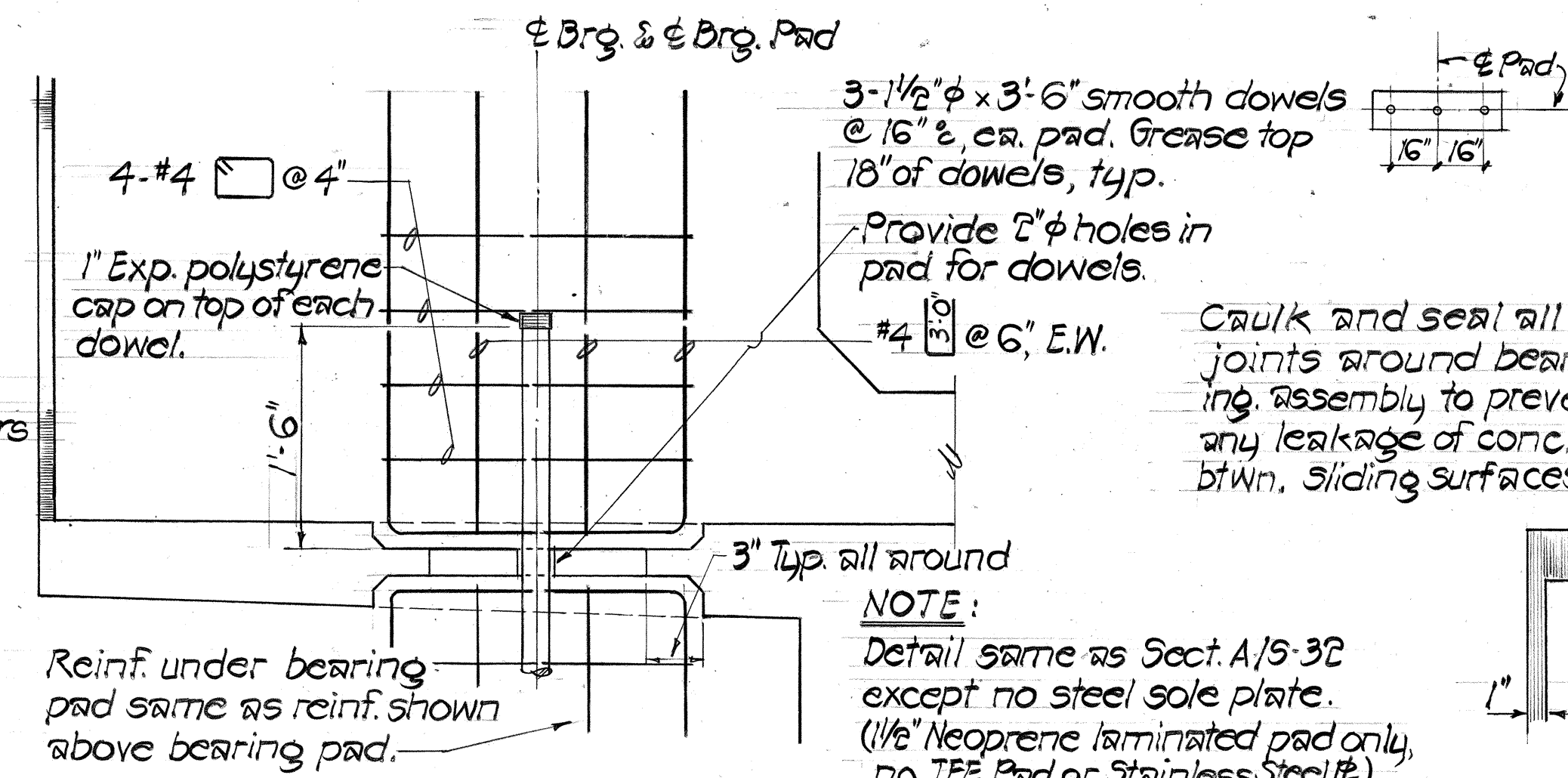


TYPICAL SECTION - ABUT. NO. 2
Scale: 1 1/2" = 1'-0"
S-32

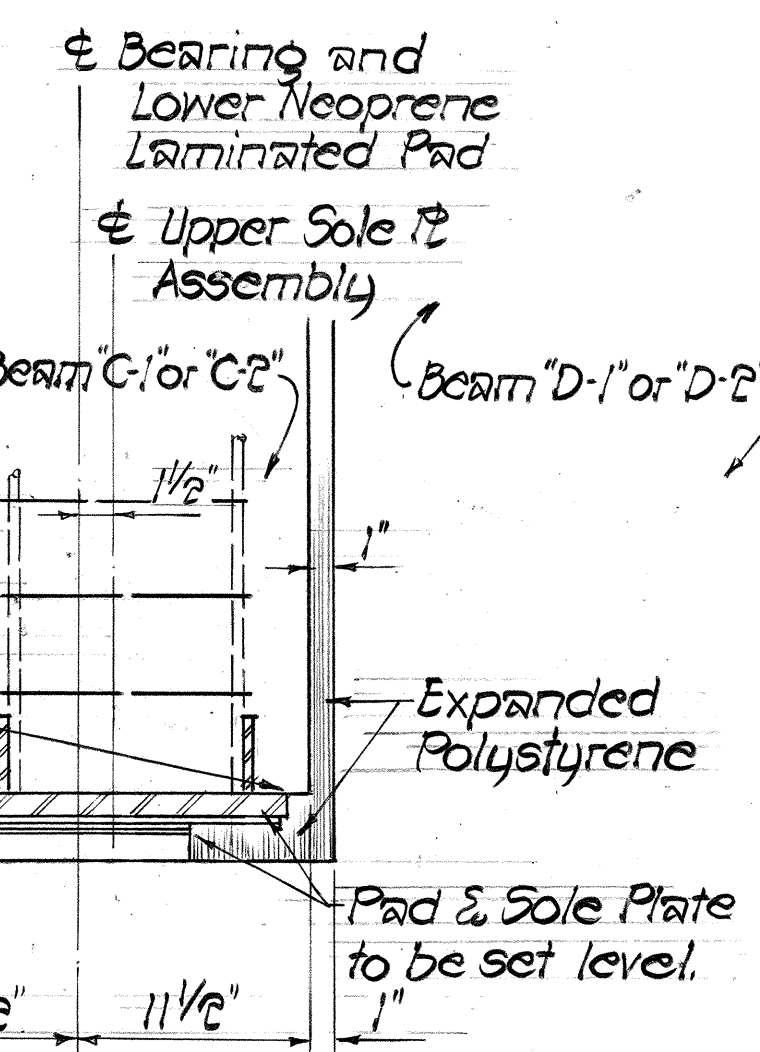


NOTE:
Galvanize assembly after fabrication.

STEEL SOLE PLATE ASSEMBLY DETAIL 2
Scale: 1 1/2" = 1'-0"
S-32



TYPICAL SECTION - ABUT. NO. 1
Scale: 1 1/2" = 1'-0"
S-32

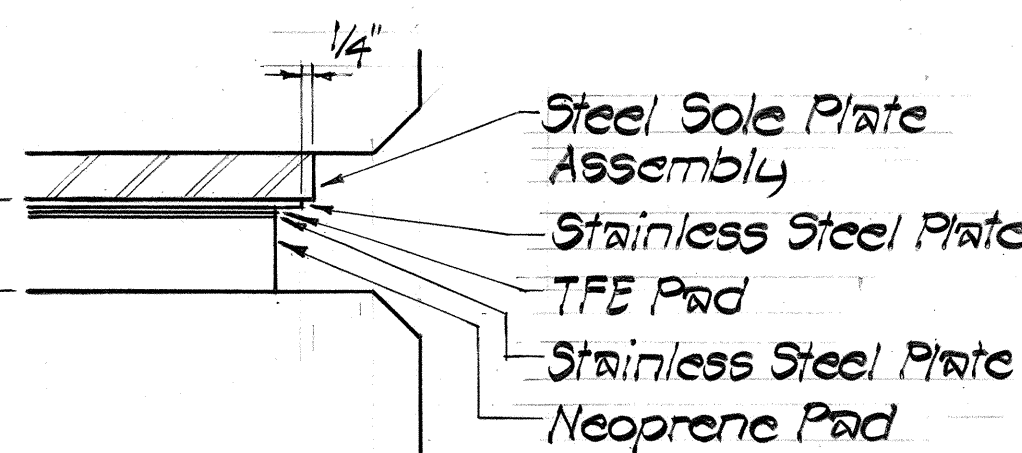


TYPICAL SECTION - EXP. JOINT
Scale: 1 1/2" = 1'-0"
S-32

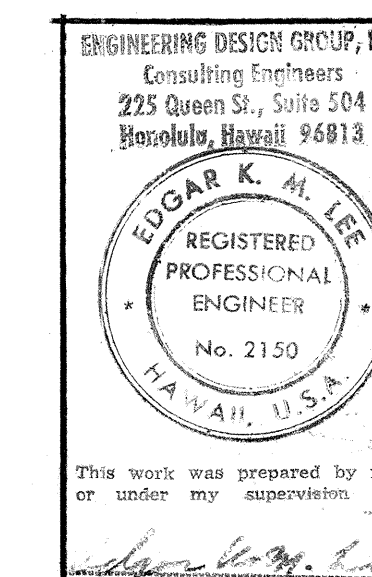
NOTE:
1. Vertical surfaces at expanded polystyrene against which concrete is to be placed shall be faced with hardboard or other suitable material which is approved by the Engineer.
2. Sole plate assembly & neoprene laminated pad same as in Det. 1/S-32.
3. Provide shop drawings of bearing assembly for review prior to fabrication.

Steel Sole Plate Assembly paid for under structural steel for H-1-K Bridge Work.

This portion of expansion joint bearing assembly paid for under Bearing & Expansion Plates Work.



DETAIL 3
Scale: 3" = 1'-0"
S-32



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

EXPANSION JOINT BEARING DETAILS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1 (157)-24

Scale: As Noted Date: 4-10-80

SHEET No. 6-32 OF 41 SHEETS

Condition @ Rail.
Sect. A1/5-33 only

*Install at min. width recommended by manufacturer.

Face of Con. Rail @ Sect. A

Top of Deck

Coat w/ Bitumastic Compound before installing

Galv. Anchor Bolts (1/2" x 6" @ 9" c)

Coat w/ epoxy just prior to making closure pour.

6.0" ±

1" ±

1-#4 cont. ea. side of anchor bolts

Waterstop Gasket

Aluminum Frame (6061-T6)

Neoprene

4.0" *

1.0"

Varies

1/25"

1/25"

8.0" ±

Limits of Exp. Jt. Sealer

A1/5-33

A 5-33

PARTIAL ELEVATION

EXPANSION JOINT

Scale: 1/8" = 1'-0"

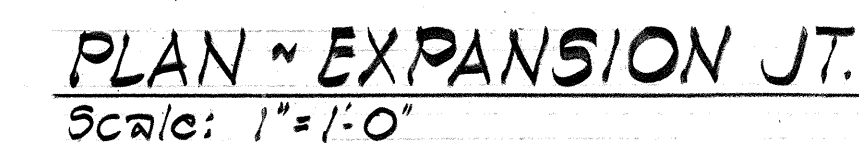
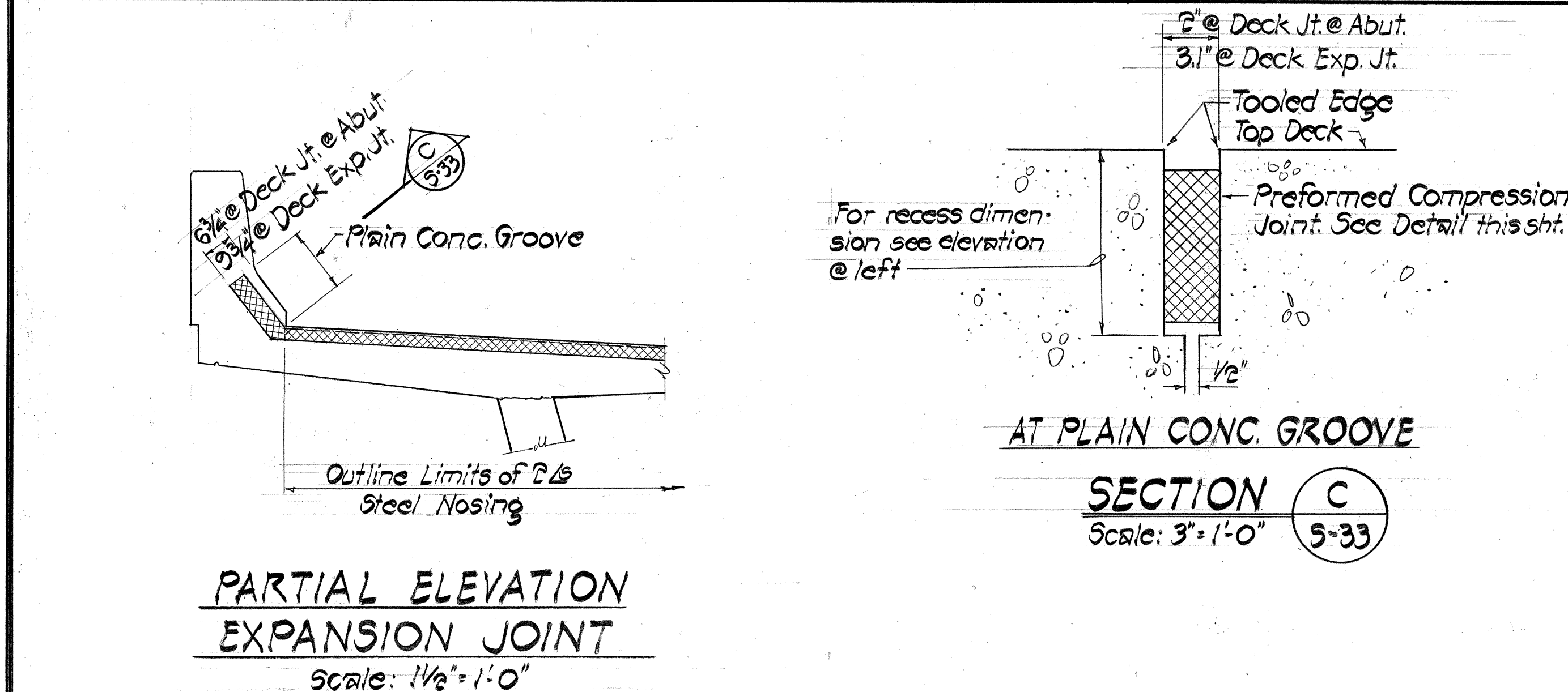
SECTION A

SECTION A1

Scale: Half Scale

Sim. as noted

NOTE:
The deck expansion joint illustrated is based on the Delastiflex Type SM400 joint. The exact joint configuration will be dependent on the actual type used.

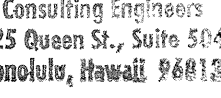


- NOTES:
1. The details shown indicate the general deck expansion joint installation requirements. The exact details shall be adjusted to suit the recommendations of the manufacturer of the actual deck expansion joint used. Verify all installation requirements and submit any required revisions to the Engineer for review prior to construction.
 2. Deck expansion joints shall not be installed until 45 days after all post-tensioning work has been completed. Joint at abutment can be installed anytime after the post-tensioning work has been completed.

ORIGINAL PLAN	SURVEY PLOTTED BY _____		DATE _____
	DRAWN BY _____	BY _____	BY _____
	TRACED BY _____	DESIGNED BY _____	BY _____
NOTE BOOK	QUANTITIES BY _____		BY _____
No. _____	CHECKED BY _____	BY _____	BY _____

ENGINEERING DESIGN GROUP, INC.
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Honolulu, Hawaii 96810

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION



EXPANSION JOINT DETAILS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24

Scale: As Noted Date: 4-10-80

Edgar K. M. Lee

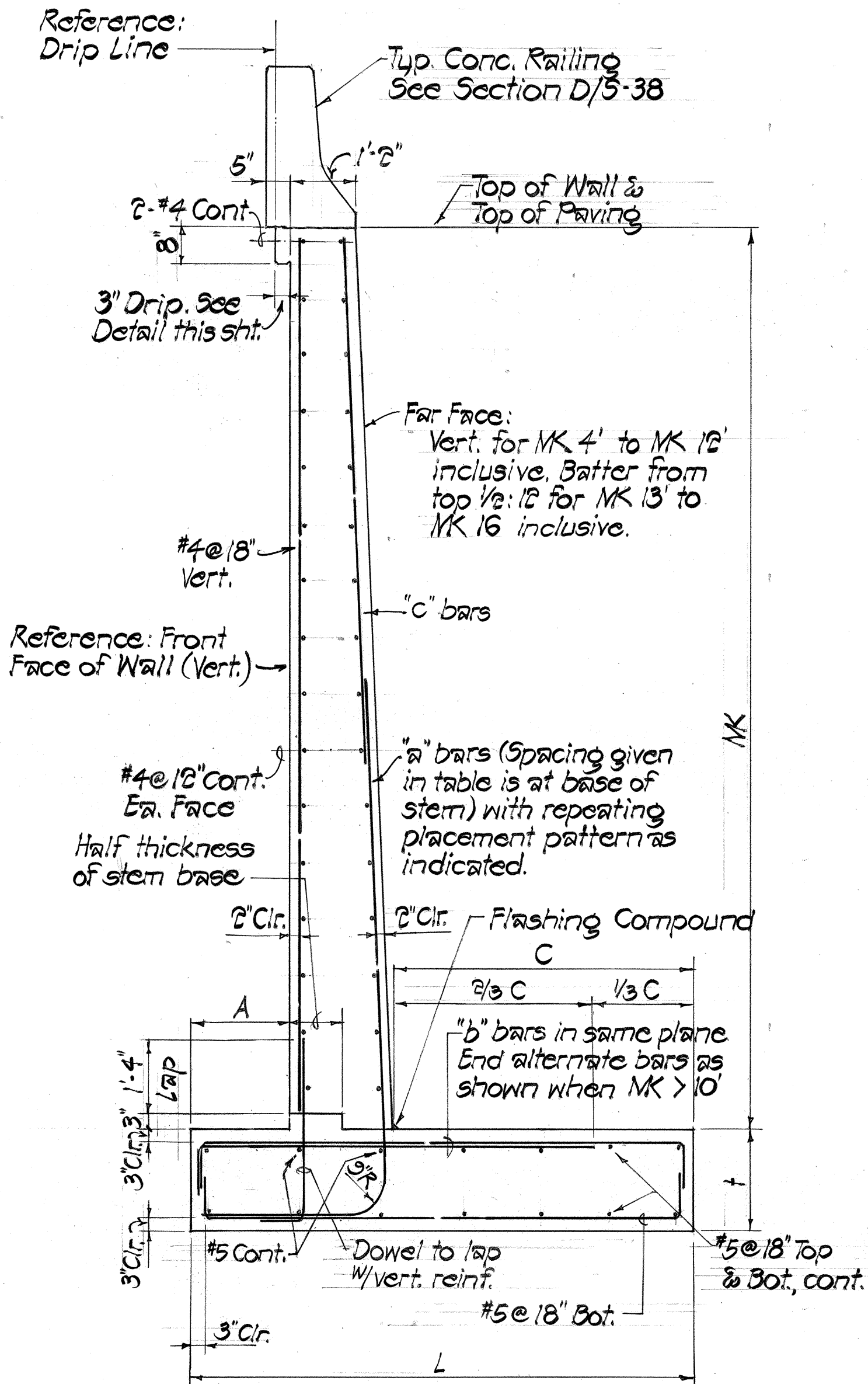
SHEET NO. 6-33 OF 6-4

SHEETS

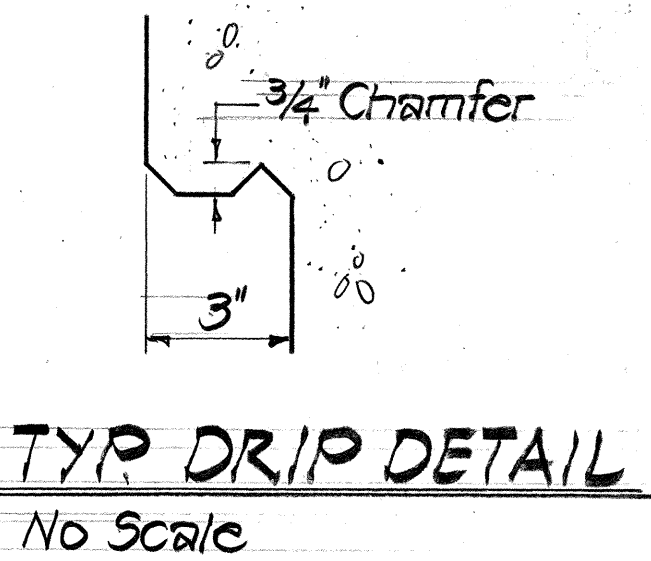
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	101	146

~ SCHEDULE OF DIMENSIONS & REINFORCING STEEL ~

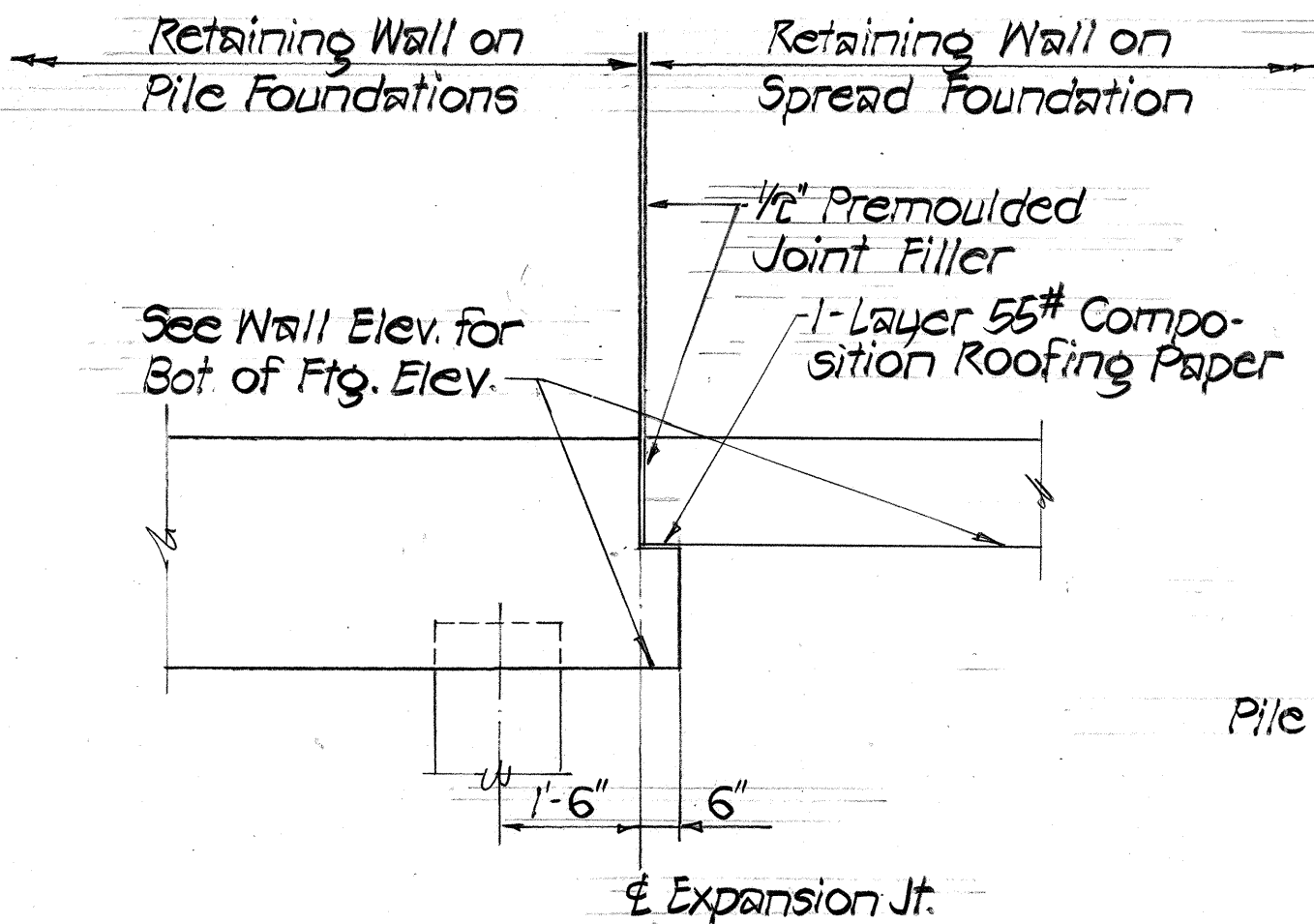
WALL ON SPREAD FOUNDATIONS							WALL ON PILE FOUNDATIONS						
Maximum "MK"	A	t	L	"a" bars	"b" bars	"c" bars	Max. "MK"	A	t	L	"a" bars	"b" bars	"c" bars
4	6"	14"	3.0'	#5@9"	#5@9"	~							
5	6"	14"	3.5'	#5@9"	#5@9"	~							
6	6"	14"	4.0'	#5@9"	#5@9"	~							
7	12"	14"	5.0'	#5@9"	#5@9"	~	7P	18"	30'	7.0'	#5@9"	#5@9"	~
8	12"	14"	5.5'	#5@9"	#5@9"	~	8P	18"	30'	7.0'	#5@9"	#5@9"	~
9	12"	14"	6.0'	#5@7"	#5@7"	~	9P	18"	30'	7.0'	#5@7"	#5@7"	~
10	12"	14"	7.0'	#6@7"	#6@7"	~	10P	18"	30'	7.5'	#6@7"	#5@7"	~
11	12"	14"	7.0'	#6@6"	#6@6"	#6@12"	11P	18"	30'	8.0'	#6@6"	#5@6"	#6@12"
							12P	18"	30'	8.0'	#6@5"	#5@5"	#6@10"
							13P	18"	30'	9.0'	#7@8"	#6@8"	#7@16"
							14P	18"	30'	9.0'	#7@6"	#6@6"	#7@18"
							15P	18"	30'	9.5'	#7@6"	#6@6"	#7@18"
							16P	18"	30'	9.5'	#8@6"	#7@6"	#7@18"



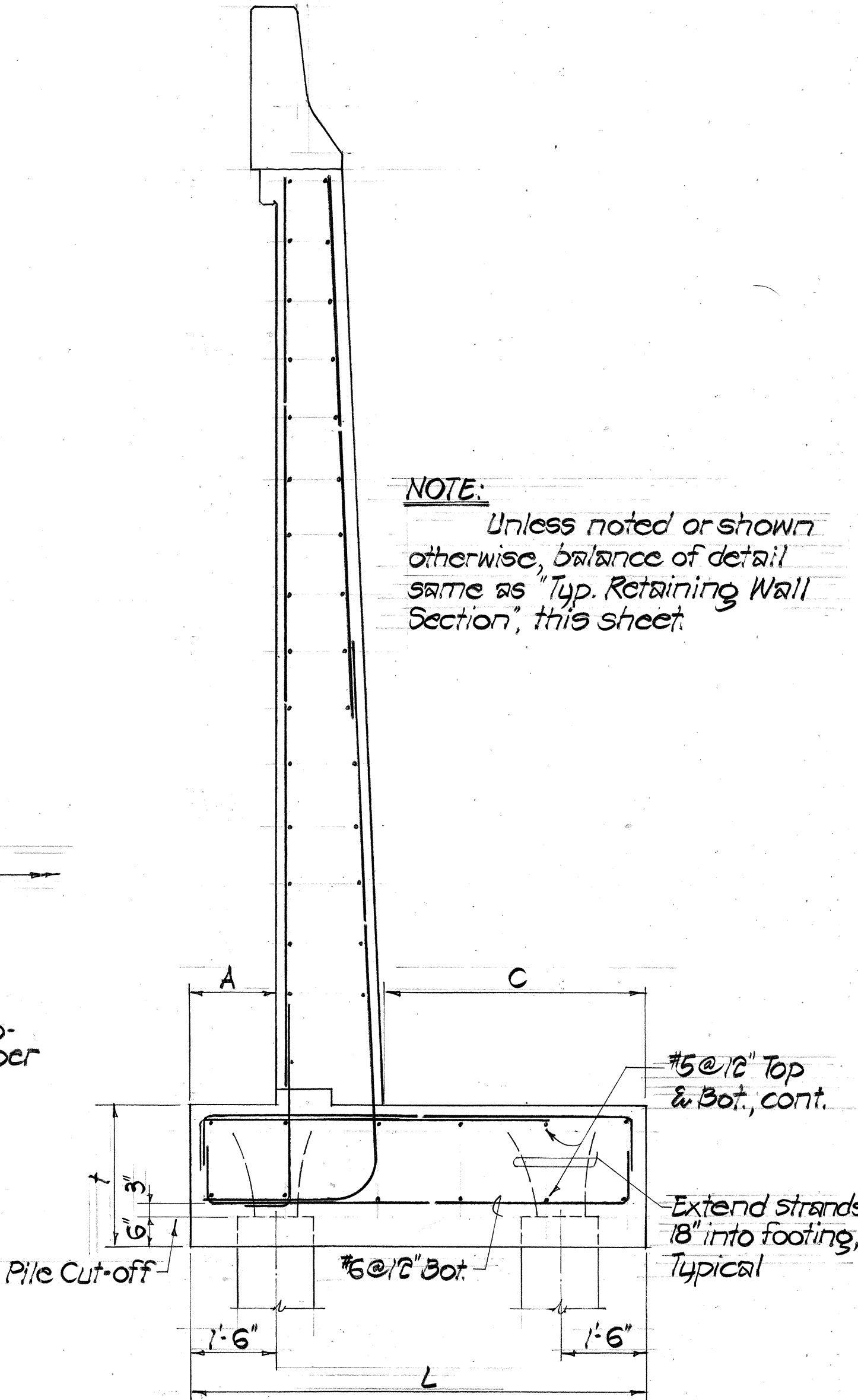
TYPICAL RETAINING WALL SECTION
No Scale



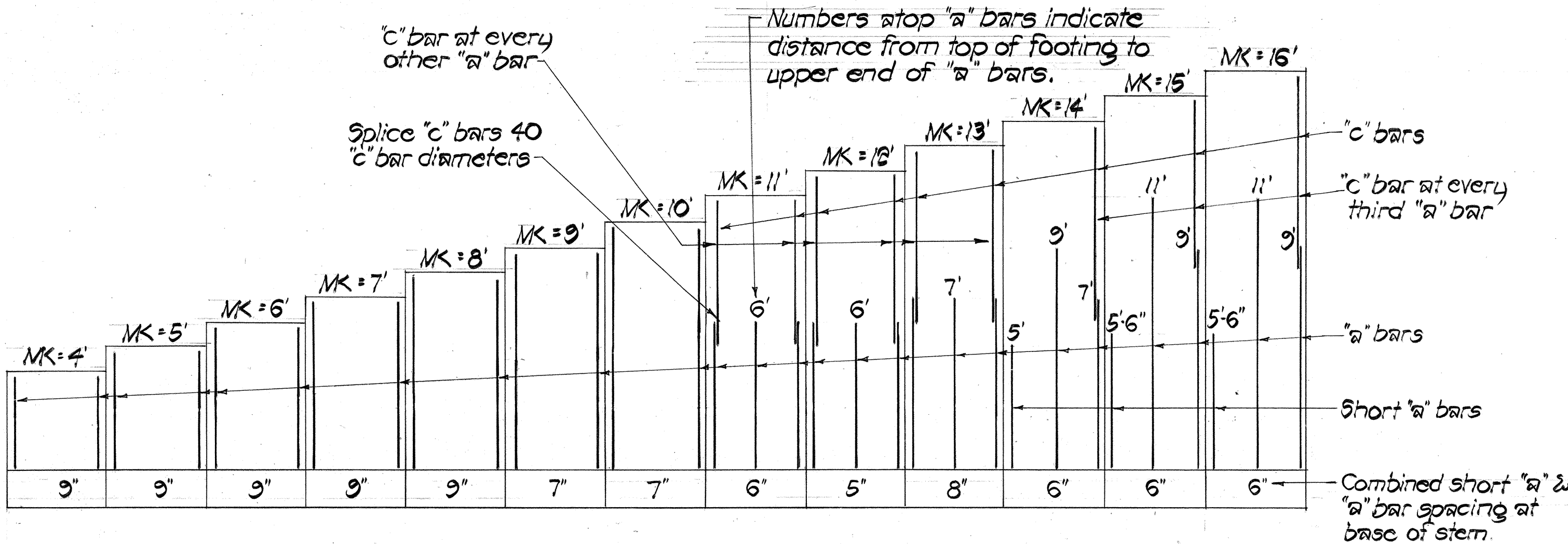
TYP DRIp DETAIL
No Scale



TRANSITION BETWEEN PILE & SPREAD FOUNDATION
No Scale

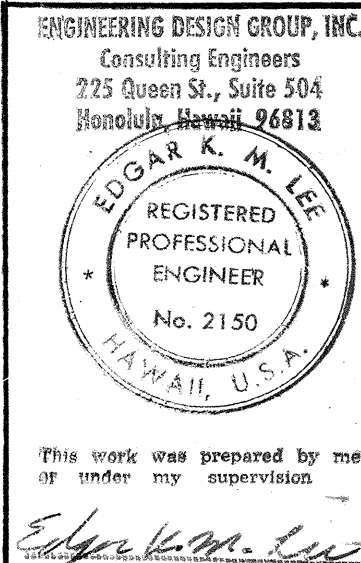


TYPICAL RETAINING WALL ON PILE FOUNDATION
No Scale

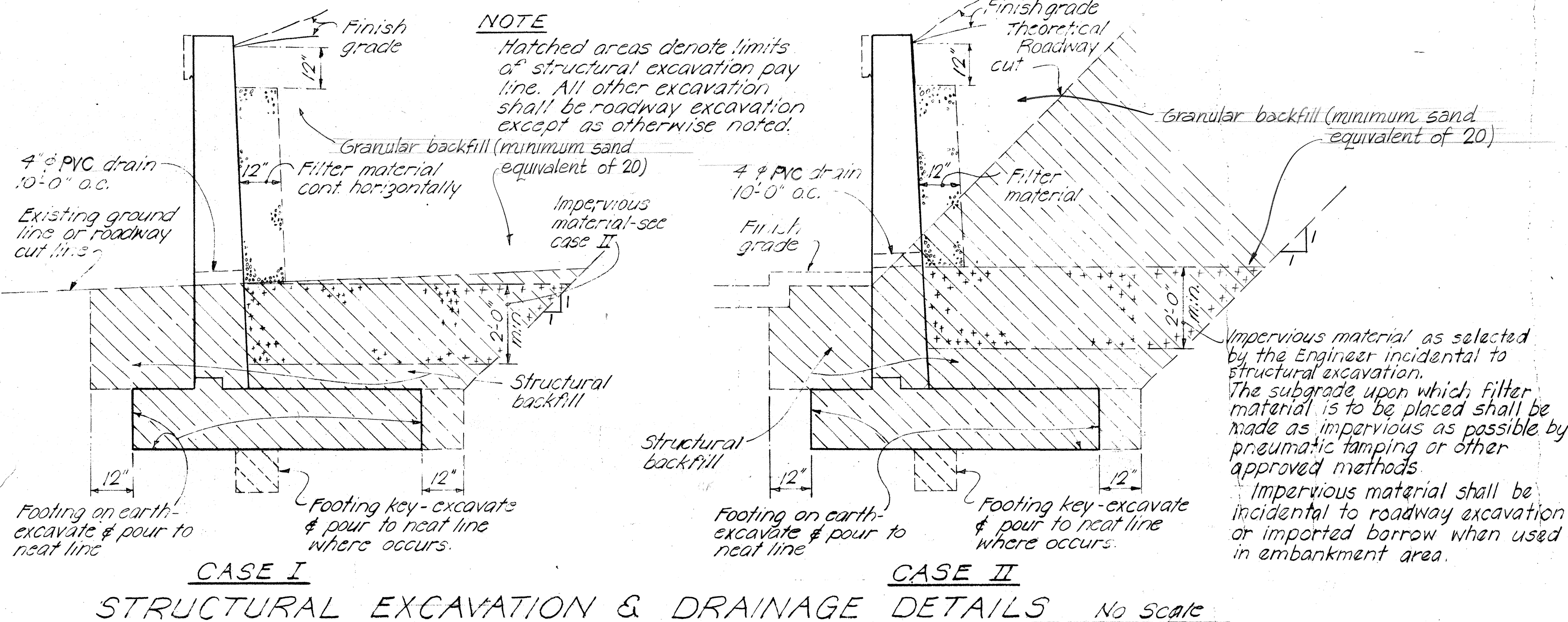
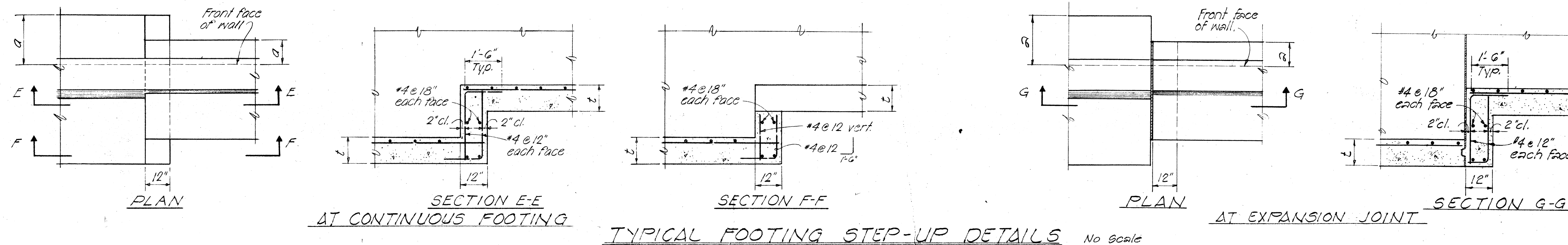
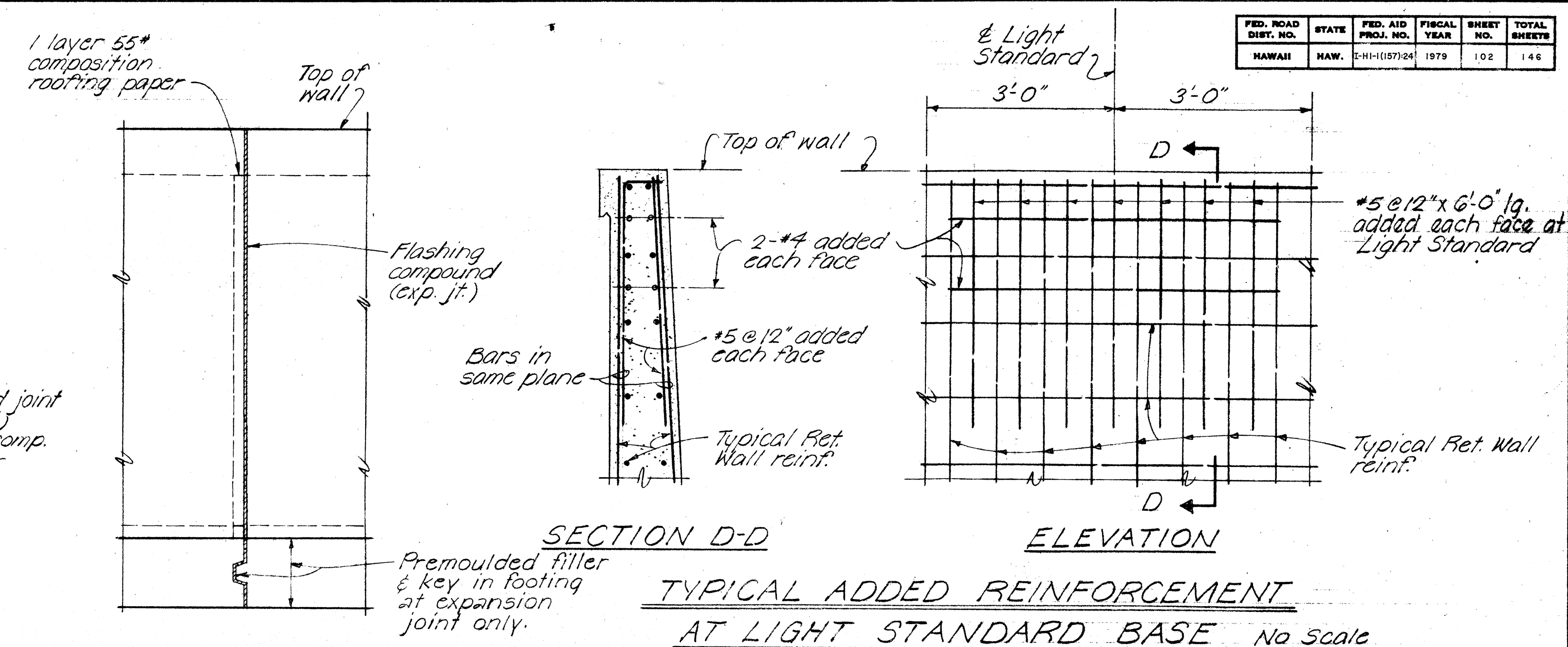
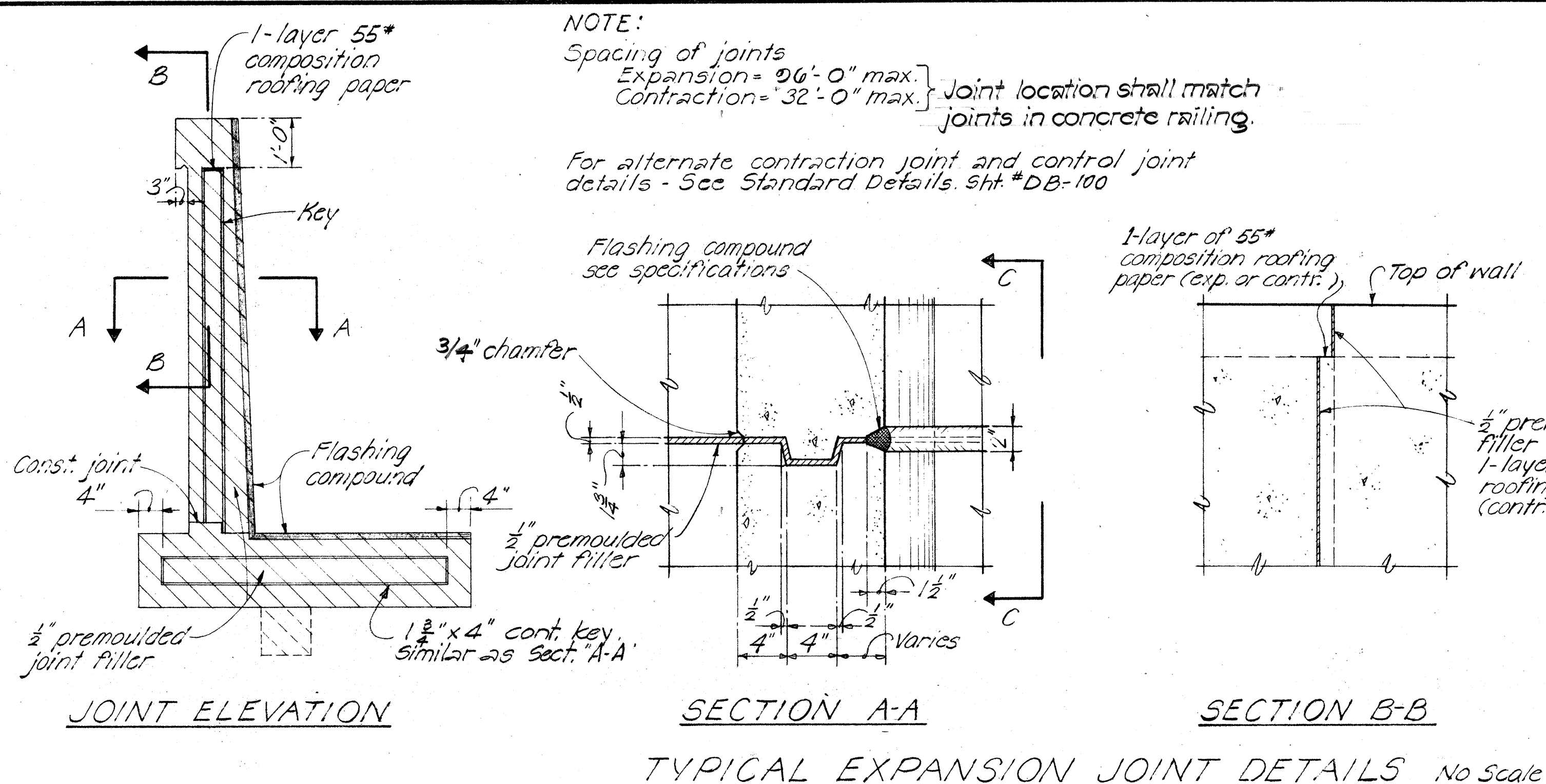


VERTICAL REINFORCING STEEL ARRANGEMENT
No Scale

NOTE:
For Typical Retaining Wall Details, see Sheet No. S-35.



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
RETAINING WALL SECTION, SCHEDULE AND DETAILS
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24
Scale: As Noted Date: 4-10-80
SHEET No. 6-34 OF 6-41 SHEETS



NO.	REVISION	APPROVED BY	DATE

ENGINEERING DESIGN GROUP, INC.
Consulting Engineers
225 Queen St., Suite 504
Honolulu, Hawaii 96813

REGISTERED PROFESSIONAL ENGINEER
No. 2150
HAWAII, U.S.A.

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

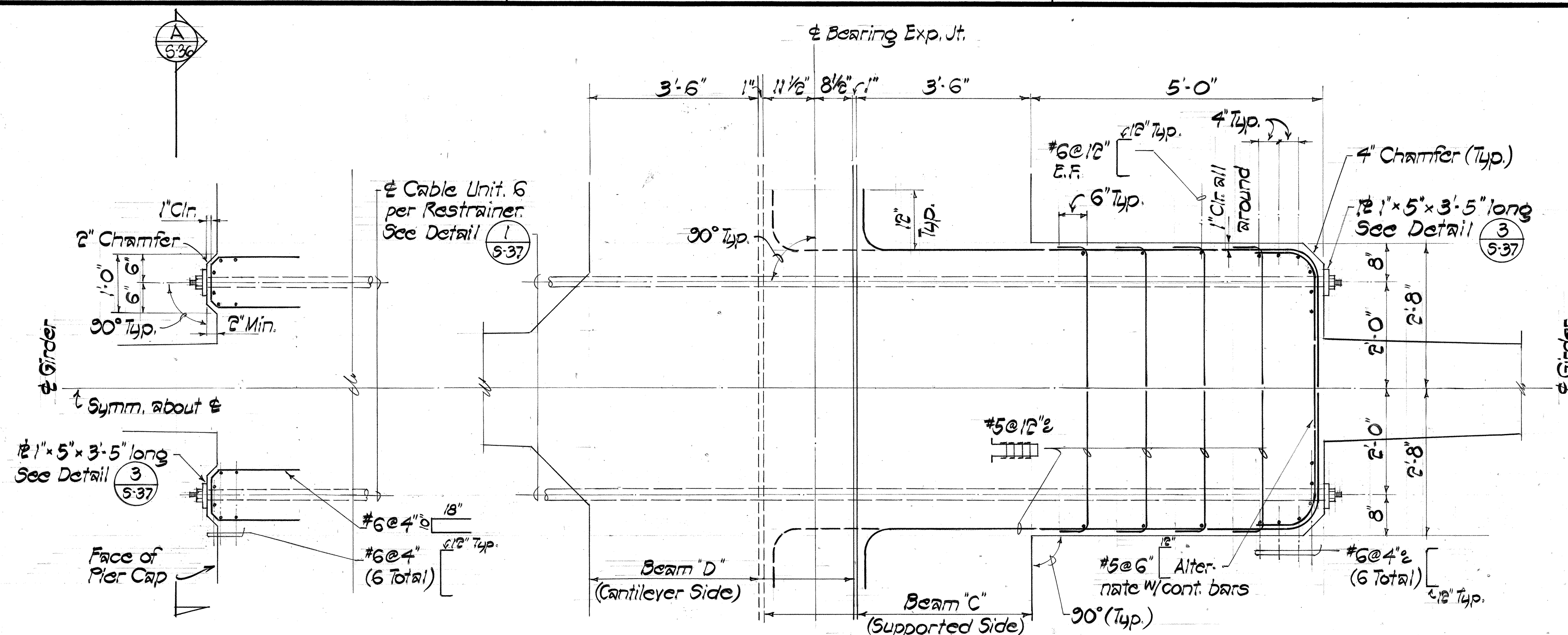
RETAINING WALL DETAILS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. 1-HI-1 (157) 24

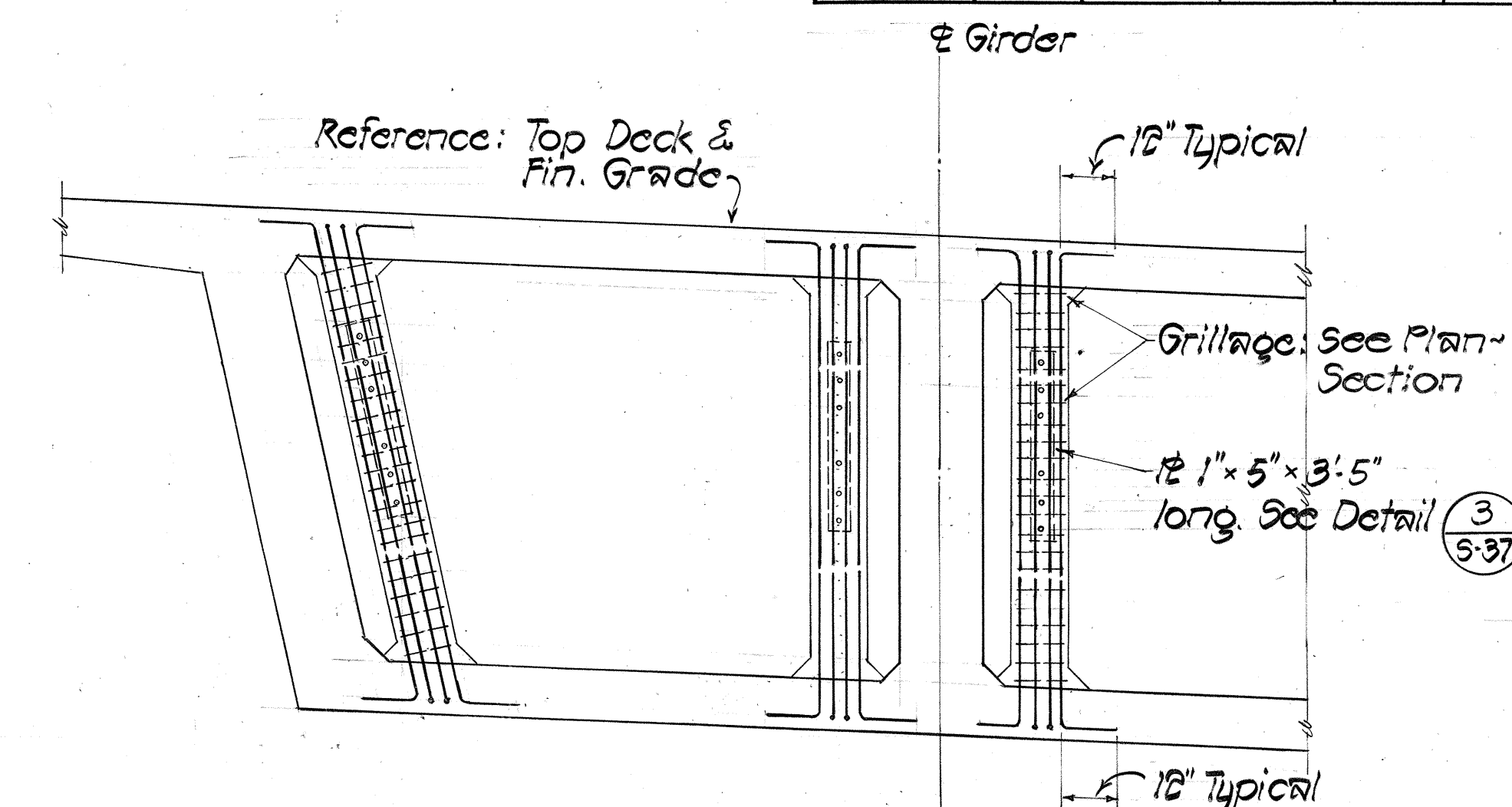
Scale: None Date: 4-10-80

SHEET No 5-35 OF 5-4 SHEETS

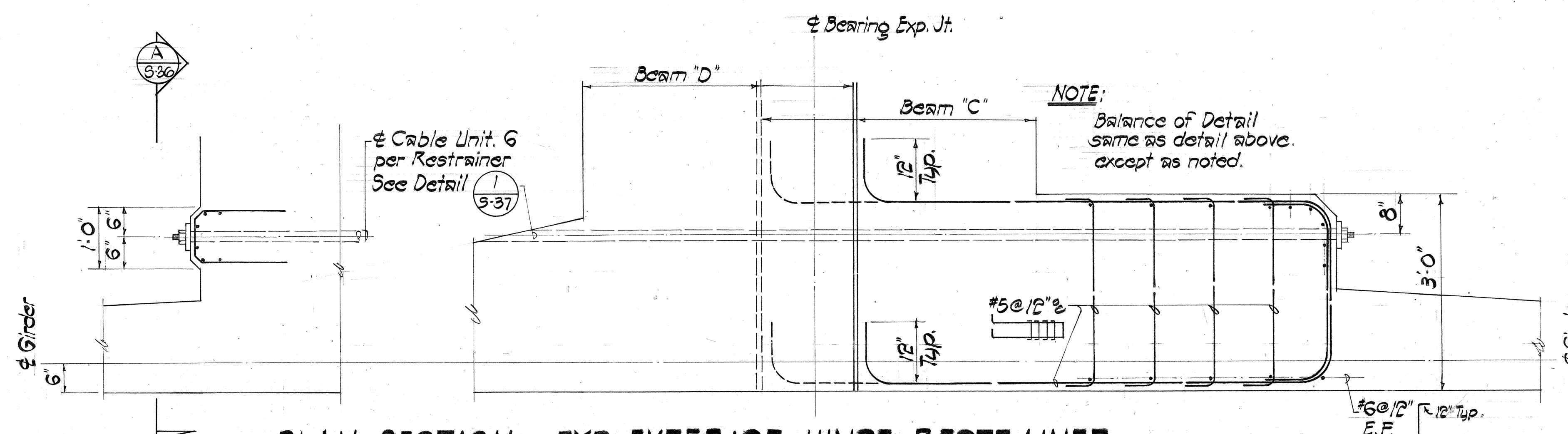
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-H1-(157)-24	1979	103	146



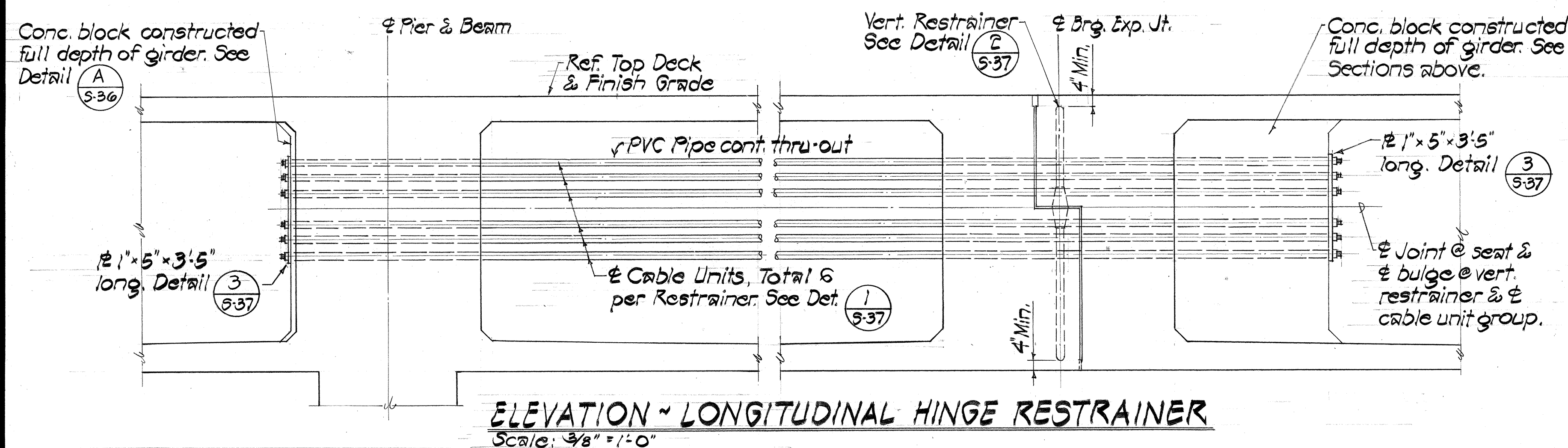
PLAN-SECTION @ TYP INTERIOR HINGE RESTRAINER
Scale: 3/4" = 1'-0"



SECTION A
Scale: 3/8" = 1'-0" 5-36



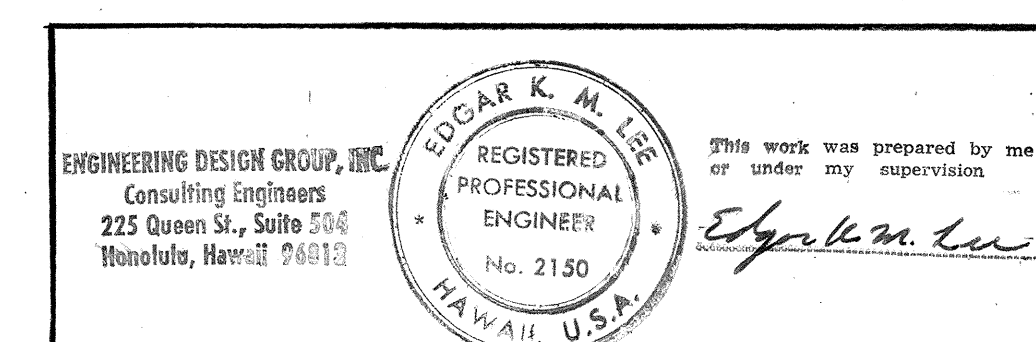
PLAN-SECTION @ TYP EXTERIOR HINGE RESTRAINER
Scale: 3/4" = 1'-0"



ELEVATION ~ LONGITUDINAL HINGE RESTRAINER
Scale: 3/8" = 1'-0"

NOTES:

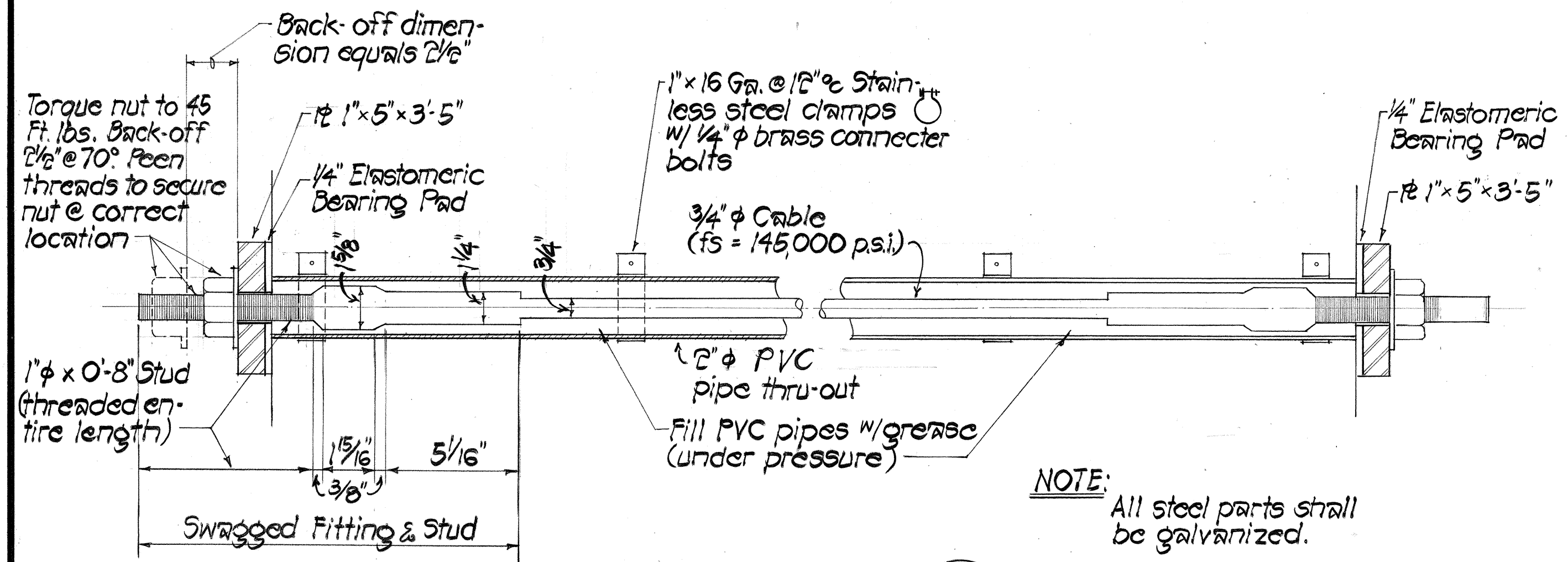
1. Cable units shall not be anchored at both ends prior to prestressing.
2. Location of access openings shall be the Contractor's option, maximum size of access openings shall be 2'-0" x 2'-0". See Std. Dwg. Sht. #DB 101-3, form removal opening similar.
3. All steel parts shall be galvanized.
4. Longitudinal hinge restrainers shall be installed at 90° to expansion joint except where otherwise indicated on Deck Framing Plans.
5. Hinge restrainer shown is at Sta. 8+94. Hinge restrainer at Sta. 4+96 sim., but opposite hand.



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
LONGITUDINAL HINGE RESTRAINER
PLAN, SECTIONS & ELEVATIONS
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. 1-H1-(157)-24
Scale: As Noted Date: 4-10-80
SHEET No. 5-36 OF 5-40 SHEETS

DATE	_____
DESIGNED BY	_____
CHECKED BY	_____
NOTED BY	_____
QUANTITIES BY	_____
ORIGINAL PLAN	_____
NO.	_____

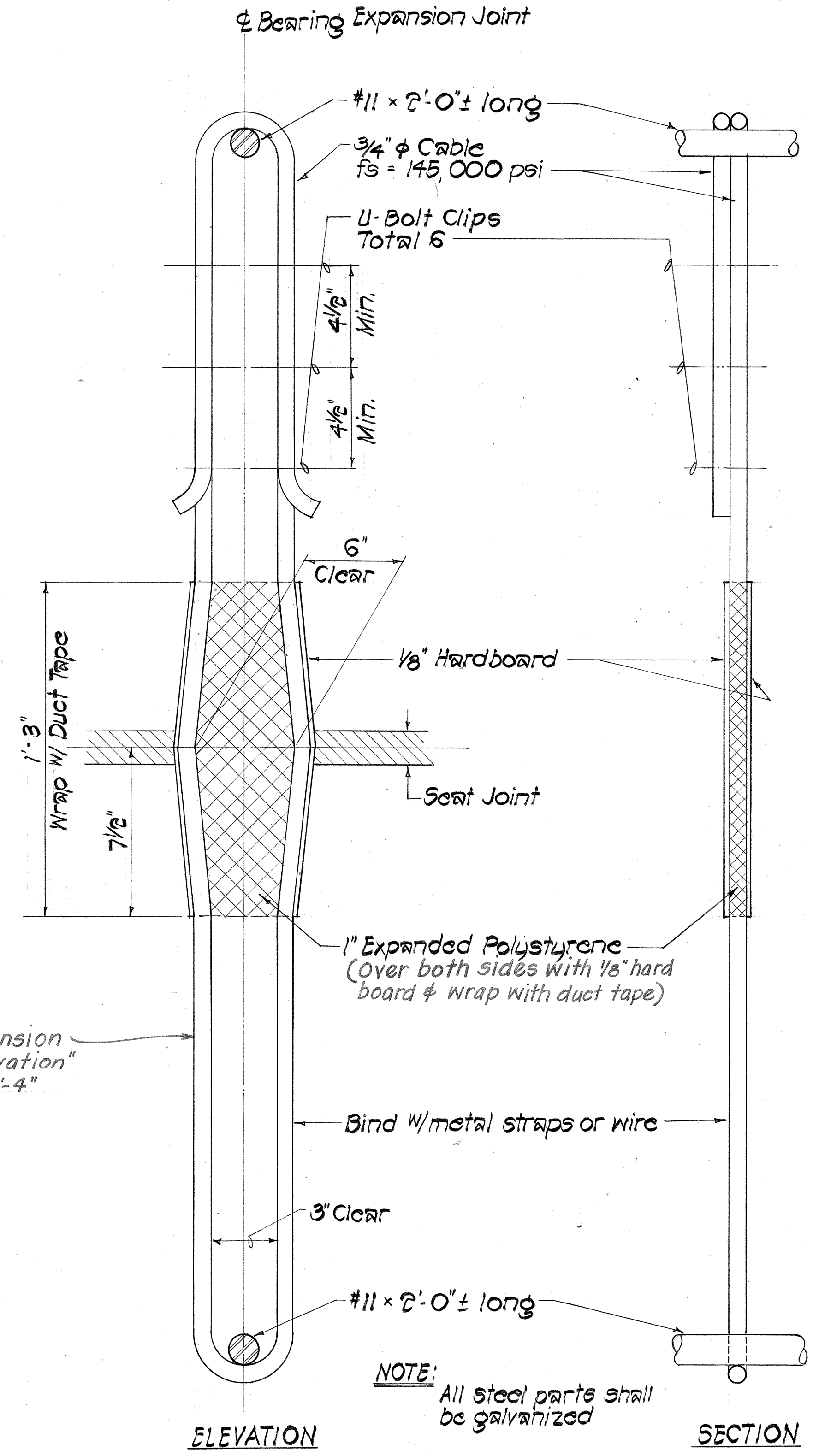
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	104	146



CABLE UNIT 1
Scale: 3" = 1'-0" 5-37

NOTE:
1. Grease (heavy duty) horizontal restrainer cables and encase greased cables with 2-inch PVC ducts. All joints in the PVC ducts shall be sealed water tight as approved by the engineer.

NOTE:
All steel parts shall be galvanized.



VERTICAL RESTRAINER 2
Scale: 3" = 1'-0" 5-37

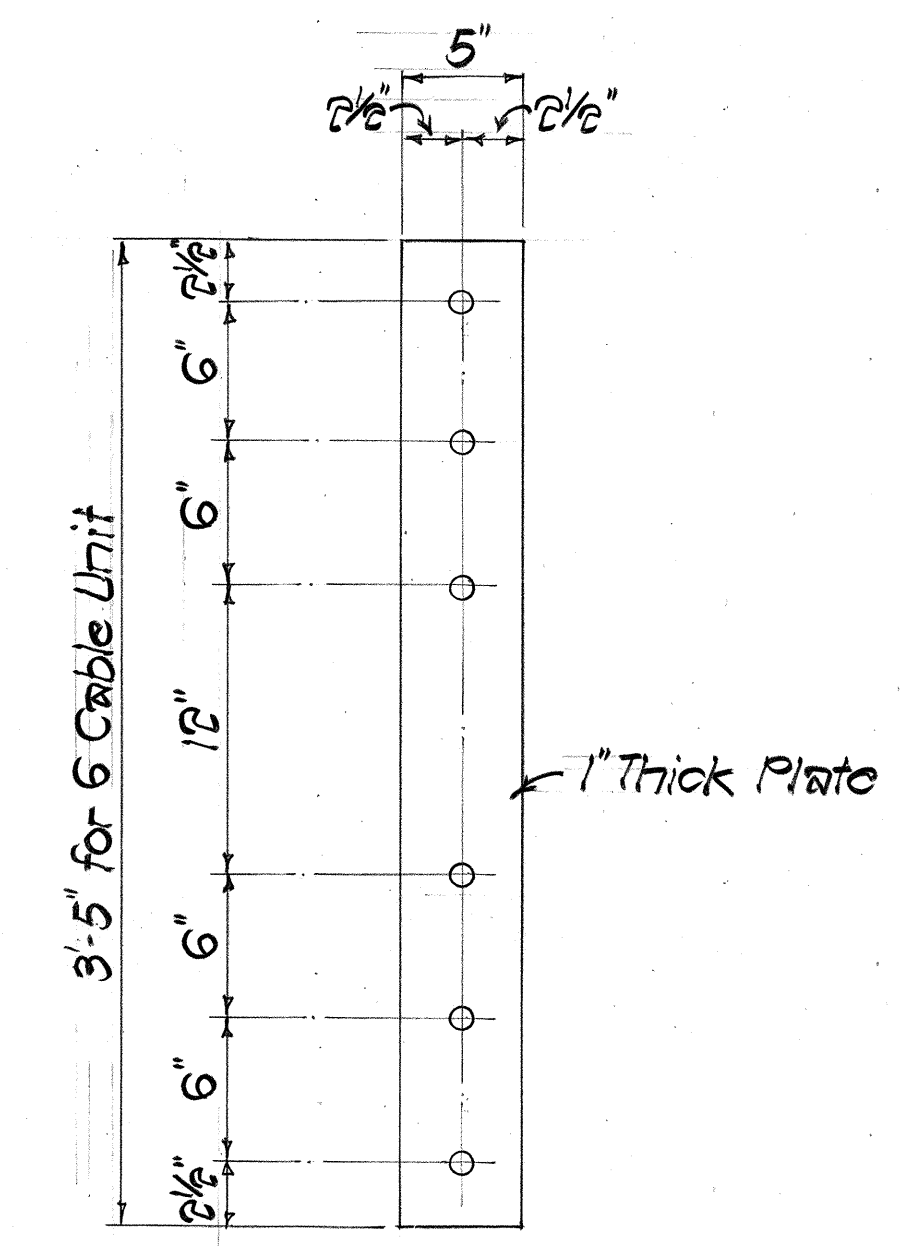


PLATE DETAIL 3
Scale: 1 1/2" = 1'-0" 5-37

NOTE:
Plates shall be considered part of restrainer unit work.

NOTE:
Shop Drawings of all longitudinal & vertical restrainers shall be submitted for review.

SURVEY PLOTTED BY	DATE
DRAWN BY	
CHECKED BY	
NOTED BY	
QUANTITIES BY	
CHECKED BY	

6/16/80	On Detail 537 revised a note & added note No. 1, per Addendum No. 2.
DATE	REVISION

EDGAR K. M. LEE
REGISTERED PROFESSIONAL ENGINEER
No. 2150
HAWAII, U.S.A.

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

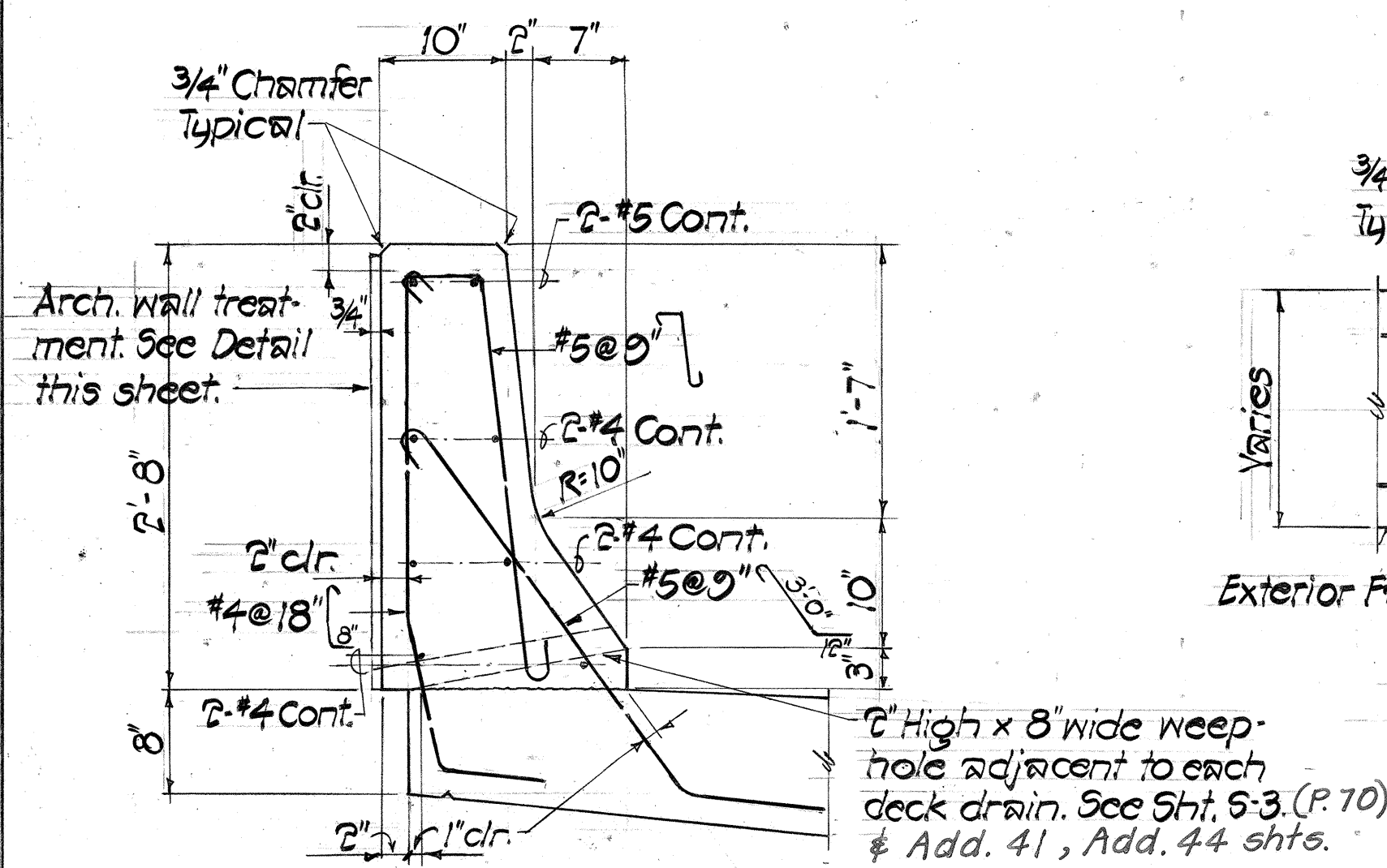
VERTICAL RESTRAINER - ELEVATION & SECTION, CABLE UNIT DETAIL

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
P.A.I. PROJ. NO. I-HI-1(157)-24

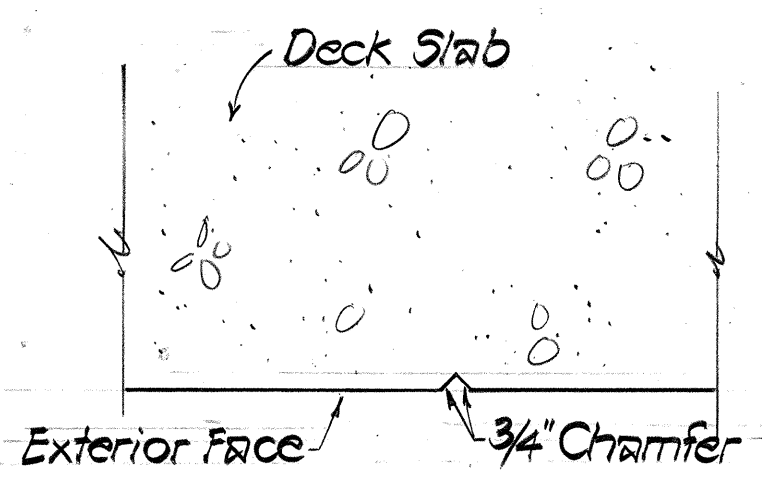
Scale: As Noted Date: 4-10-80

SHEET No. 5-37 OF 6-41 SHEETS

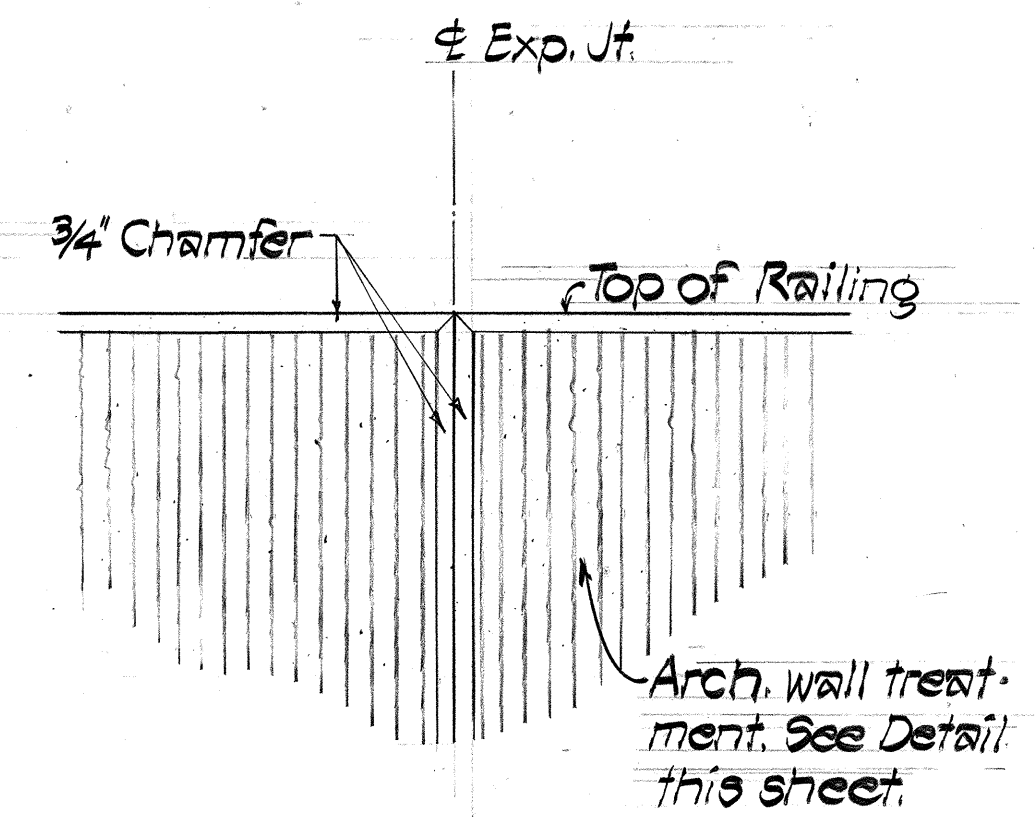
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	105	146



AT EXPANSION JOINT
SECTION B
Scale: 1 1/2" = 1'-0" 5-38



GROOVE IN DECK SLAB BELOW RAILING EXPANSION JOINT
SECTION C
Scale: 1 1/2" = 1'-0" 5-38

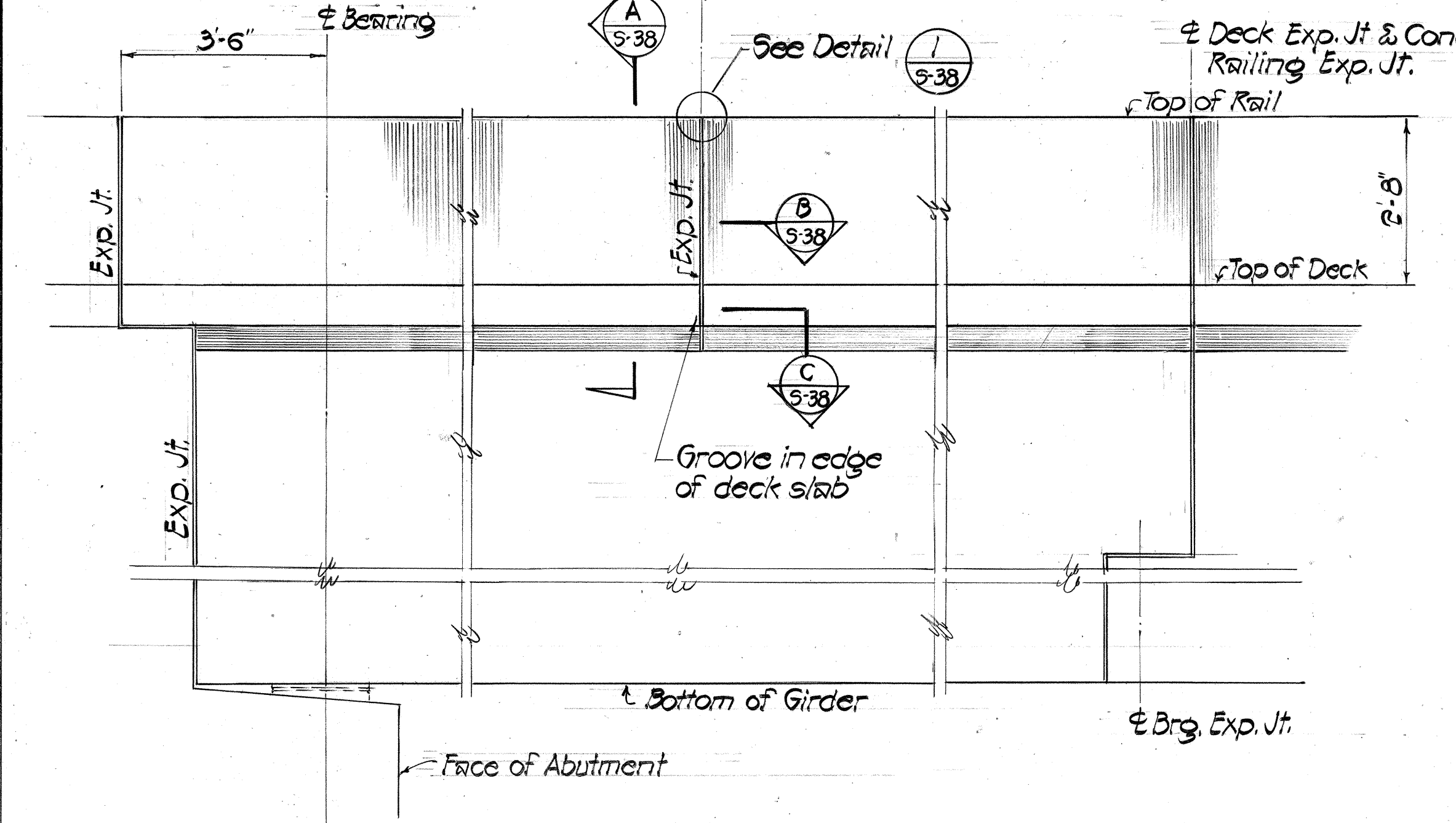


DETAIL 1
Scale: 1 1/2" = 1'-0" 5-38

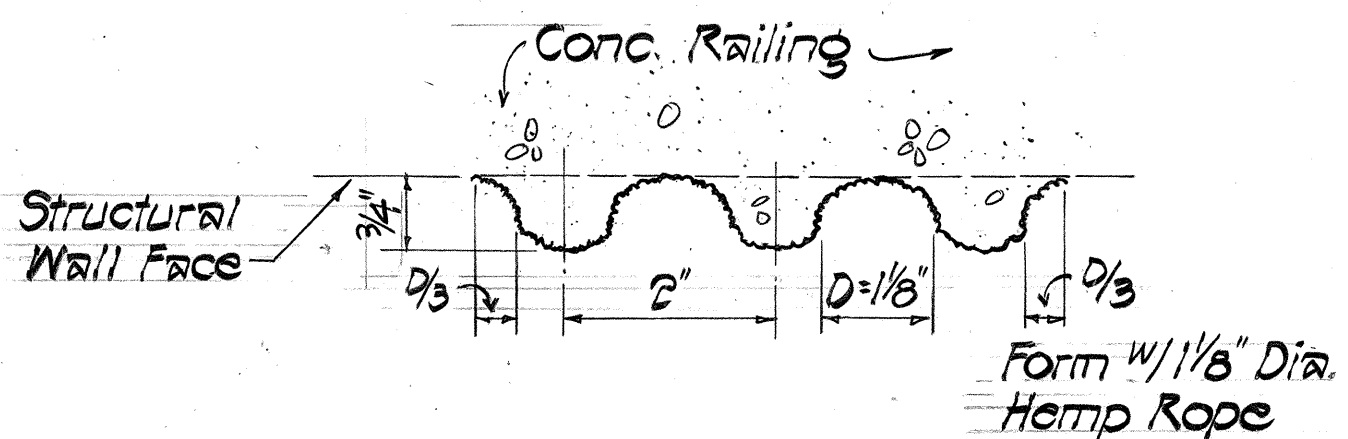
TYP. CONC. RAILING SECTION A
Scale: 1" = 1'-0" 5-38

NOTE: See Sht. 5-41 for modified conc. railing where light pole brackets occur.

Space conc. rail exp. joint evenly between bridge deck exp. joints but not more than 32'. Railing exp. joints to be at right angle to roadway grade. Top. Where light poles occur, adjust exp. joint spacing so that joints are at least 3' away from poles. See Det. 1/5-41.

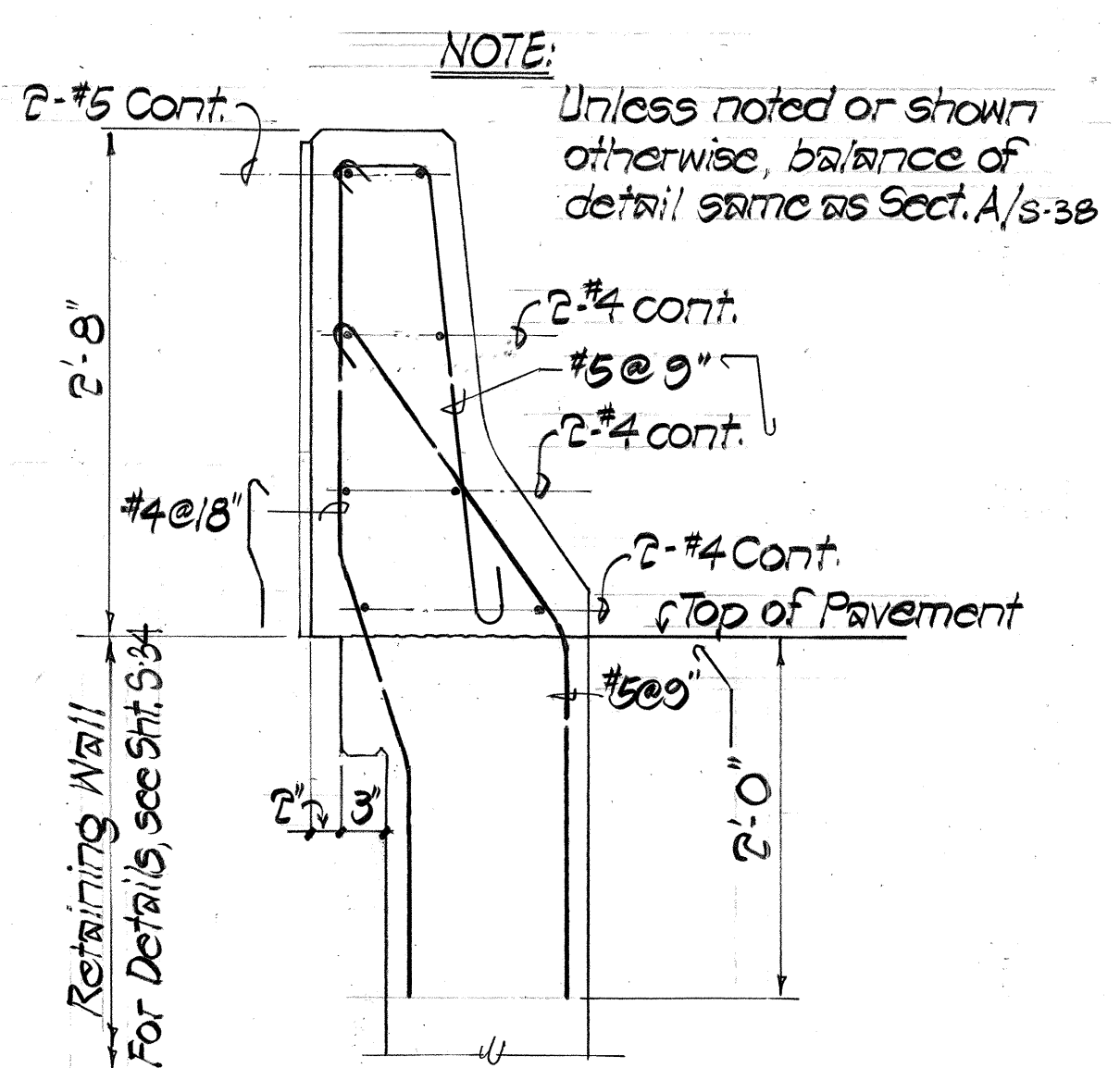


ELEVATION ~ TYP. CONC. RAILING
Scale: 1/2" = 1'-0" 5-38

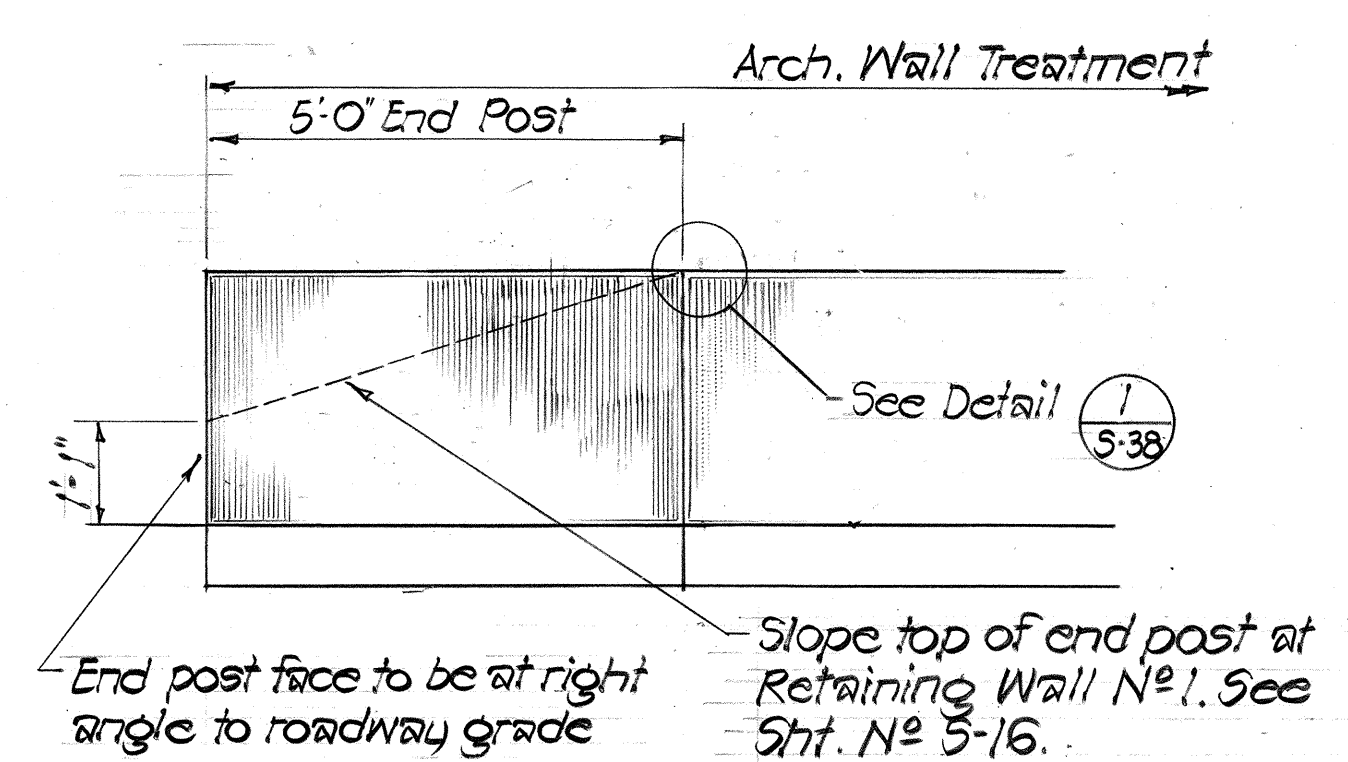


ARCH. WALL TREATMENT DETAIL 2
Not to Scale 5-38

NOTE: Cost of arch. wall treatment on pier columns shall be incidental to conc. work & arch. wall treatment on conc. railings shall be incidental to conc. railing work.



TYP. CONC. RAILING SECTION AT RETAINING WALL D
Scale: 1" = 1'-0" 5-38



ELEVATION ~ TYP. END POST
Scale: 1/2" = 1'-0" 5-38

SURVEY PLOTTED BY	DATE
DRAWN BY	
CHECKED BY	
NOTE BOOK	
NO.	

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Consulting Engineers
225 Queen St., Suite 504
Honolulu, Hawaii 96813

EDGAR K. M. LEE
REGISTERED PROFESSIONAL ENGINEER
No. 2150
HAWAII

THIS WORK WAS DESIGNED BY THE
FIRM UNDER THE SUPERVISION OF
EDGAR K. M. LEE

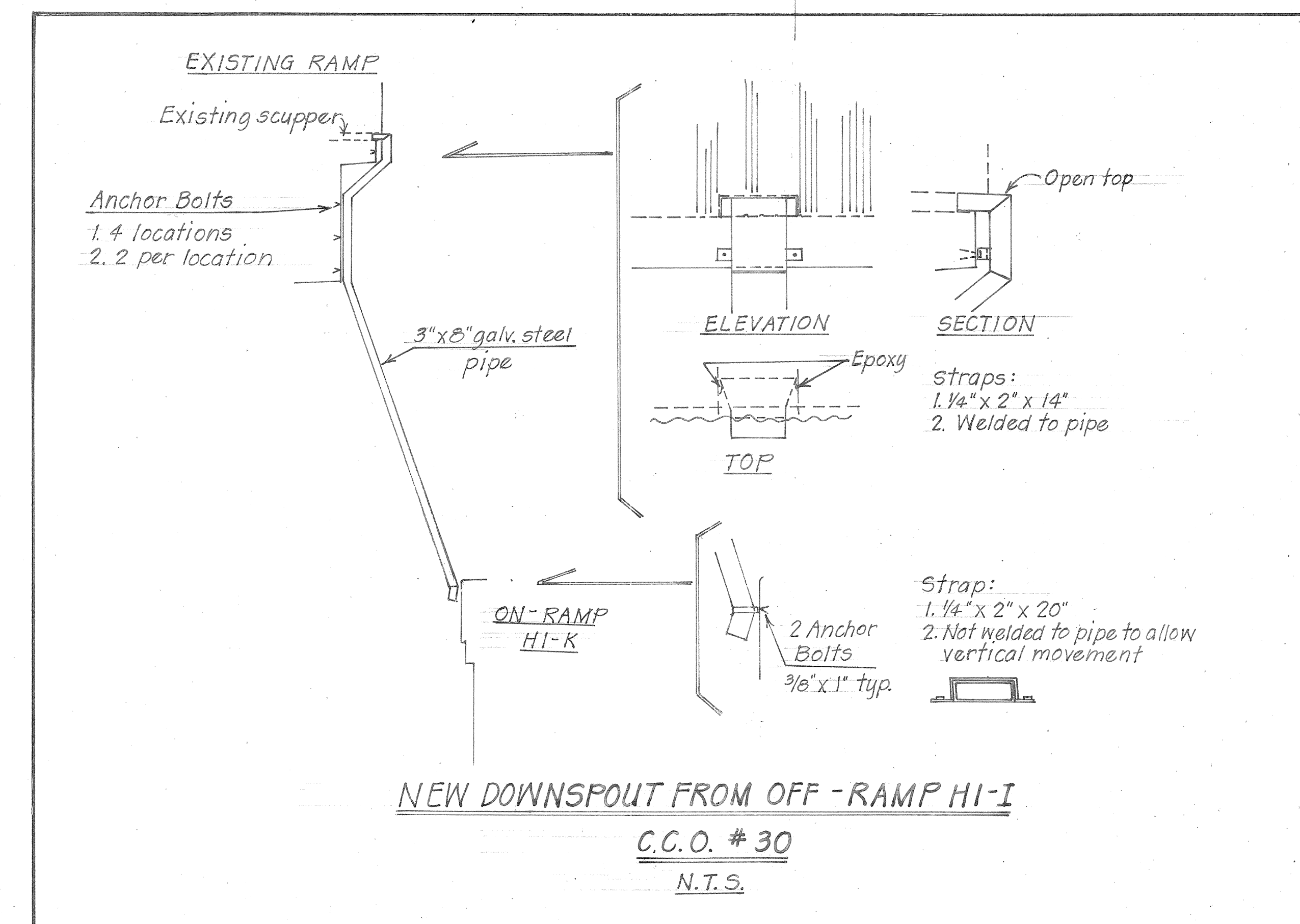
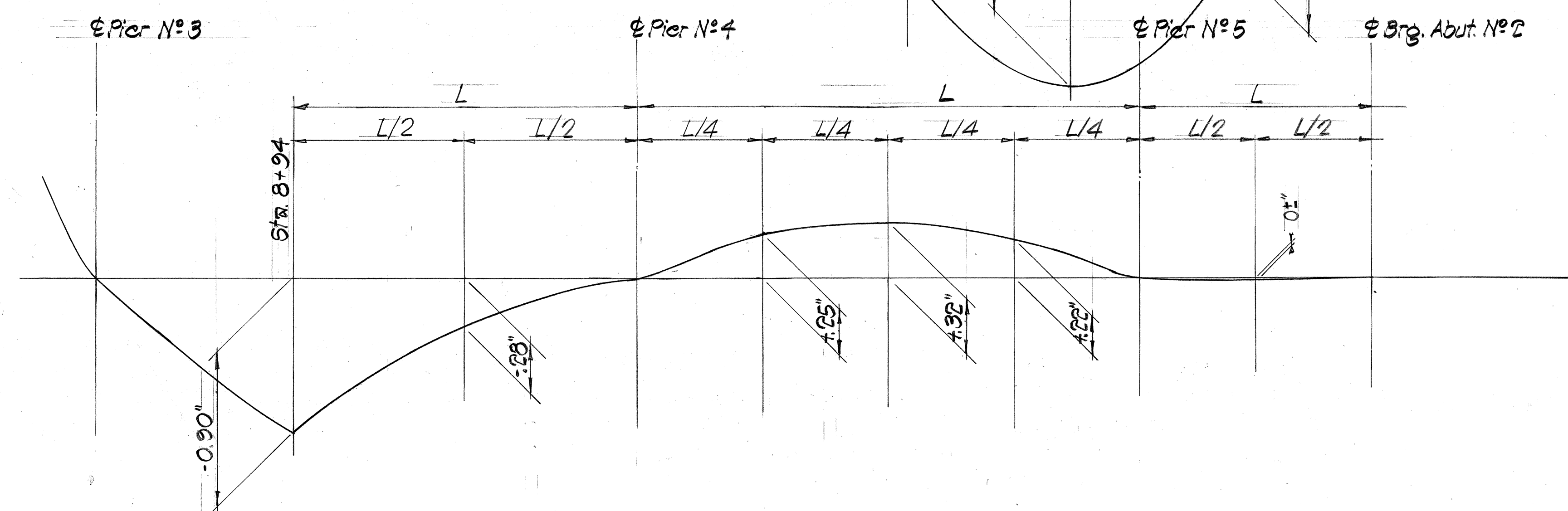
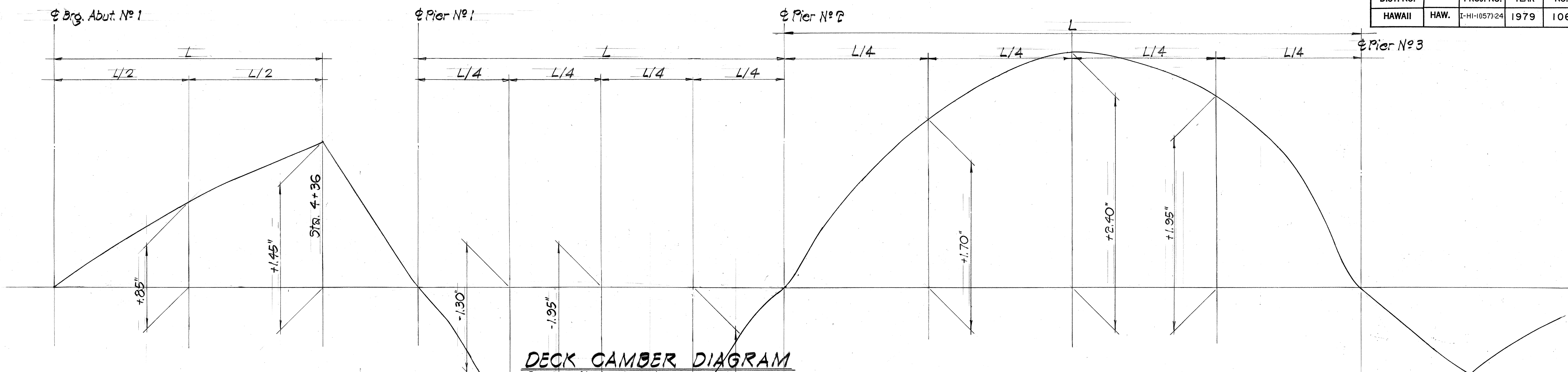
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

CONCRETE RAILING
SECTIONS & DETAILS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24
Scale: As Noted Date: 4-10-80

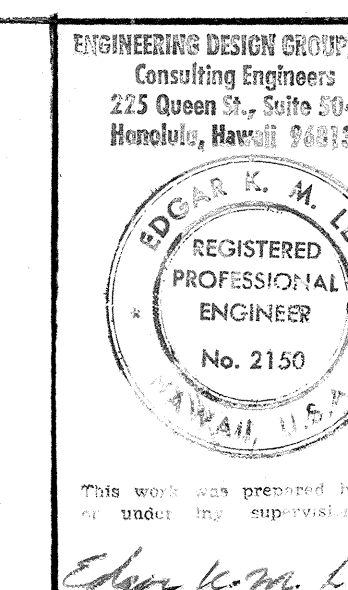
SHEET No. 5-38 OF 5-41 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	106	146



DECK CAMBER DIAGRAM
Scales: Horiz. 1" = 60'
Vert. 1" = 0.5'

ORIGINAL PLAN	SURVEY PLOTTED BY	DATE
NOTE BOOK	DESIGNED BY	
QUANTITIES BY	CHECKED BY	
No.		

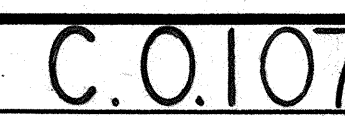


STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

DECK CAMBER DIAGRAM

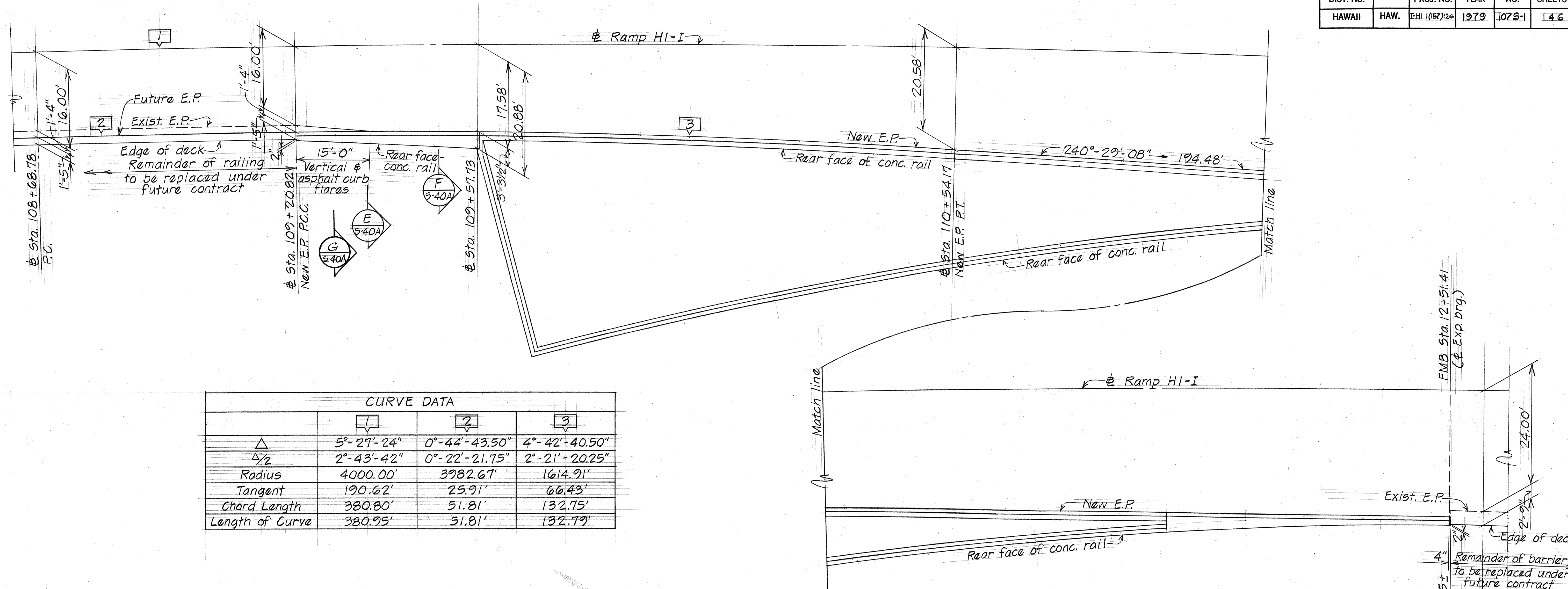
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
P.A.I. PROJ. NO. I-HI-1(157)-24
Scale: As Noted Date: 4-10-80
SHEET No. 5-3 OF 5-4 SHEETS

{ } Denotes changes to sht #107.



B-2-5

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	107S-1	1979	107S-1	146



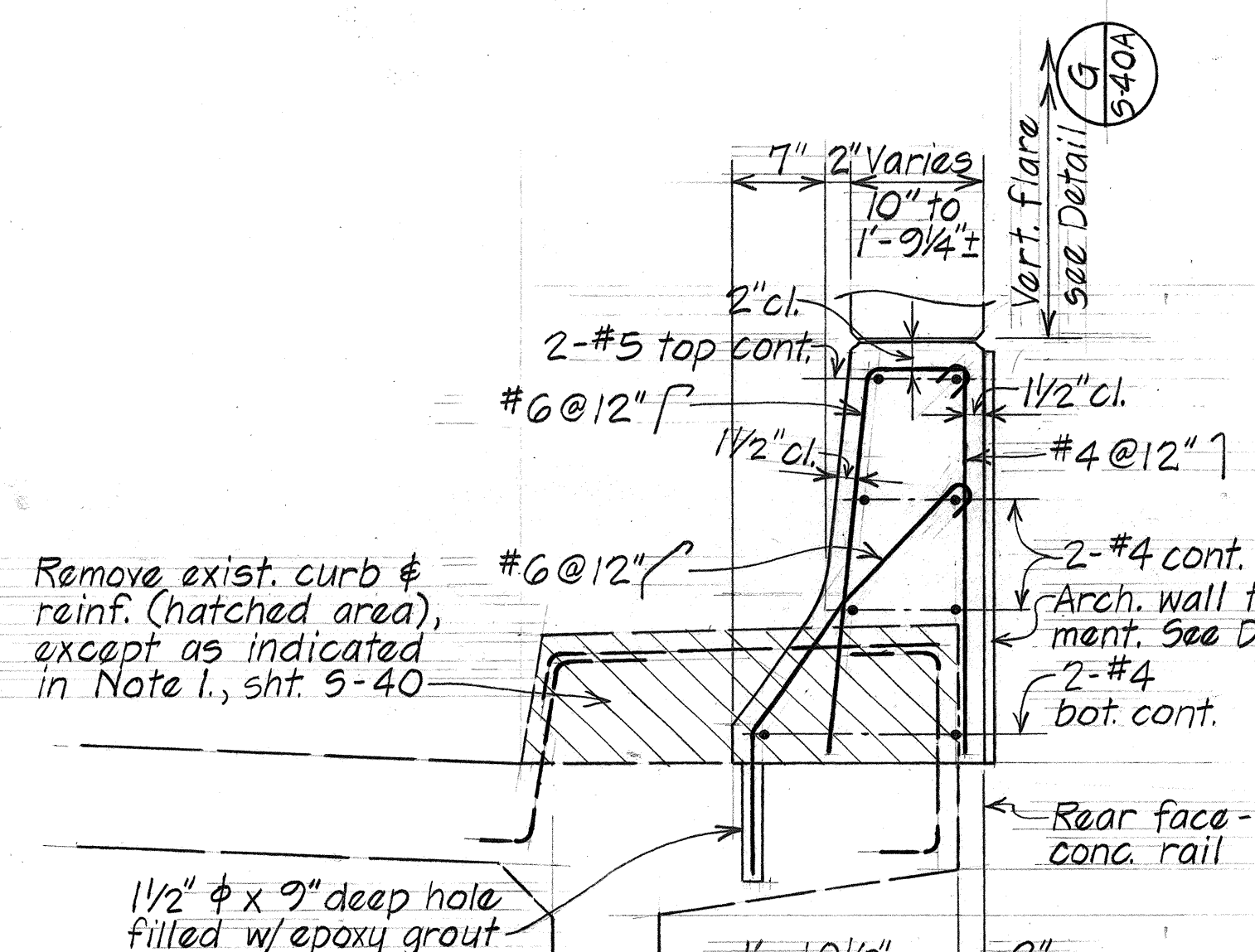
CURVE DATA			
	1	2	3
Δ	5°-27'-24"	0°-44'-43.50"	4°-42'-40.50"
$\Delta/2$	2°-43'-42"	0°-22'-21.75"	2°-21'-20.25"
Radius	4000.00'	3782.67'	1614.91'
Tangent	190.62'	25.91'	66.43'
Chord Length	380.80'	51.81'	132.75'
Length of Curve	380.95'	51.81'	132.79'

PARTIAL PLAN

Scale: 1" = 10'-0"

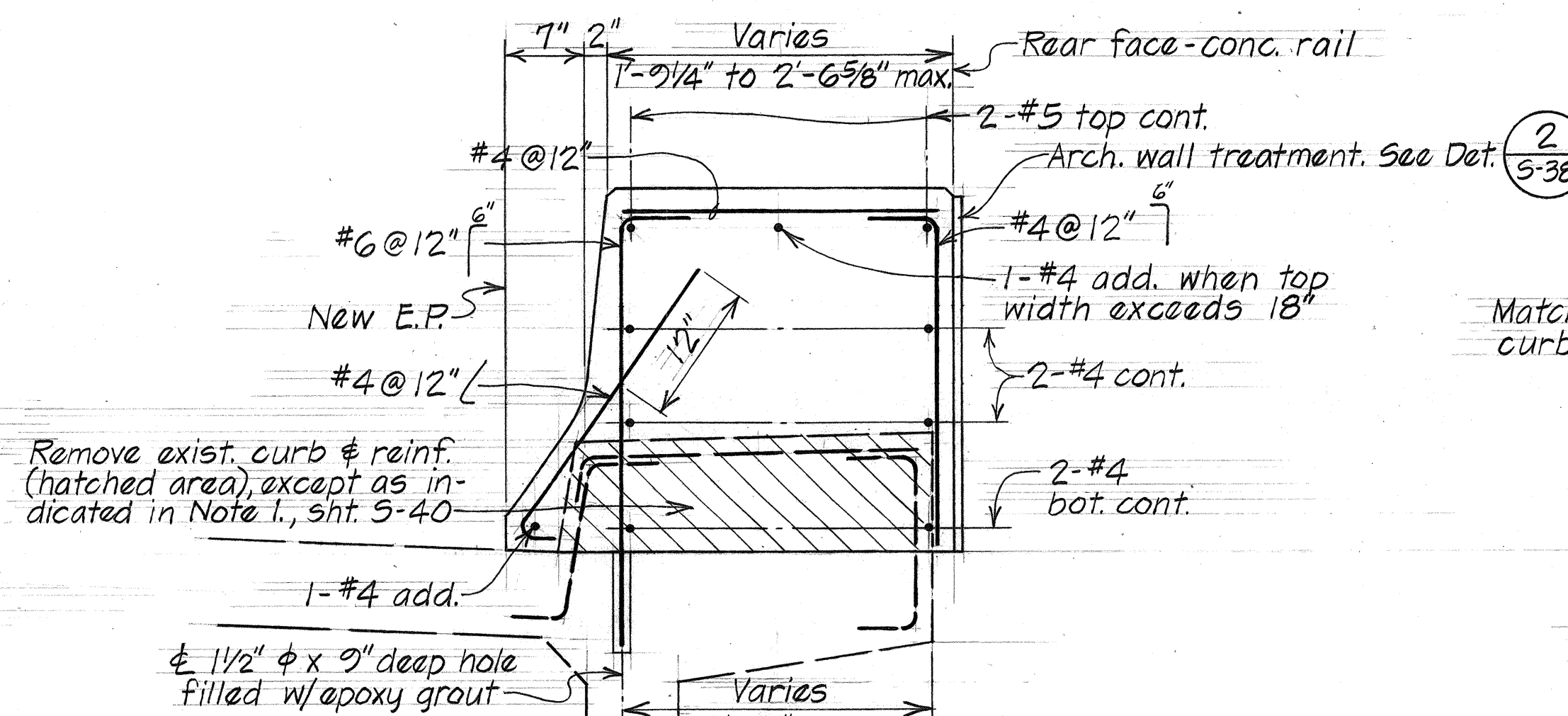
Note: For additional details see Sht. S-40

DATE	_____
DESIGNED BY	_____
CHECKED BY	_____
NOTED BY	_____
QUANTITIES BY	_____
CHECKED BY	_____



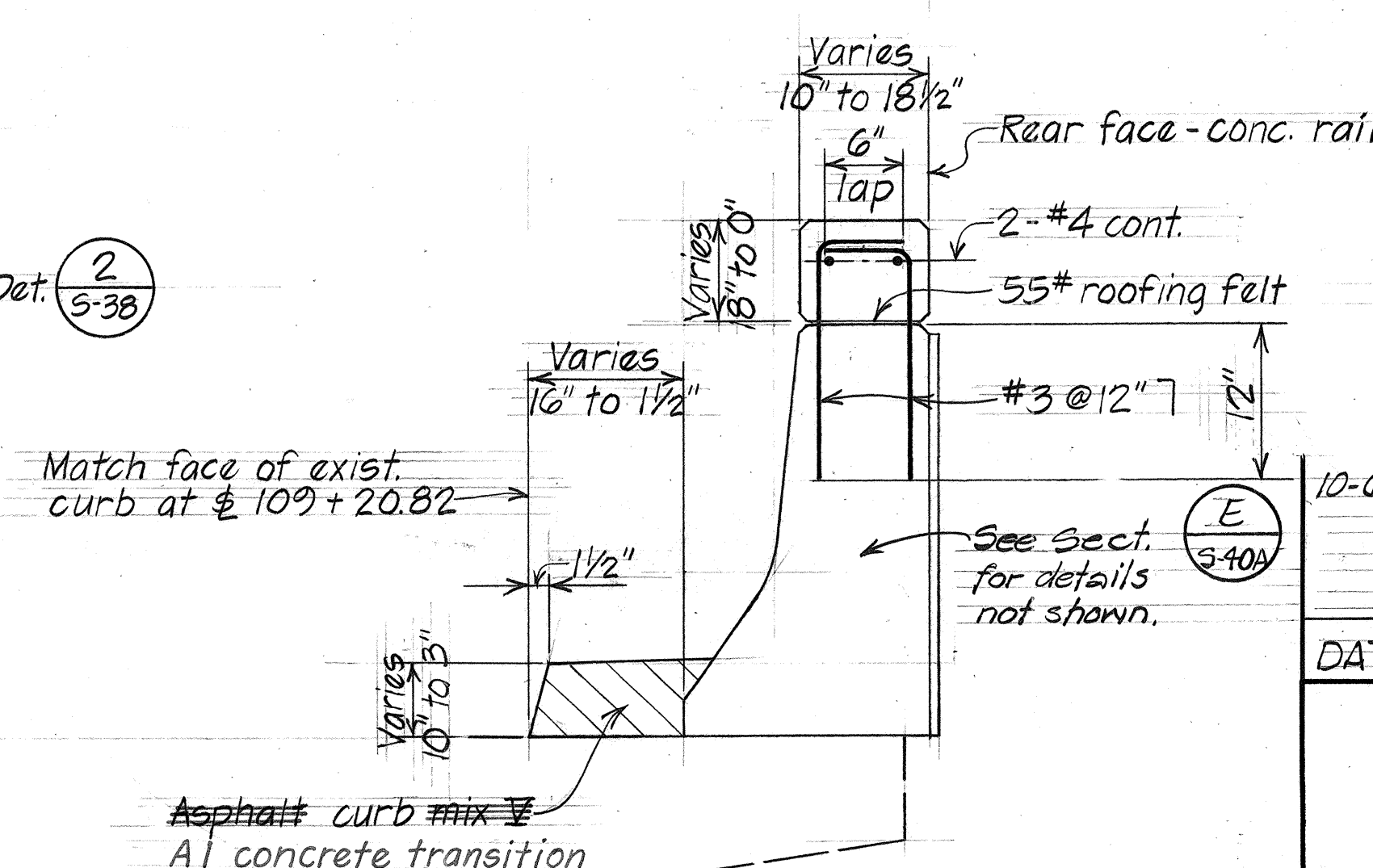
SECTION E-5-40A

RAMP HI-I STA. 109+28.02 TO 109+41
Scale: 1" = 1'-0"



SECTION F-5-40A

RAMP HI-I STA. 109+41 TO 109+57.13
Scale: 1" = 1'-0"



TEMPORARY VERTICAL FLARE & ASPHALT CURB FLARE DETAIL

RAMP HI-I STA. 109+20.82 TO 109+30.82
Scale: 1" = 1'-0"

10-6-80 Sheet added to Contract Plan set for additional details required on sheet no. C.O.107.

DATE REVISION

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION

EXISTING RAMP MODIFICATIONS

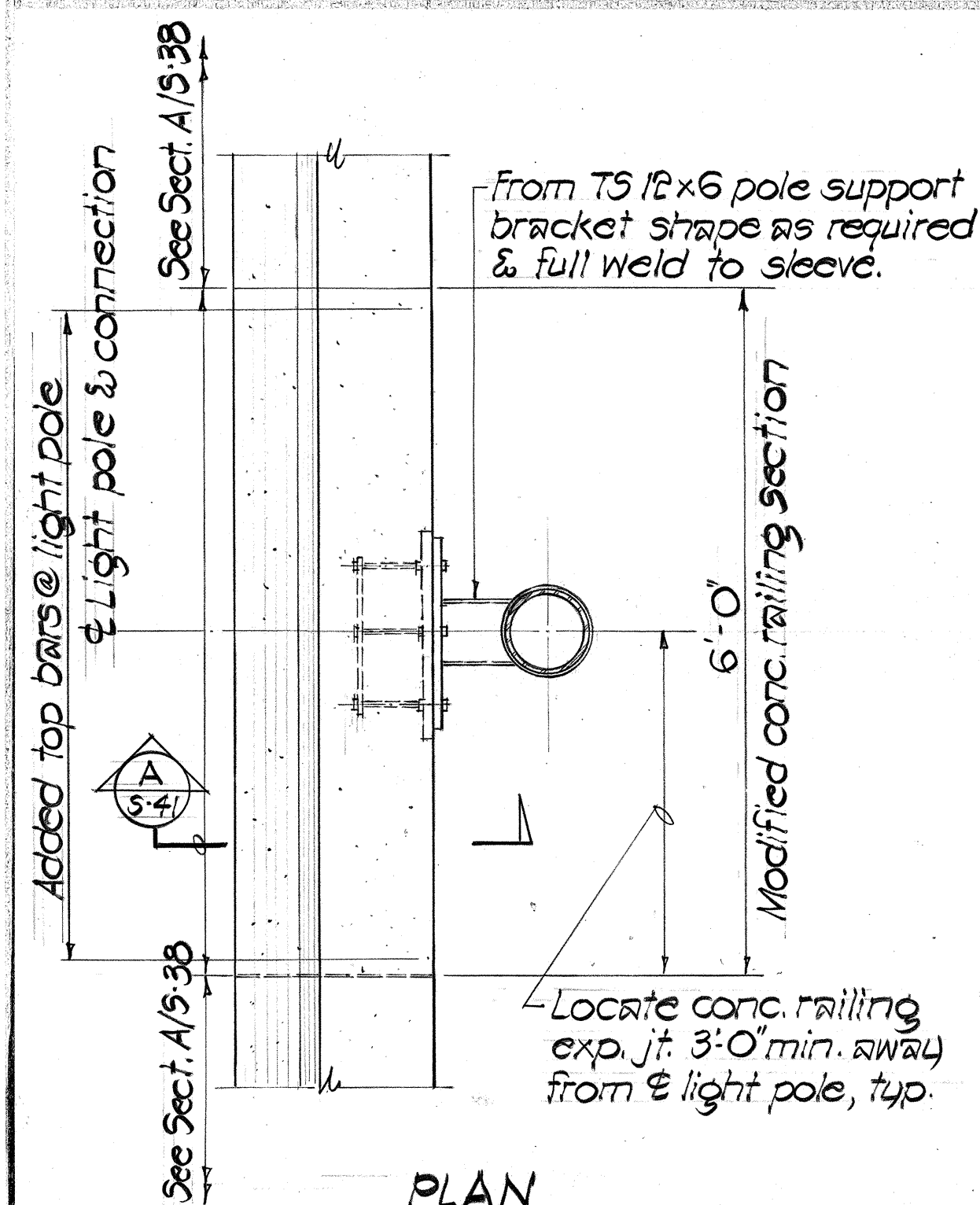
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157):24

Scale: As Noted Date: _____

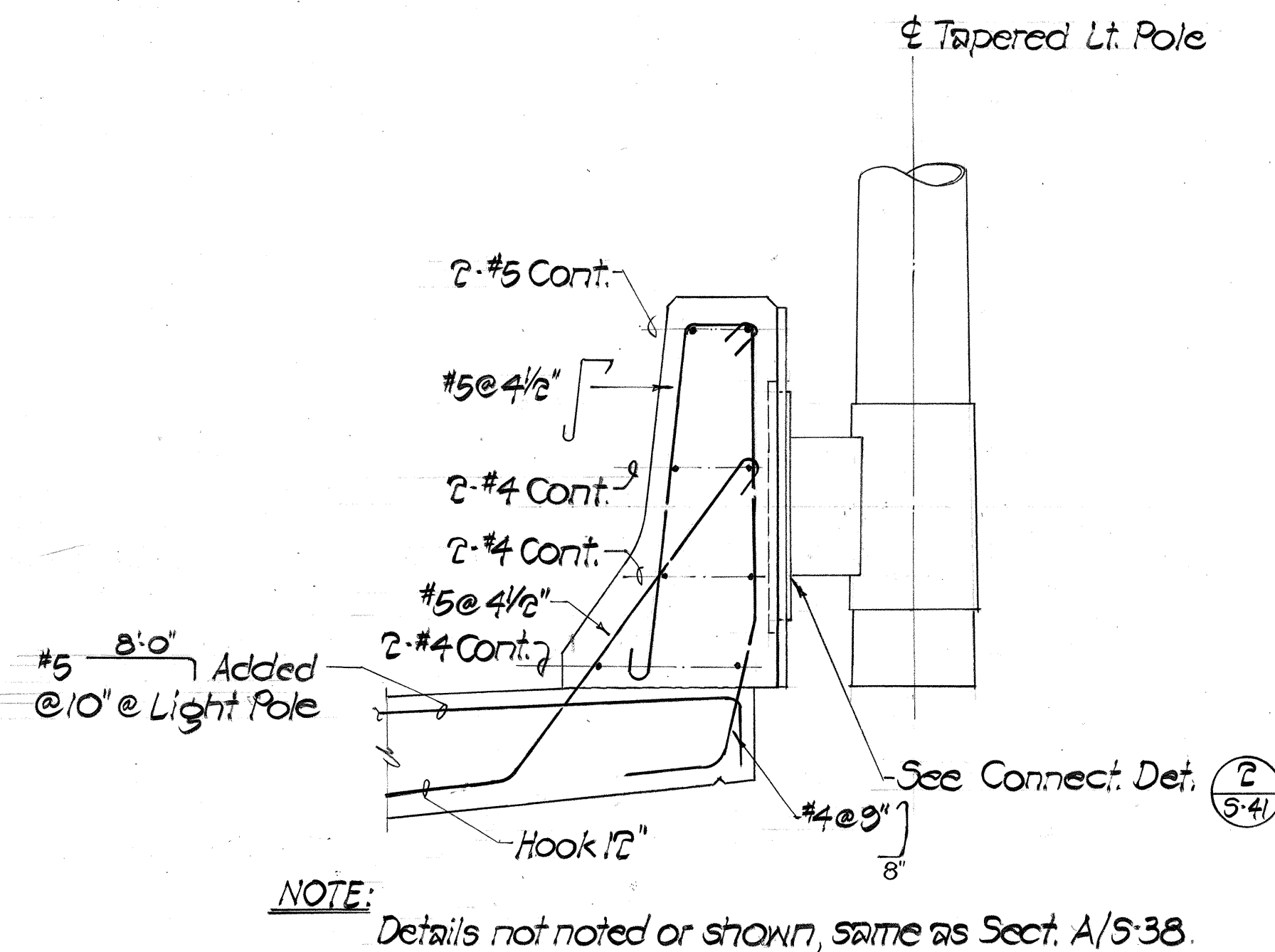
SHEET No. S-40A OF S-41 SHEETS

C.O.107S-1

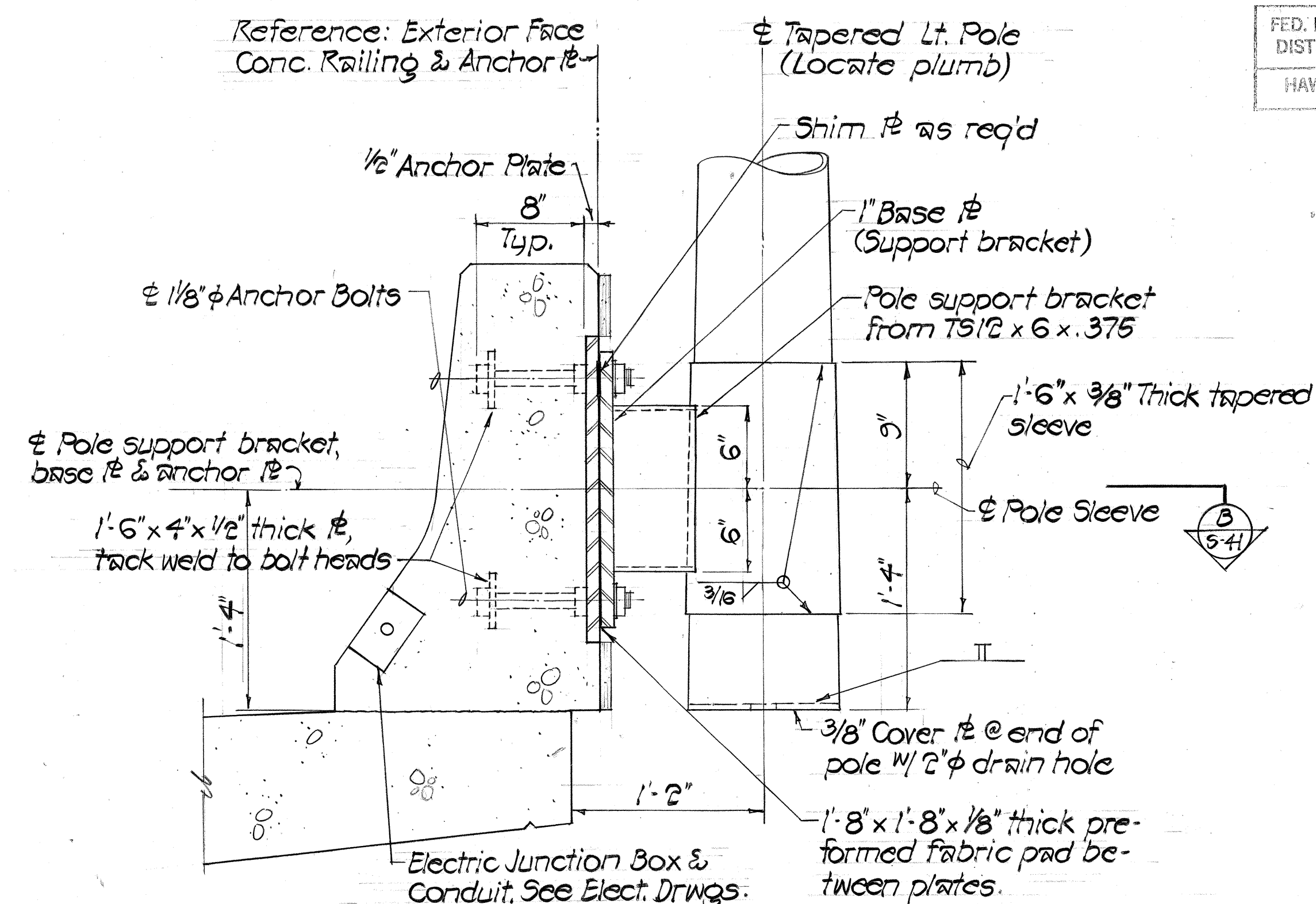
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(57):24	1979	108	146



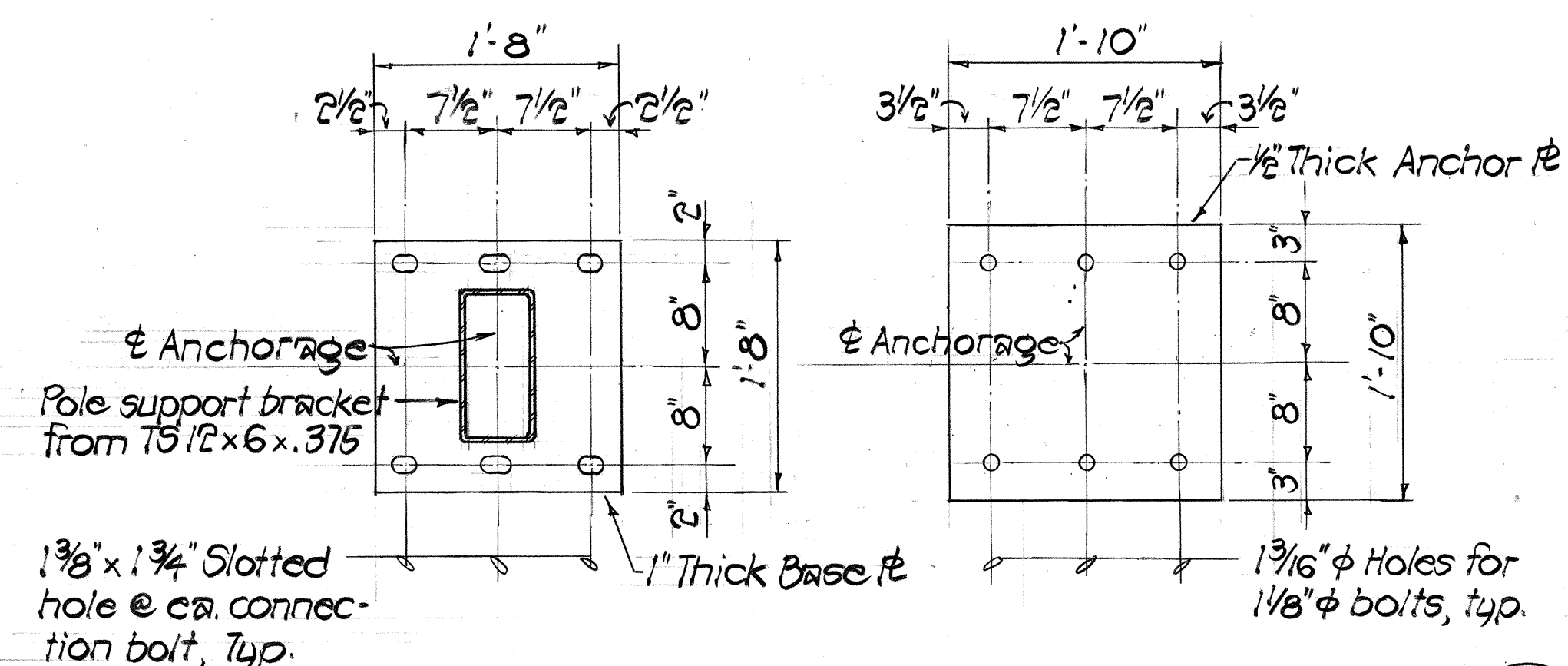
LIGHT POLE CONNECTION (1)
Scale: $\frac{3}{4}" = 1'-0"$ (S-41)



SECTION A
Scale: 1" = 1'-0" 5-41

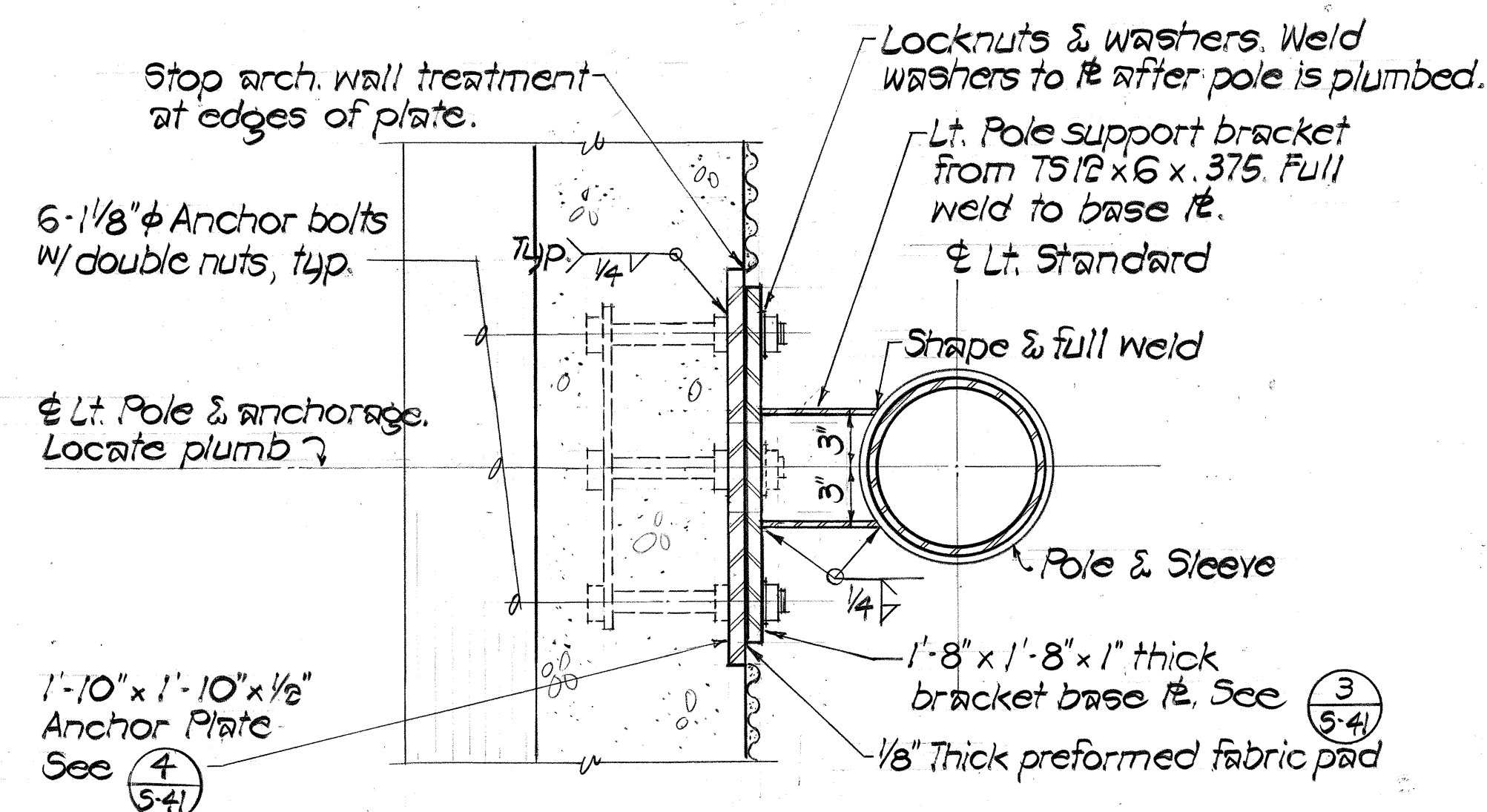


CONNECTION DETAIL (?)
Scale: 1/8" = 1'-0" (5-41)



POLE SUPPORT BRACKET
& BASE PL ASSEMBLY (3)
Scale: 1" = 1'-0" (5-4)

BRACKET ANCHOR Φ 4
Scale: 1" = 1'-0" (S-4)



SECTION B
Scale: 1/2" = 1'-0" S-41

NOTE: Light pole support bracket & base plate assembly, bracket anchor plate, bolts, nuts, washers and anchor plates to be hot dip galvanized after fabrication.

ORIGINAL PLAN

SURVEY PLOTTED BY _____ DATE _____

DRAWN BY _____

TRACED BY _____

DESIGNED BY _____

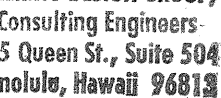
QUANTITIES BY _____

CHECKED BY _____

No. _____

ENGINEERING DESIGN GROUP, INC.
 Consulting Engineers
 725 Queen St., Suite 504
 Honolulu, Hawaii 96813

STATE OF HAWAII
 DEPARTMENT OF TRANSPORTATION
 LAND TRANSPORTATION FACILITIES DIVISION



LIGHT POLE CONNECTION DETAILS

INTERSTATE ROUTE H-1
 KAPIOLANI INTERCHANGE ON-RAMP

F.A.I. PROJ. NO. I-HI-1(157)-B4

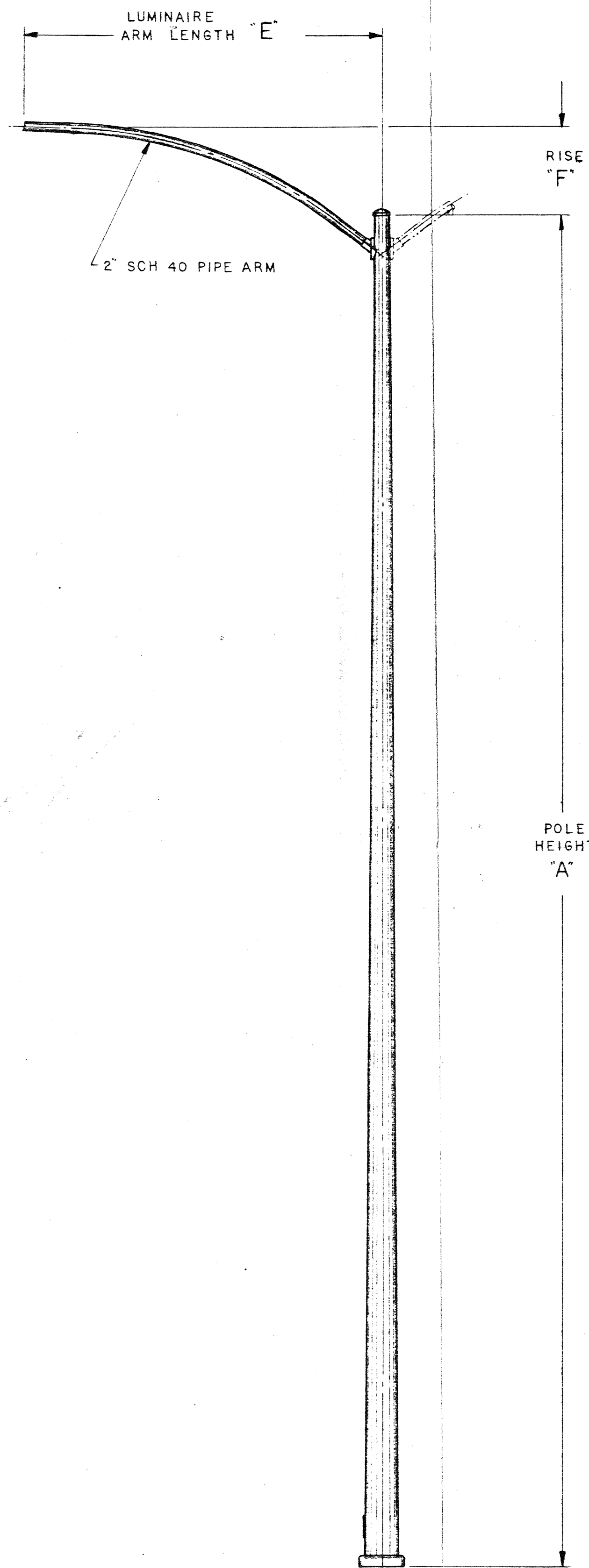
This work was prepared by me
or under my supervision

Scale: As Noted

Date: 4-10-80



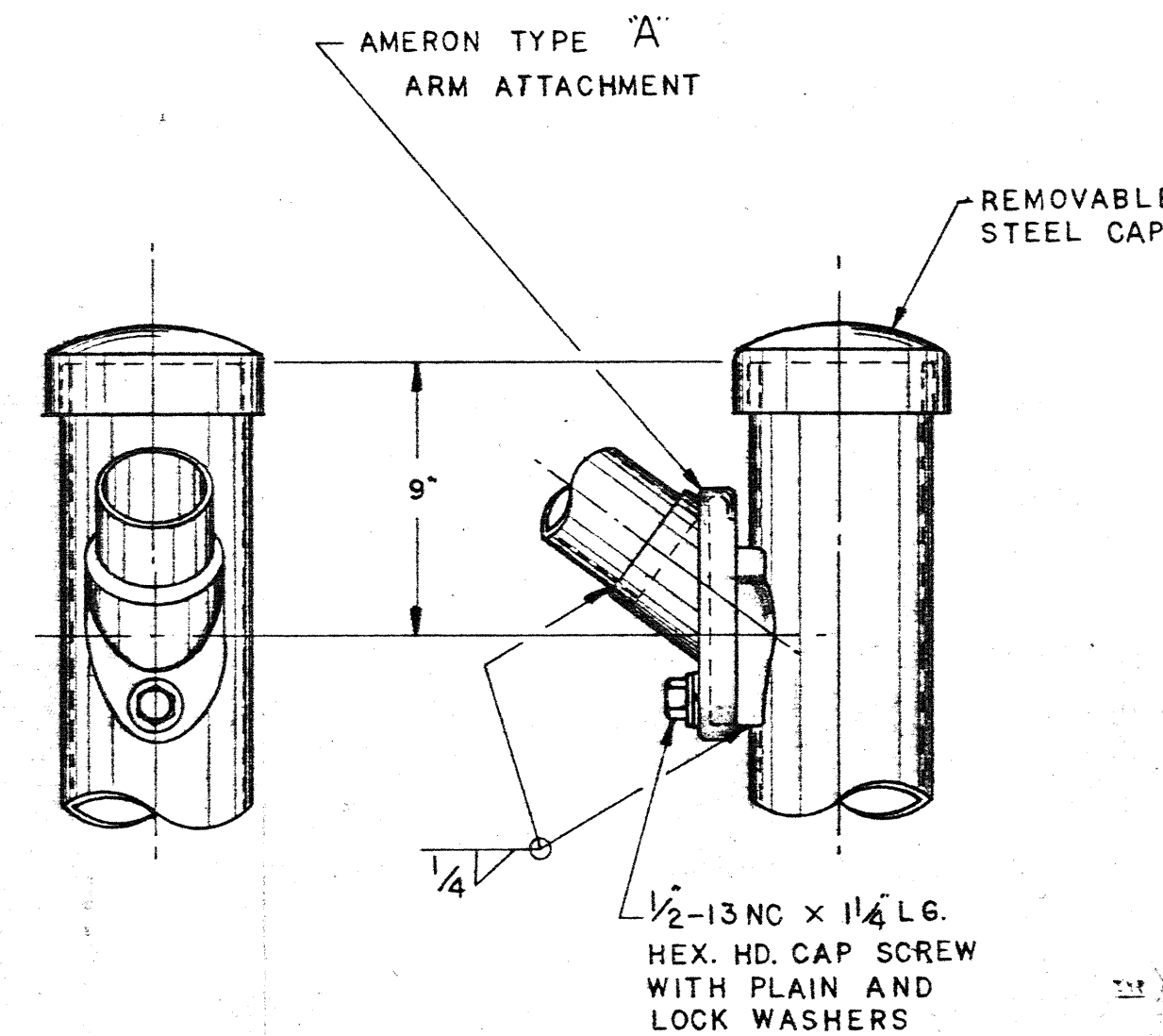
SHEET NO. 5-41 OF 5-41 SHEETS



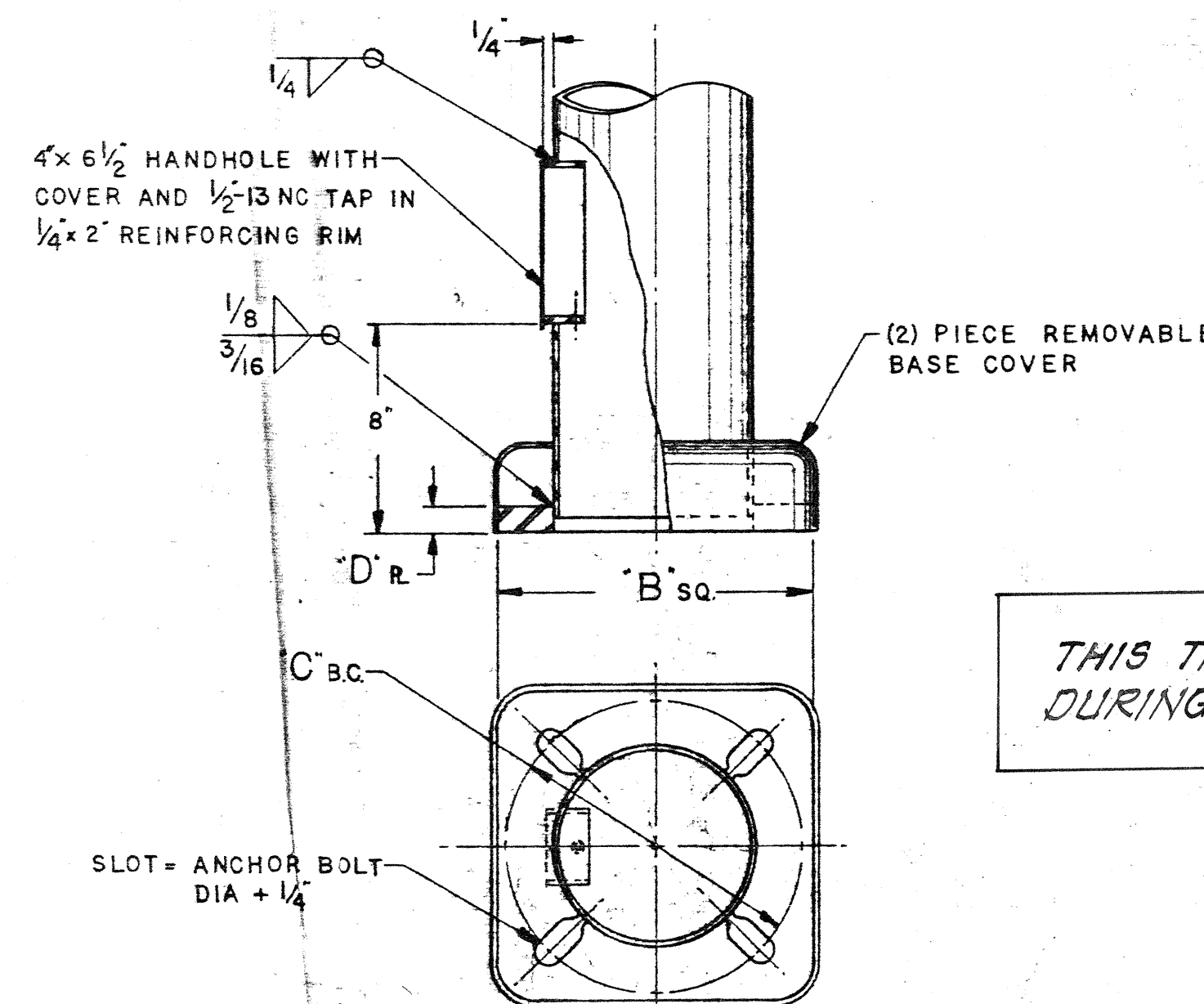
CATALOG NUMBER	POLE DATA							ANCHOR BOLT (ASTM A-307)
	'A'	TOP OD x BASE OD x GA	'B'	'C'	'D'			
A-250-024	25'-0"	3 7/8" x 7 5/16" x 10	11 1/2"	11 1/2"	1"			1" x 36" x 4"
A-280-024	28'-0"	3 7/8" x 7 3/4" x 10	11 1/2"	11 1/2"	1"			1" x 36" x 4"
A-300-024	30'-0"	3 7/8" x 8" x 10	11 1/2"	11 1/2"	1"			1" x 36" x 4"
A-350-024	35'-0"	3 7/8" x 8 1/16" x 10	11 1/2"	11 1/2"	1 1/4"			1" x 36" x 4"
A-400-024	40'-0"	3 7/8" x 9 3/8" x 10	13 1/2"	12 1/2"	1 1/4"			1" x 36" x 4"
A-450-024	45'-0"	3 7/8" x 10 1/16" x 10	16"	14 1/2"	1 1/4"			1" x 36" x 4"
A-260-024	26'-6"	3 7/8" x 7 1/2" x 10	11 1/2"	11 1/2"	1"			1" x 36" x 4"

ARM DATA	
'E'	'F'
4'-0"	9"
5'-0"	11"
6'-0"	1'-3"
8'-0"	2'-3"

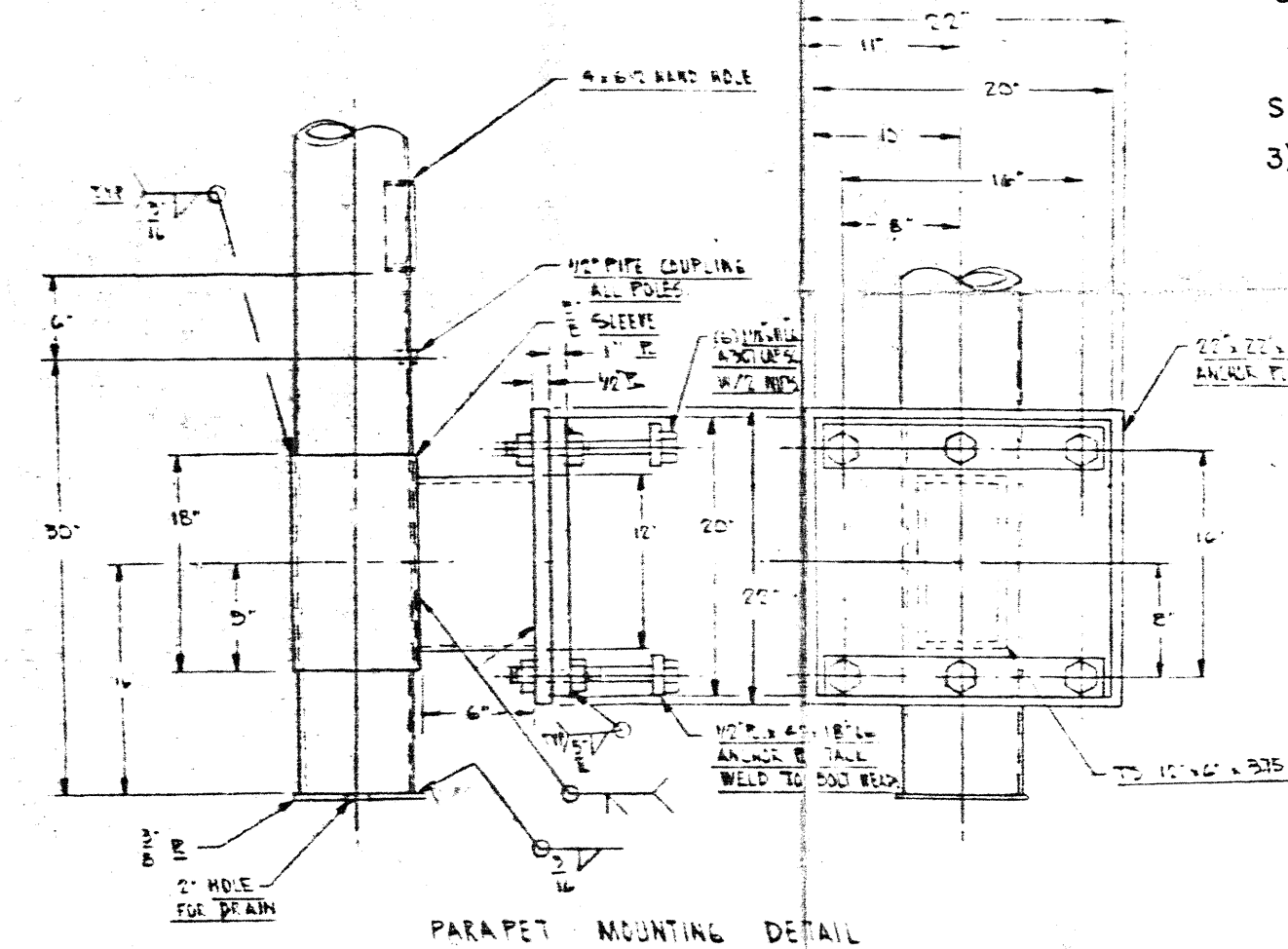
FLAG	ITEM NO.	QUANTITY THIS DWG.	DESCRIPTION			
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS	
HAWAII	HAW.	I-HI-1(137)-24	1970	1085-1	146	



ARM CONNECTION DETAIL



BASE DETAIL



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

LIGHT POLE CONNECTION DETAILS

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(137)-24
SCALE: AS NOTED Date: 10/29/60

SHEET NO. 544 OF 541 SHEETS

NOTES

- POLE SPECIFICATIONS
MATERIAL
SHAFT: STEEL OF 40,000 P.S.I. MINIMUM YIELD AFTER FABRICATION.
BASE PLATE: ASTM A-36 STEEL.
PIPE: ASTM A-53 GRADE 'B' STEEL.
WELD: (ALL BUTT WELDS TO BE GROUND FLUSH WITH BASE METAL).
LONGITUDINAL: BUTT WELD BY THE SUBMERGED ARC PROCESS.
CIRCUMFERENTIAL: BUTT WELD WITH PERMANENT BACK UP RING.
FINISH
GALVANIZE PER ASTM A-123 OR PRIME PAINT BY DIP PROCESS.
- CATALOG NUMBERING PROCEDURE
SERIES A-308-024
LUM. ARM LENGTH
SHAFT LENGTH
DRWG. NUMBER
- FOR DOUBLE ARM POLES ADD THE LETTER "D" BEHIND THE ARM LENGTH IN THE CATALOG NUMBER. EXAMPLE: A-308 D-024

APPROVED

2/3/81
DISTRICT ENGR. - OAHU

AGENCY	STATE OF HAWAII
PROJECT	KAPIOLANI INTERCHANGE
DISTRIBUTOR	WESTINGHOUSE ELECTRIC SUPPLY
CONTRACTOR	
CUSTOMER ORDER NO.	6501-08077-10
AMERON M.S.A. NO.	

MATERIAL LIST	
QTY	CATALOG NUMBER AND DESCRIPTION
1	A-306-024 W/ ANCHOR BOLTS
1	A-306-024 W/ PARAPET MOUNT.
1	LENGTH OF POLE

A	ADDED 26'6" SHAFT	12-76 G.A.H.	
	REDRAWN	1-76 G.A.H.	
SYMBOL	REVISION	DATE	BY APPVD.

Ameron POLE PRODUCTS DIVISION

"A" SERIES LIGHTING STANDARD

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DRAWN	G.A.H.	1-76	SCALE:	024
APPVD.	S.A.S.	1-76		

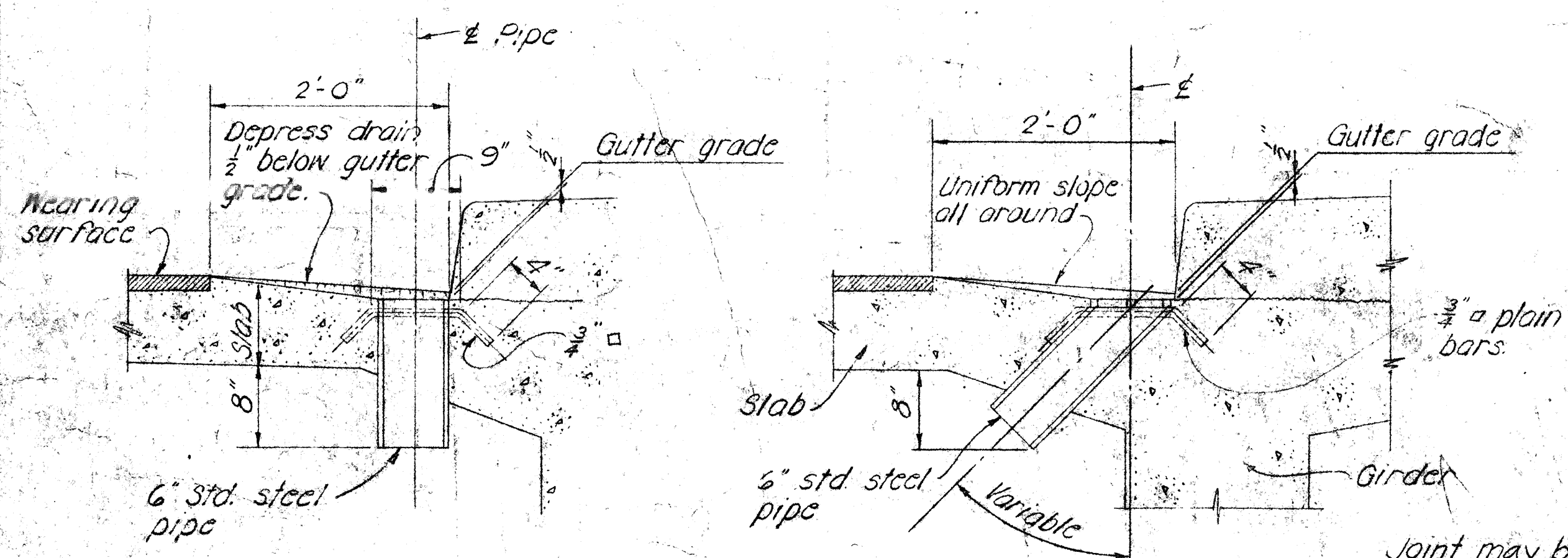
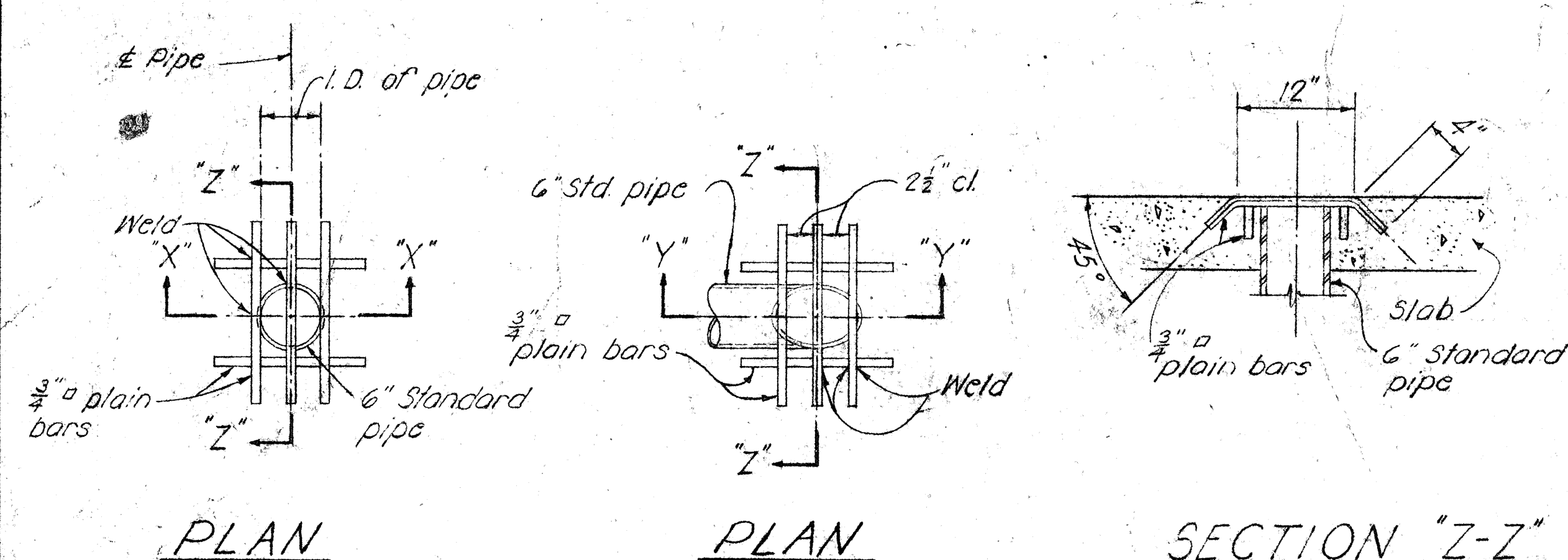
108 S-1

GENERAL NOTES

- All concrete shall be Class A unless otherwise noted.
- Reinforcing shall be detailed in accordance with A.C.I. Manual of Standard Practice for Detailing Reinforced Concrete Highway Structures unless otherwise noted.
- Clearance for reinforcing bars shall be as follows, unless otherwise noted:
 - 1/2" For stirrups of beams & pier caps.
 - 2" For ties of columns.
 - 2" For formed walls.
 - 3" For footings.
- Minimum spacing between parallel bars shall be 2 1/2 times the diameter of bars, but in no case shall the clear distance between the bars be less than 1 1/2 times the maximum size of the coarse aggregate.
- All dimensions relating to reinforcing are to centers of bars unless otherwise noted.
- Reinforcing bars shall be securely tied at all intersections & lap splices & shall be held in place during pouring to maintain their proper locations as shown on plans.
- Vertical column bars shall be arranged in such a manner as to miss pier cap bars above as directed by the Engineer.
- Except as otherwise noted on drawings, all exterior corners & re-entrant angles in concrete work shall be chamfered 3/4" x 3/4".
- Concrete seats receiving steel plates, lead plates or neoprene pads shall be poured monolithically with supporting structures & top of concrete seats shall be finished with a steel trowel to a smooth level surface at the elevation shown on the plans.
- Elastomeric Pads: Bottom of bridge bearing pads shall be secured against displacement with adhesives approved by the Engineer. Pads shall be incidental to concrete and will not be paid for separately.
- Forms for all exposed surfaces of separation structures and surfaces of retaining walls visible from a traveled way or from a populated area shall be plywood except that metal forms may be permitted provided a surface finish satisfactory to the Engineer can be obtained and the use of such forms is approved in writing by the Engineer. Forms for round columns shall be made of materials that will give a smooth finish and true dimensions as given in the plans. Forms for prestressed concrete members shall be either metal or plywood.
- Gothic letters and figures approximating dimensions shown will be acceptable if approved by the Engineer.
- The contractor shall verify the location of all existing utility lines and notify the respective owners before commencing work of excavation or driving of piles.
- Standard Detail Drawings refer to all structures in general, except for modifications as may be required for special conditions. For such modifications refer to the corresponding detailed drawings.

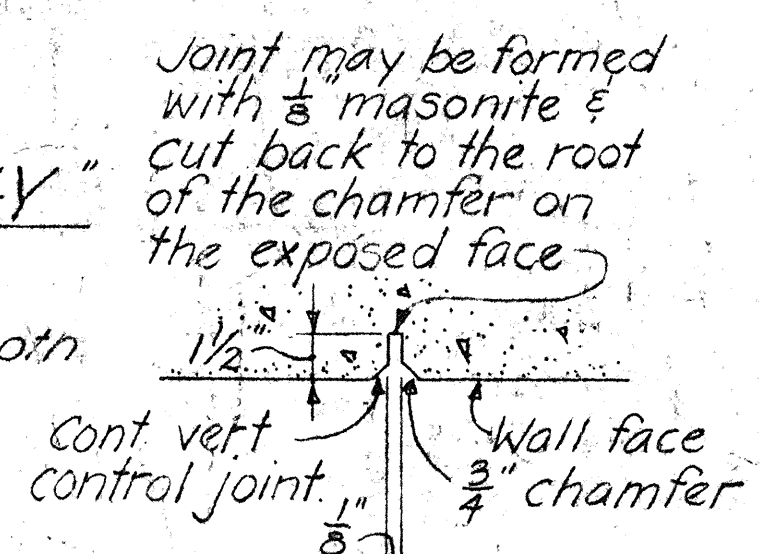
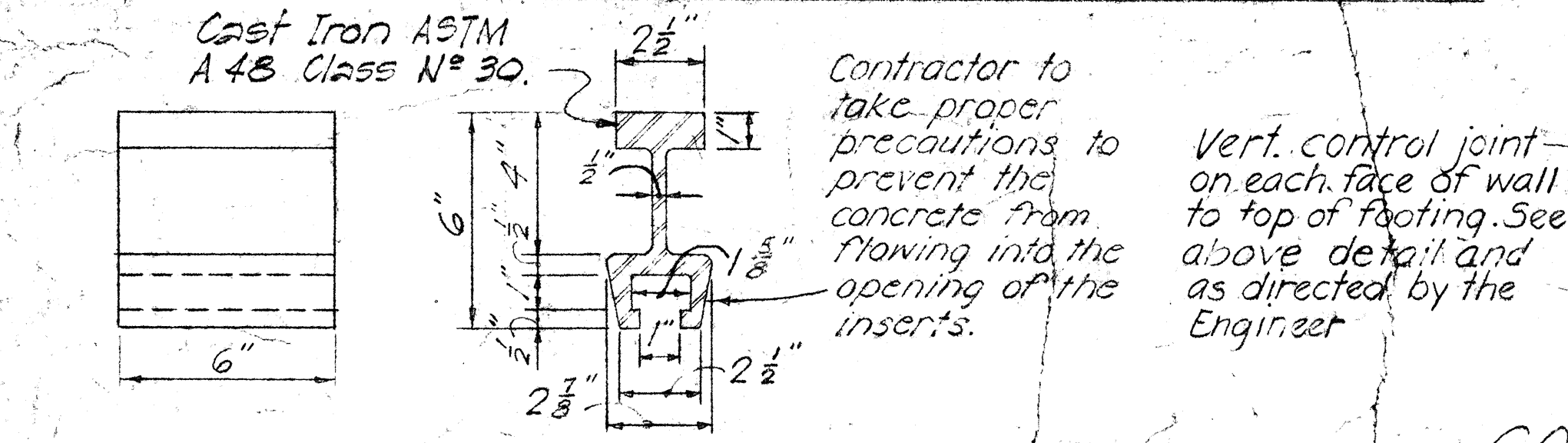
TYPICAL CONCRETE GROOVE DETAIL

DRIP DETAIL



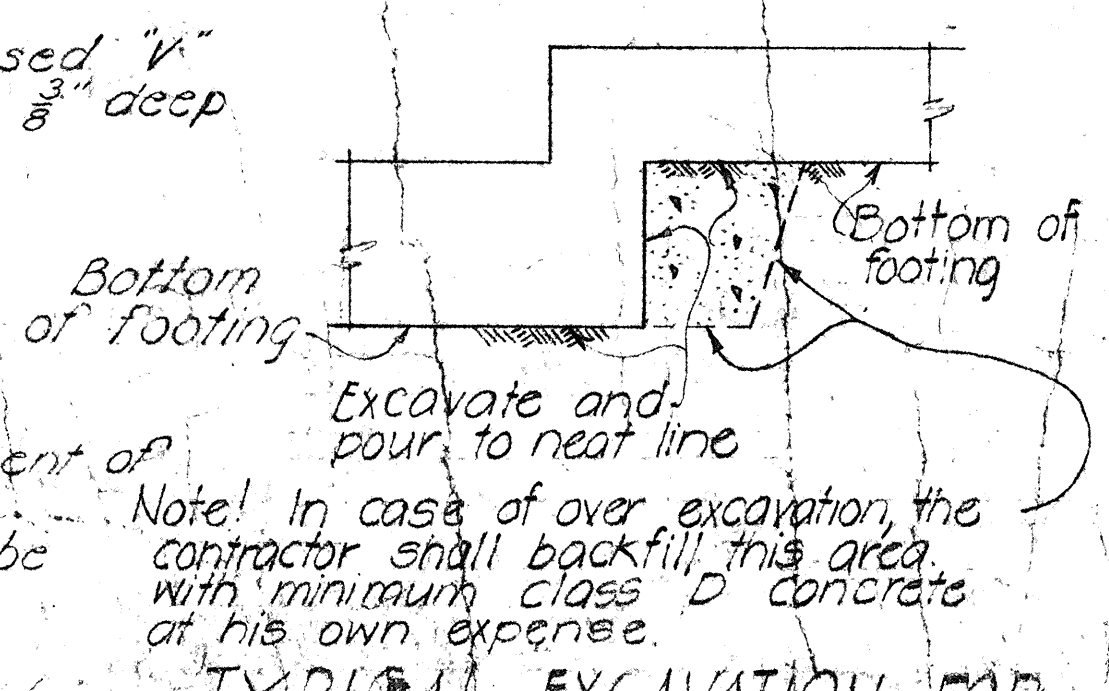
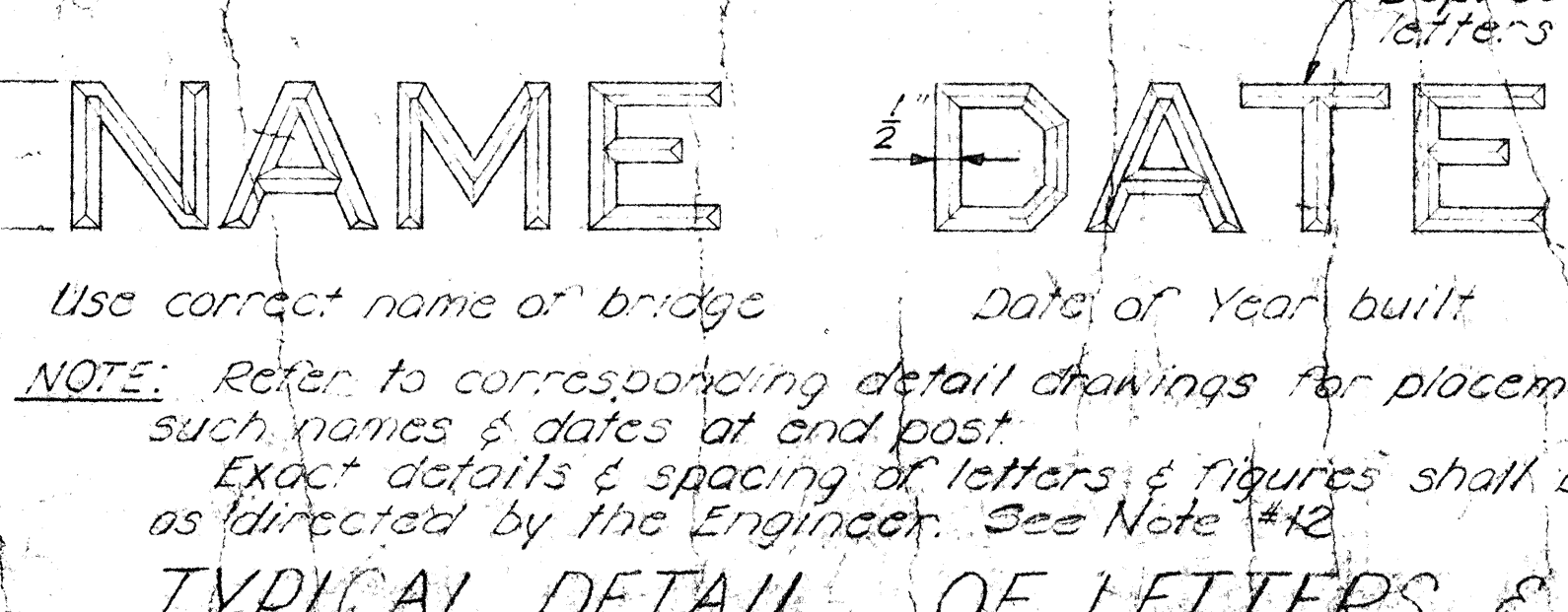
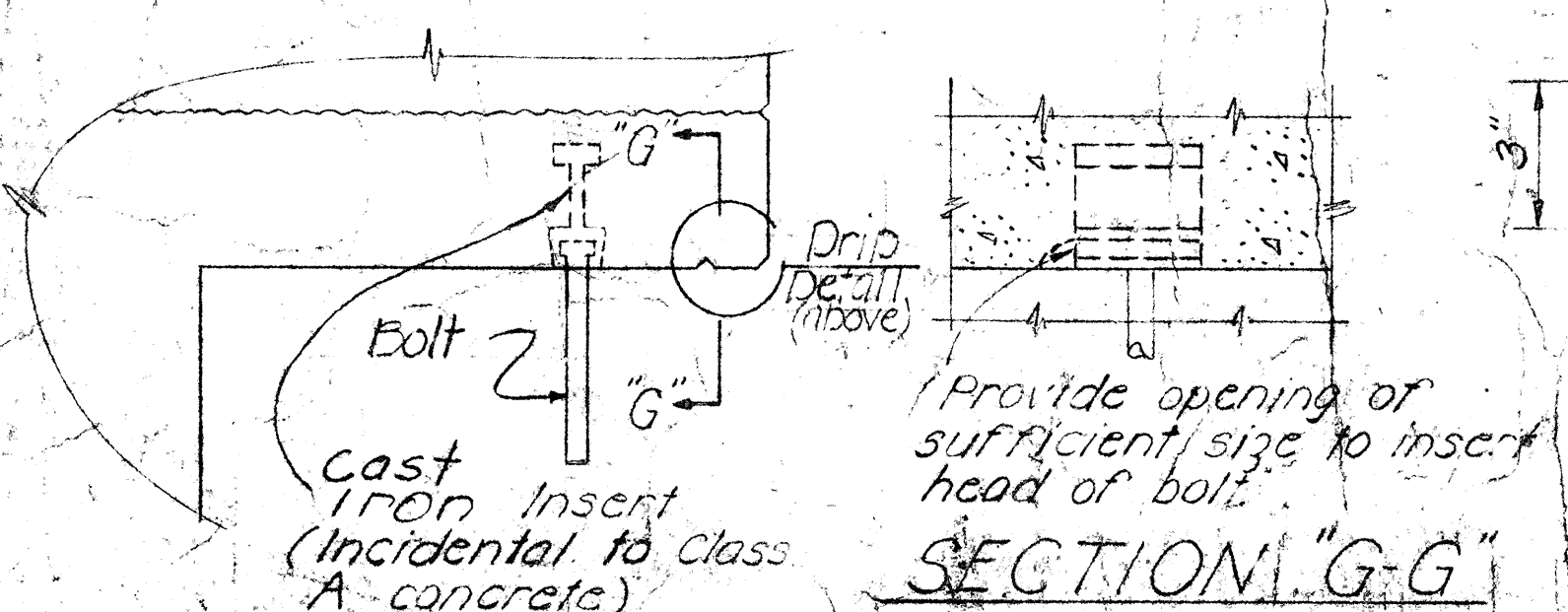
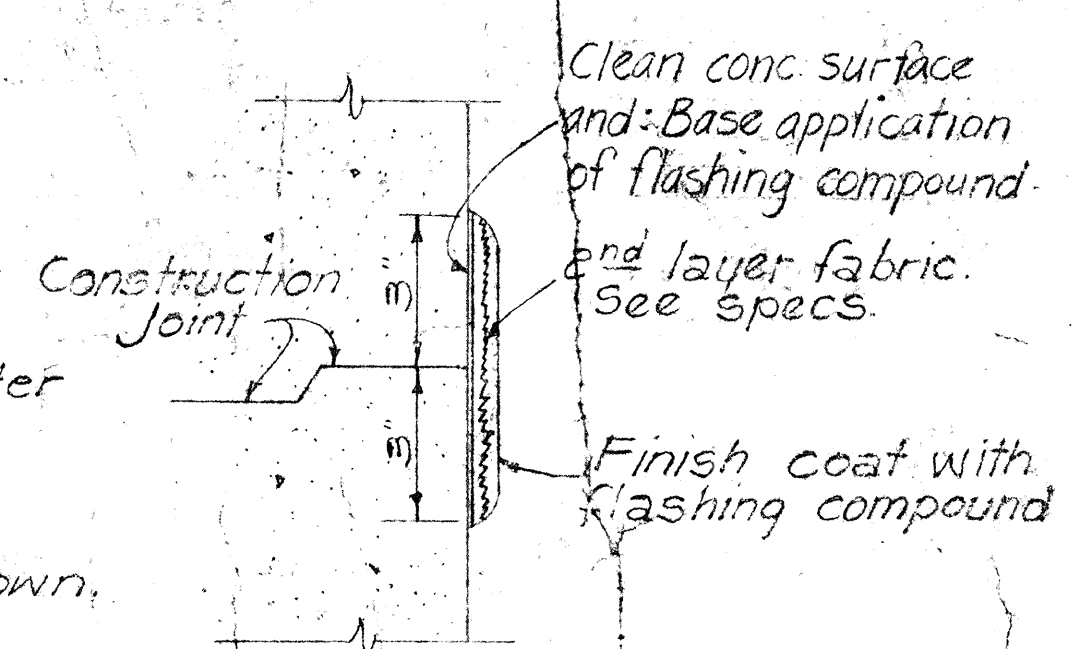
NOTE: Metal drains shall be considered incidental to Class A Concrete in both concrete and steel bridges and will not be paid for separately. Assembly of grillage and pipe shall be hot-dip galvanized after all welding is completed.

TYPICAL DRAINAGE DETAILS



CONTROL JOINT DETAILS

TYP. FLASHING COMPOUND WATERPROOFING DETAIL



APPROVAL RECOMMENDED:
 Paul T. Yamashita
 BRIDGE DESIGN ENGINEER
 1-23-76
 DATE

APPROVED
 Paul T. Yamashita
 ASSISTANT CHIEF, ENGINEERING
 1-28-76
 DATE

NO.	REVISION	APPROVED BY	DATE

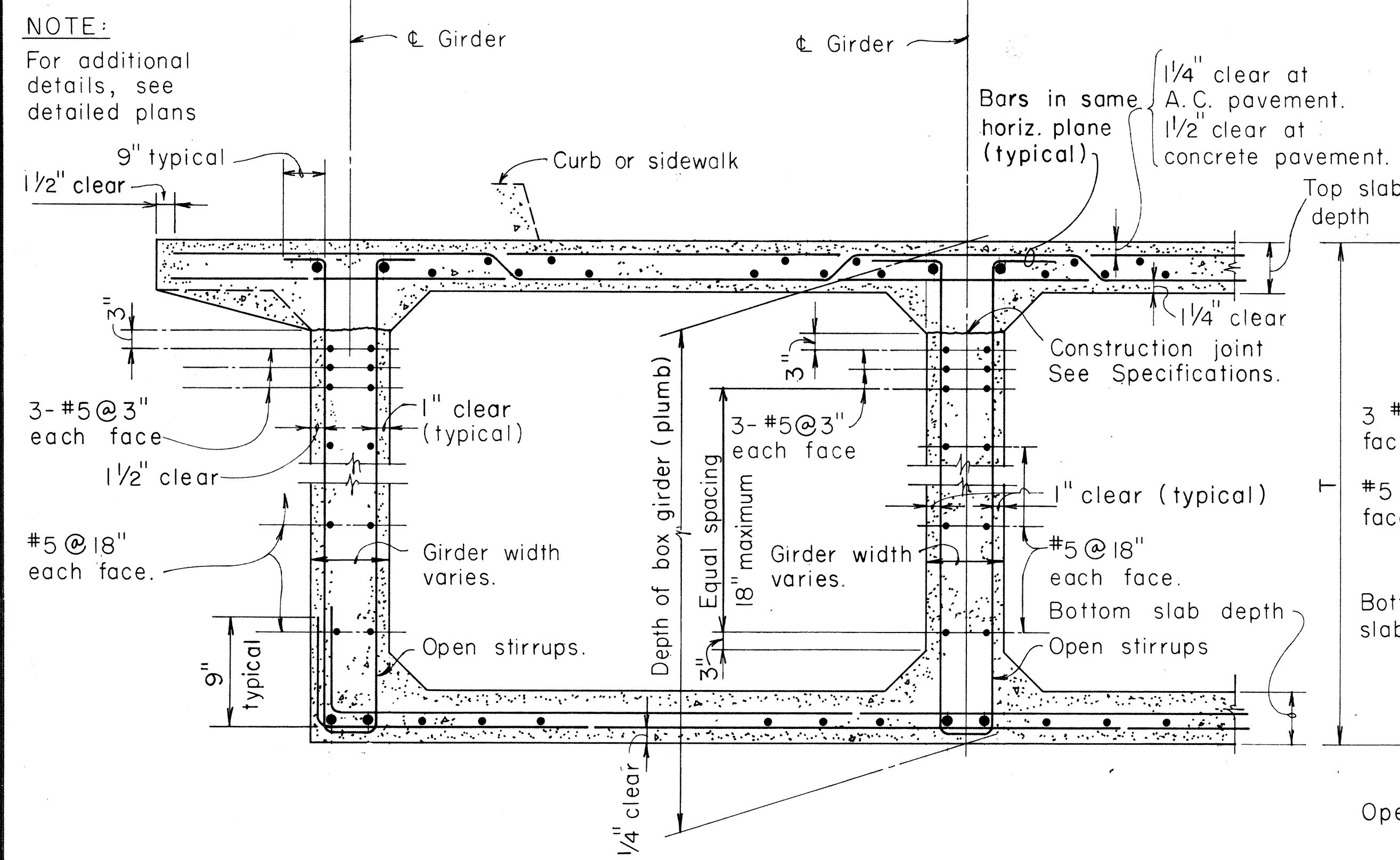
STATE OF HAWAII
 DEPARTMENT OF TRANSPORTATION
 HIGHWAYS DIVISION
STANDARD DETAILS
 NOTES AND MISCELLANEOUS
 DETAILS
 Date: October 1969
 SHEET NO. 109 OF SHEETS DB-109

DRAWN BY: []
 CHECKED BY: []
 DATE: []

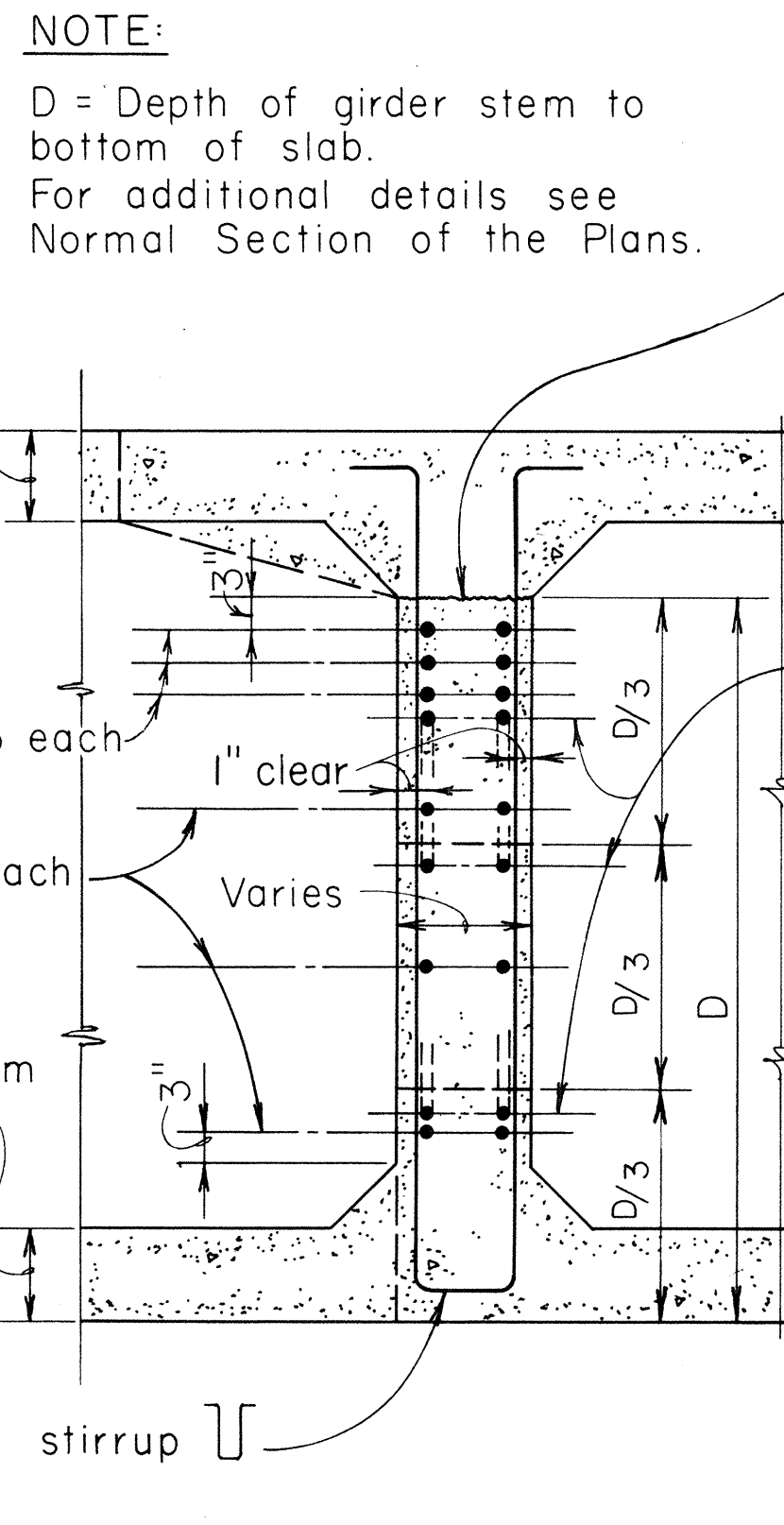
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	DRAWN BY _____	41
	TRACED BY _____	42
	DESIGNED BY _____	43
	QUANTITIES BY _____	44
NOTE BOOK	CHECKED BY _____	45
	No. _____	46



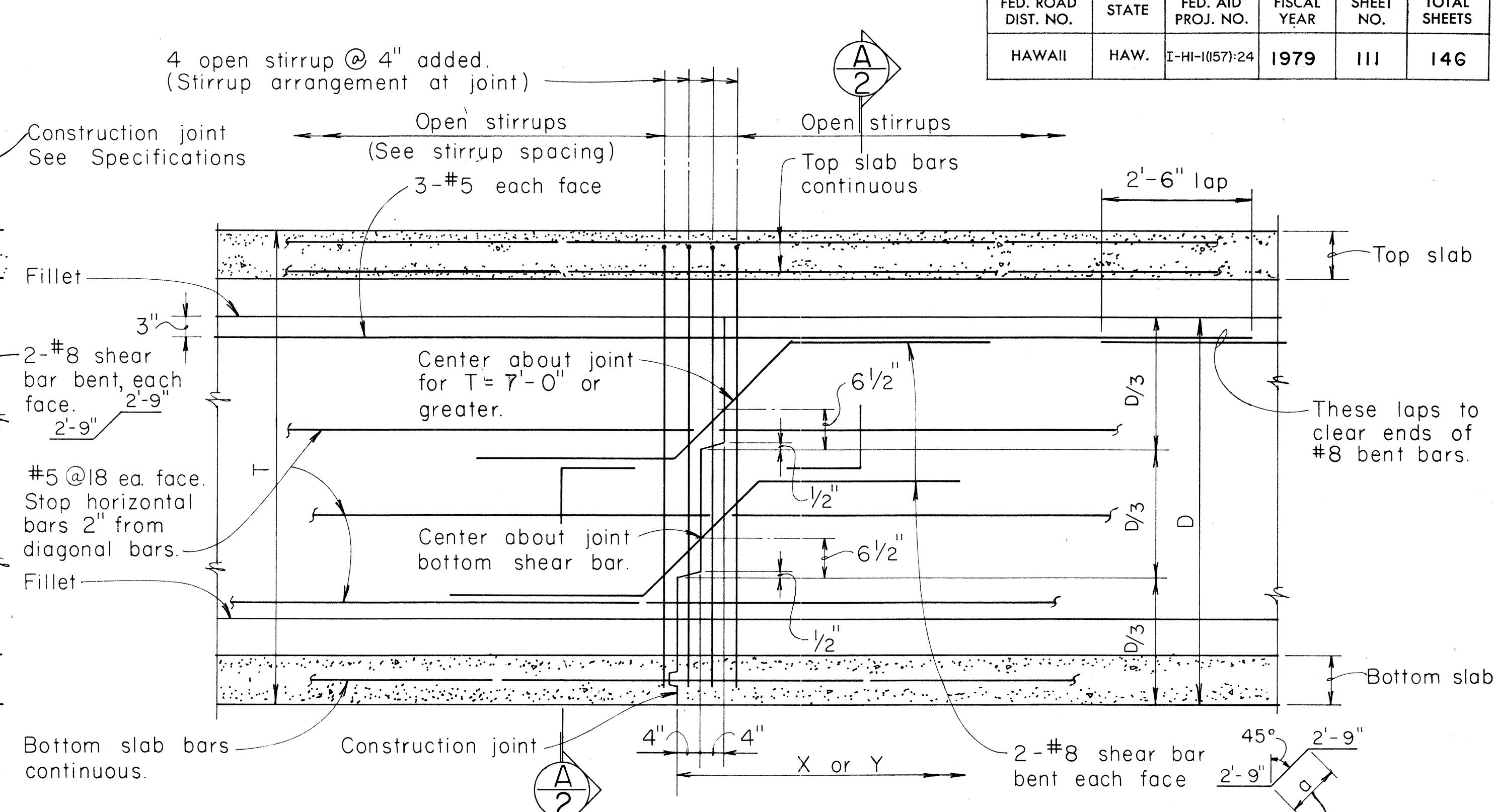
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-10571-24	1979	III	146



EXTERIOR GIRDER
TYPICAL NORMAL BOX GIRDER SECTION

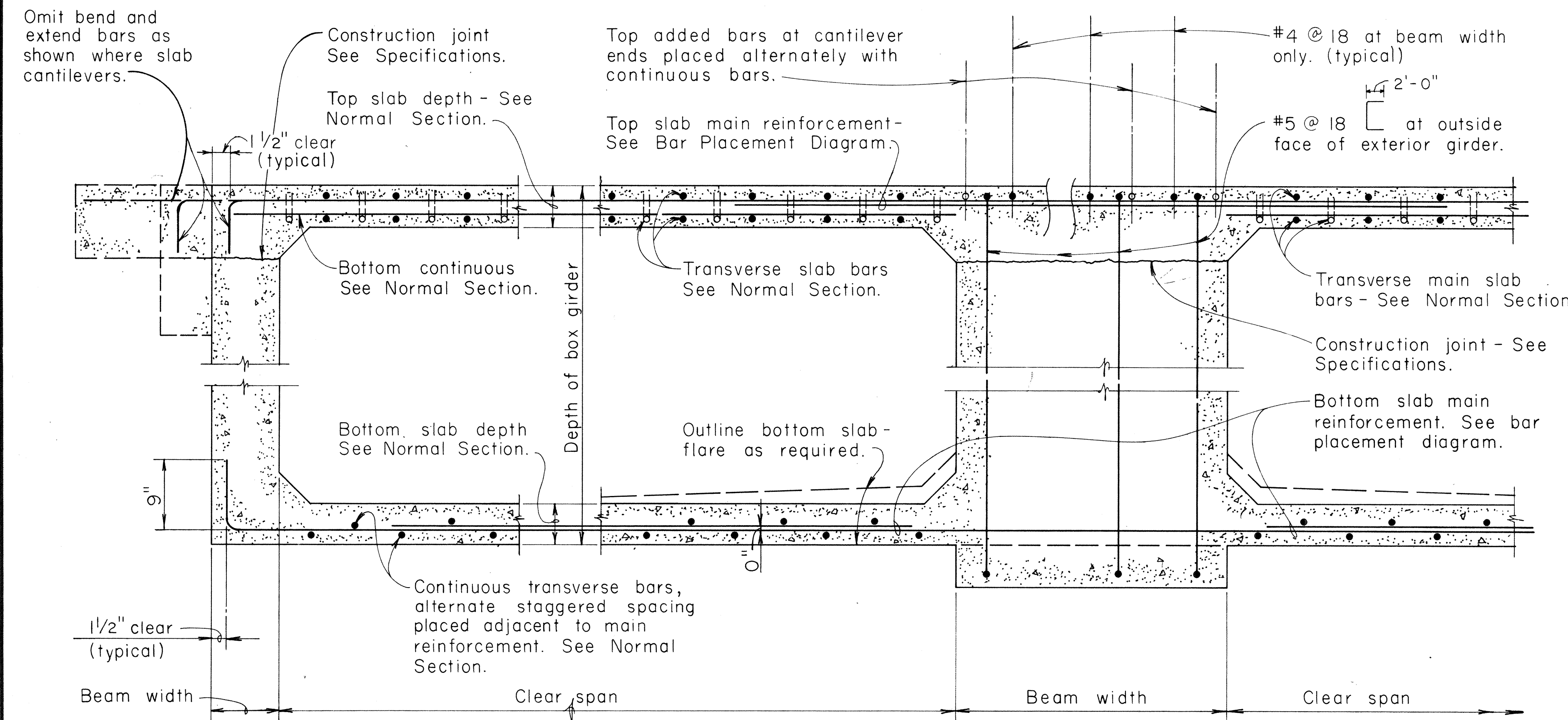


SECTION A/2



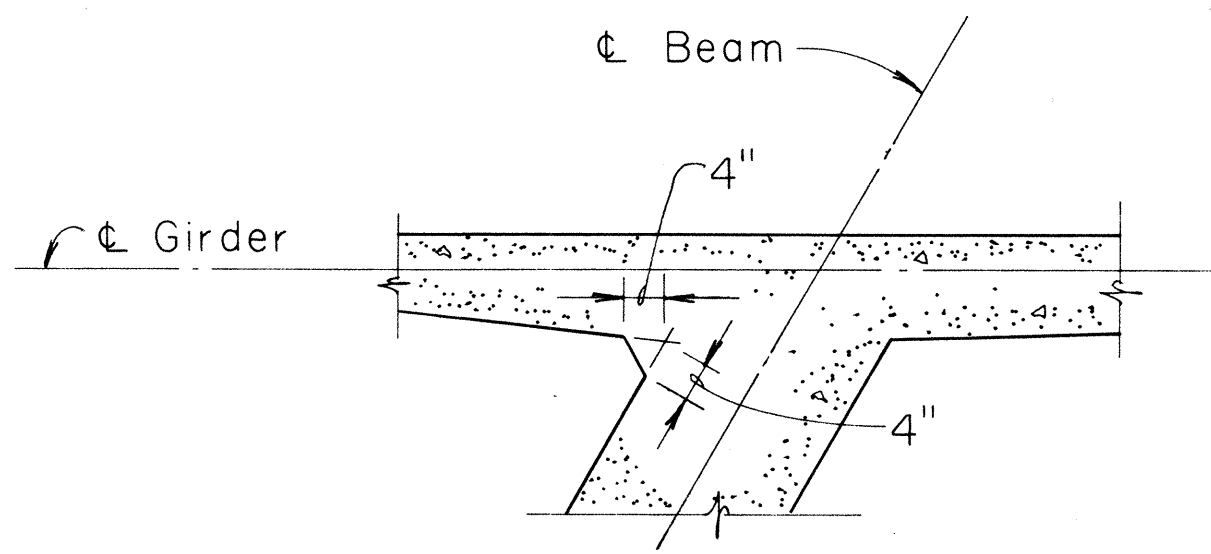
GIRDER STEM CONSTRUCTION JOINT

Depth T	a
4'-11" and smaller	1'-9"
5'-0" to 5'-11"	2'-0"
6'-0" to 6'-11"	2'-3"
7'-0" and greater	3'-0"



AT NON-CONTINUOUS END AT CONTINUOUS SUPPORT

TYPICAL LONGITUDINAL DECK SECTION
ALONG GIRDER AT NORMAL OR RADIAL SUPPORTS



TYPICAL DETAIL OF VERTICAL CHAMFER AT ACUTE INTERSECTION OF SKEWED CONCRETE MEMBER

APPROVAL RECOMMENDED:
Clarence R. Yamamoto 6-1-76
BRIDGE DESIGN ENGINEER DATE

APPROVED
Shiroshi 6-2-76
ASSISTANT CHIEF, ENGINEERING DATE

NO.	REVISION	APPROVED BY	DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

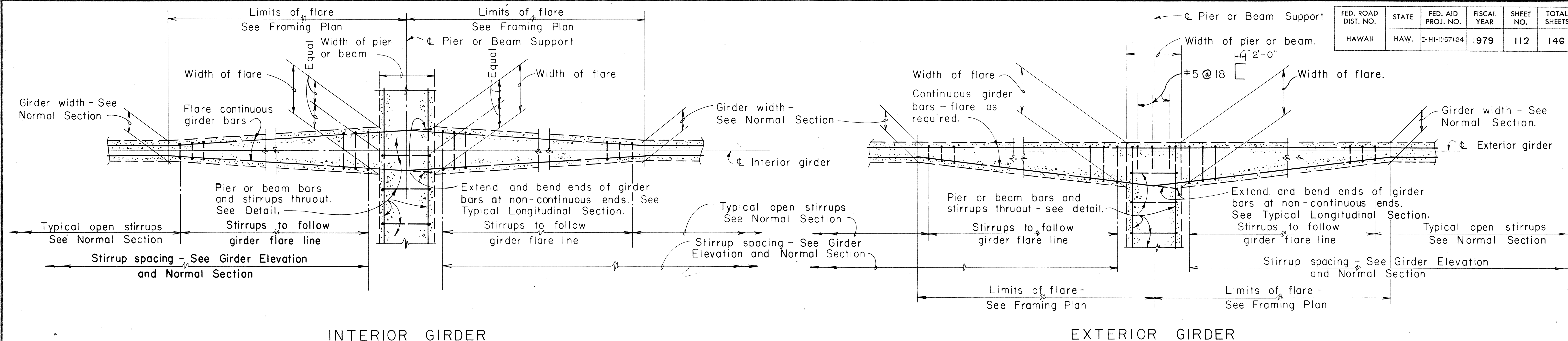
STANDARD DETAILS
CONCRETE BOX GIRDER

Not To Scale Date: May 1976

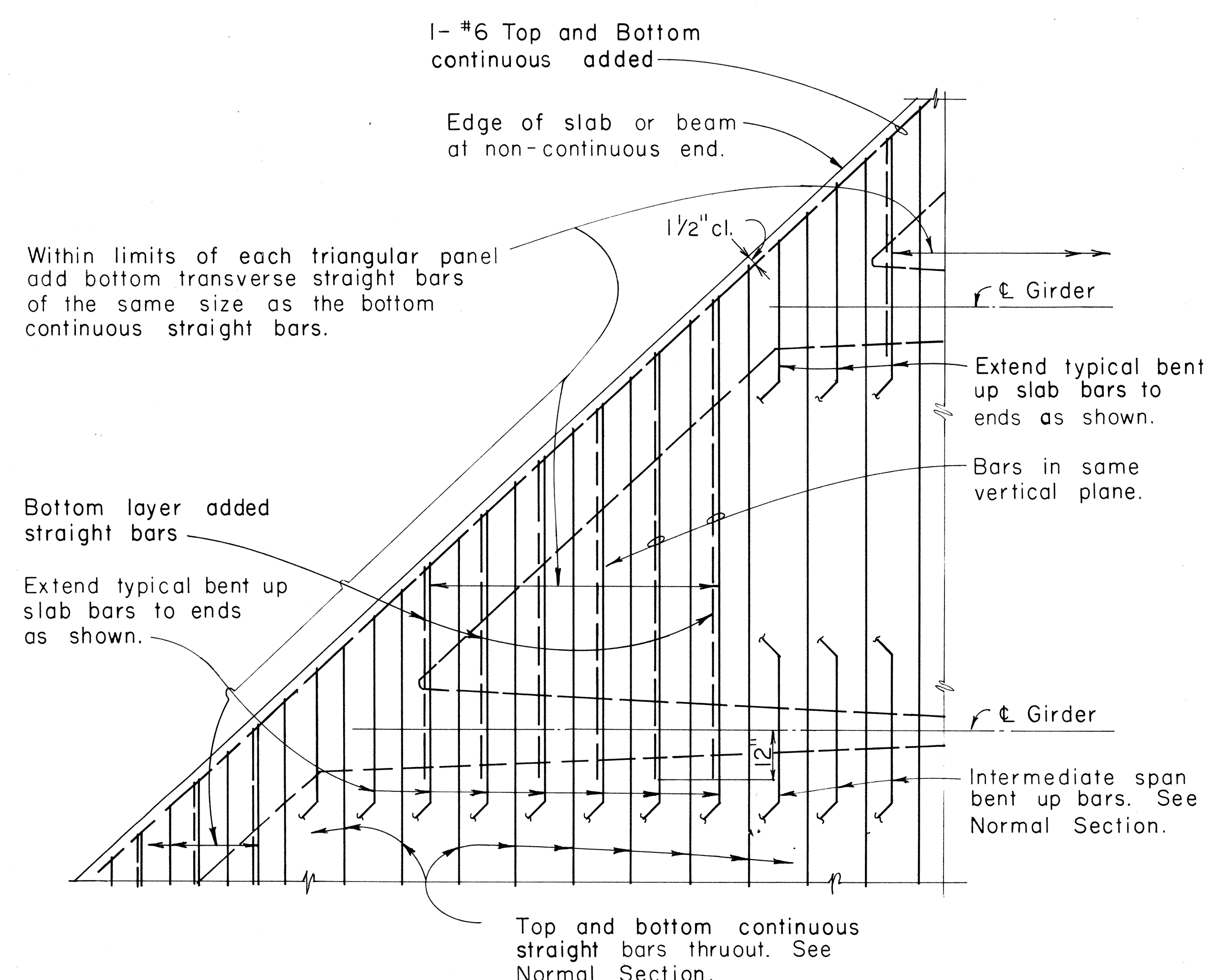
SHEET No. 2 OF 3 SHEETS DB-101-2

ORIGINAL PLAN
DESIGNED BY
CHECKED BY
DATE
NO.

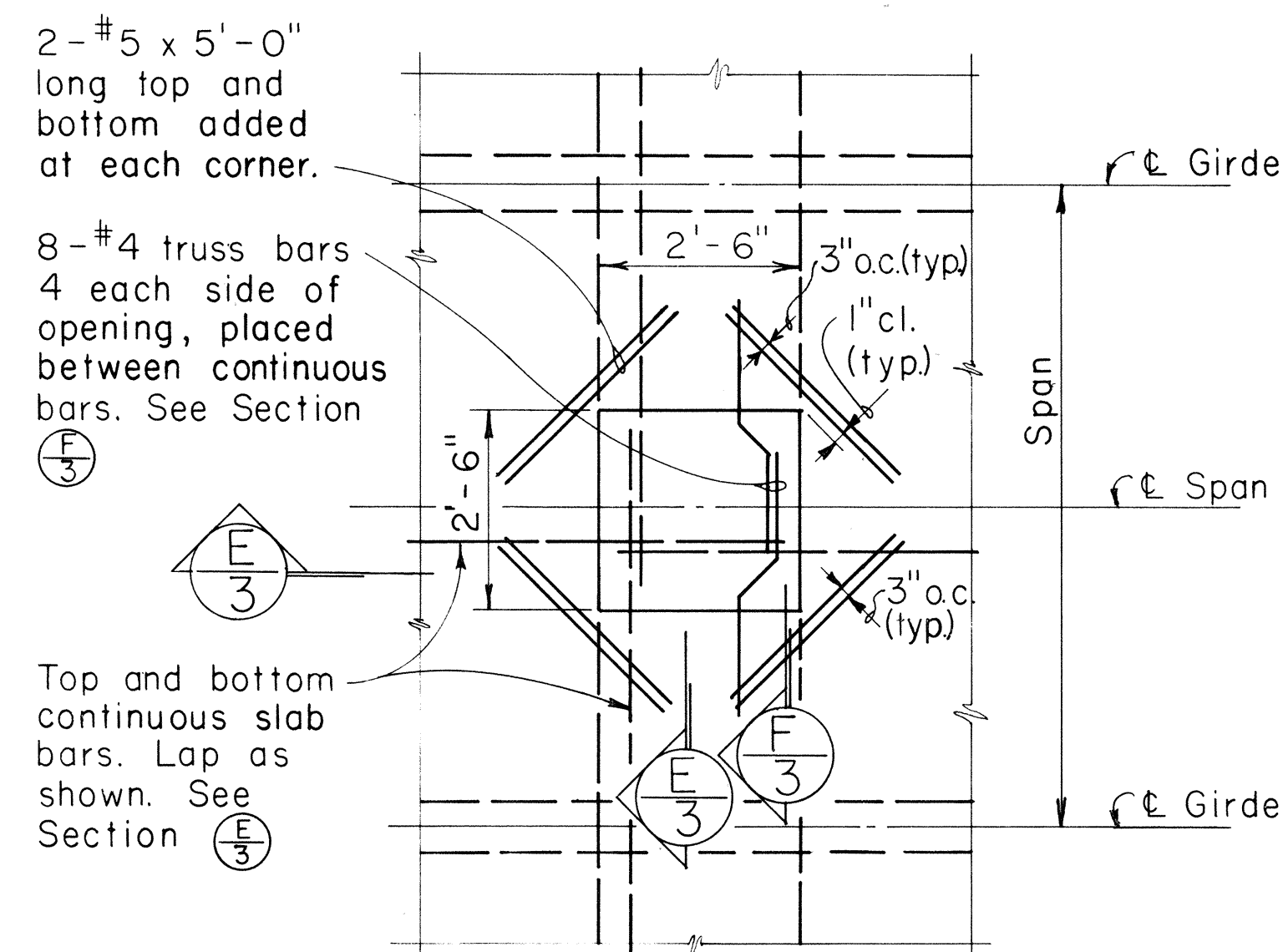
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1157-24	1979	112	146



**TYPICAL DETAILS OF GIRDER FLARE AT NORMAL OR RADIAL SUPPORTS - CONTINUOUS ENDS AS SHOWN
(NON-CONTINUOUS ENDS SIMILAR EXCEPT AS OTHERWISE NOTED)**



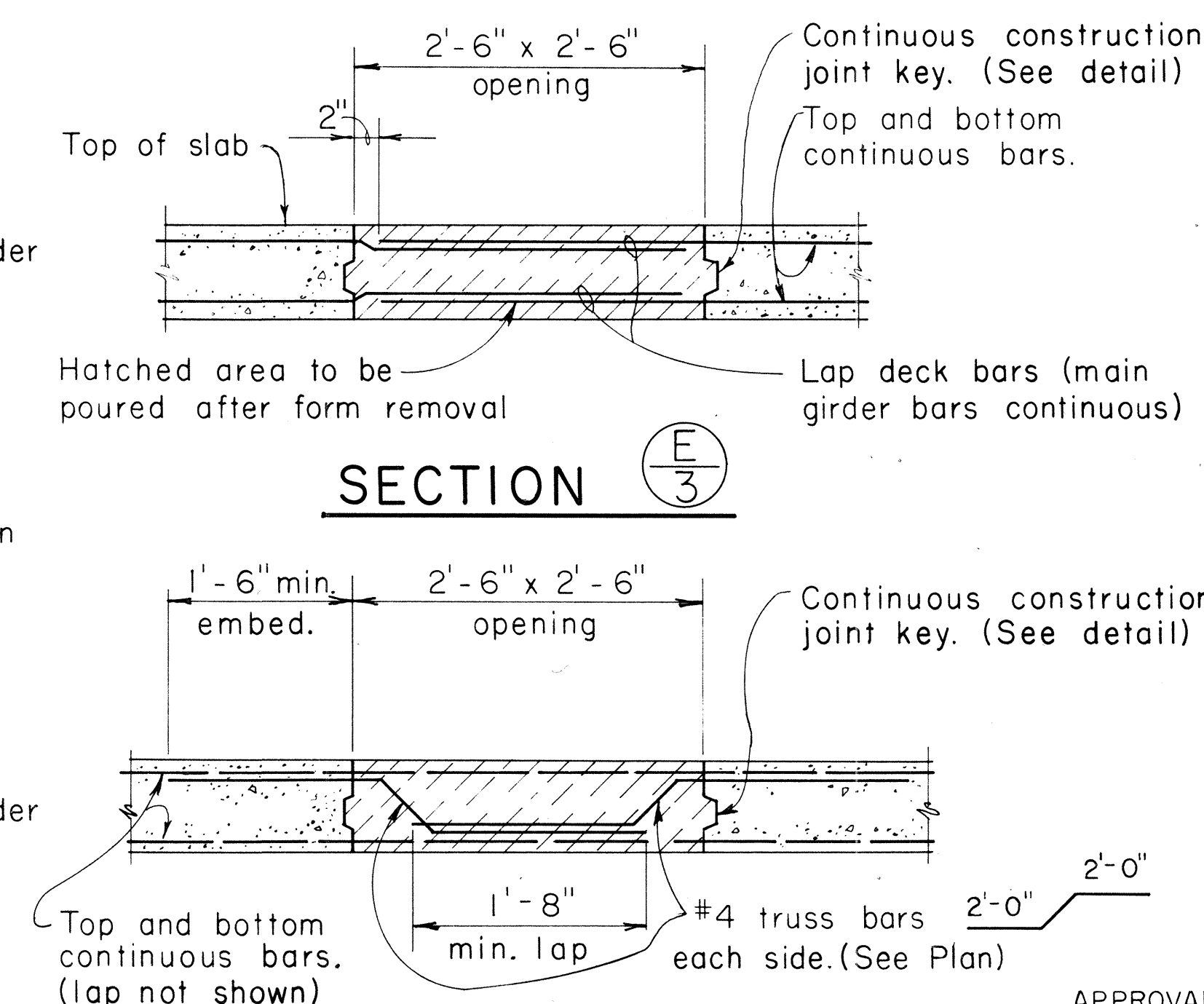
PART PLAN - TYPICAL ARRANGEMENT OF TOP SLAB REINFORCEMENT AT SKEWED ENDS



NOTES

1. Final location of opening shall be determined by the Engineer.
2. Form removal opening at top slab shall not be located where main girder bars occur.
3. Form removal opening may extend from edge of deck.

PART PLAN - DECK SHOWING FORM REMOVAL OPENING



SECTION F/3

APPROVAL RECOMMENDED
Calvin R. Yamamoto 5-1-76
BRIDGE DESIGN ENGINEER DATE

APPROVED
Robert J. Salas 5-2-76
ASSISTANT CHIEF, ENGINEERING DATE

NO.	REVISION	APPROVED BY	DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

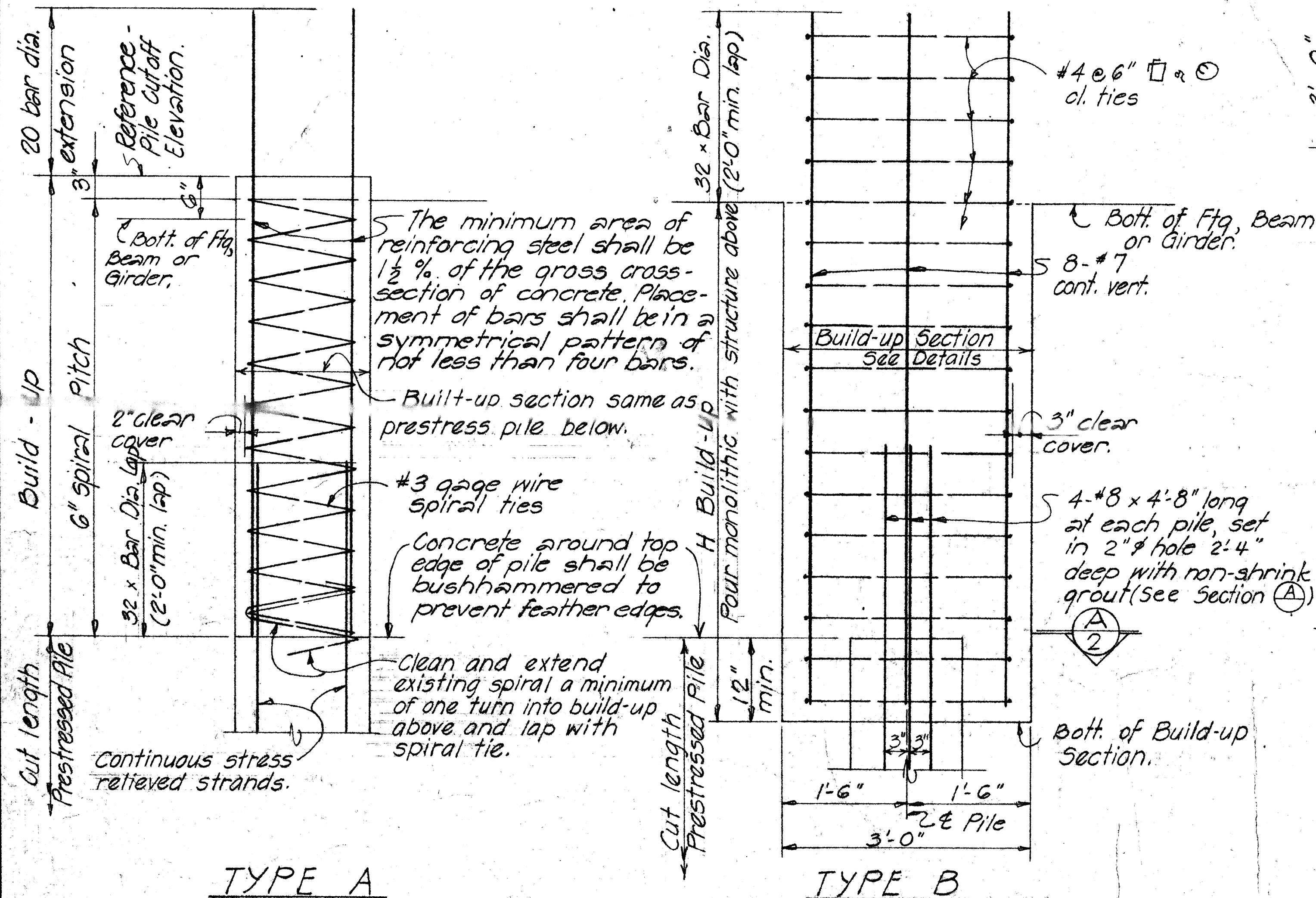
STANDARD DETAILS
CONCRETE BOX GIRDER

Not To Scale: Date: May 1976

SHEET No. 3 OF 3 SHEETS DB-101-3

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-(157)-24	1979	114	146

2'-6" min. unless otherwise directed by the Engineer.
Remove concrete after driving, leaving reinf. exposed.
Chamfer - See Typical Pile Detail.



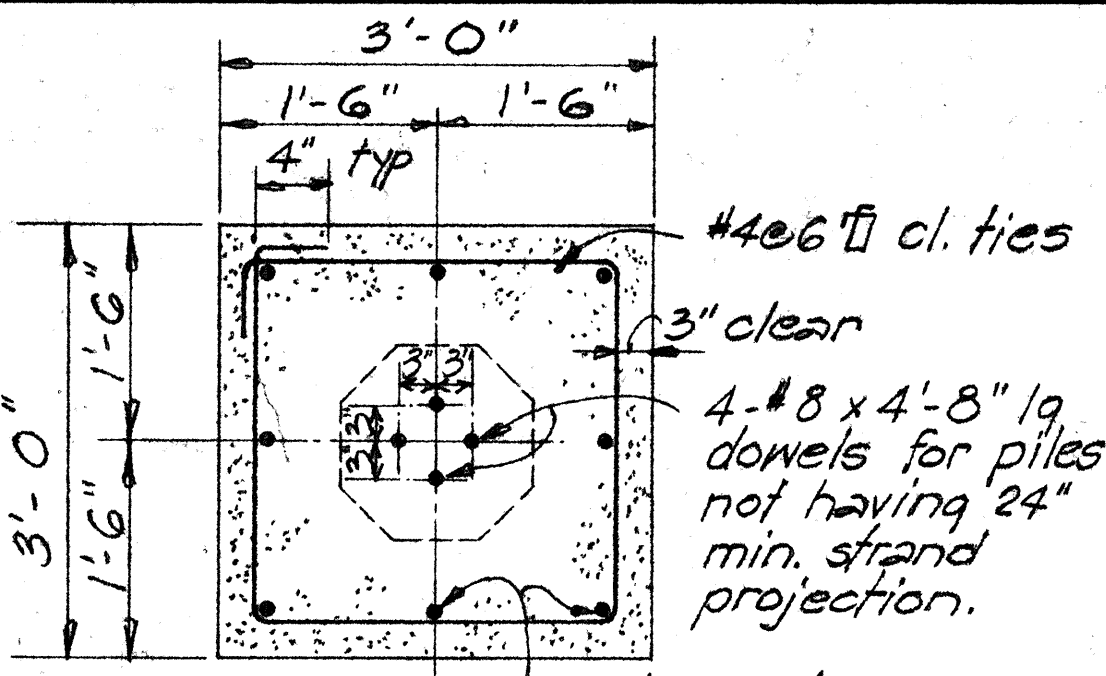
TYPE A

TYPE B

Notes:

Selection of pile build-up type without driving to be determined by the Engineer.
When height of depressed portion of footing (H) exceeds 4", the depressed section shall be poured separately from the main body of the footing.
Depressed portion of footing may be omitted for pile heads embedded at least 4" into footing provided at least 90% of the piles in each footing have the minimum embedment into footing as shown in contract plans.

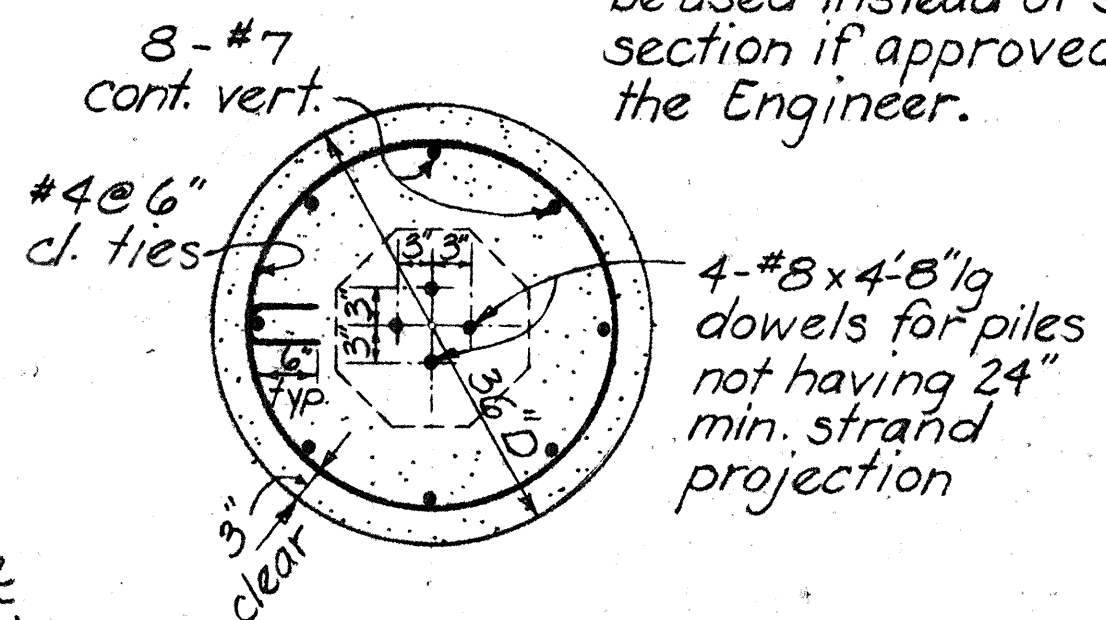
WITHOUT DRIVING TYPICAL PILE BUILD-UP DETAILS



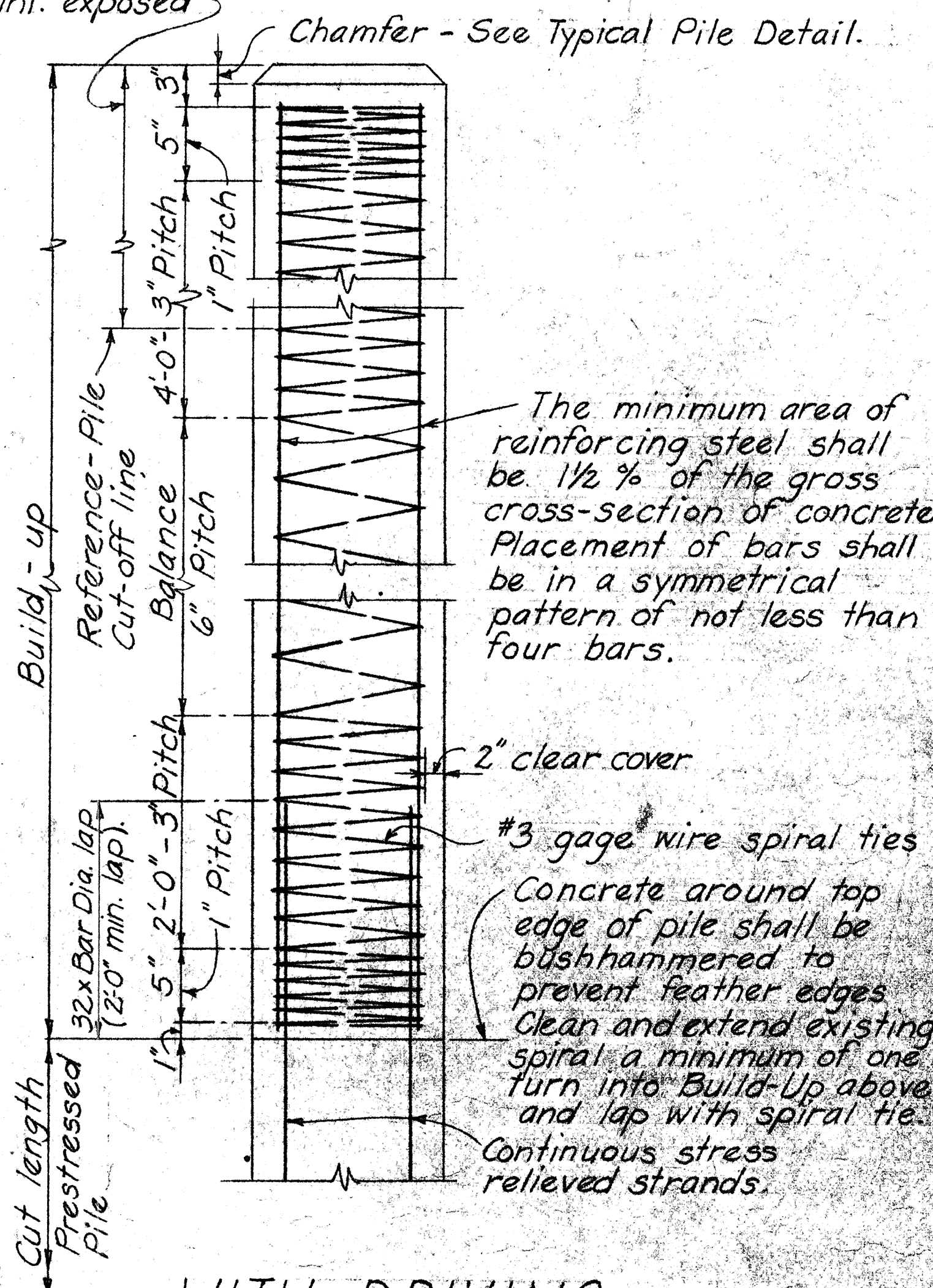
SQUARE BUILD-UP

Note:

Circular section may be used instead of square section if approved by the Engineer.



CIRCULAR BUILD-UP
SECTION A/2



WITH DRIVING TYPICAL PILE BUILD-UP DETAIL

GENERAL NOTES

1. Work of cutting off prestressed conc. piles or concrete build-ups shall be performed in such a manner as to avoid spalling or damaging the pile below cut-off. In case of such damage, the pile shall be replaced or repaired by the Contractor at his expense as directed by the Engineer.
2. Portion of pile embedded into footing shall be of sound concrete. Damaged portions shall be removed and pile cut-off elevation lowered as required by the Engineer.

APPROVAL RECOMMENDED:
Paul T. Yamashita
BRIDGE DESIGN ENGINEER
DATE 1-23-70

APPROVED
Paul T. Yamashita
For ASSISTANT CHIEF, ENGINEERING
DATE 1-28-70

NO.	REVISION	APPROVED BY	DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STANDARD DETAILS
PRESTRESSED CONCRETE PILES

Date: Oct. 1968
SHEET No. 2 OF 2 SHEETS DB-300.2

DATE	
SURVEY PLANNED BY	
DRAWN BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
NO.	

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION
HONOLULU, HAWAII

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
SECTION I REPAIRS

FEDERAL AID INTERSTATE PROJECT NO. I-HI-1 (157): 24

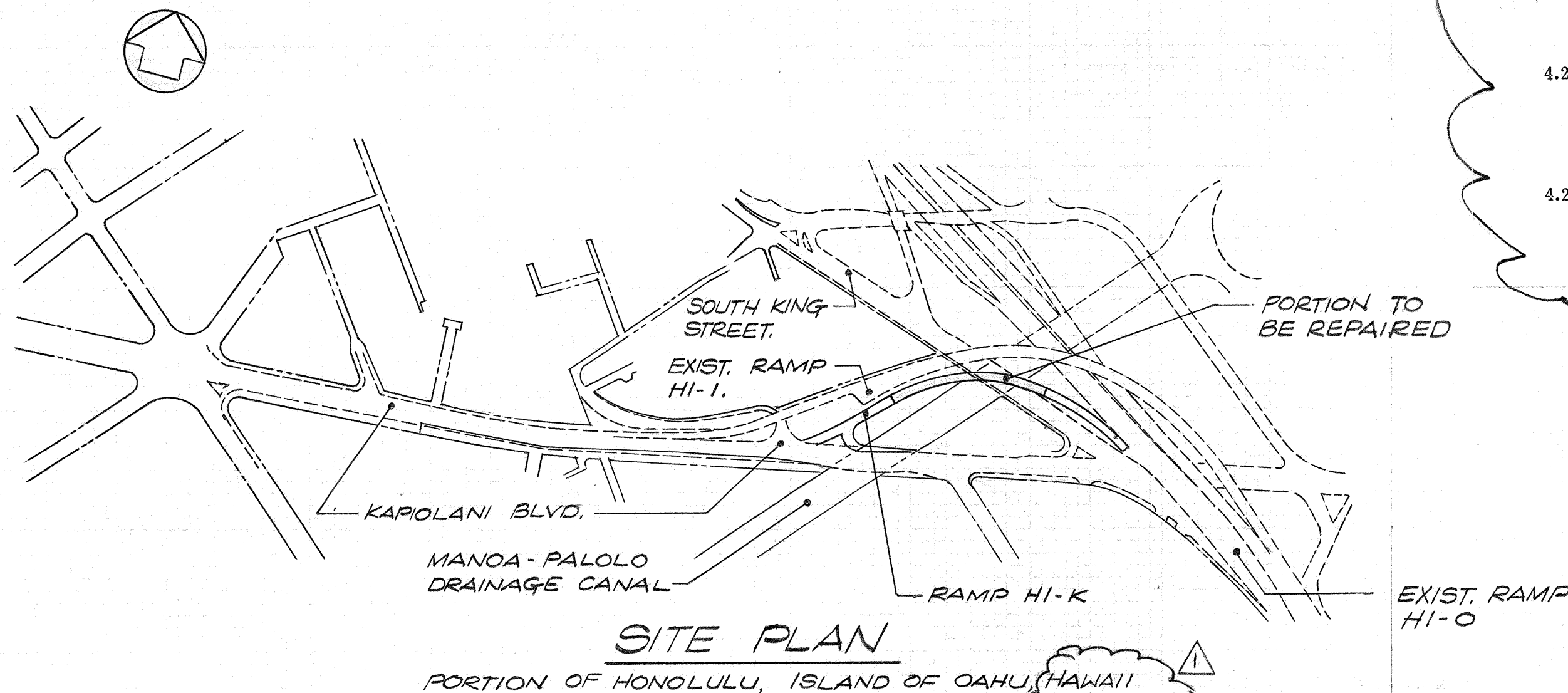
DISTRICT OF HONOLULU
ISLAND OF OAHU

DRAWING LIST

SR-1	GENERAL NOTES
SR-2	LAYOUT PLAN & LONGITUDINAL SECTION
SR-3	DEMOLITION PLAN
SR-4	WEB ELEVATION
SR-5	TYPICAL SOUTH WEB SECTION ELEVATION & DETAILS
SR-6	SOUTH WEB SECTIONS
SR-7	CENTER WEB SECTIONS
SR-8	NORTH WEB SECTIONS
SR-9	DOWEL DETAILS
SR-10	MISCELLANEOUS DETAILS
SR-11	ADDED POST-TENSIONING DETAILS

QUANTITY SCHEDULE

TYPE	REFERENCE	NOTE	NUMBER	WEIGHT
HOOKED DOWELS				
# 5 EMBEDDED	DETAIL 2/SR-9		1078	3230#
# 5 EMBEDDED	DETAIL 2A/SR-9		139	490
# 6 EMBEDDED	DETAIL 4/SR-9		408	1400
# 5 THREADED	DETAIL 3/SR-9		319	850
# 5 THREADED	DETAIL 7/SR-9	@ NORTH WEB ONLY	401	900
# 6 THREADED	DETAIL 1/SR-9		935	3550
# 6 THREADED	DETAIL 3/SR-9	@ NORTH WEB ONLY	513	1760
STIRRUPS				
# 5 HOOKED TOP & BOT.		@ SOUTH WEB ONLY	49	470
# 5 HOOKED @ BOT. ONLY		@ SOUTH WEB ONLY	70	720
# 5 HOOKED @ TOP ONLY		CENTER & NORTH WEBS	70	645
# 5 STRAIGHT		CENTER & NORTH WEBS	120	1140
# 6 HOOKED TOP & BOT.		@ SOUTH WEB ONLY	194	2020
# 6 HOOKED @ BOT. ONLY		@ SOUTH WEB ONLY	293	4240
# 6 HOOKED @ TOP ONLY		CENTER & NORTH WEBS	410	4910
# 6 STRAIGHT		CENTER & NORTH WEBS	578	7450
DOWELS				
# 5 x 3'-0"	DETAIL 3/SR-10	@ PIERS & BEAMS	32	101
# 5 x 6'-0"	DETAIL 4/SR-10	@ DIAPHRAGMS	20	120
# 4 x 3'-0"	DETAIL 3/SR-10	@ PIERS & BEAMS	80	161
# 4 x 6'-0"	DETAIL 4/SR-10	@ DIAPHRAGMS	50	201
HORIZONTAL WEB REINFORCING				
# 4 CONT.				9480
# 5 CONT.				5490
REINFORCING AT ADDED TENDON BLISTERS				
# 5 TIES	SECTIONS A & B/SR-11			1230
# 8 DOWELS	SECTIONS C & D/SR-11			1010
SPLICE BARS				
# 5	SECTION A/SR-5			450



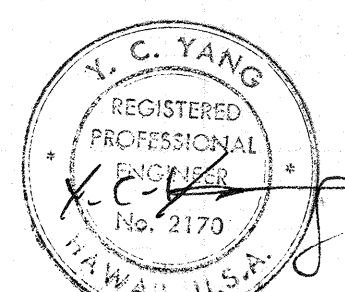
REPAIR WORK

- 1.0. Access Openings (SR-10)
- 1.1. Provide temporary openings in the Deck Slab for workmen and materials. Openings to be 4'-6" x 2'-6" centered between webs. Edge to have 1" deep saw cut surface at top. Leave 4" extensions of deck rebar at edge to allow weld splicing for repair. (Detail 1/SR-10)
 - 1.1.1. Repair openings after post-tensionings. Replace all rebar by weld splicing new rebar of the same size to the 4" extensions. Epoxy bond existing to new concrete. See specifications Section 9.3.5.
- 1.2. Provide permanent openings in the cells for both diaphragms. Center between webs. Finished opening 3'-0"x4'-0" with 4" corner chamfers, 18" clear of Bottom Slab. Initial openings 4'-0"x3'-0" to allow hooking existing bars and additions of new rebar. Cut-set top for concrete placement. (Detail 2/SR-10 and Section A/SR-10)
- 2.0. Selective Demolition (SR-3)
 - 2.1. Remove existing deck soffit forms. Reuse for repair forms.
 - 2.2. Remove Unsound Concrete.
 - 2.2.1. Remove spalled concrete still in place on webs.
 - 2.2.2. Remove highly fractured concrete at base of webs.
 - 2.2.3. Remove minor amounts of unsound concrete in various locations on webs.
 - 2.3. Remove exposed web reinf. at the South face of the webs in the spalled areas on the South Web. Cut off exposed rebar 3" from sound embedment. (Section A/SR-5)
 - 2.4. Remove damaged duct in spalled areas on the South Web. Remove concrete to expose sound embedded duct to provide an exposed length for coupling. Coordinate length with P.T. system supplier.
- 3.0. Concrete Pour and Vent Holes (SR-5)
 - 3.0.1. Provide holes through the deck for concrete placement and vibrator access. These holes also provide space for the embedment and epoxy bonding of stirrup bars. Determine and mark the location of existing top rebar on the deck surface before cutting forms. Vary hole locations slightly to avoid cutting rebar. (Detail 1/SR-5)
 - 3.1. Pour holes are 45" o.c. Use 4" diameter cores and pneumatic tools. The basic type is 5"x12" (South Web) or 4"x12" (North and Center Web) thickness of the concrete. Centered on one exposed deck rebar, and containing 2 stirrup bars.
 - 3.2. Vent holes are 45" o.c., spaced between pour holes, and contain one stirrup bar each. Use 3" diameter core holes. Vent holes must be at least 12" clear from pour holes.
 - 3.3. Provide an additional vent hole, without stirrup bar, adjacent to the surfaces of end forms and diaphragms where a standard spaced pour or vent hole is further than 12" from that surface.
 - 4.0. Prestress Tendons (SR-4) (SR-11)
 - 4.0.1. Provide materials, tensioning and grouting to complete the work to meet original and repair requirements, and be compatible with the existing conditions. Each web contains 6 tendons, each with a capacity of 31 strands (1/2" # 270K). Tendons materials are 4-1/2" diameter duct, bearing plates, wedge plates, wedges, strand, miscellaneous attachments, and grout. Those tendons having been detensioned shall be removed. New strands (1/2" # 270K) must be supplied. Use 31 strands in each tendon. Bearing plates are undamaged and are to be reused in place. New duct is to be supplied to replace the damaged duct in spalled areas of South Web. Tendons must be grouted.
 - 4.0.2. Check permanent ducts for damage and blockages before beginning repair, and before and after new concrete placement. Perform check by passing a mandrel, not less than 3-1/2" diameter, through the duct. Correct any damaged or blocked duct before proceeding further with the work.
 - 4.1. Existing duct in sound concrete is to remain in place.
 - 4.2. Provide new 4-1/2" galvanized duct in spalled areas to replace the damaged duct. Couple the new duct to existing embedded duct. The paths of new ducts shall match existing profile.
 - 4.2.1. Remove existing concrete from around new ducts to provide clearances for the concrete pour. Minimum clearances between duct and existing surface to be 1" horizontally, 1-1/2" vertically. At coupling locations, the minimum clearances are 2" horizontally, 3" vertically. (Section A/SR-5)
 - 4.2.2. Provide inspection holes through web where new ducts have less than 1-1/2" clear between them in the South Web. Use 1" diameter drill holes at the top, and bottom of pattern 12" o.c. along web. (Section A/SR-5)

- 4.2.2.1. Inspect holes during concrete pour. Plug flowing holes until concrete set, then remove plug and dry pack. If no flowing occurs inject grout to fill void after the pour.
- 4.3. After the new concrete is placed, pull in new strand and prepare for tensioning.
- 4.4. Tension the tendons in the required sequence when the field cured concrete test cylinders have a tested strength of 4000 psi.
- 4.5. After the field tensioning records have been approved by the Engineer and State, remove the excessive strand extensions and grout the duct.
- 4.6. For longitudinal additional tendons (3-12-1/2" # strand tendons). (SR-11)
 - 4.6.1. Chip off the deck slab at the vicinity of the proposed new anchorage block. Drill holes to seat reinforcing as called out on the drawings. (Sect. A/SR-11) (Sect. B/SR-11)
 - 4.6.2. Epoxy grout reinforcing in pre-drilled holes and install stirrup tie reinforcing after placement of anchorage assembly at both ends of the tendon.
 - 4.6.3. Provide rigid steel pipe at front end of the anchorage trumpet, securing the joint with steel sleeve. The rigid steel pipe will be joined with plastic P.E. sheath with plastic sleeve and watertight joint sealer at the joint. The P.E. pipe shall be rated to withstand 150 psi for 1-1/2 hours. Contractor shall submit related material and connection details for State approval. (SR-11)
- 5.0. Roughen Surfaces (SR-3, 6, 7, 8)
 - 5.1. Roughen all existing concrete surfaces which will contact new concrete. Use pneumatic, or sand blast and pneumatic tools. Remove all surface paste, expose aggregate, provide roughness equal to 1/8" undulation in any 2" distance. A groove of 1/4" depth in any 6" distance will be considered equivalent for roughness.
- 6.0. Keys (SR-3) (SR-10)
 - 6.1. Provide continuous keys in the Bottom Slab and Sides of Diaphragms, Peris and Expansion Joint Beams, at North and Center Webs, 1" deep x 4" wide, with a 3/4" deep saw cut edge.
- 7.0. Rebar (SR-4 through SR-11)
 - 7.1. Stirrups: #5 and #6, 1-1/2" clear from new South face, 1" clear for North and Center Web. Embedment of some bars in the concrete pour and vent holes is required. Uniform spacings shown on the drawings are nominal only and are to be used as an equivalency criteria, in place spacings may be between 6" minimum, 12" maximum. Embedment of bars in the Bottom Slab at the North and Center Webs is required, as well as in pour and vent holes.
 - 7.1.1. Two stirrup bars are embedded a minimum of 6" in each Pour Hole, and one in each Vent Hole. The top 6" of bar is to be epoxy bonded. Use tape to protect the area during initial stage of concrete pour. When pour reaches the bottom of the Deck remove tape and coat surface with epoxy, then complete the pour to the top of the Deck.
 - 7.1.2. Stirrup bars are embedded in Bottom Slab at the North and Center webs in 4" deep epoxy grouted drill holes in the key. (Detail 6/SR-9)
 - 7.1.3. All stirrup bar ends not embedded in Deck are to have 90° degree hooks 3" long.
 - 7.2. Dowels in Webs: All hooked 90° degrees one end, 1-1/2" clear from new South face, 1" from new North and Center Web face; other end is embedded in existing web in epoxy grouted drill holes. Drill the holes with a rotary-impact drill. Layout locations in advance to insure no conflicts with prestress ducts or other rebar. (SR-4)
 - 7.2.1. Standard arrangement is in stacks of 4 dowels. Stacks are 18" o.c. along webs. (SR-4) Align vertical locations with those of dowels in adjacent stacks as much as possible to allow mutual use of horizontal bars inside the dowel hooks. Locate dowels not closer than 12" to Deck Slab, 6" in Bottom Slab. Do not locate dowels directly beneath centers of Concrete Pour or Vent Holes.
 - 7.2.1.1. First dowel adjacent to duct either top or bottom to be #6 bar, the dowels between duct groups to be #6 bar for exposed ducts and #5 for the rest of the webs. Add dowels at 36" c. along lines adjacent to ducts. All (SR-4) #6 and #5 dowels adjacent to ducts are in 1-1/4" and 1" diameter holes respectively drilled through the concrete. Each dowel is threaded 4" and has a nut, washer and bearing plate attached at the North face of the web. The dowels in the web flare area will have an embedment length of 12". On the North Web bearing plate is inserted in a 1-1/2" deep pocket. Epoxy grout all drill holes and bearing plates. Epoxy coat all steel surfaces of the bearing plate assembly after installation. Epoxy coat pocket surfaces on the North Web, and dry pack the pockets. (North and Center Web: SR-7 and SR-8, South Web: SR-5 and SR-6.)

- 7.2.1.2. At flare area of South Web the dowels adjacent to the ducts are only the #6's at the stack line, and are in 12" deep, 1-1/4" diameter drill holes, epoxy grouted. (Detail 4/SR-9)
- 7.2.1.3. Remainder of dowels in the stack are #5 bars, embedded in 1" diameter 9" deep drill holes, epoxy grouted. (Detail 2/SR-9)
- 7.2.2. Provide #5 or #6 stirrups in 1" and 1-1/4" diameter respectively 9" deep (into sound concrete) drill holes, epoxy grouted, through the South Web into the Bottom Slab at 4" from the soffit. Nominal spacing is 9" o.c. along the South Web. (Detail 2/SR-5)
- 7.3. Horizontal bars in the web repairs: Lap to straight dowels embedded in diaphragms piers and beams at Center and North Webs. Lap 2'-0" at South Web. Hook 90° at formed ends of web repairs at South Web. (3/SR-10, 4/SR-10)
- 7.4. Provide straight bars, #4, 5 and 6, for Access Opening repair, miscellaneous placements and support bars. Field cut, bend and weld splice as necessary. (Detail 1/SR-10)
- 7.5. Field bending and Welding. (Weld Table-Detail 5/SR-10)
 - 7.5.1. Welders must be prequalified per AWS D1.4.
 - 7.5.2. Hot bending may be used if approved by the State.
- 8.0. Other Items
 - 8.1. Provide forming and concrete for the repairs as required in the specifications.
 - 8.2. Provide epoxy injection bonding of the cracks in the webs as stated in the specifications. Coordinate with the State.
- 9.0. Suggested Sequence for Coordination of Work
 - 9.1. Cut out the deck access openings (1.0.). Perform the selective demolition (2.0.). Check the ducts for obstruction (4.0.2.). Cut out diaphragm access openings. (Detail 2/SR-10)
 - 9.2. Locate and mark on the deck surface the position of the existing top rebar in the pour and vent hole areas, then cut out the openings (3.0.).
 - 9.3. Locate and mark the extent of the duct on the existing web surfaces, for both embedded and new patterns (4.2.1., 4.2.2.). Cut the concrete to provide duct clearances (4.2.1.) and openings (1.0.).
 - 9.4. Use a string line from the pour and vent holes to mark the location of new embedded stirrup bars on the web surface (7.1., 7.1.1., 7.1.2.). Mark out and drill holes for the stirrup bars in the Bottom Slab at the North and Center Webs (7.1.2.).
 - 9.5. Mark the intended location of web dowels on the web surface (7.2.). Check for patterning clearances, and for no interference with stirrups. Drill the dowel holes. Make the bearing plate pockets in the North Web. Place the web dowels in their holes and check that the cover and hole depth requirements are met.
 - 9.6. Mark the intended location of the horizontal bar diaphragm dowels (7.3.). Check to insure no interference with web dowels or stirrups. Drill the dowel holes.
 - 9.7. Epoxy grout the diaphragm dowels, then the web dowels and their bearing plates.
 - 9.8. Drill the inspection holes in the web (4.2.2.). Place the new tendon ducts (4.2.). Weld splices of new reinforcing to existing extensions where web reinforcing has been cut and removed (2.3.). Inspect epoxy grouted dowels.
 - 9.9. Loose place the horizontal bars for the webs (7.3.). Epoxy grout the stirrup bars into the bottom slab. Tie-in the horizontal bars.
 - 9.10. Check the placements, complete unfinished work, and close the web forms.
 - 9.11. Check the ducts for obstructions (4.0.2.). Wet the existing concrete surface in the repair area for 24 hours prior to the concrete pour.
 - 9.12. Place concrete for webs and epoxy bond stirrups (7.1.1.). Make test cylinders. Check the inspection holes on the webs (4.2.2.1.). Start the cure.
 - 9.13. Complete prestress work (4.3., 4.4., 4.5.) and any other remaining work.
 - 9.14. Weld splice new rebar in the deck access openings (1.1.1.).
 - 9.15. Close deck access openings with 5000 psi concrete, texture and cure.

C.O. 114 S-1



Location of Dowels - From 8" to 6" in Bottom Slab 7-23-82
7-2-82

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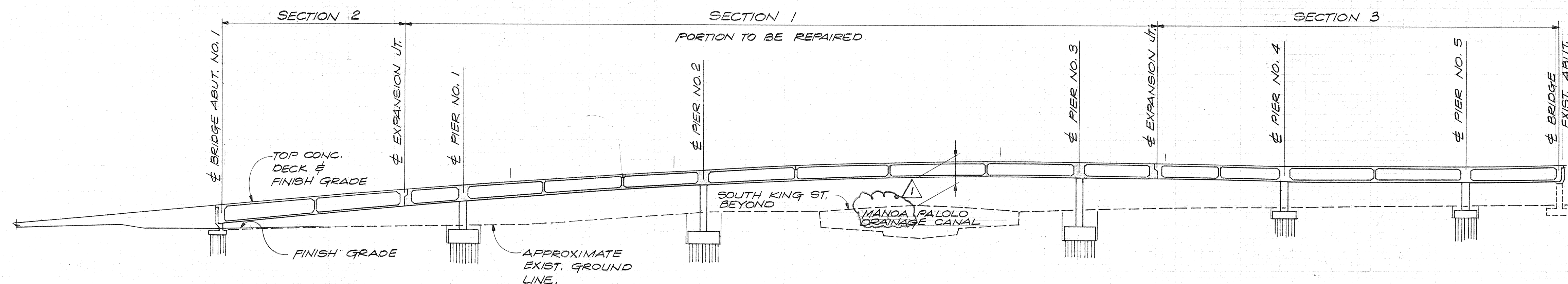
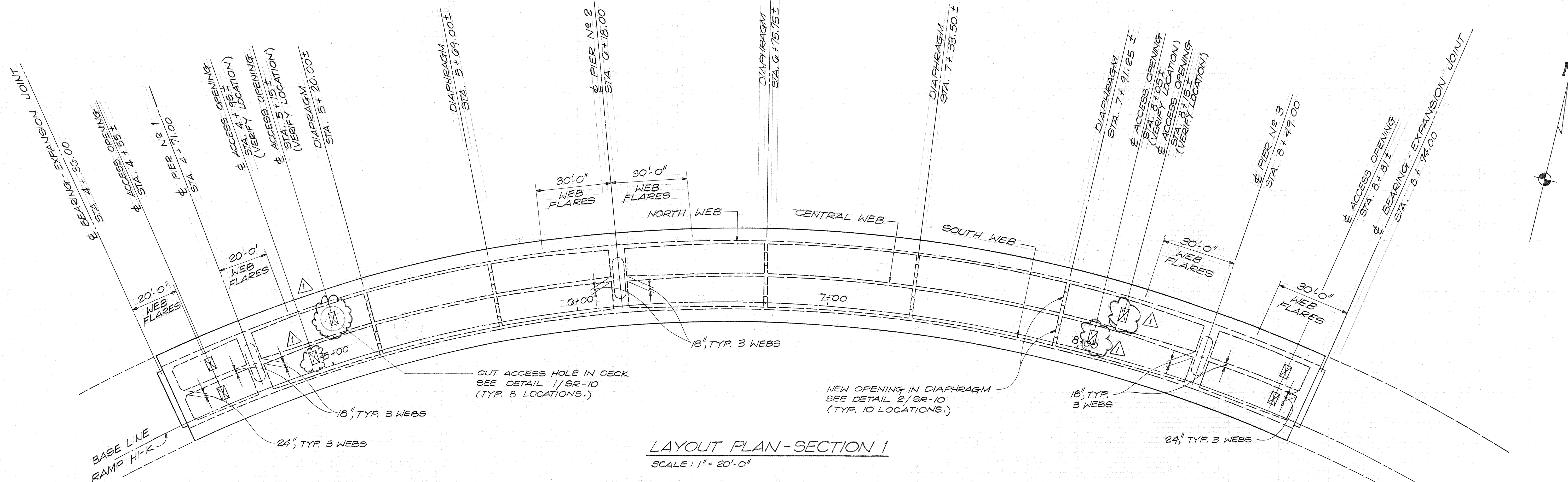
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SHEET TITLE:	GENERAL NOTES
PROJECT:	KAPIOLANI INTERCHANGE ON-RAMP REPAIRS F.A.I. PROJECT NO. I-HI-1 (157): 24

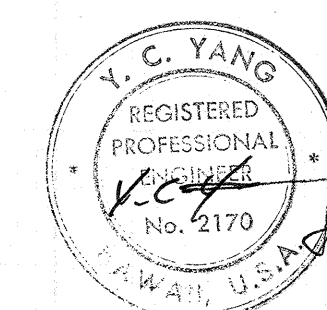
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SHEET 1 OF 12 SHEETS	

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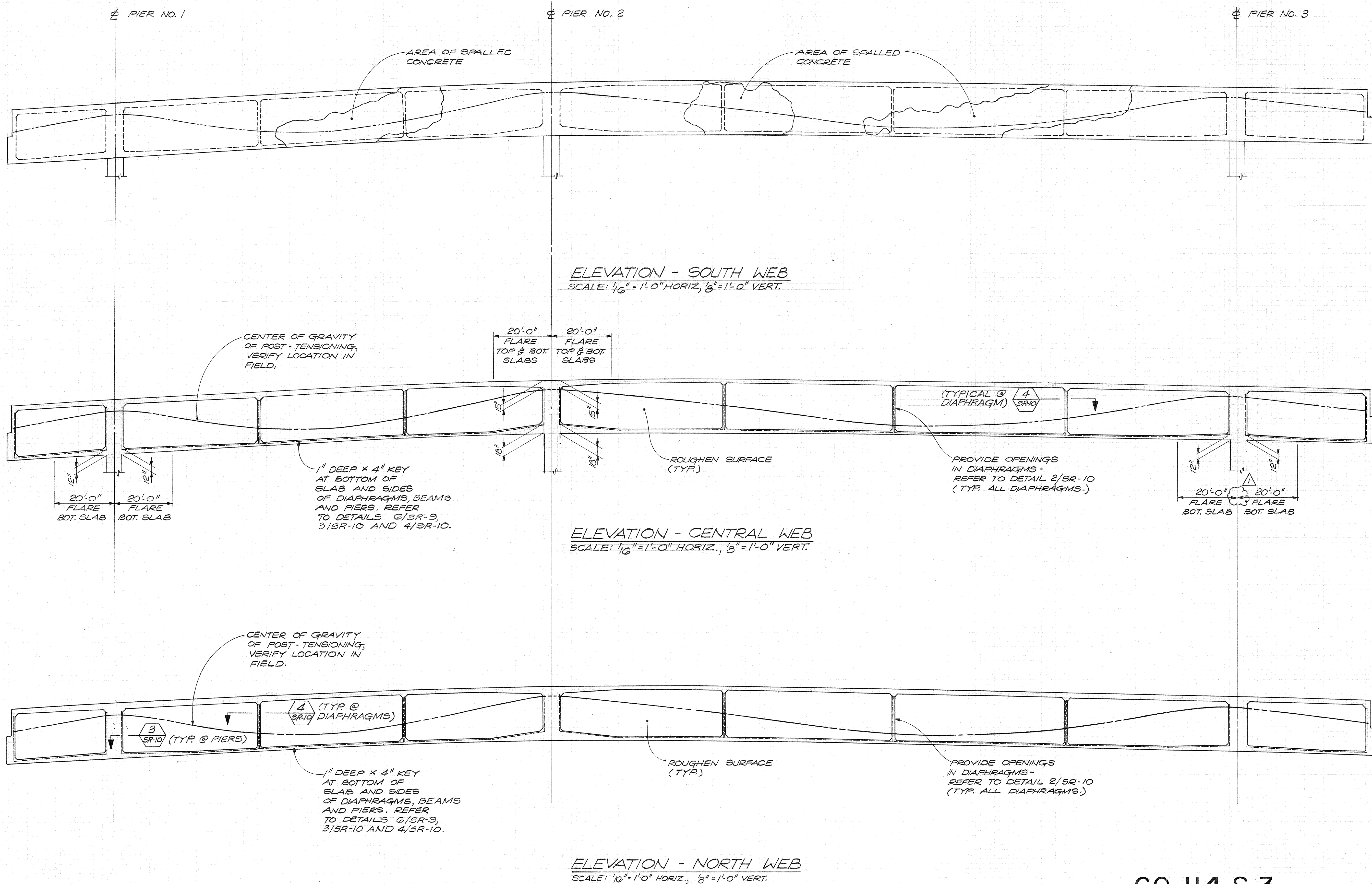
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REVIEW	5-82	D.C.

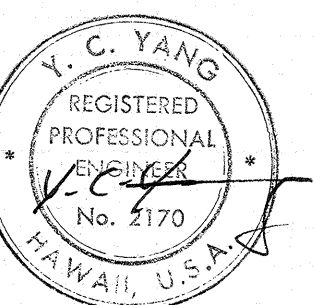
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F.A.I. PROJECT NO. I-HH (157): 24

SHEET NO.
SR-2
SHEET 2 OF
12 SHEETS

PROJECT NO. DESIGN DRAWN BY: B. BANSUELO CHECKED



C.O. 114 S-3



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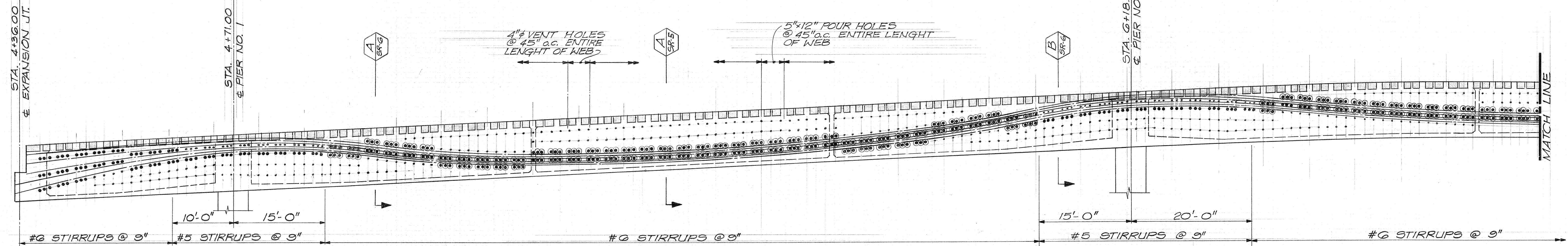
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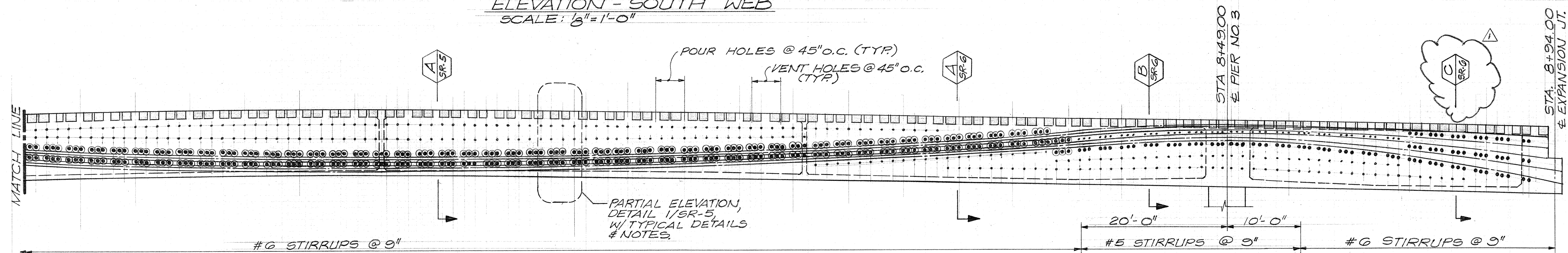
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REVIEW	5-82	D.C.

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PROJECT:	KAPIOLANI INTERCHANGE ON-RAMP REPAIRS F.A.I. PROJECT NO. I-HI-1 (157) : 24

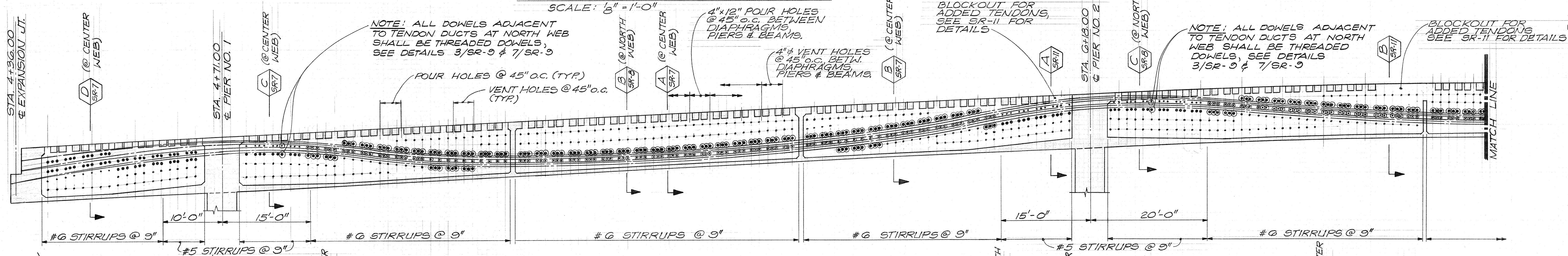
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SR-3
SHEET 3 OF
12 SHEETS



ELEVATION - SOUTH WEB
SCALE: 1/8" = 1'-0"



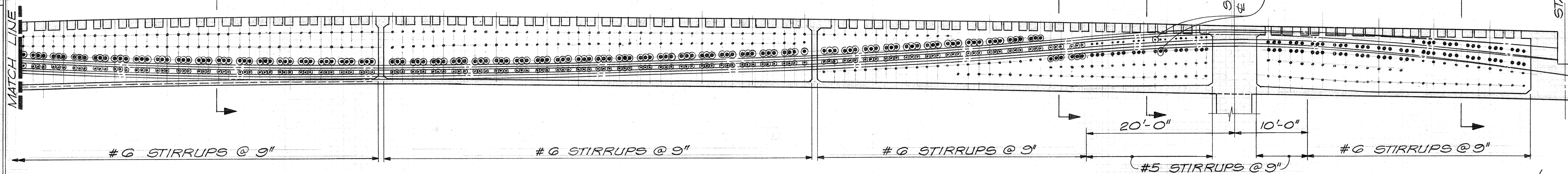
ELEVATION - SOUTH WEB (CONTINUED)
SCALE: 1/8" = 1'-0"



ELEVATION - NORTH & CENTER WEB
(CENTER WEB SHOWN; NORTH WEB SIMILAR EXCEPT AS NOTED)
SCALE: 1/8" = 1'-0"

LEGEND

- POST-TENSIONING DUCT (EXISTING)
- #6 EMBEDDED DOWELS (REFER TO DETAIL 4/SR-9)
- #5 EMBEDDED DOWELS (REFER TO DETAILS 2/SR-9 & 2A/SR-9)
- #6 THREADED DOWELS (REFER TO DETAIL 1/SR-9 FOR CENTER & SOUTH WEB; DETAIL 3/SR-9 FOR NORTH WEB)
- #5 THREADED DOWELS (REFER TO DETAIL 5/SR-9 FOR CENTER & SOUTH WEB; DETAIL 7/SR-9 FOR NORTH WEB)



ELEVATION - NORTH & CENTER WEB (CONTINUED)
(CENTER WEB SHOWN; NORTH WEB SIMILAR EXCEPT AS NOTED)
SCALE: 1/8" = 1'-0"

DOWEL CONFIGURATION SHOWN FOR CENTER WEB - FOR CONFIGURATION AT NORTH WEB SEE PARTIAL ELEVATION C/SR-10.

C.O. 114 S-4



7-23-82
7-2-82

NO. REVISION DATE



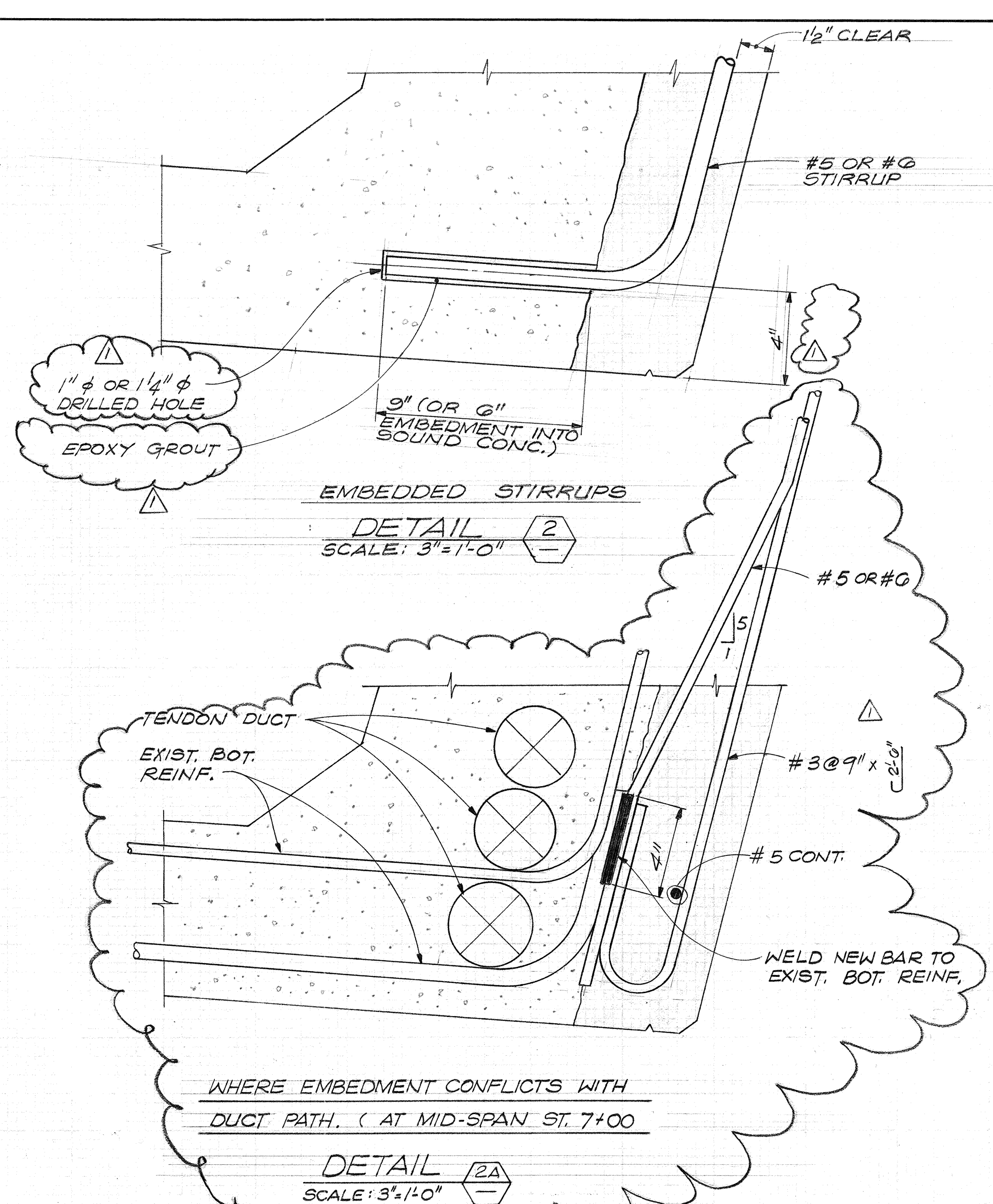
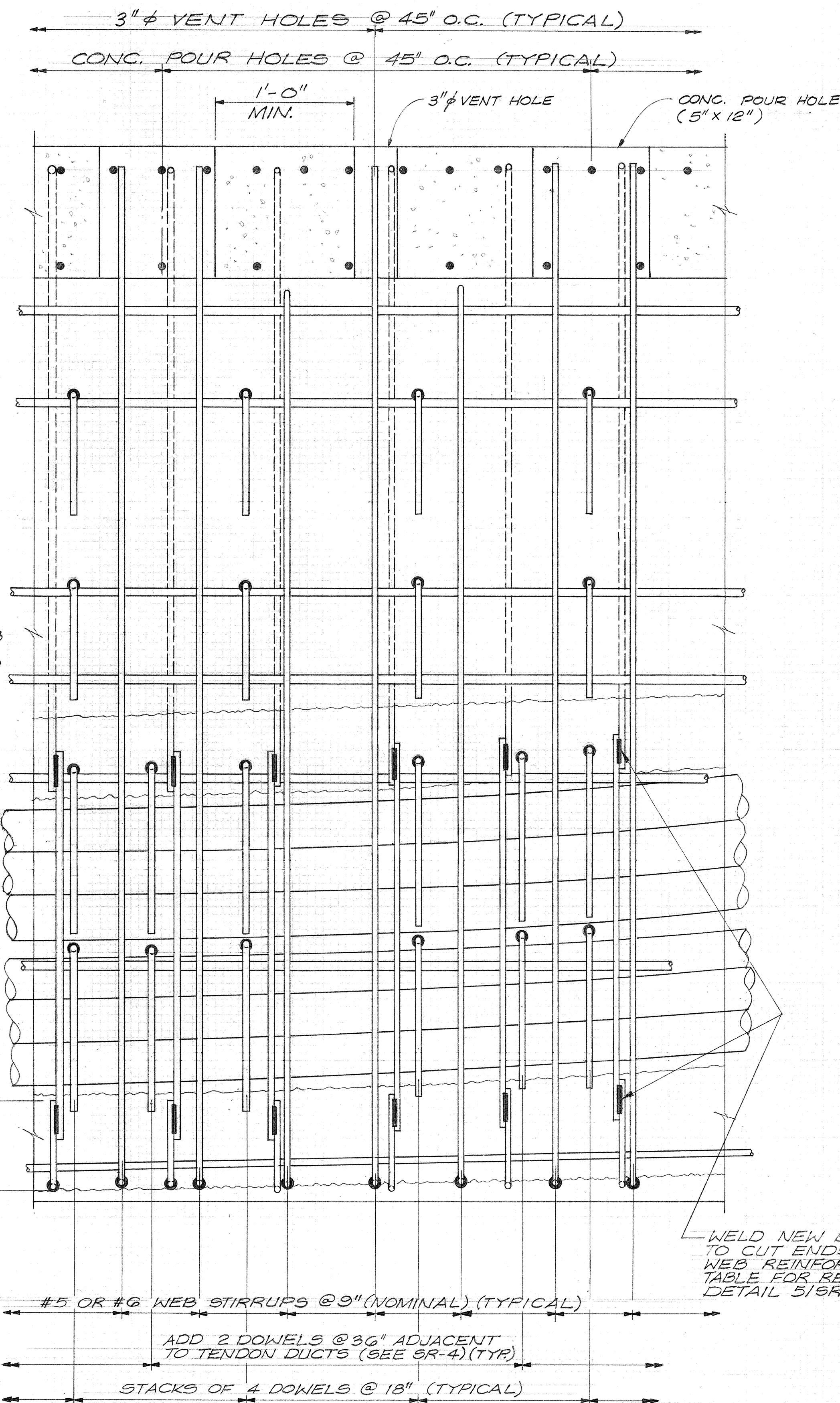
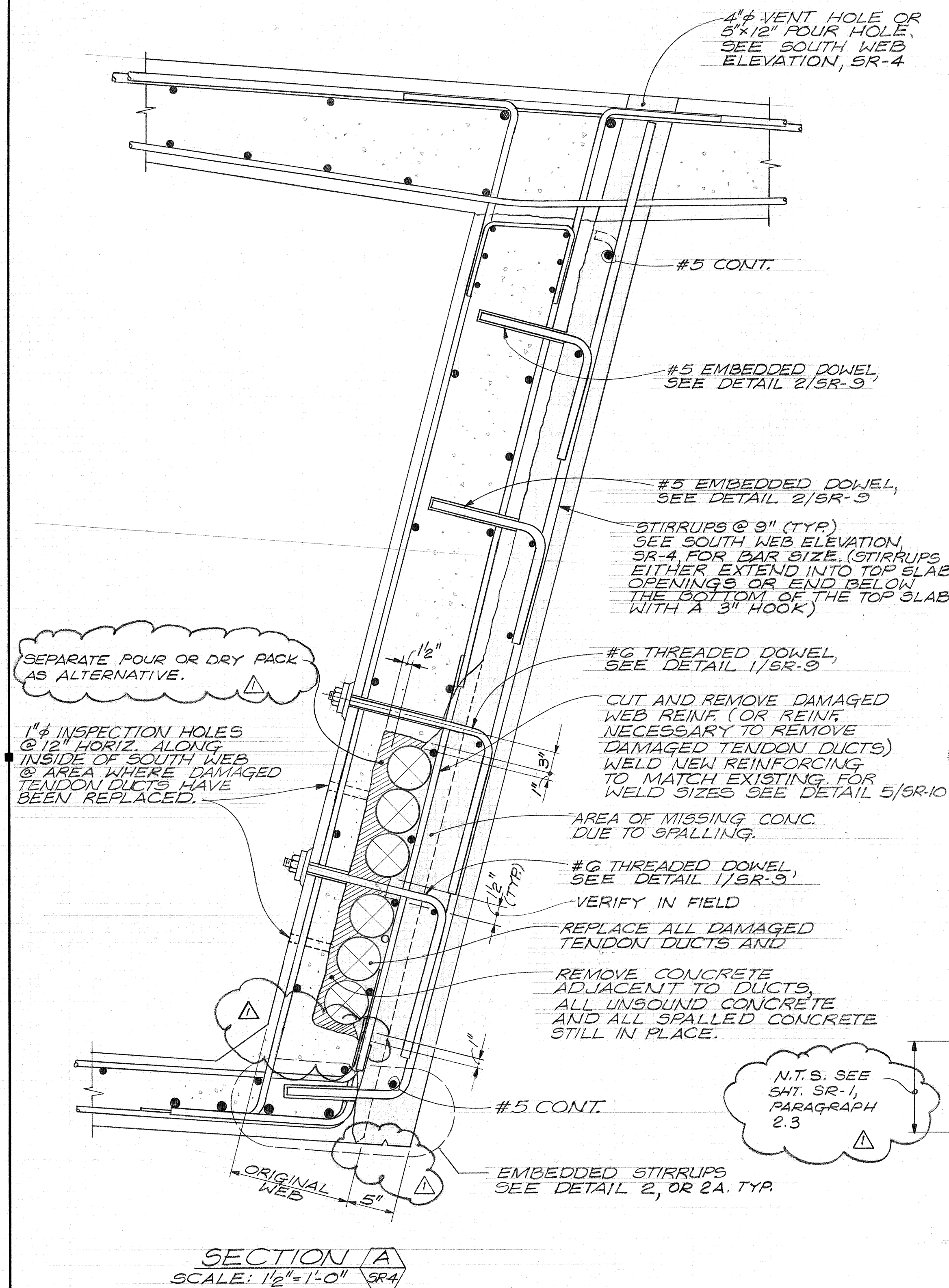
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REVIEW	5-82	DC

SHEET TITLE:
WEB ELEVATIONS - REPAIRS

PROJECT:
KAPIOLANI INTERCHANGE ON-RAMP REPAIRS
F.A.I. PROJECT NO. I-HI-1 (157) : 24

SHEET NO.
SR-4
SHEET 4 OF
12 SHEETS



ELEVATION - PORTION OF SOUTH WEB

DETAIL 1
SCALE: 1/2"=1'-0" SR-5

C.O. 114 S-5

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NO. REVISION DATE

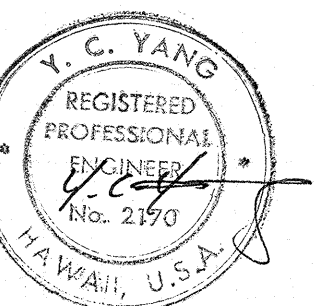
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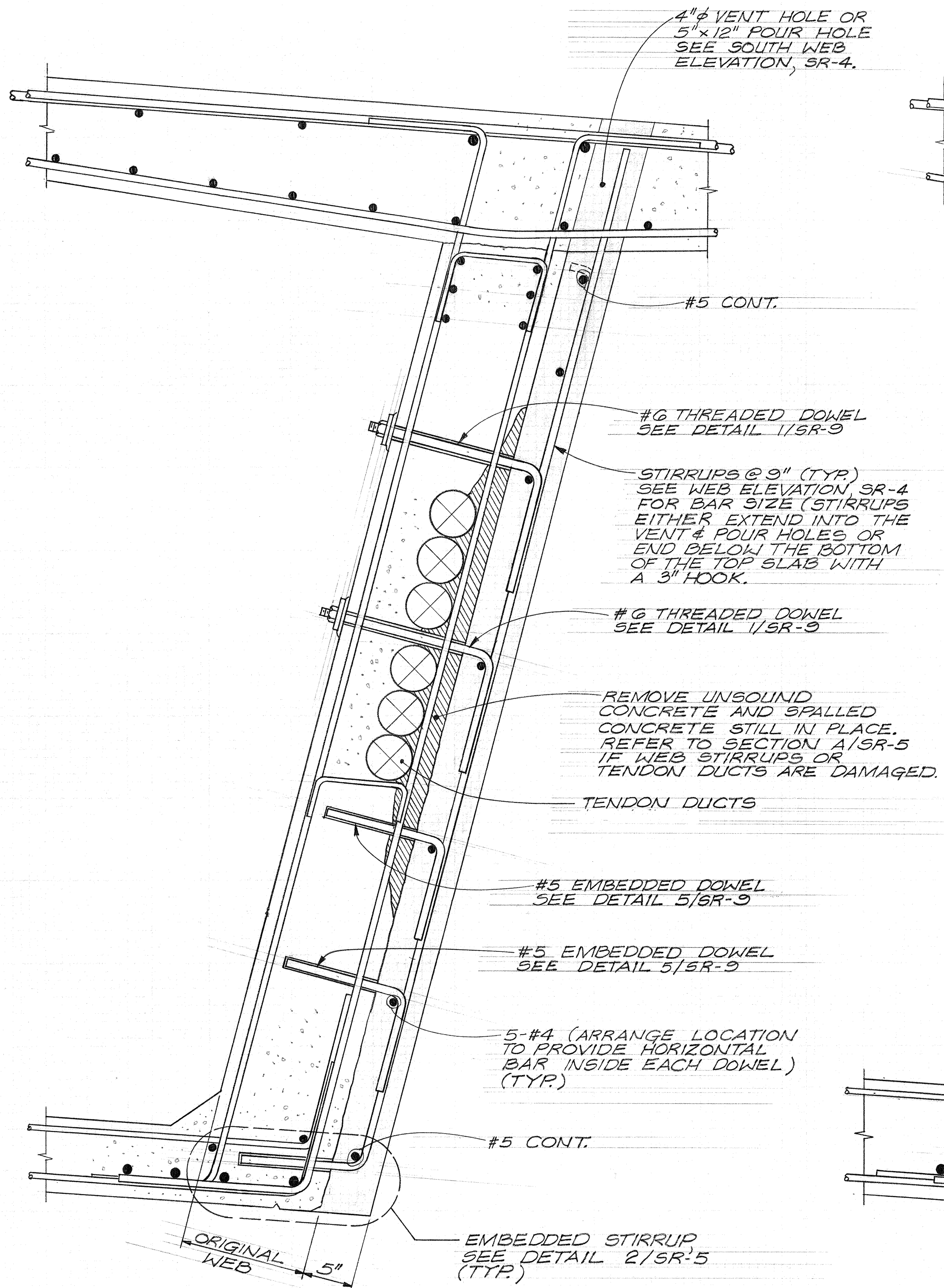
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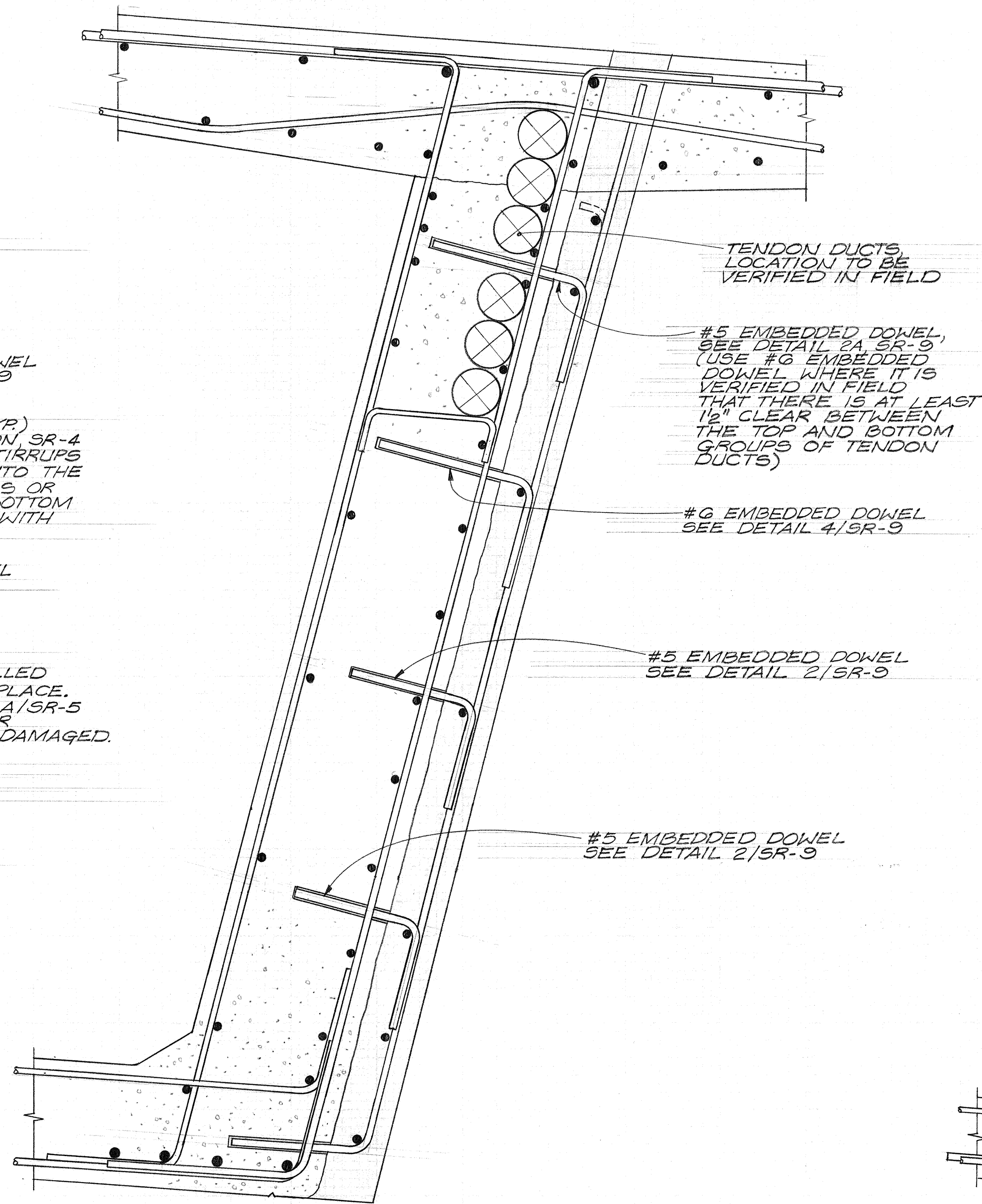
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PROJECT:	KAPIOLANI INTERCHANGE ON-RAMP REPAIRS F.A.I. PROJECT NO. I-HI-1 (157) : 24

SHEET NO.
SR-5
SHEET 5 OF 12 SHEETS

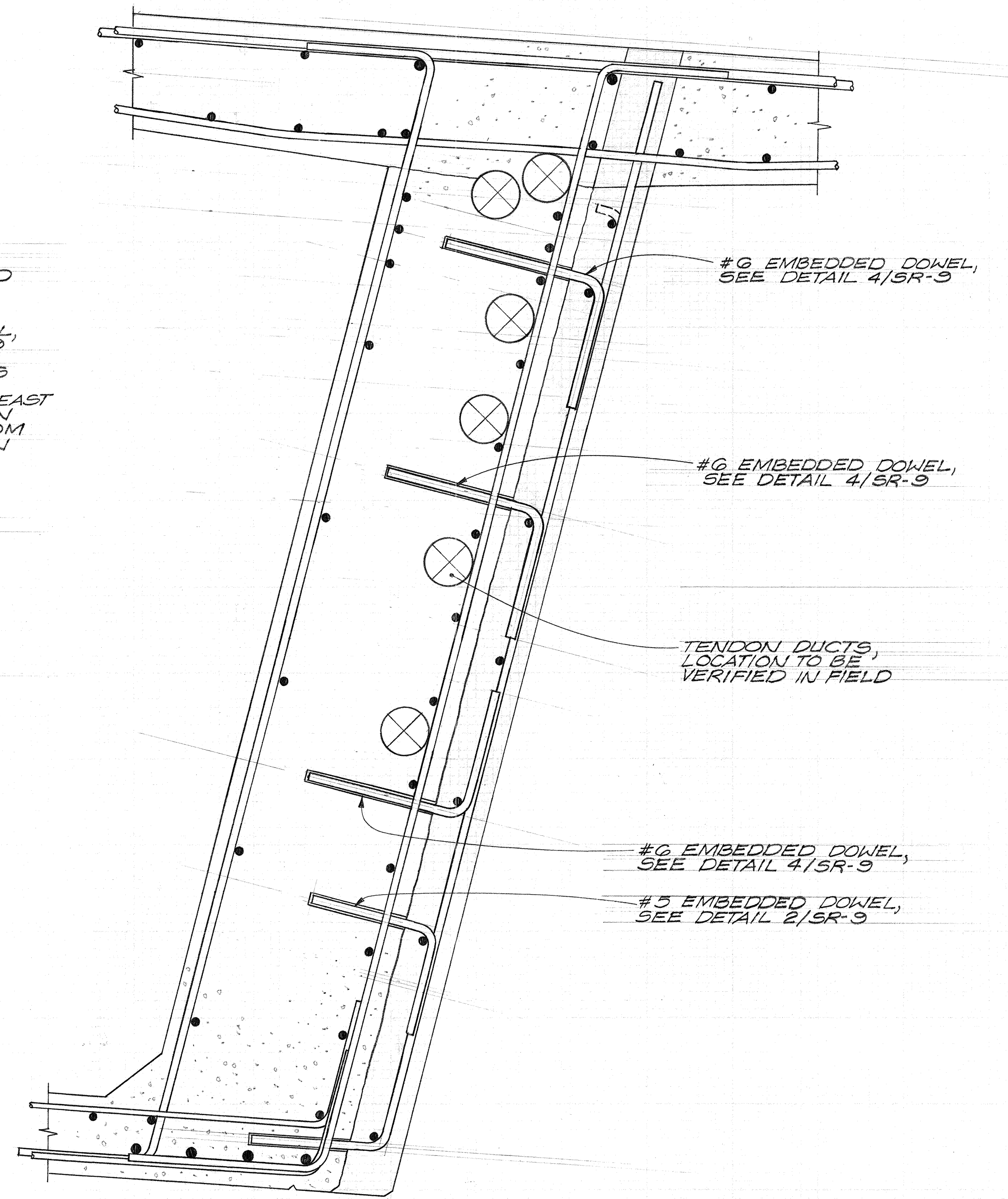




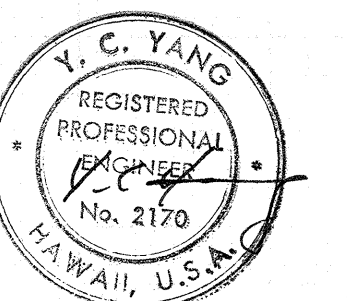
SECTION A
SCALE: 1 1/2" = 1'-0"



SECTION B
SCALE: 1 1/2" = 1'-0"



SECTION C
SCALE: 1 1/2" = 1'-0"



C.O. 114 S-6

7-2-82
NO. REVISION DATE



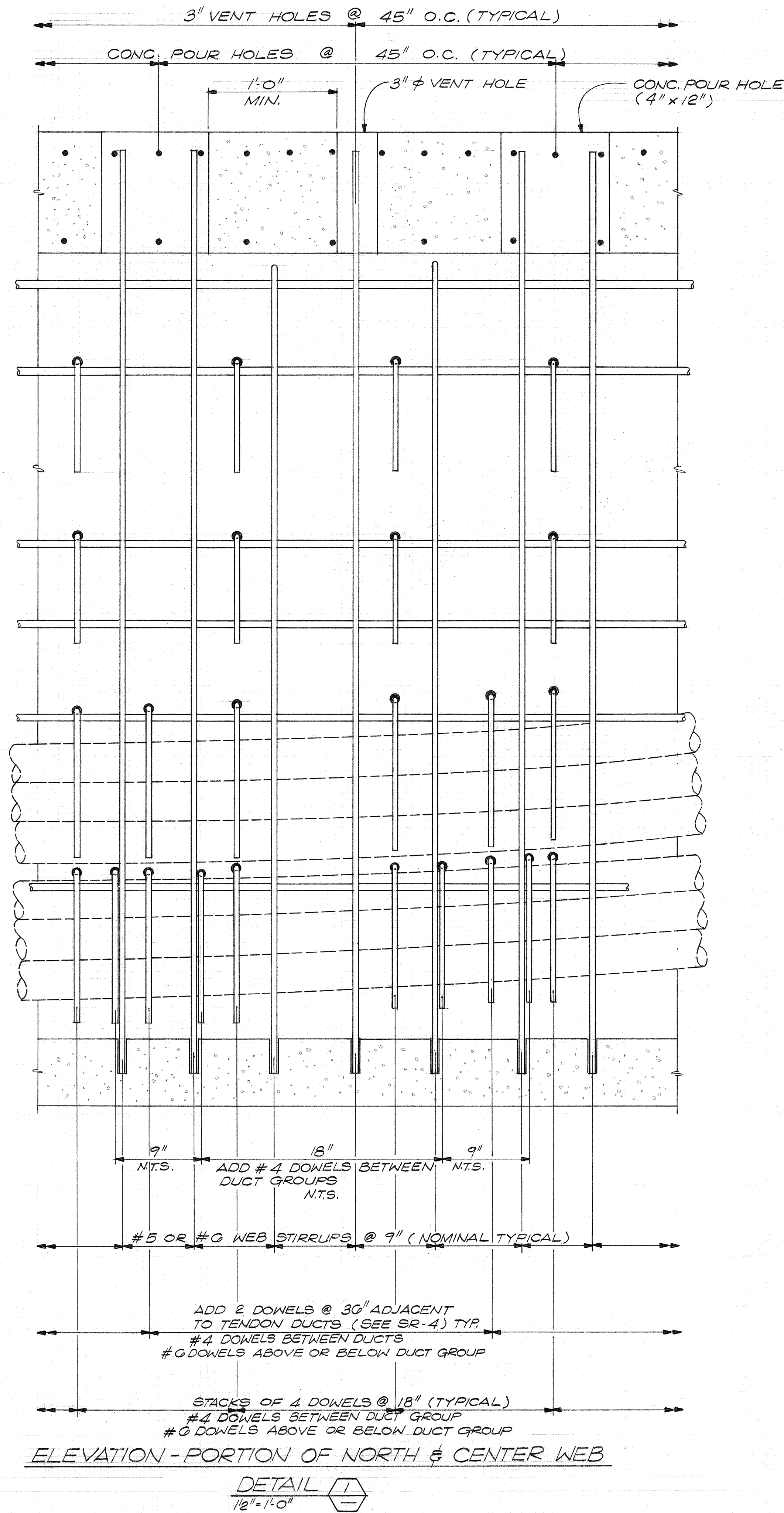
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REVIEW	5-82	D.C.

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PROJECT:	KAPIOLANI INTERCHANGE ON-RAMP REPAIRS F.A.I. PROJECT NO. I-HH(157) : 24

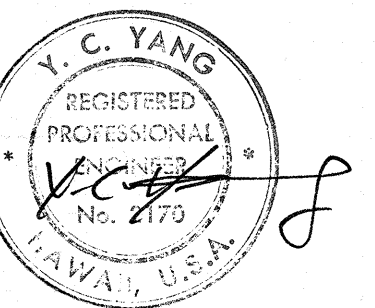
SHEET NO.
SR-6
SHEET 6 OF
12 SHEETS

PROJECT NO. DESIGN DRAWN BY: BANSUELO CHECKED



ELEVATION - PORTION OF NORTH & CENTER WEB

DETAIL 1
1/2" = 1'-0"



C.O. 114 S-7

NO.	REVISION	DATE

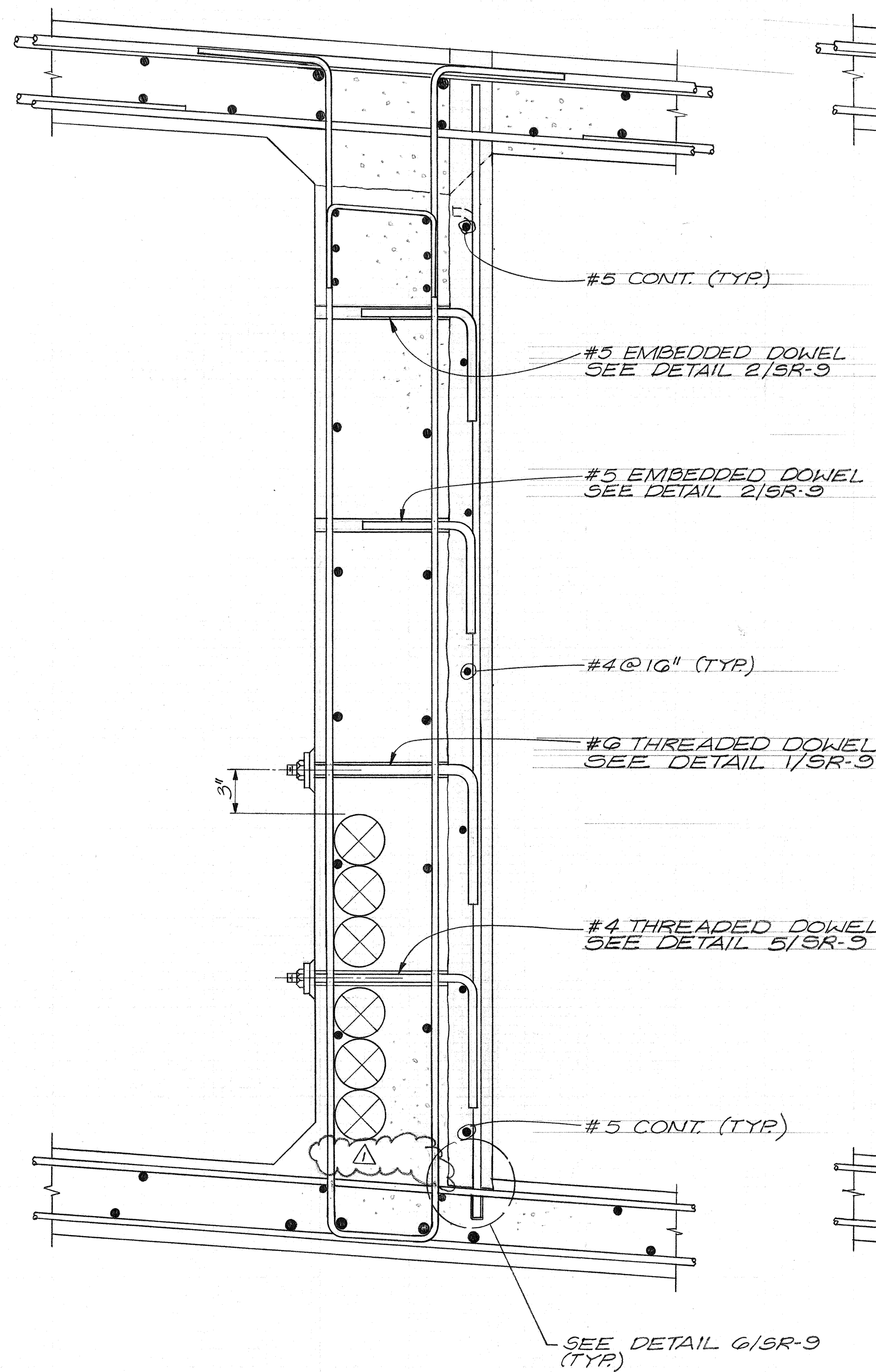


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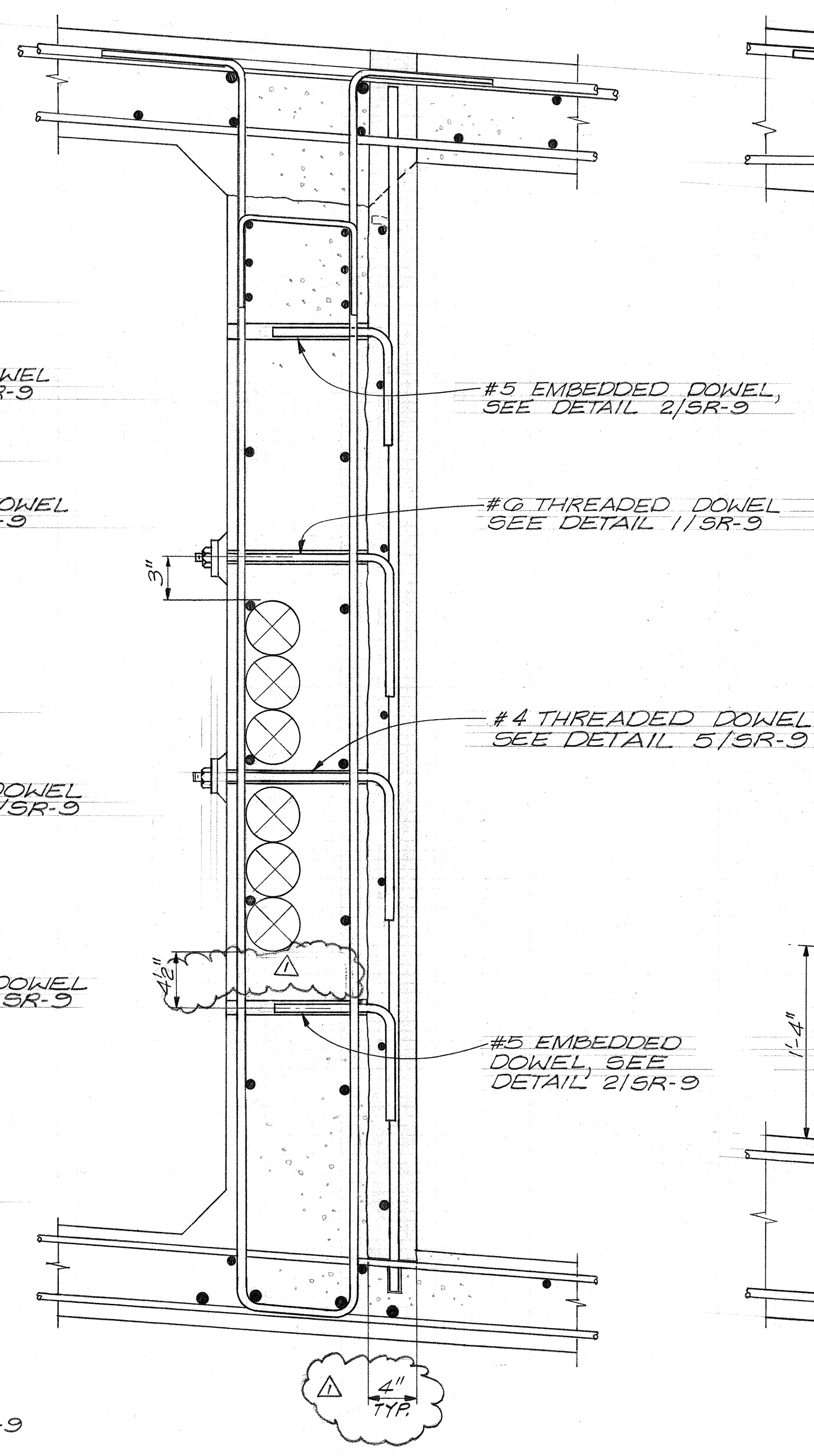
Issued For	Date	By

SHEET TITLE: NORTH & CENTER WEB ELEVATION
PROJECT: KAPIOLANI INTERCHANGE ON-RAMP REPAIRS
F.A.I. PROJECT NO. I-HI-1 (157) : 24

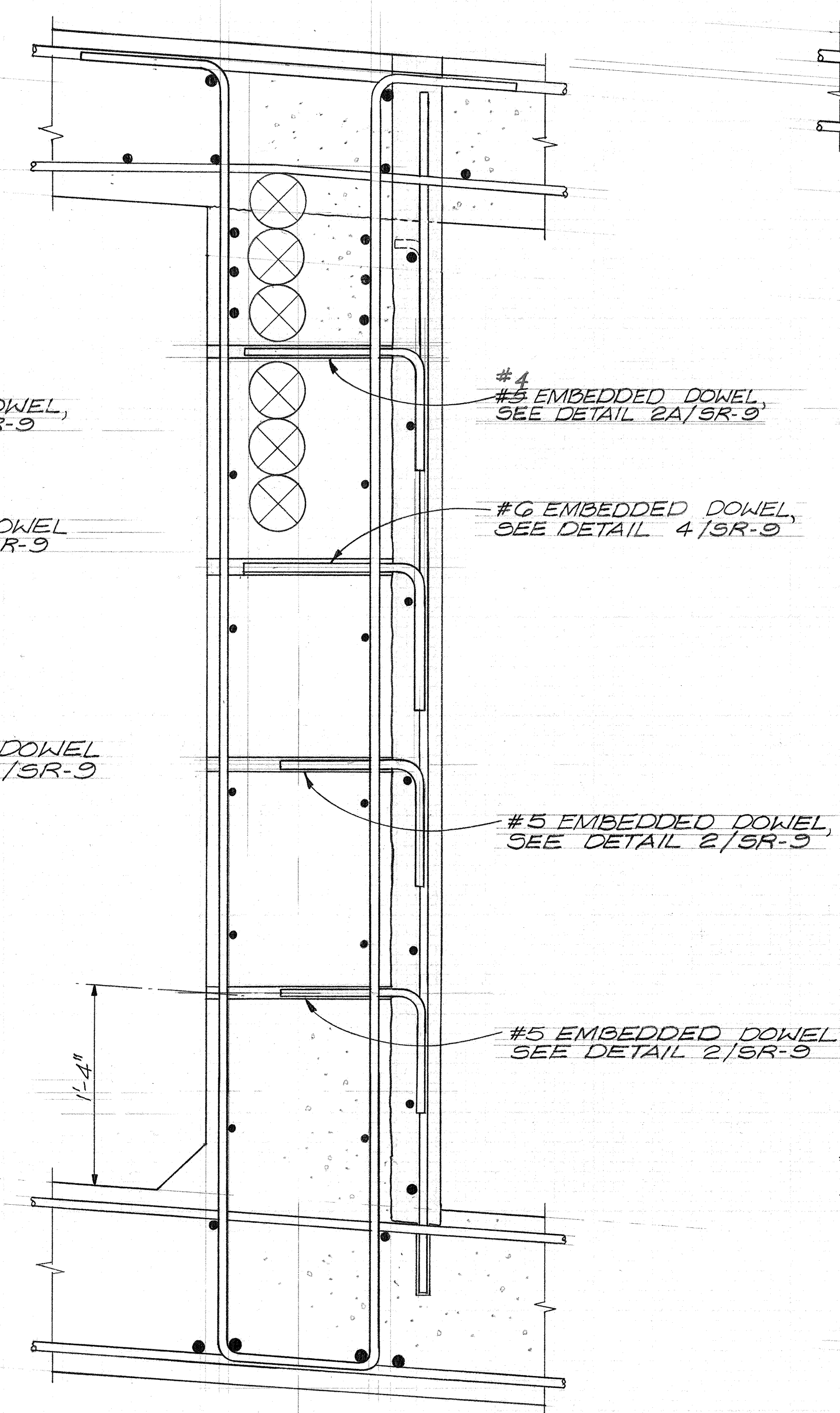
SHEET NO.
SR-6A
SHEET 6A OF
12 SHEETS



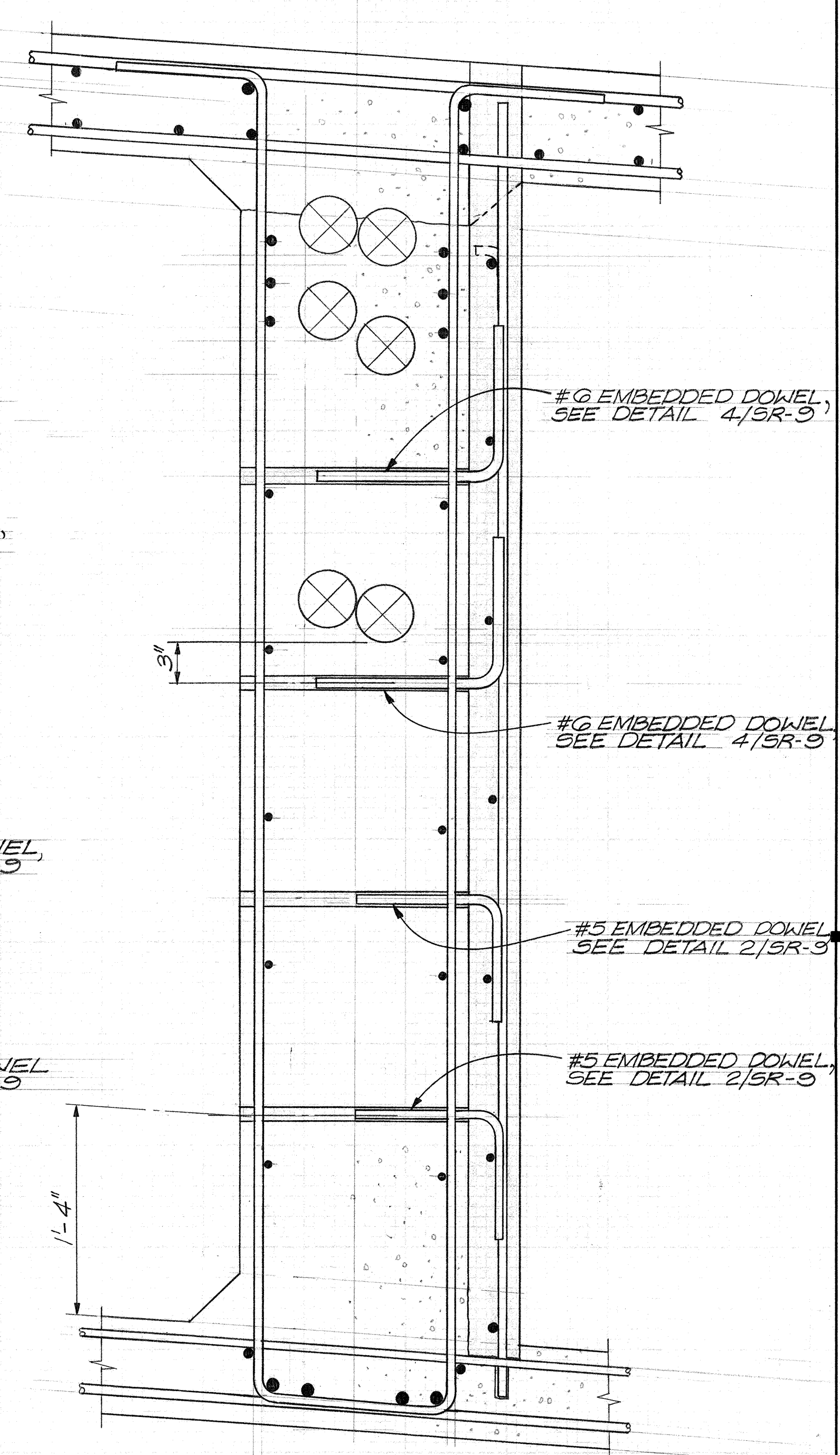
SECTION **A**
SCALE: 1/2" = 1'-0" **SR-4**



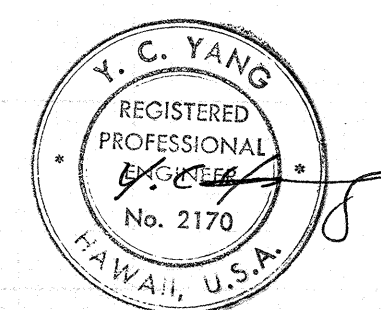
SECTION **B**
SCALE: 1/2" = 1'-0" **SR-4**



SECTION **C**
SCALE: 1/2" = 1'-0" **SR-4**



SECTION **D**
SCALE: 1/2" = 1'-0" **SR-4**



C.O. 114 S-8

NO.	REVISION	DATE



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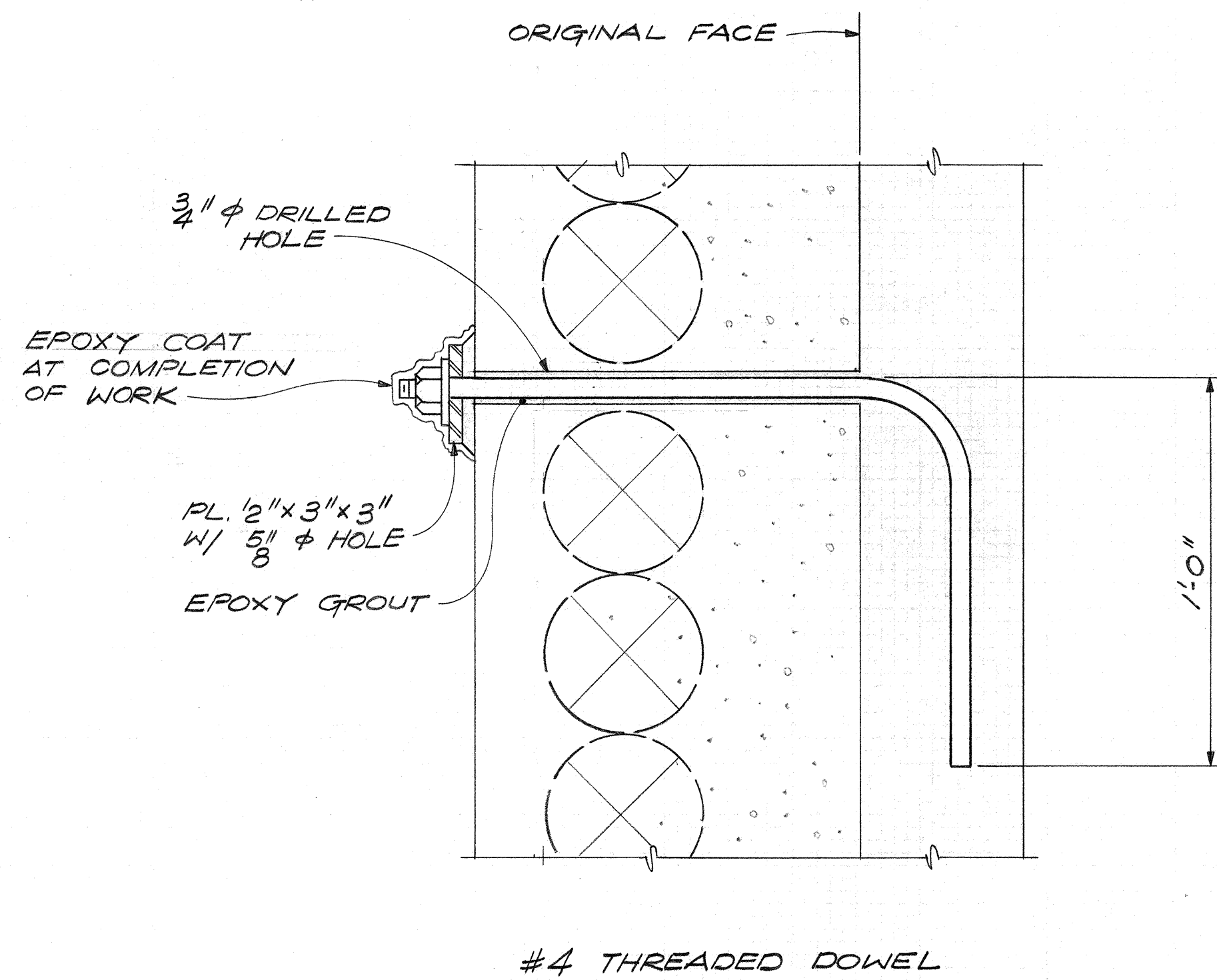
Issued For	Date	By
REVIEW	5-82	D.C.

SHEET TITLE:	CENTER WEB SECTIONS
PROJECT:	KAPIOLANI INTERCHANGE ON-RAMP REPAIRS F.A.I. PROJECT NO. I-HI-1(157) : 24

SHEET NO.	SR-7
SHEET 7 OF 12 SHEETS	

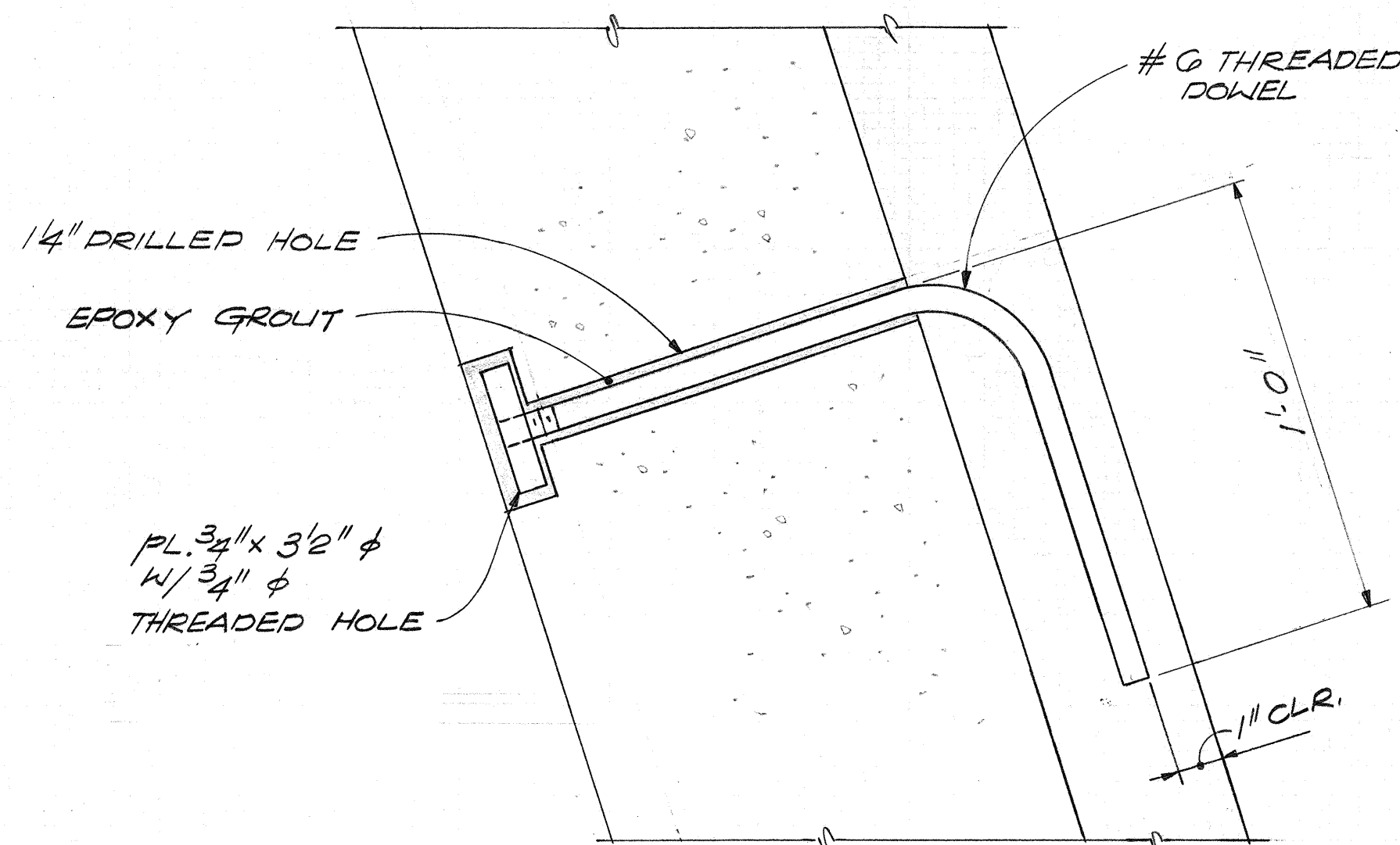
PROJECT N°:

SHEET N°
SR-8
SHEET 8 OF
12 SHEETS



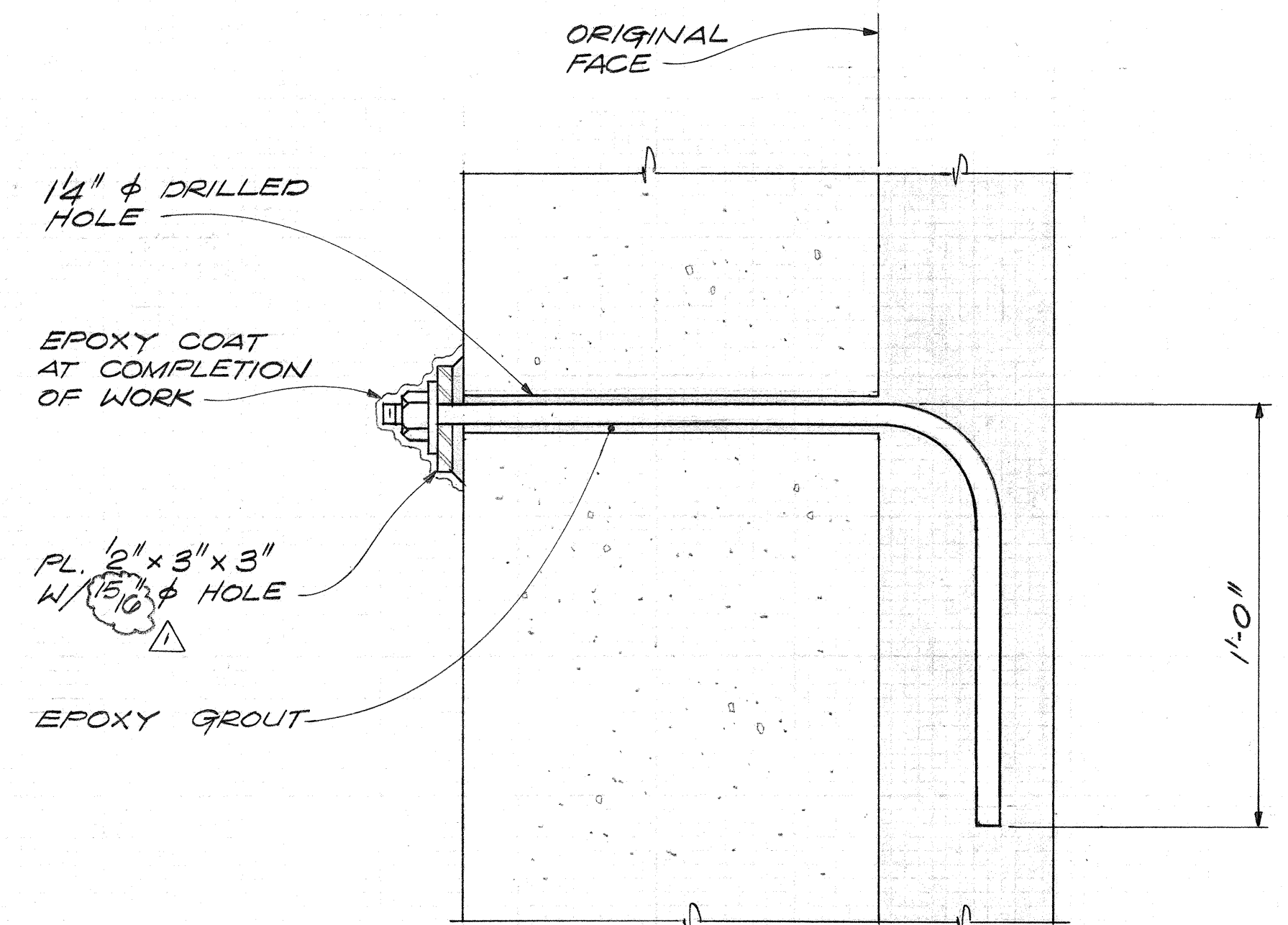
#4 THREADED DOWEL

DETAIL 5
SCALE: 3"=1'-0" SR-7



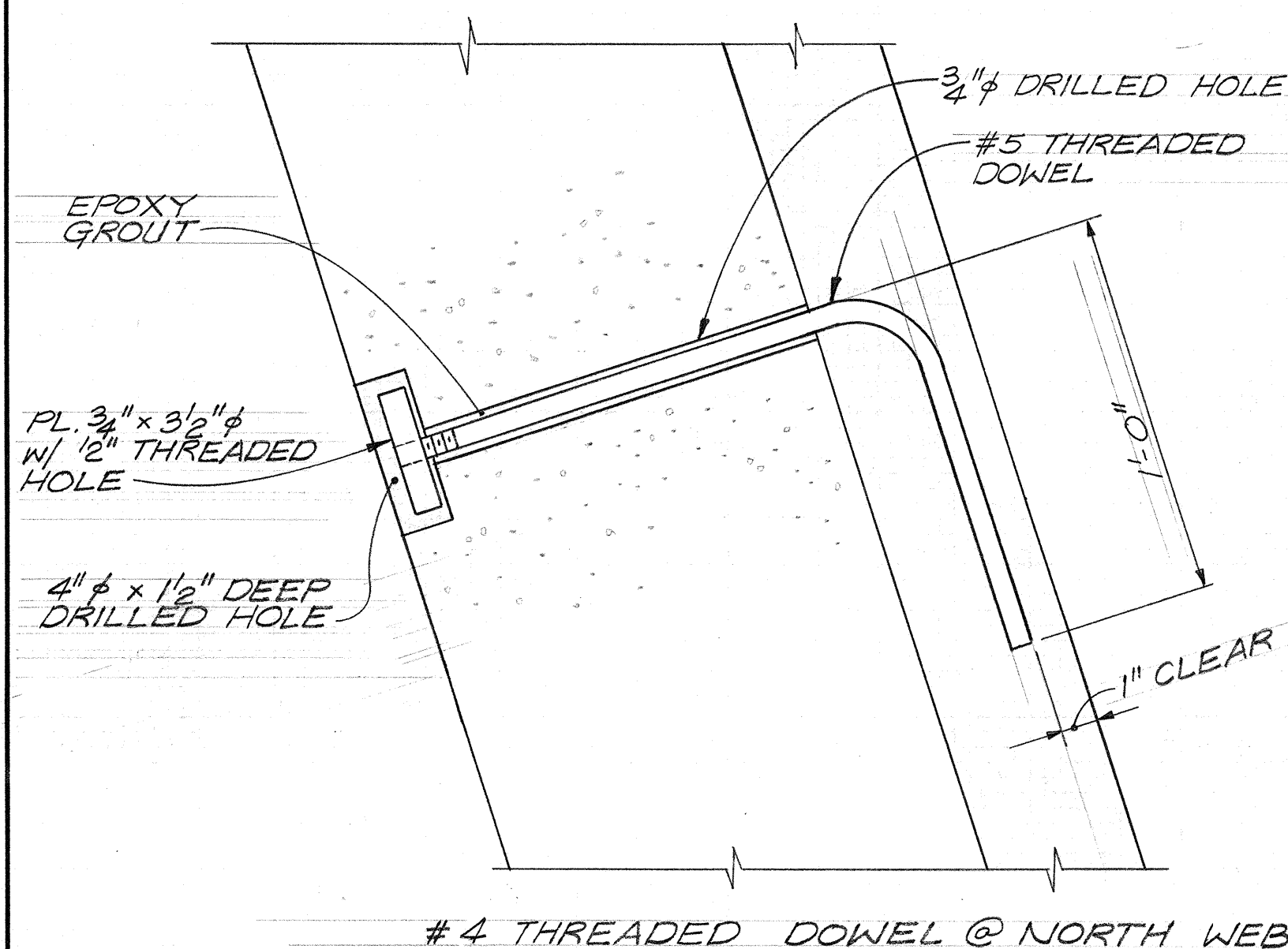
#6 THREADED DOWEL
@ NORTH WEB.

DETAIL 3
SCALE: 3"=1'-0" SR-8



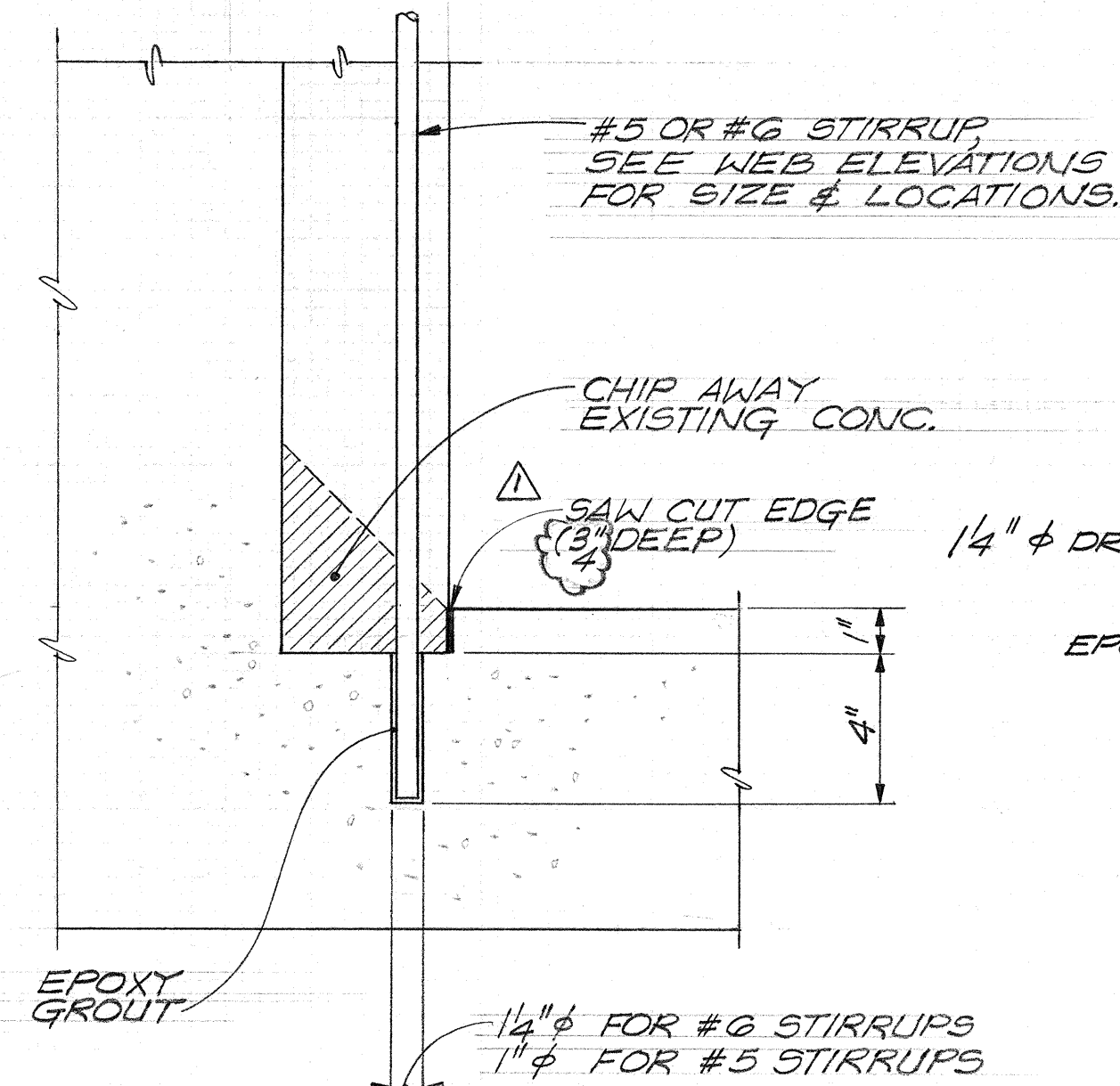
#6 THREADED DOWEL

DETAIL 1
SCALE: 3"=1'-0" SR-5

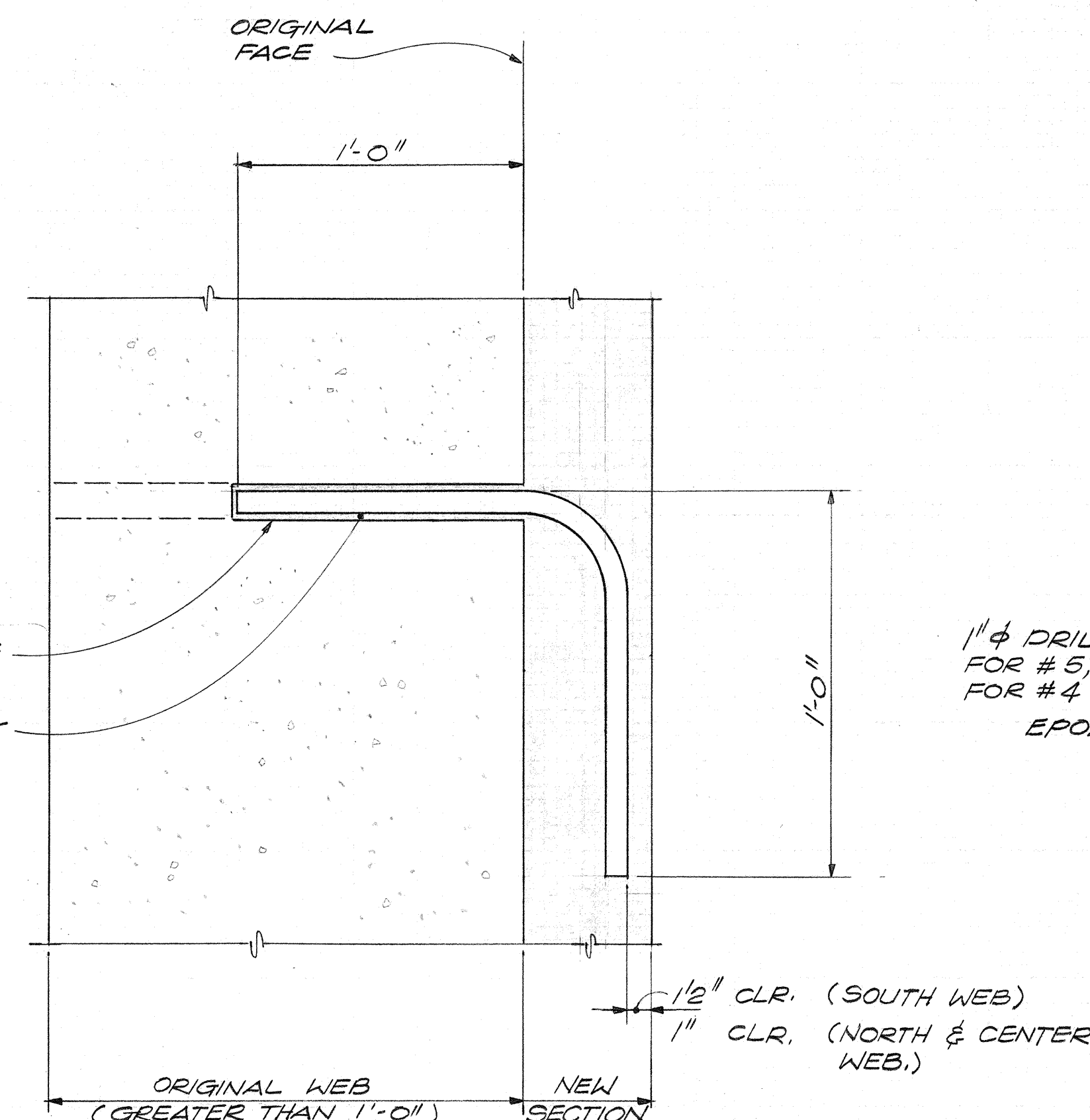


#4 THREADED DOWEL @ NORTH WEB

DETAIL 7
SCALE: 3"=1'-0" SR-3

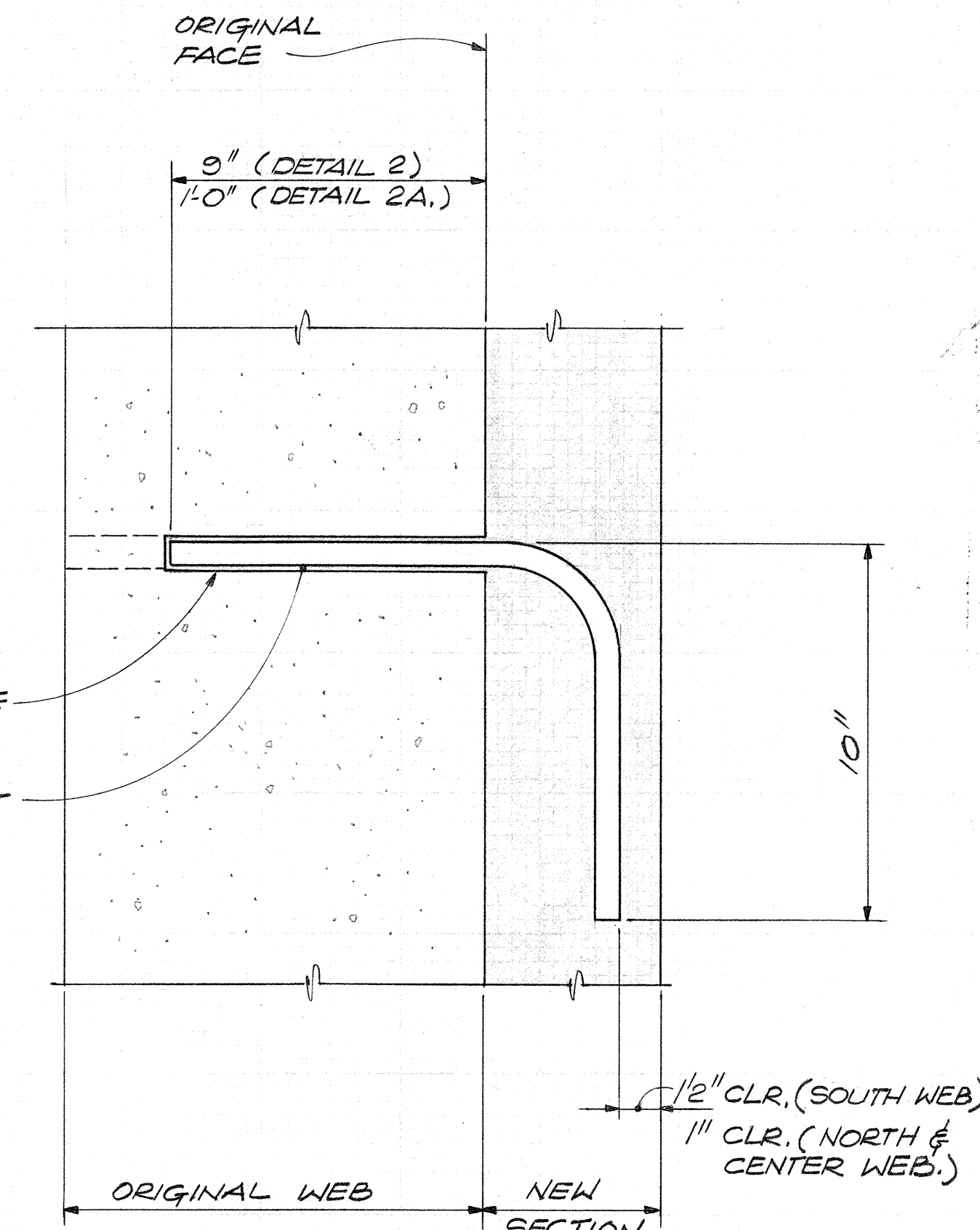


DETAIL 6
SCALE: 3"=1'-0" SR
TYPICAL KEY AND STIRRUP
EMBEDMENT AT BOTTOM SLAB,
@ NORTH AND CENTER WEBS.



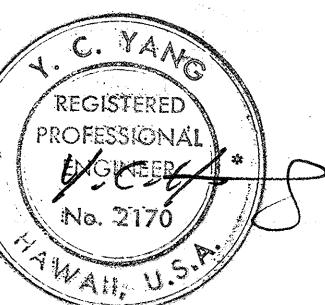
#6 EMBEDDED DOWEL

DETAIL 4
SCALE: 3"=1'-0" SR-6



#5 EMBEDDED DOWEL

DETAIL 2A
SCALE: 3"=1'-0" SR-5 SR-7



C.O. 114 S-10

7-2-82
NO. REVISION DATE

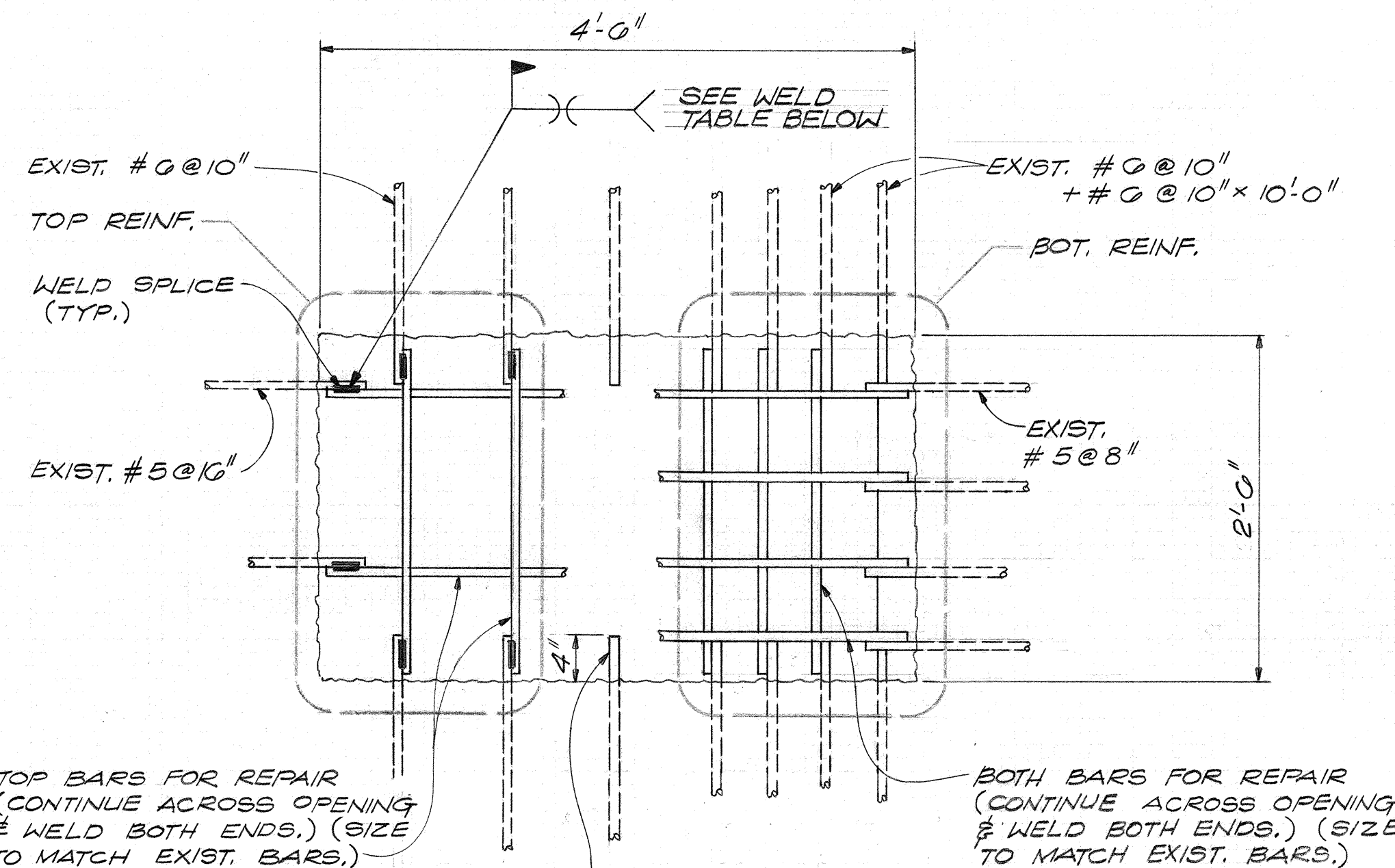


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Issued For	Date	By
REVIEW	5-82	D.C.

SHEET TITLE: DOWEL DETAILS
PROJECT: KAPIOLAN INTERCHANGE ON-RAMP REPAIRS
F.A.I. PROJECTS NO. I-HI-1 (157) v24

SHEET NO.
SR-9
SHEET 9 OF
12 SHEETS



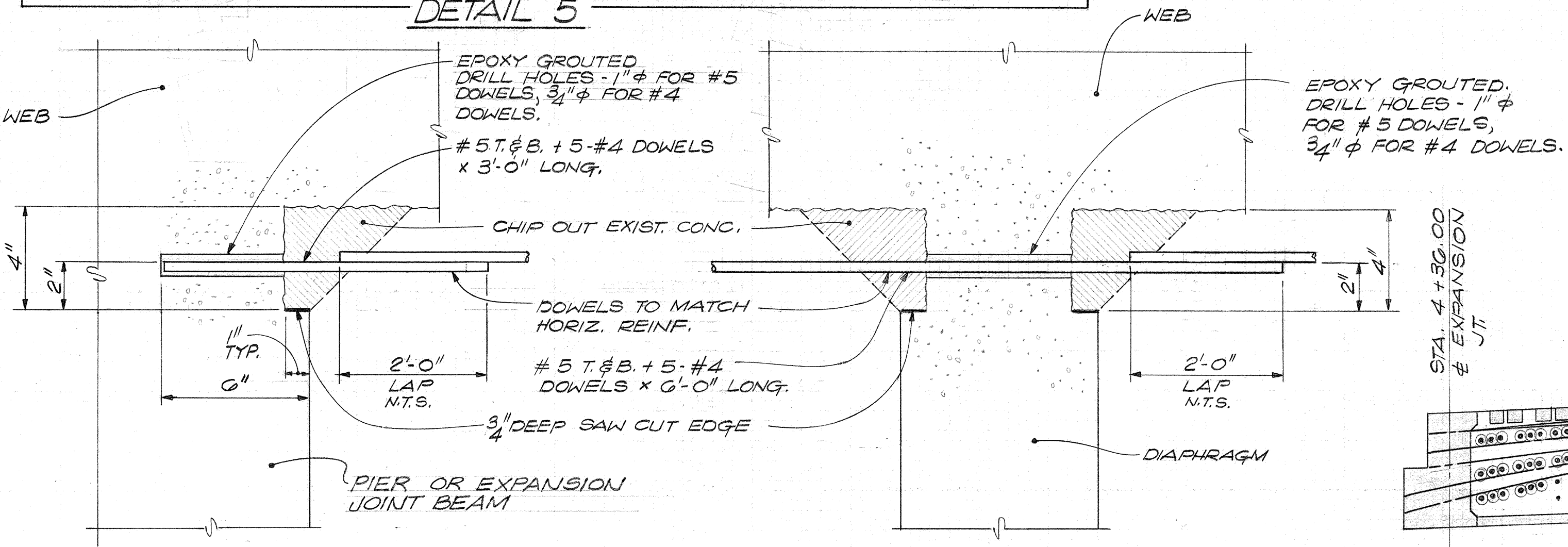
TEMPORARY ACCESS OPENING
TYPICAL TEMPORARY ACCESS OPENING IN TOP DECK

DETAIL 1
SCALE: 1"=1'-0" SR-2

BAR SIZE	A. DBL. GROOVE THROAT	B. SGL. GROOVE THROAT	LGTH.	LGTH.
#6	3/8"	1/2"	3/8"	2 1/2"
#5	1/4"	1/4"	1/4"	2"
#4	1/4"	1/4"	1/4"	1 3/4"

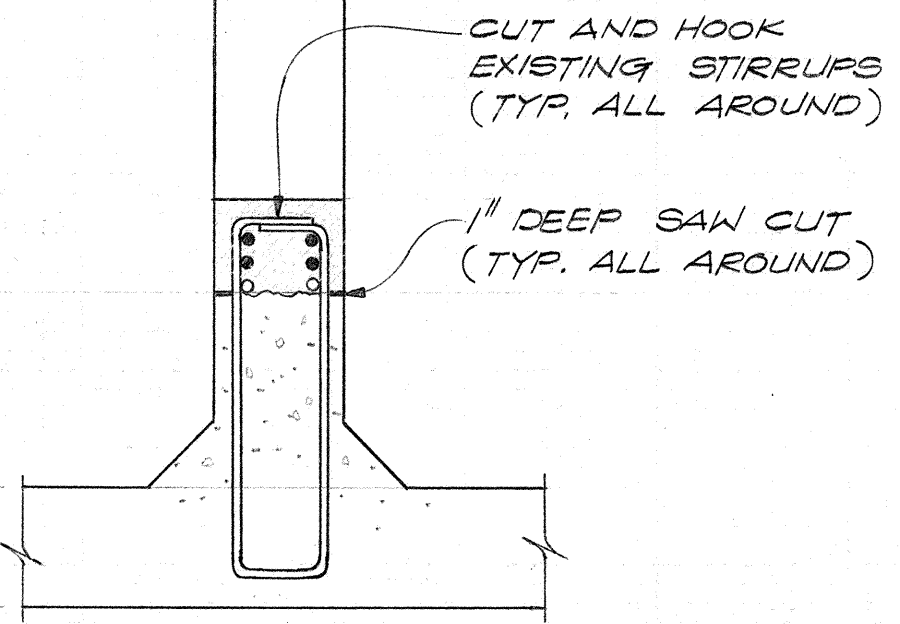
NOTE:
1. DO NOT WELD WITHIN 2 BAR DIAMETERS OF COLD BENT AREA.
2. USE DBL. GROOVE WELDS UNLESS ONE SIDE IS UNACCESSIBLE.
3. USE E70 ELECTRODES FOR GRADE 60 AND E70 ELECTRODES GRADE 40.
4. THE WELDING GROUND SHALL BE ATTACHED DIRECTLY TO THE STEEL BEING WELDED.

DETAIL 5

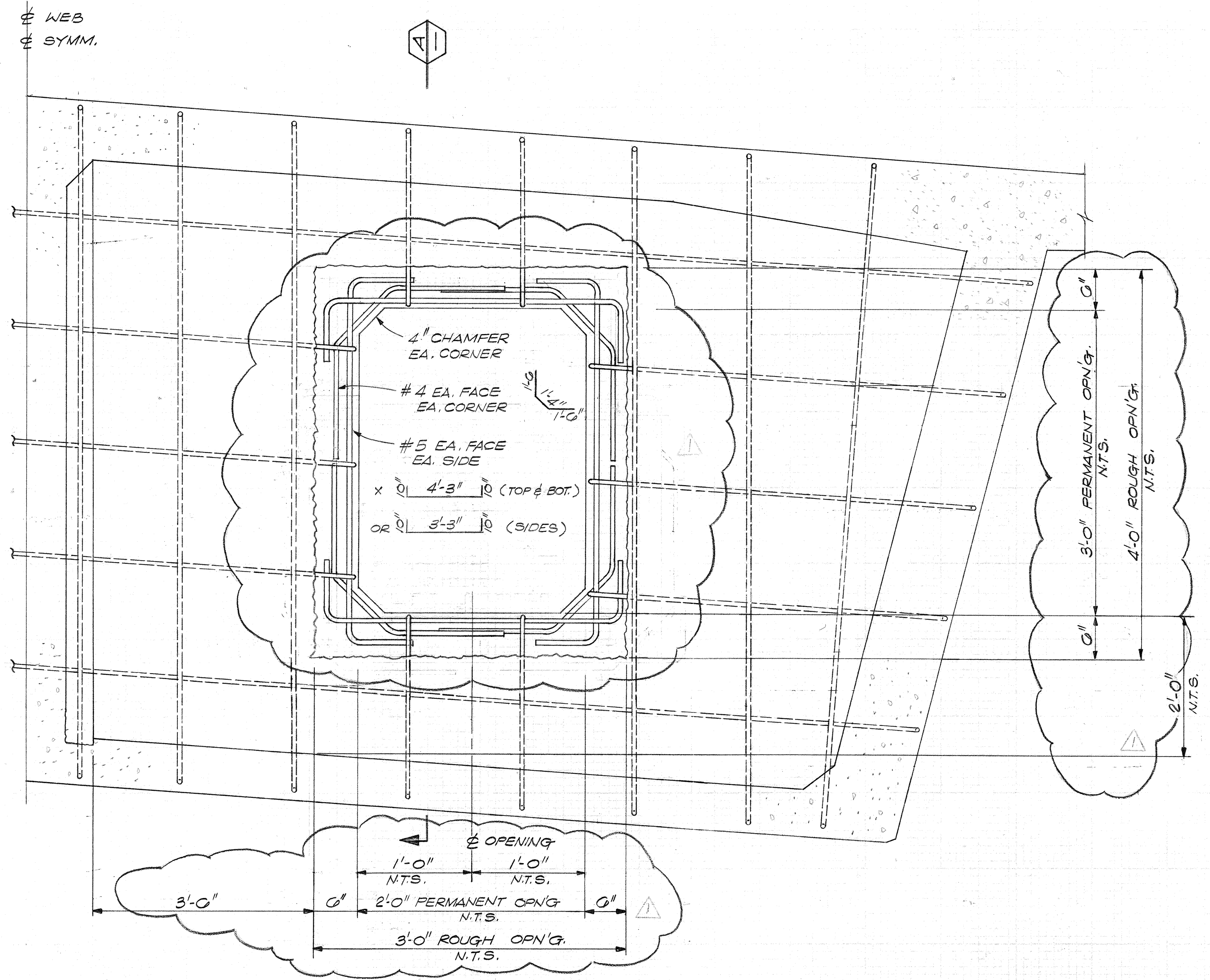


PLAN DETAIL 3
SCALE: 3/8"=1'-0" SR-3

PLAN DETAIL 4
SCALE: 3/8"=1'-0" SR-3

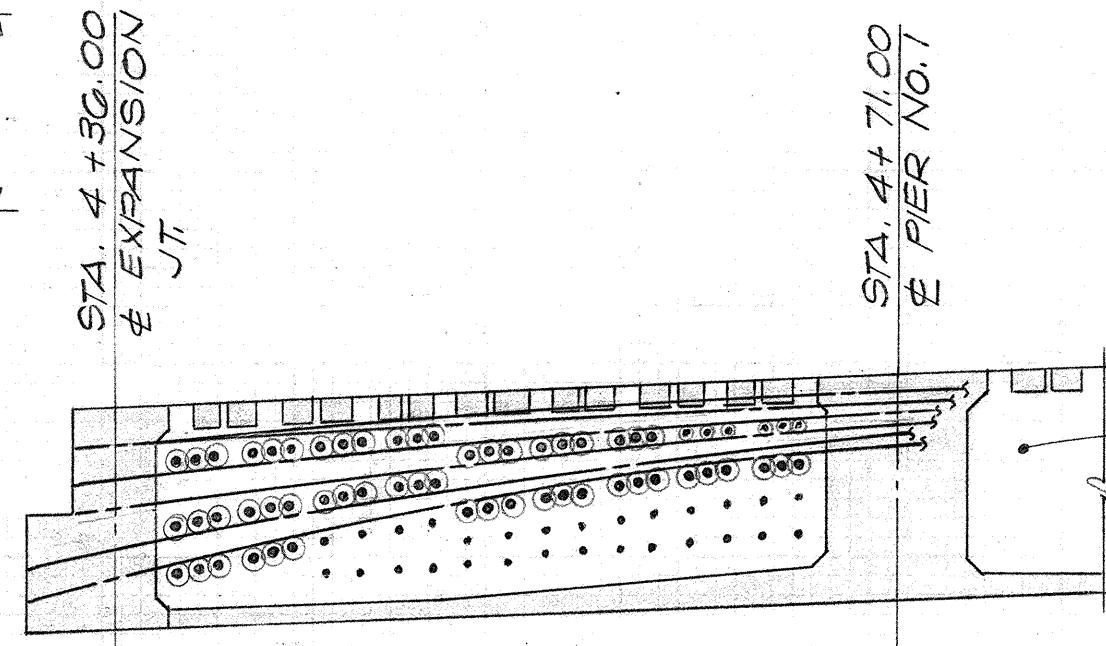


SECTION A
1"=1'-0"

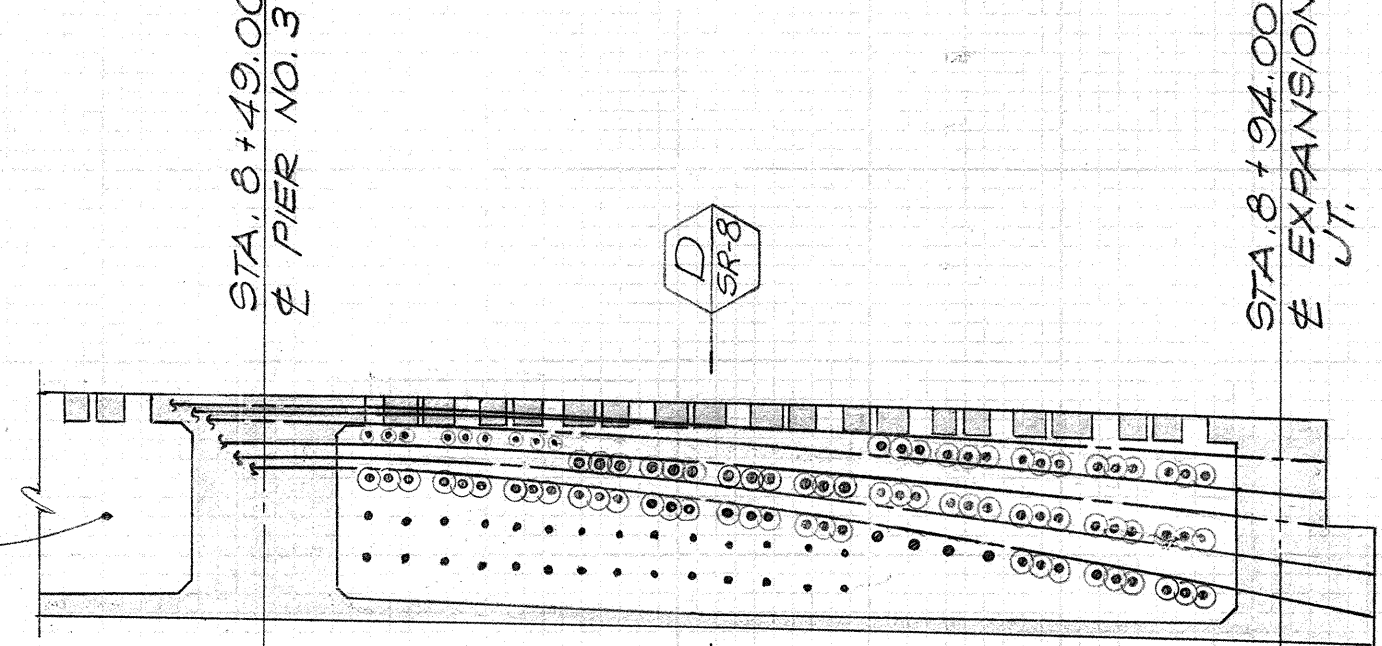


ADDED 4'-0" x 3'-0" PERMANENT DIAPHRAGM OPENING
TYPICAL EACH SIDE AT ALL 8" DIAPHRAGMS - (TOTAL 10 OPENINGS)
1"=1'-0"

DETAIL 2
SCALE: 1"=1'-0" SR-2

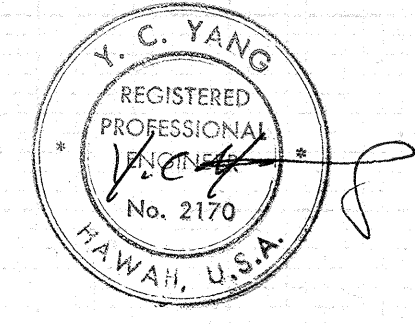


PARTIAL ELEVATION B - NORTH WEB
SCALE: 1/8"=1'-0"



PARTIAL ELEVATION C - NORTH WEB
SCALE: 1/8"=1'-0"

C.O. 114 S-II

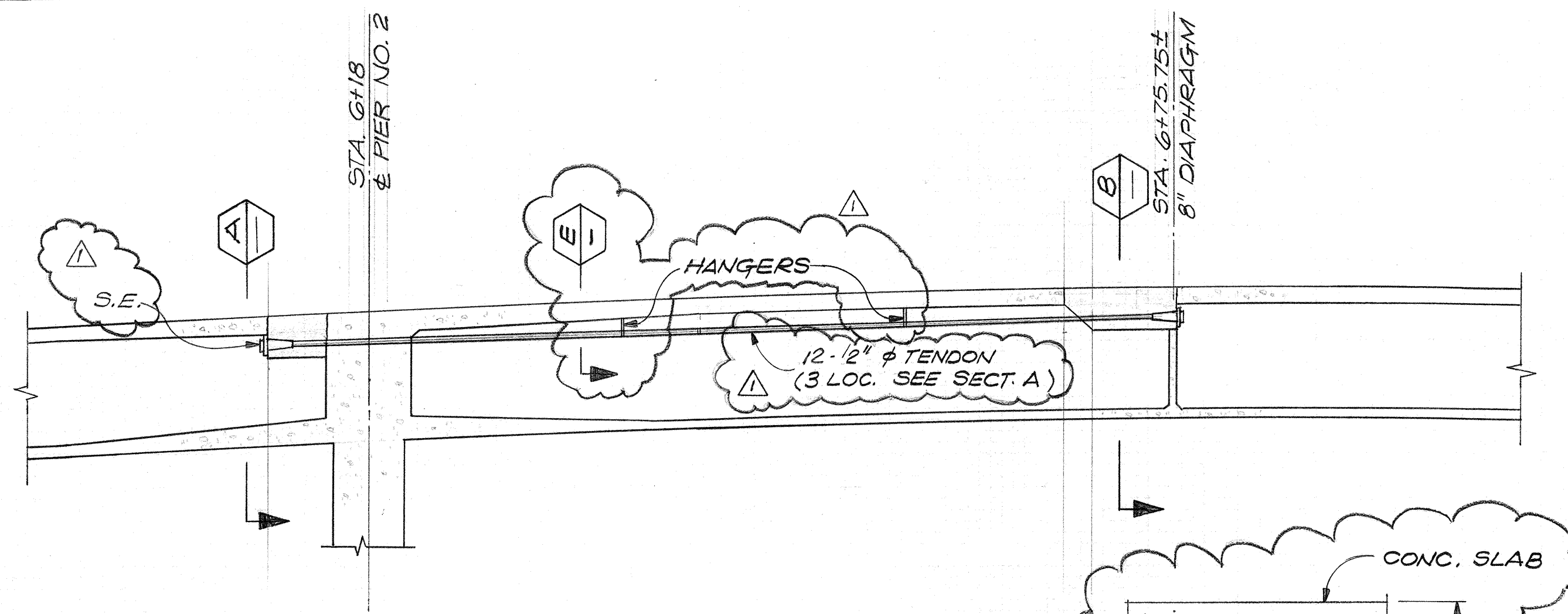


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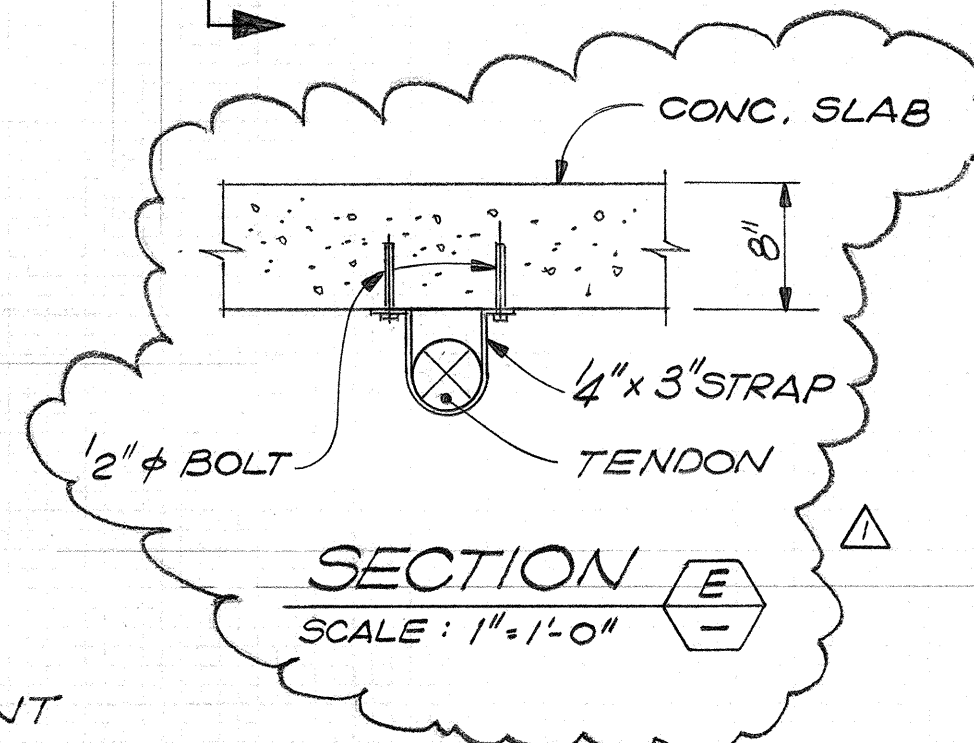
Issued For	Date	By
REVIEW	5-82	D.C.

SHEET TITLE:	
MISCELLANEOUS DETAILS	
PROJECT:	
KAPIOLANI INTERCHANGE ON-RAMP REPAIRS F.A.I. PROJECT NO. I-H-I (157): 24	

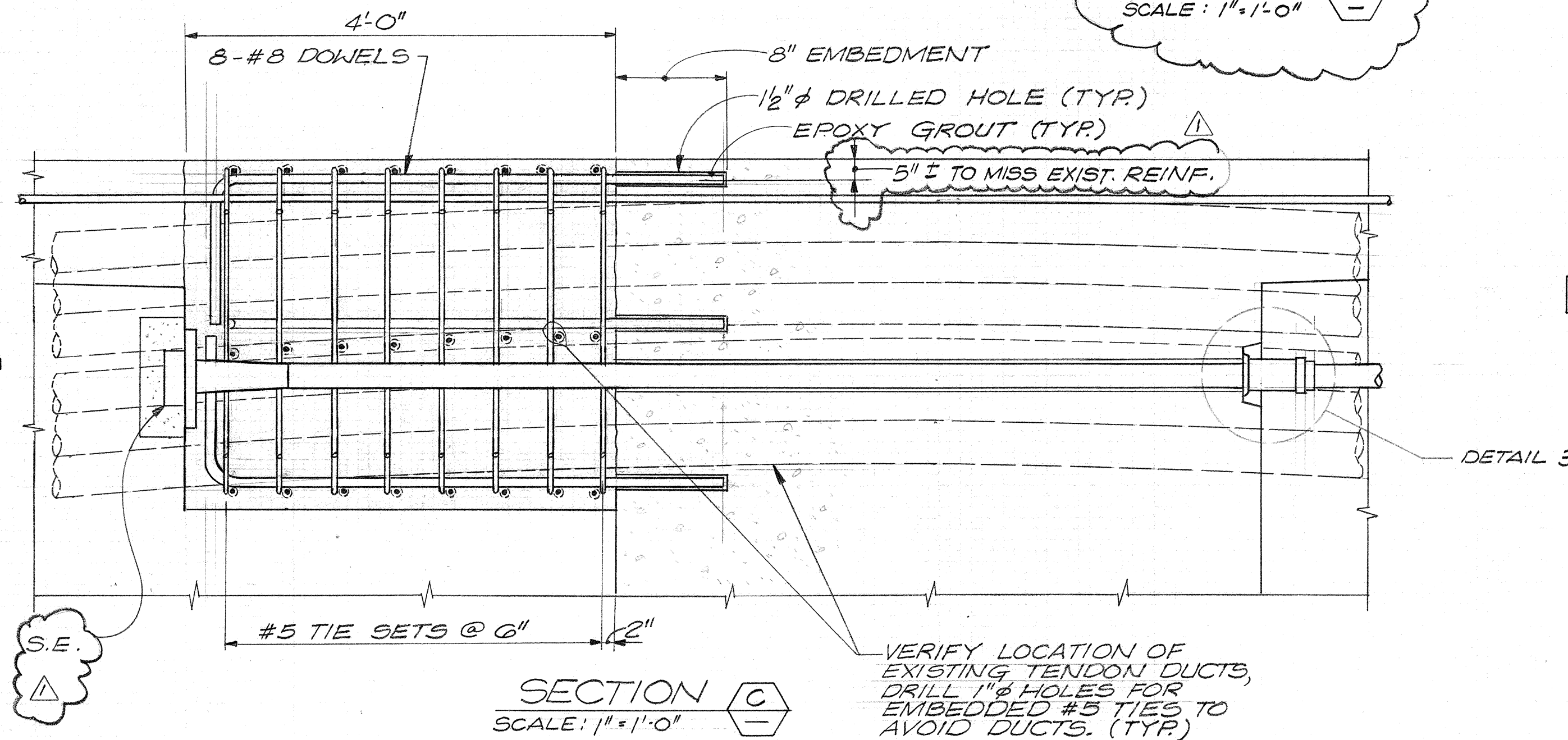
SHEET NO.	
SR-10	
SHEET 10 OF 12 SHEETS	



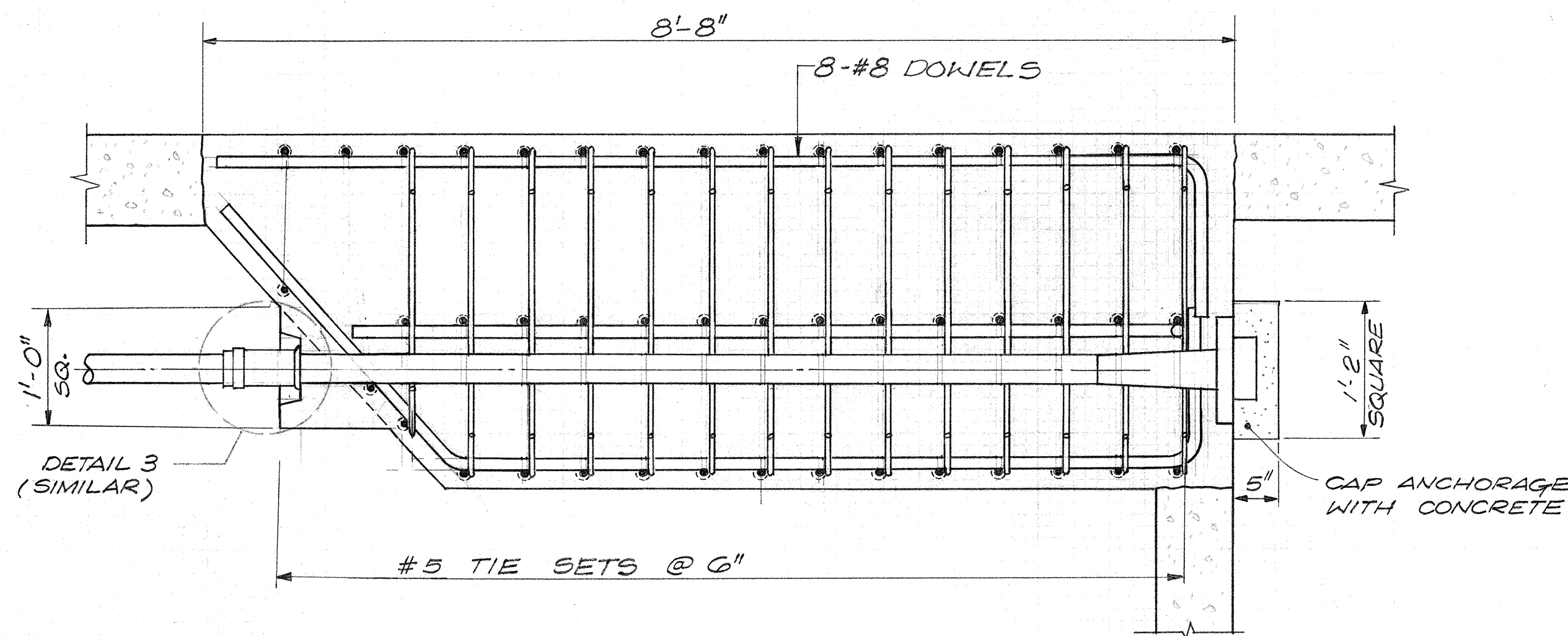
PARTIAL ELEVATION E
SCALE: 1/8" = 1'-0"



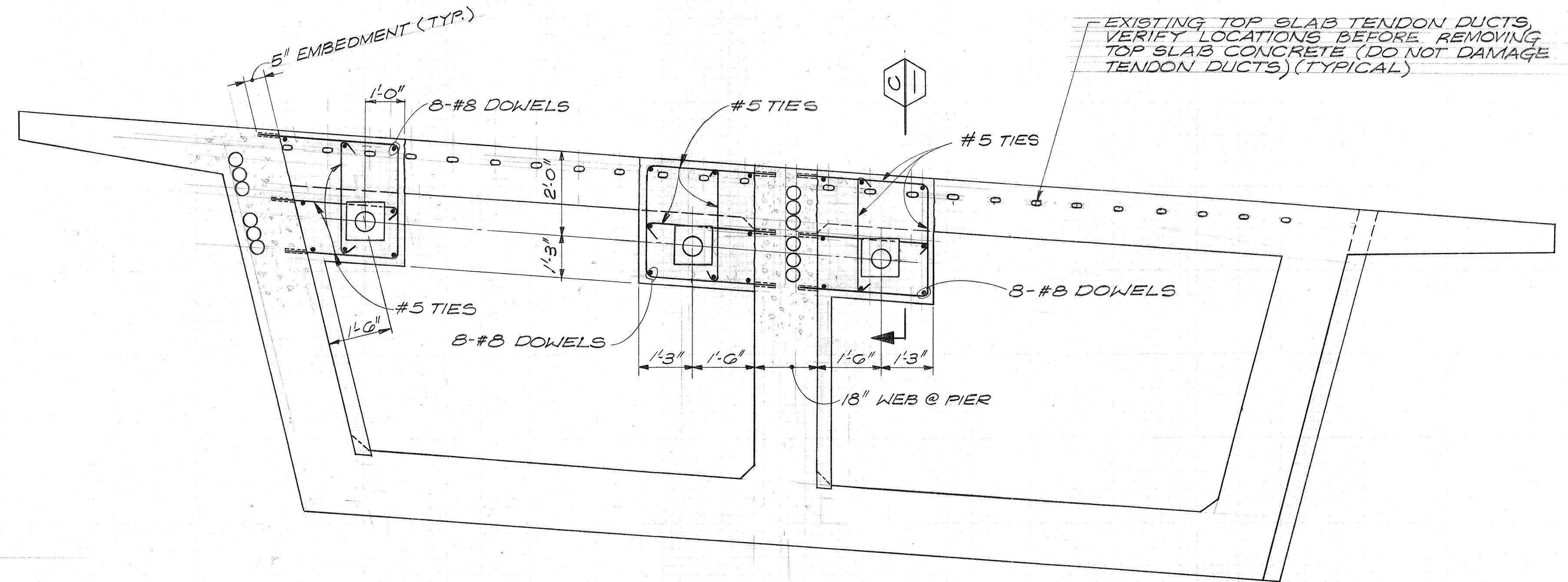
SECTION E
SCALE: 1" = 1'-0"



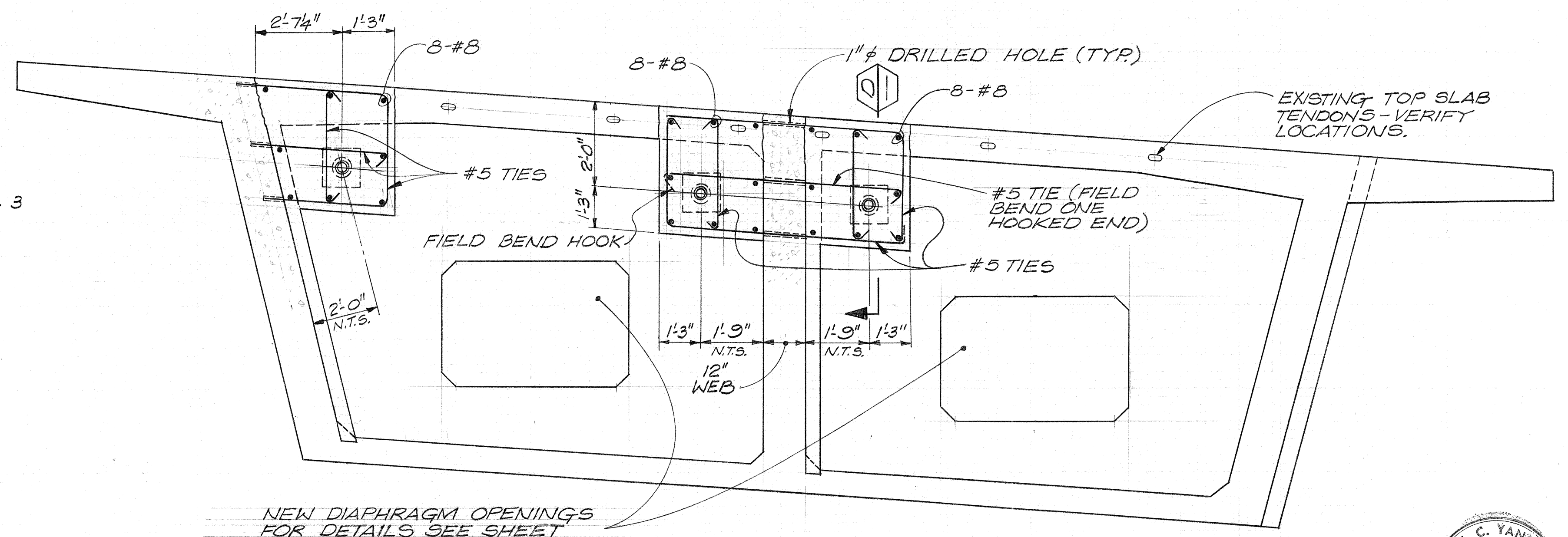
SECTION C
SCALE: 1" = 1'-0"



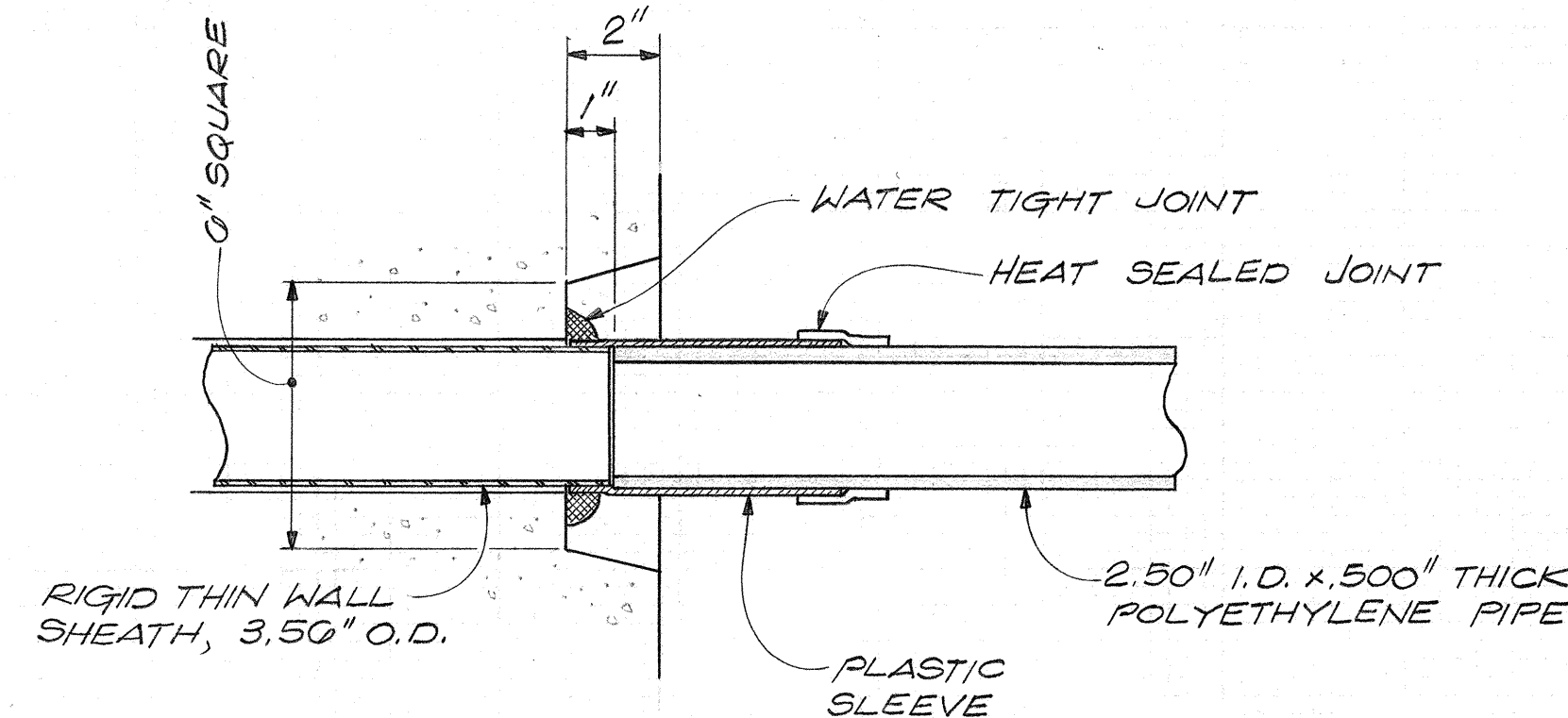
SECTION D
SCALE: 1" = 1'-0"



SECTION A (@ PIER NO. 2)
SCALE: 1/2" = 1'-0"



SECTION B (@ DIAPHRAGM)
SCALE: 1/2" = 1'-0"



DETAIL 3
SCALE: 3" = 1'-0"

POST-TENSIONING SEQUENCE

1. STRESS WEB TENDONS AS SHOWN AND NOTED ON ORIGINAL SHOP DRAWINGS BY V.S.L. CORP.
2. STRESS THE ADDED TENDON NORTH OF THE CENTER WEB FIRST, THEN THE ADDED TENDON SOUTH OF THE CENTER WEB, FINALLY STRESS THE ADDED TENDON @ THE NORTH WEB. STRESS THESE TENDONS AFTER THE WEB TENDONS HAVE BEEN FULLY GROUTED.
3. GROUT THE ADDED TENDONS.
4. CLOSE THE DECK ACCESS OPENINGS (SR-10)
5. STRESS THE TOP SLAB TENDONS AT PIER NO. 2 AS PER THE ORIGINAL SHOP DRAWINGS.

C.O. 114 S-12

Issued For	Date	By
REVIEW	5-82	D.C.

SHEET TITLE:	ADDED POST-TENSIONING DETAILS
PROJECT:	KAPIOLANI INTERCHANGE ON-RAMP REPAIRS F.A.I. PROJECT NO. I-HH (157): 24

SHEET NO.	SR-11
SHEET 11 OF 12 SHEETS	

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	115	146

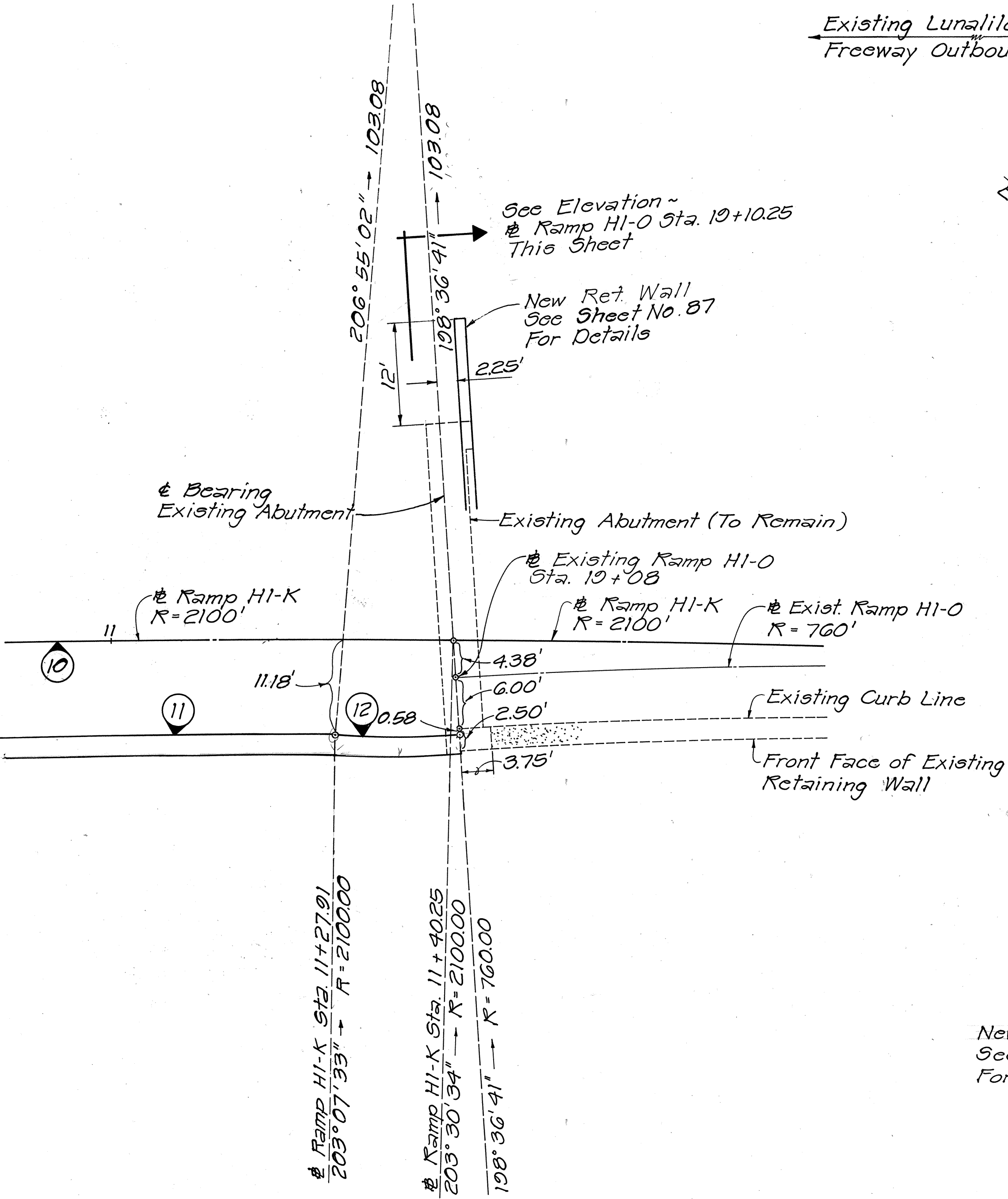
Ramp HI-K Curve Data

10

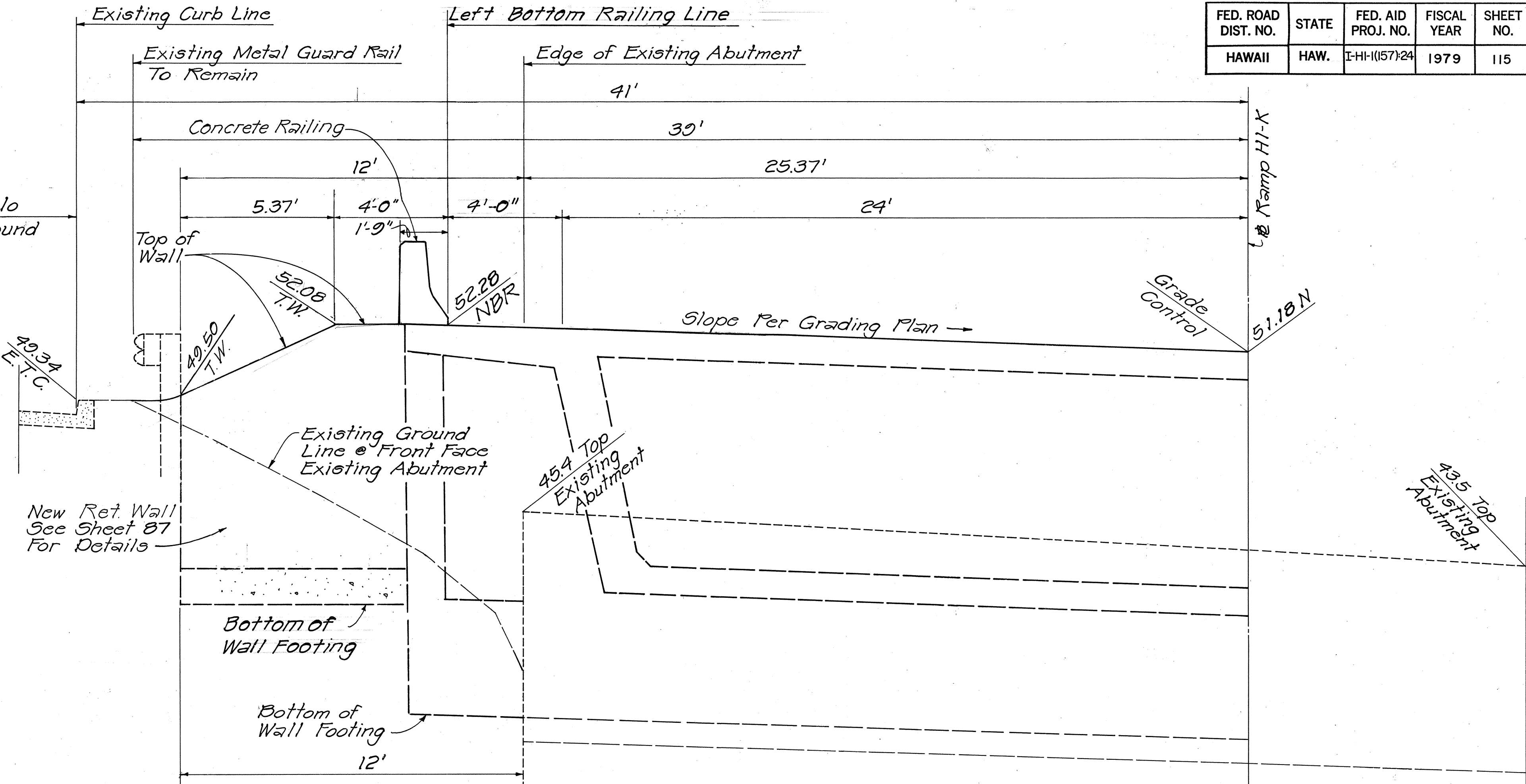
11

12

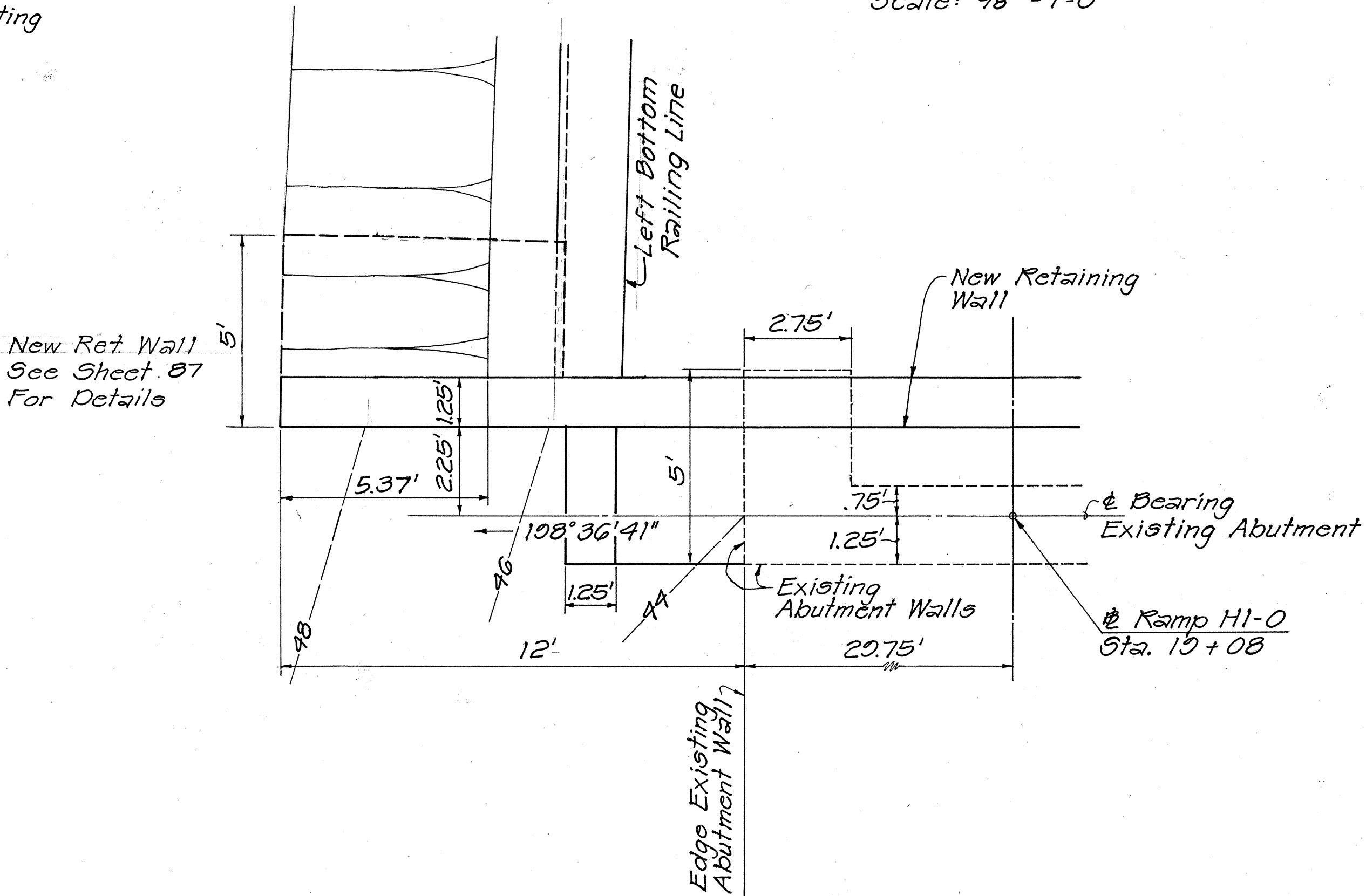
Δ	7°22'10"	7°59'17"	8°18'20.7"
R	2100.00	1100.00	103.08
C	270.01	153.24	14.93
T	135.28	76.80	7.48
Lc	270.20	153.36	14.94



LAYOUT PLAN @ RAMP HI-K STA. 11+40.25
Scale: 1" = 10'



ELEVATION ~ RAMP HI-K STA. 11+40.25
Scale: 3/8" = 1'-0"



PLAN ~ RAMP HI-K STA. 11+40.25
Scale: 3/8" = 1'-0"

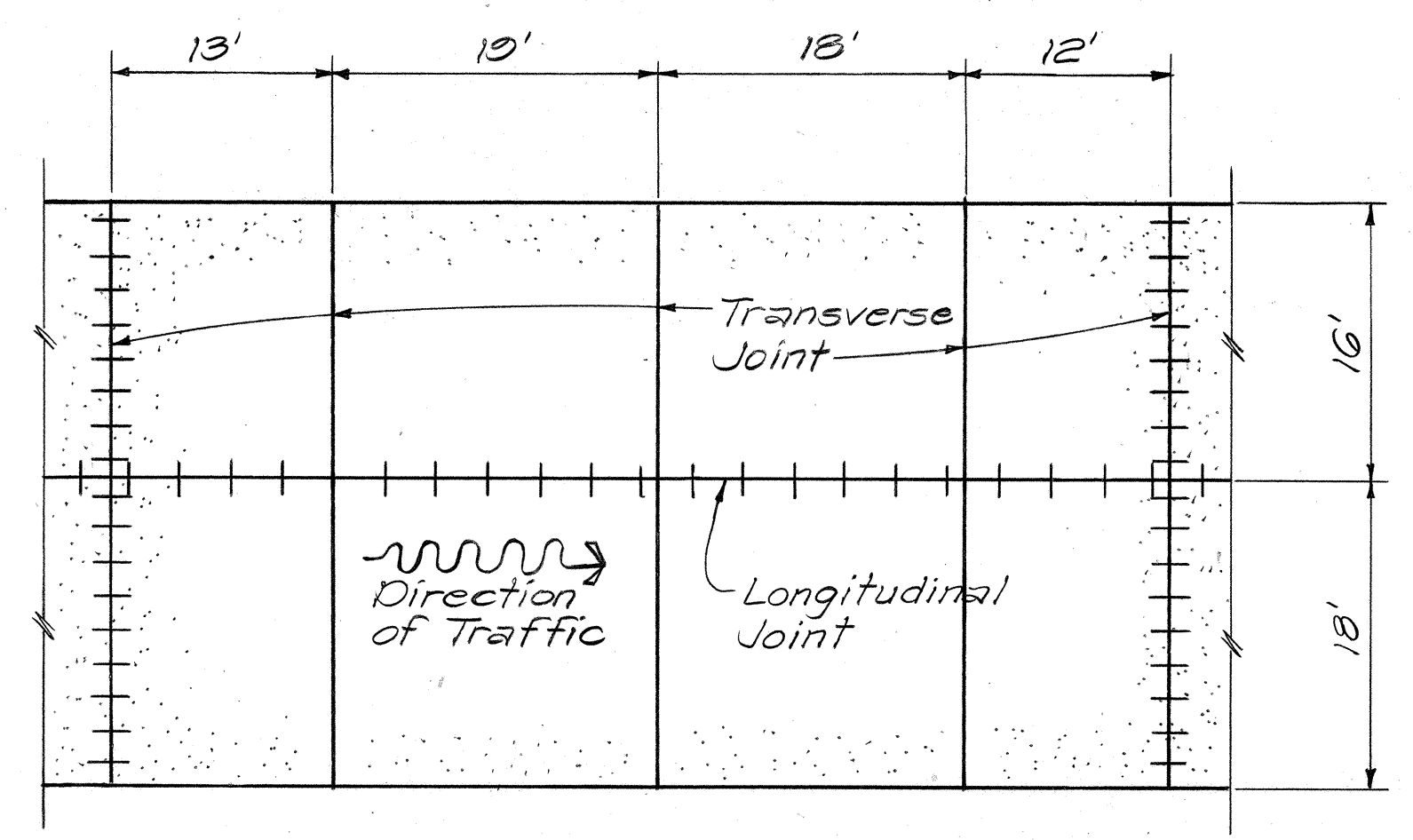
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION

Donald Y. H. Kim

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
LAYOUT AND MISCELLANEOUS DETAILS
RAMP HI-K
INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24
Scale: As Shown Date: 10-15-79
SHEET NO. 1 OF 1 SHEETS

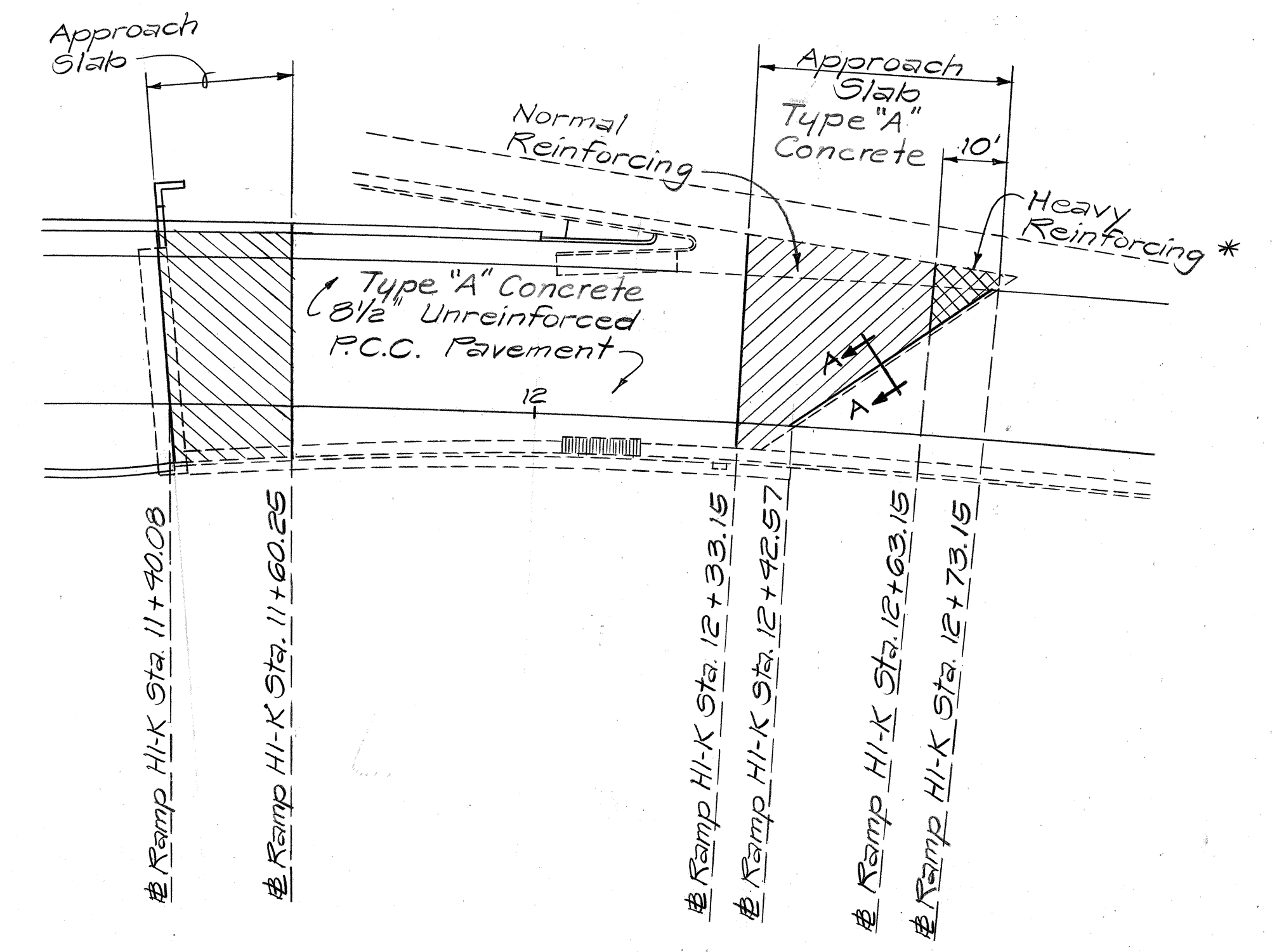
SURVEY PLOTTED BY	DATE
DRAWN BY	
DESIGNED BY	
NOTED BOOK	
QUANTITIES BY	
CHECKED BY	
No.	

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1157-24	1979	116	146

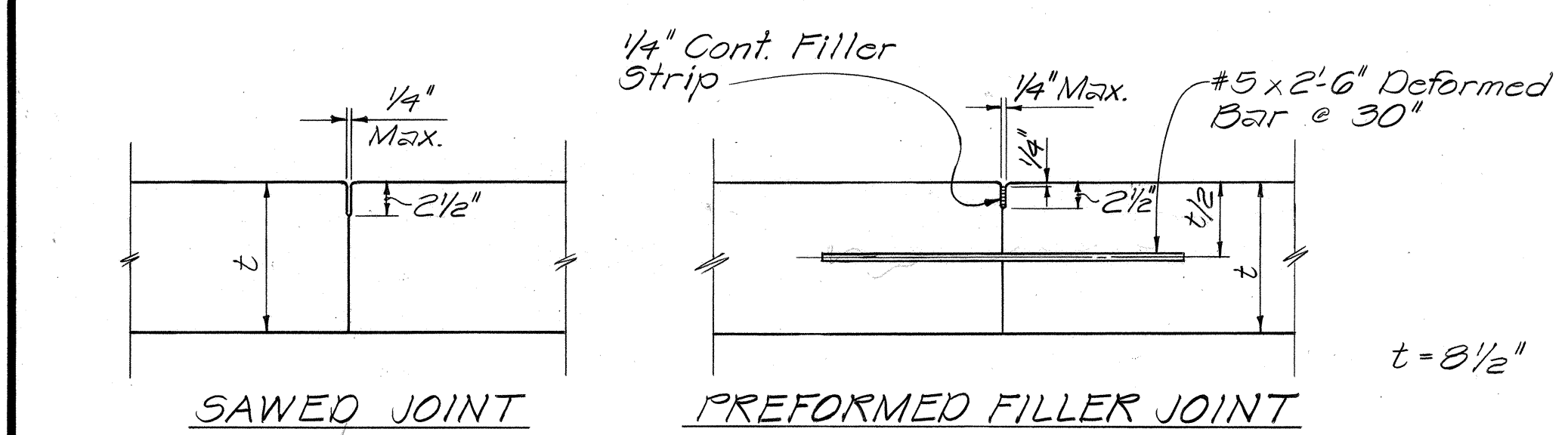


TYPICAL JOINT LOCATION AND LAYOUT PLAN
RAMP STA. 11+60.25 to 12+42.57
 Scale: 1"=10'

- CONSTRUCTION NOTES**
1. The longitudinal weakened plane joint detail shall be applicable at the intermediate traffic lane edge when two or more lanes are paved in one continuous pour. The straight tie bars shall be placed mechanically to the depth shown on the detail. All other longitudinal joints shall be constructed in accordance with the contact joint detail shown on this plan.
 2. Transverse weakened plane joints shall be sawed diagonally as shown, unless otherwise directed by the Engineer.
 3. Transverse weakened plane joints shall be spaced at successive intervals of 13', 19', 18', and 12' in the direction of traffic. Repeat for the remaining joints.
 4. Transverse contact joints shall be located at a minimum distance of 10' from the nearest planned transverse weakened plane joint.
 5. At the outer edge of the P.C.C. pavement, 1/2" radius rounding shall be utilized longitudinally.

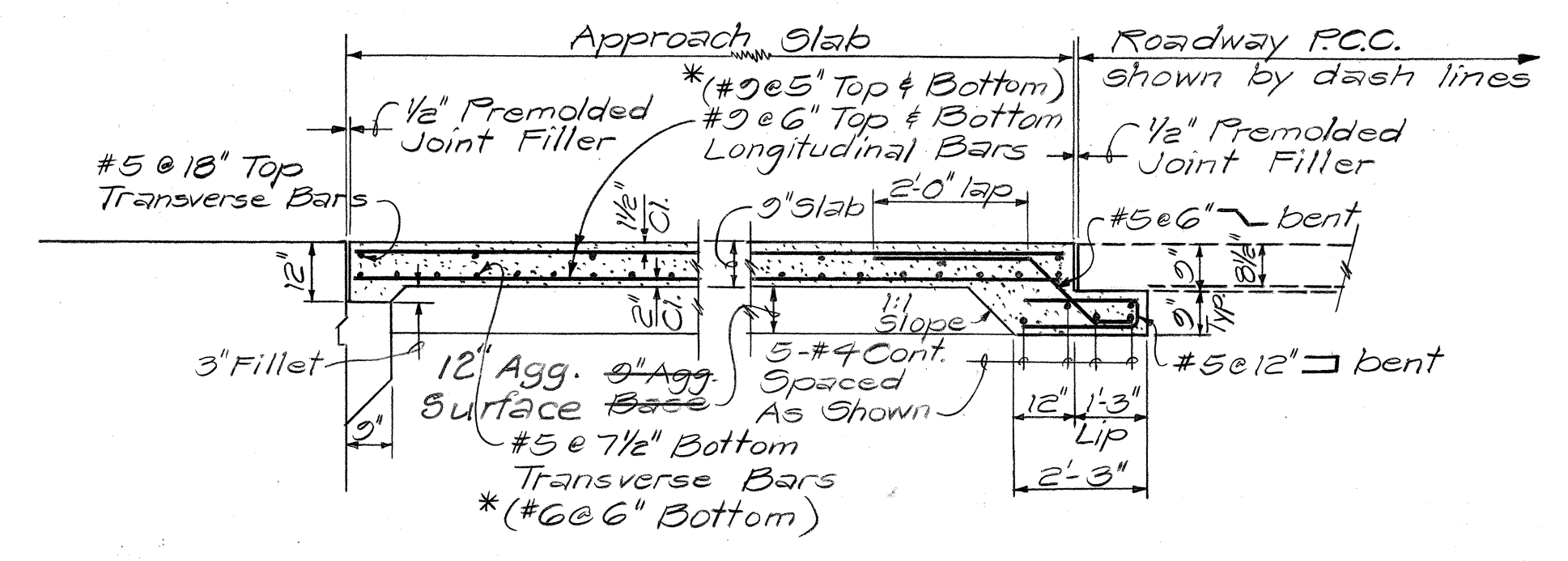


P.C.C. - APPROACH SLAB PLAN
Ramp Sta. 11+40.08 to Sta. 12+73.15
 Scale: 1"=20'

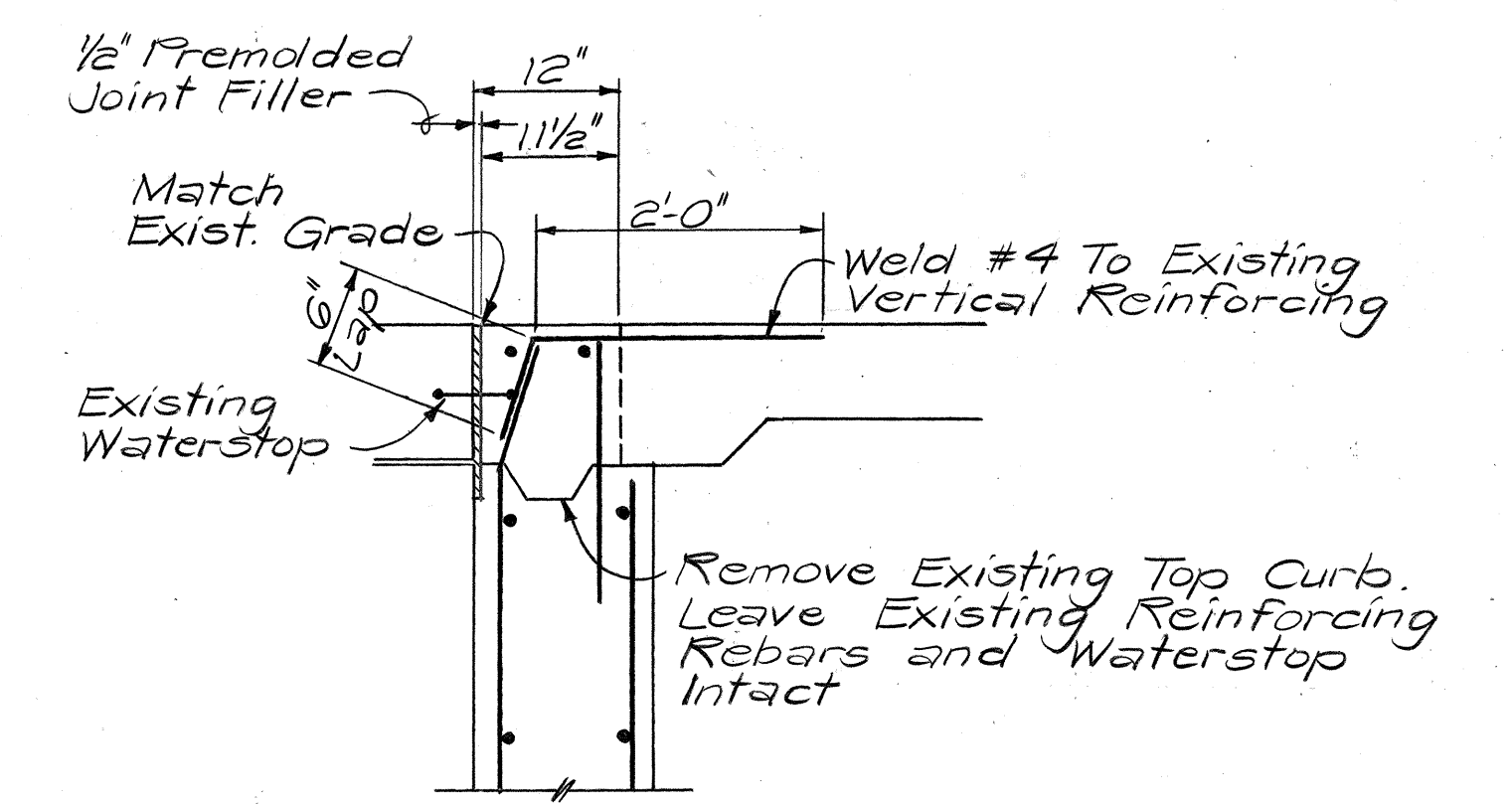


TRANSVERSE WEAKENED PLANE (CONTRACTION) JOINT
 Scale: 1"=1'-0"

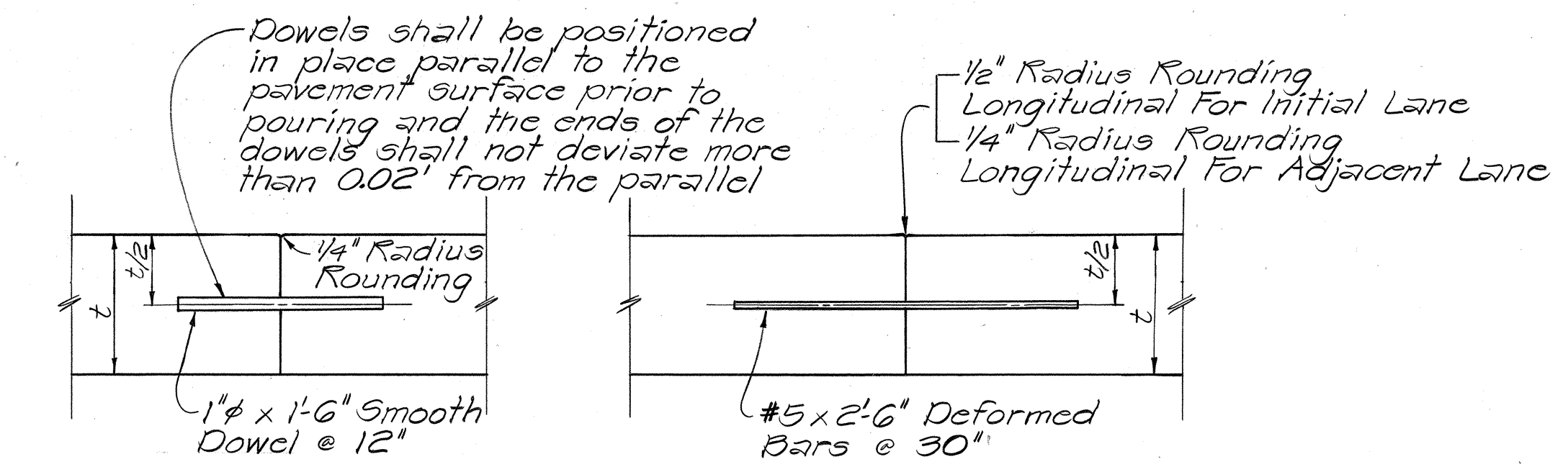
LONGITUDINAL WEAKENED PLANE (CONTRACTION) JOINT
 Scale: 1"=1'-0"



TYPICAL APPROACH SLAB
 Not To Scale

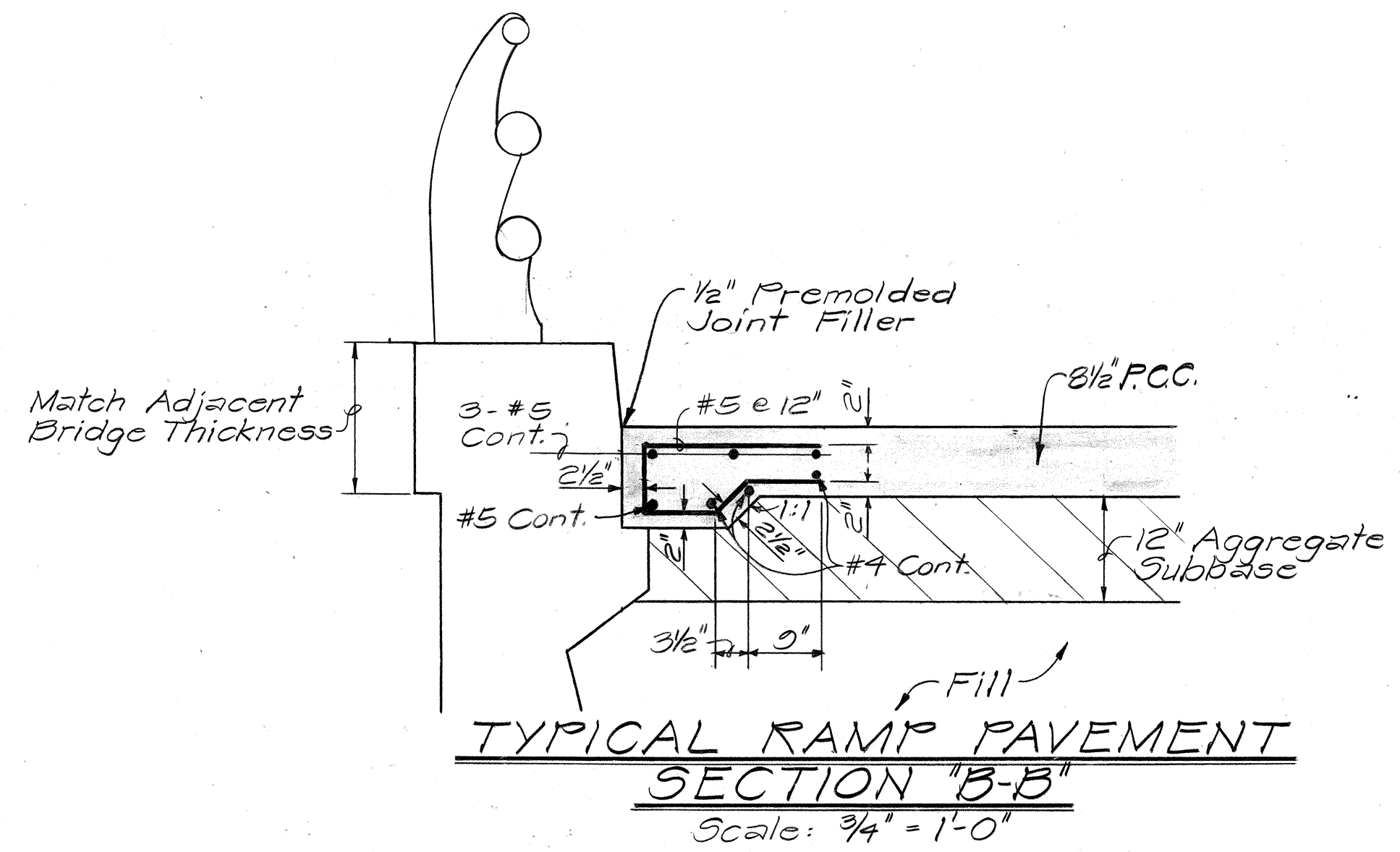


SECTION "A-A"
DETAIL @ EXISTING ABUTMENT
 Scale: 3/4"=1'-0"



TRANSVERSE CONSTRUCTION (CONTACT) JOINT
 Scale: 1"=1'-0"

LONGITUDINAL CONSTRUCTION (CONTACT) JOINT
 Scale: 1"=1'-0"



TYPICAL RAMP PAVEMENT
SECTION "B-B"
 Scale: 3/4"=1'-0"

SURVEY PLOTTED BY	DATE
DRAWN BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
No.	

6/12/80	Added Approach Slab
	Revised P.C.C. Pavt Details
Date	Revision

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 Donald Y. H. Kaim

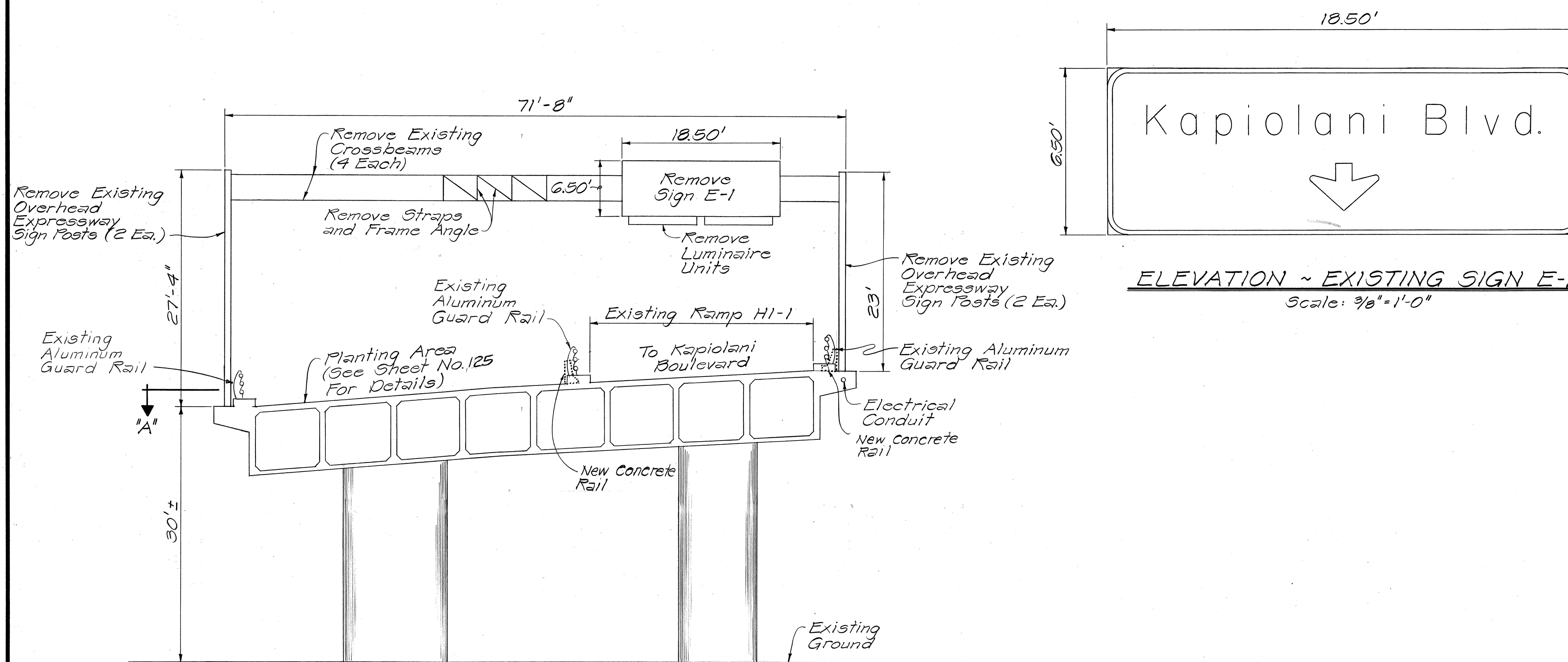
STATE OF HAWAII
 DEPARTMENT OF TRANSPORTATION
 LAND TRANSPORTATION FACILITIES DIVISION
P.C.C. PAVEMENT DETAILS
 RAMP HI-K

INTERSTATE ROUTE H-1
 KAPIOLANI INTERCHANGE ON-RAMP
 F.A.I. PROJ. NO. I-HI-1157-24
 Scale: As Noted Date: 10-15-79

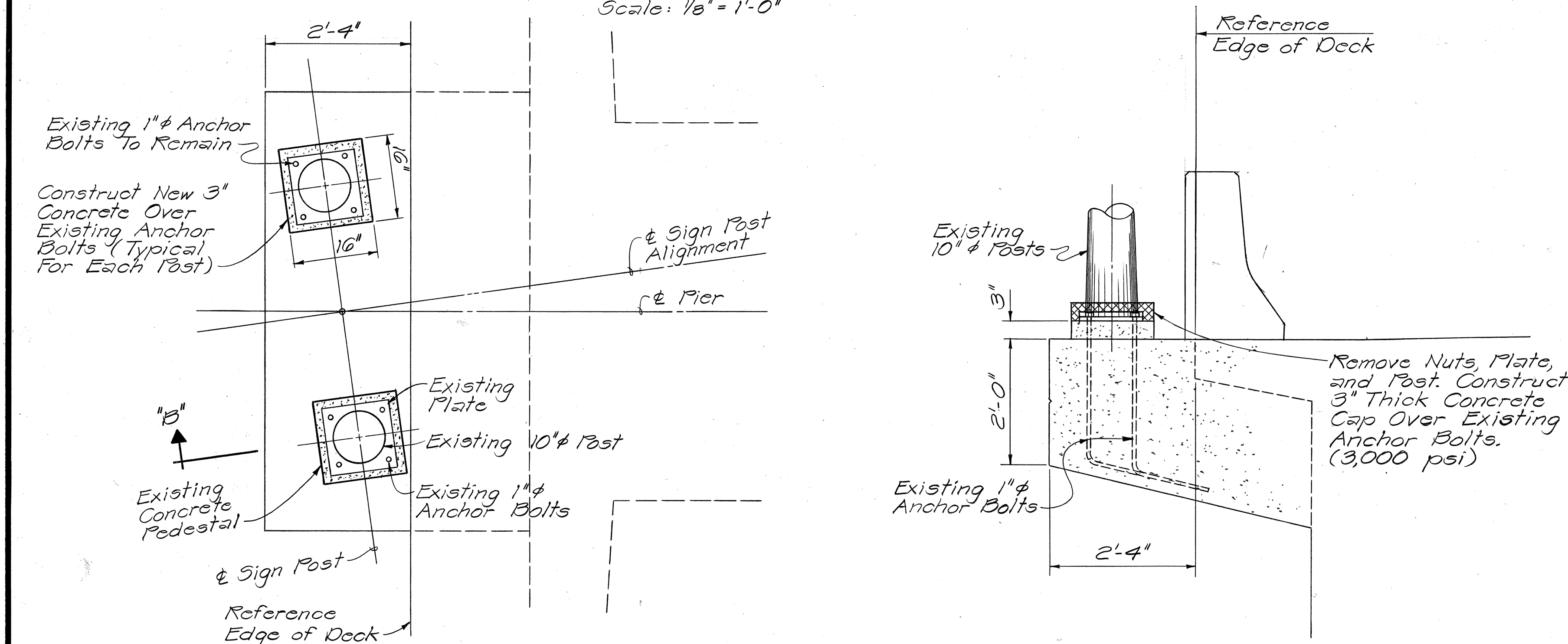
SHEET NO. 1 OF 1 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(157)-24	1979	118	146

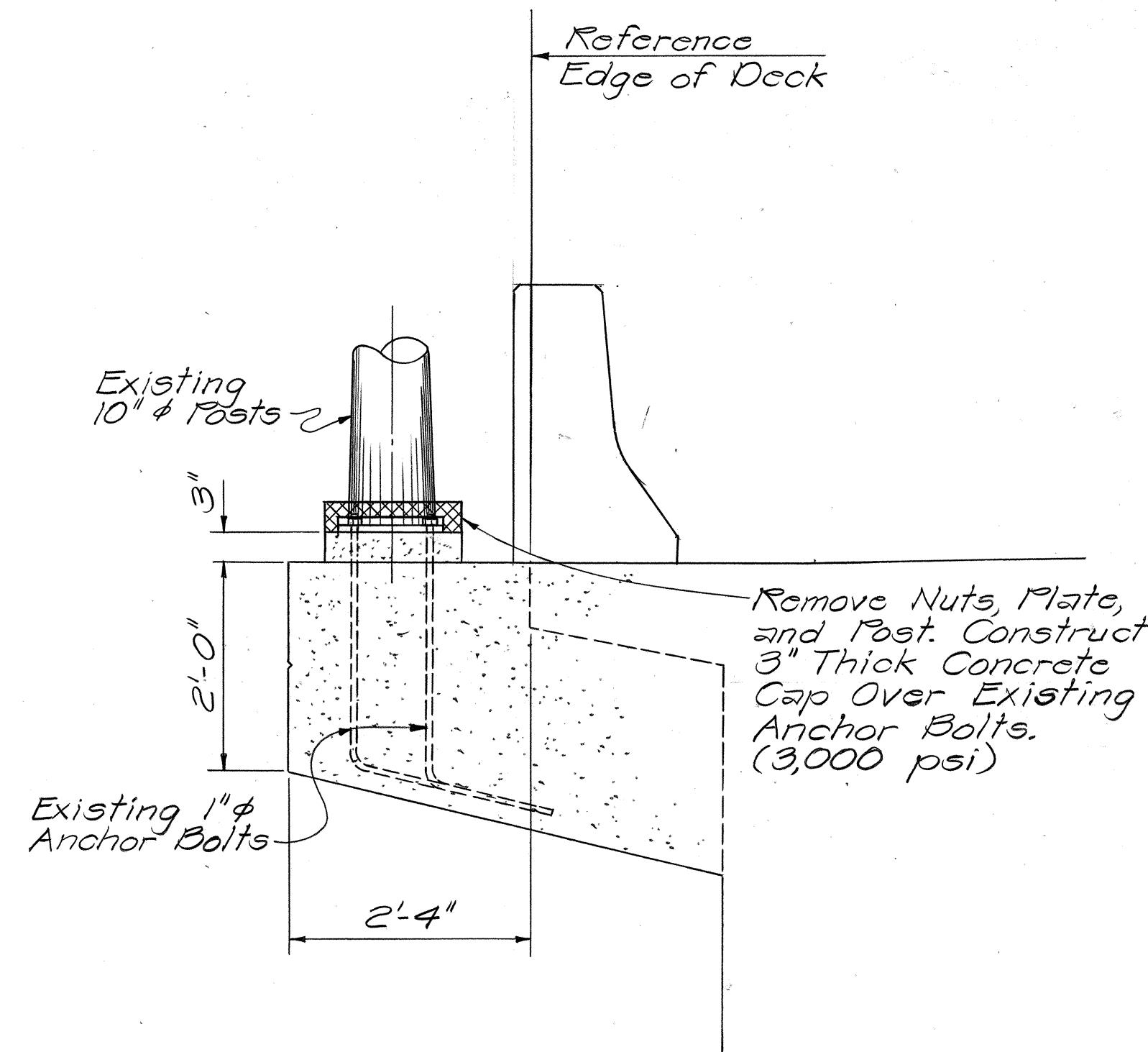
NOTES:
1. Contractor to verify size of pipe structures to be removed.



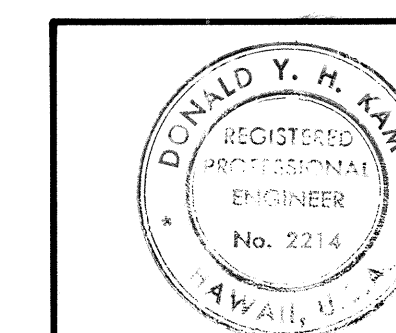
ELEVATION ~ EXISTING SIGN E-1
Scale: 3/8" = 1'-0"



SECTION "A" ~ PLAN
EXISTING SIGN POST BRACKET
Scale: 3/4" = 1'-0"



SECTION "B"
Scale: 3/4" = 1'-0"



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ME OR UNDER MY SUPERVISION
Donald Y. H. Kam

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
LAND TRANSPORTATION FACILITIES DIVISION
**EXISTING RAMP SIGN
REMOVAL DETAILS**

INTERSTATE ROUTE H-1
KAPIOLANI INTERCHANGE ON-RAMP
F.A.I. PROJ. NO. I-HI-1(157)-24

Scale: As Noted Date: 10-15-79

SHEET NO. 2 OF 2 SHEETS