

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(131):17	1977	332	

1. GENERAL SPECIFICATIONS: HAWAII DEPARTMENT OF TRANSPORTATION, STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, 1976, TOGETHER WITH SPECIAL PROVISIONS PREPARED FOR THIS CONTRACT.
2. DESIGN SPECIFICATIONS: A.A.S.H.T.O. 1973 STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES AND ITS SUBSEQUENT INTERIM SPECIFICATIONS.
3. LOADS:
- (A) DEAD LOAD: AN ALLOWANCE OF 18 P.S.F. FOR FUTURE WEARING SURFACE OF ASPHALT CONCRETE HAS BEEN PROVIDED FOR IN THE DESIGN.
- (B) LIVE LOAD: HS20-44 OR INTERSTATE LOADING.
4. MATERIALS:
- (A) ALL CONCRETE SHALL BE CLASS A UNLESS OTHERWISE NOTED.
- (B) ALL REINFORCING STEEL SHALL BE A.S.T.M. A615, GRADE 40 UNLESS OTHERWISE NOTED.
- (C) ALL STRUCTURAL STEEL SHALL BE A.S.T.M. A-36, HOT DIP GALVANIZED, UNLESS OTHERWISE NOTED.
- (D) ALL ANCHOR BOLTS, WASHERS AND NUTS SHALL BE A.S.T.M. A-307, HOT DIP GALVANIZED, UNLESS OTHERWISE SPECIFIED.
- (E) FOR MATERIALS OF PRESTRESSED CONCRETE BEAM SEE PRESTRESSED CONCRETE NOTES.
5. DESIGN STRESSES: SHALL FOLLOW A.A.S.H.T.O. STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES IN ADDITION TO THOSE LISTED BELOW.

CONCRETE	CLASS A	ALL OTHERS (EXCEPT PRESTRESSED CONCRETE BEAMS AND UNLESS OTHERWISE NOTED ON THE PLANS)
F'C	3000 P.S.I.	5000 P.S.I.
FC	1200 P.S.I.	2000 P.S.I.
N	9	7

FOR PRESTRESSED CONCRETE BEAMS, SEE PRESTRESSED CONCRETE NOTES.

6. REINFORCEMENT:
- (A) THE MINIMUM COVERING MEASURED FROM THE SURFACE OF THE CONCRETE TO THE FACE OF ANY REINFORCING BARS SHALL BE AS FOLLOWS:
- A.1. DECK SLABS (FOR BOX GIRDERS, T-BEAMS, PRESTRESSED BEAMS)
- A. TOP BARS = 1 1/2"
- B. BOTTOM BARS = 1 1/4"
- A.2. BOX GIRDERS
- A. 1 1/4" TO STIRRUP FROM OUTSIDE FACES
- B. 1" TO STIRRUP FROM INSIDE FACES.
- A.3. FOR PRESTRESSED CONCRETE BEAMS SEE PRESTRESSED CONCRETE BEAM DETAILS.
- A.4. ABUTMENTS, PIERS AND RETAINING WALLS = 2"
- A.5. COLUMNS = 2" TO TIES
- A.6. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH = 3".
- A.7. CONCRETE EXPOSED TO SEA WATER = 4".
- (B) REINFORCING BARS SHALL BE DETAILED IN ACCORDANCE WITH A.C.I. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE HIGHWAY STRUCTURES UNLESS OTHERWISE NOTED.
- (C) MINIMUM CLEAR SPACING BETWEEN PARALLEL BARS SHALL BE 1 1/2 TIMES THE DIAMETER OF BARS (FOR NON BUNDLED BARS), BUT IN NO CASE SHALL THE CLEAR DISTANCE BETWEEN THE BARS BE LESS THAN 1 1/2 TIMES THE MAXIMUM SIZE OF THE COARSE AGGREGATE.
- (D) ALL DIMENSIONS RELATING TO REINFORCING BARS (EG. SPACING OF BARS, ETC.,) ARE TO CENTERS OF BARS UNLESS OTHERWISE NOTED.
- (E) REINFORCING BARS SHALL BE SECURELY TIED AT ALL INTERSECTIONS AND LAP SPLICES AND SHALL BE HELD IN PLACE DURING POURING TO MAINTAIN THEIR PROPER LOCATIONS AS SHOWN ON THE PLANS.
- (F) VERTICAL COLUMN BARS SHALL BE ARRANGED IN SUCH A MANNER AS TO AVOID INTERFERENCE WITH PIER CAP BARS ABOVE AS DIRECTED BY THE ENGINEER.
7. ELASTOMERIC PADS: BOTTOM OF BRIDGE BEARING PADS SHALL BE SECURED TO THE CONCRETE SEATS, TO PREVENT DISPLACEMENT, WITH ADHESIVES APPROVED BY THE ENGINEER.

8. CONSTRUCTION METHODS.
- (A) SEE STANDARD SPECIFICATIONS AND SPECIAL PROVISIONS.
- (B) IN GENERAL TOP OF CONCRETE DECK SLAB SHALL BE CONSTRUCTED TO FOLLOW THE ROADWAY VERTICAL AND HORIZONTAL CURVES.
- (C) FOR CONCRETE FINISH SEE STANDARD SPECIFICATIONS AND FOR ARCHITECTURAL TREATMENT OF COLUMNS, SEE DETAILS.
- (D) GIRDER CONCRETE SEATS RECEIVING ELASTOMERIC PADS SHALL BE POURED MONOLITHICALLY WITH SUPPORTING STRUCTURE. TOP OF CONCRETE SEATS SHALL BE FINISHED WITH A STEEL TROWEL TO A SMOOTH LEVEL SURFACE TO THE ELEVATION SHOWN ON THE PLANS.
- (E) SPREAD FOOTINGS SHALL BE EXCAVATED AND POURED NEAT AGAINST UNDISTURBED GROUND. IN CASE OF OVER EXCAVATION, SPACE BETWEEN FOOTING AND GROUND SHALL BE FILLED WITH CONCRETE AT THE CONTRACTOR'S EXPENSE AND AS DIRECTED BY THE ENGINEER. THE MINIMUM QUALITY OF THE FILL CONCRETE SHALL BE CLASS D.
- (F) IN THE INSTALLATION OF ANCHOR BOLTS FOR BRIDGE BEARINGS, LIGHTING STANDARDS AND EXPRESSWAY BRIDGE SIGN STRUCTURES, THE CONTRACTOR SHALL PROVIDE RIGID TEMPLATES TO MAINTAIN THE PROPER LOCATIONS AND SHALL PROTECT SUCH ANCHOR BOLTS AT ALL TIMES DURING THE PERIOD OF CONSTRUCTION. METHODS SHALL BE APPROVED BY THE ENGINEER.
- (G) EXCEPT AS OTHERWISE NOTED, ALL VERTICAL DIMENSIONS ARE MEASURED PLUMB.

9. GENERAL
- (A) ALL ITEMS NOTED INCIDENTAL WILL NOT BE PAID FOR SEPARATELY.
- (B) THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UTILITY LINES AND NOTIFY THE RESPECTIVE OWNERS BEFORE COMMENCING THE WORK OF EXCAVATION OR THE DRIVING OF PILES, INCLUDING ANY TEMPORARY PILING OR SHEETING.
- (C) ALL STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION UNLESS OTHERWISE NOTED.
- (D) CONSTRUCTION JOINTS MAY BE RELOCATED OR ADDITIONAL ONES ADDED SUBJECT TO THE APPROVAL OF THE ENGINEER.
- (E) UNLESS OTHERWISE NOTED, ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4" x 3/4".
- (F) STANDARD DETAIL DRAWINGS REFER TO ALL STRUCTURES IN GENERAL, EXCEPT FOR MODIFICATIONS AS MAY BE REQUIRED FOR SPECIAL CONDITIONS. FOR SUCH MODIFICATIONS REFER TO THE CORRESPONDING DETAILED DRAWINGS.
- (G) FOR ELECTRICAL CONDUIT LOCATION DETAILS SEE ELECTRICAL DRAWINGS.

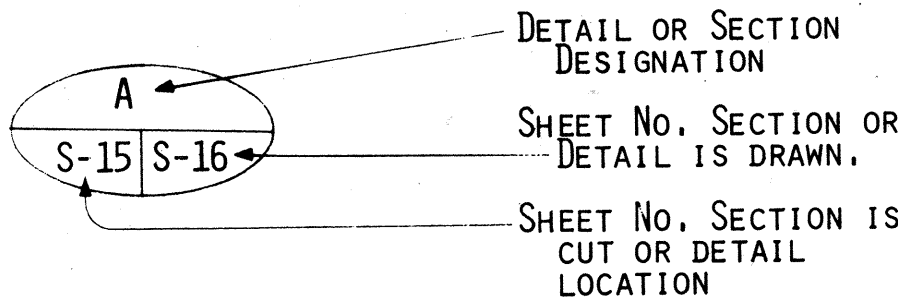
10. FOUNDATION:
- (A) DESIGN LOAD FOR ALL ~~PILES - 90 TONS~~ PILE TYPE A = 115 TONS AND PILE TYPE B = 85 TONS UNLESS OTHERWISE SPECIFIED, ON FOOTING PLANS.
- (B) FOR BORING LOGS AND THEIR LOCATION SEE SHEET NOS. B-1, B-2, ETC.
- (C) THE ESTIMATED PILE TIP ELEVATIONS SHOWN ON THE PLANS ARE BASED ON BORING DATA.
- (D) IN DRIVING EACH GROUP OF PILES, THE CONTRACTOR SHALL START FROM CENTER OF THE GROUP AND PROCEED OUTWARDS, EXCEPT FOR TEST PILES AND LOAD TEST PILES.
- (E) THE CONTRACTOR SHALL VERIFY THE TYPE OF SURFACE SOIL BEFORE STARTING TO DRIVE ANY PILE. WHERE THERE IS A SURFACE CRUST, A TEMPORARY STEEL SLEEVE SHALL BE USED TO PROTECT THE BITUMEN COATING ON PILES. THE SLEEVE SHALL EXTEND THROUGH THE DEPTH OF THE CRUST. THE SLEEVE SHALL BE REMOVED AFTER THE PILE IS DRIVEN AND THE ANNULAR SPACE AROUND THE PILE SHALL BE BACKFILLED AS SPECIFIED IN THE SPECIAL PROVISIONS. SURFACE SOIL/CRUST IS DEFINED AS THE MATERIAL BETWEEN THE GROUND LINE, OR BOTTOM OF EXCAVATION, AND THE UNIFORM SOFT CLAYS (WITHOUT THE PRESENCE OF GRANULAR MATERIALS).
- (F) LOAD TEST PILES INDICATED IN FOUNDATION PLAN SHALL BE FURNISHED 17 1/2' LONGER THAN REGULAR PILES.
- (G) TEST PILES INDICATED IN FOUNDATION PLAN SHALL BE FURNISHED 5' LONGER THAN REGULAR PILES.
- (H) SQUARE PILES ARE TO BE SO ALIGNED THAT FRONT AND REAR FACES OF PILES ARE PARALLEL TO CENTERLINE OF PIER.

* DESIGN LOAD FOR PILES EXCLUDES PILE WEIGHT AND ESTIMATED DOWNDRAG.

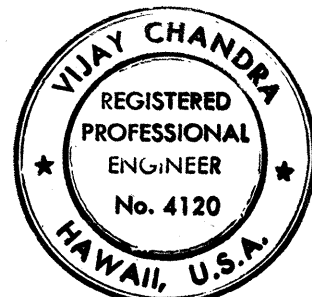
11. REFERENCES:
- (A) FOR PRESTRESSED CONCRETE NOTES, SEE DRAWINGS FOR PRESTRESSED BEAM DETAILS.
- (B) FOR SIGN STRUCTURE NOTES, SEE DRAWING NO. ES-1.
- (C) FOR ADDITIONAL PILE NOTES SEE DRAWING NO. SD-10.

12. SYMBOLS AND ABBREVIATIONS

(E) OR (EXP.)	- EXPANSION	O.C.	- ON CENTERS
(H)	- HINGE	O/S	- OFFSET
ALT.	- ALTERNATE	STR.	- STRAIGHT OR STRINGER
AZ.	- AZIMUTH	STIRR.	- STIRRUP
$\varnothing$	- CENTERLINE	TYP.	- TYPICAL
$\bar{B}$	- BASELINE	(T) OR T.	- TOP
E.F.	- EACH FACE	(B) OR B.	- BOTTOM
B.F.	- BOTH FACES	BRG.	- BEARING
F.F.	- FAR FACE	T.O.D.	- TOP OF DECK
N.F.	- NEAR FACE	W.P.	- WORKING POINT
C.J.	- CONSTRUCTION JOINT	a	- AT
CL.	- CLEAR	$\varnothing$ OR DIA.	- DIAMETER
EL., ELEV.	- ELEVATION	$\bar{P}$	- PLATE
E.P.	- EDGE OF PAVEMENT		DETAIL OR SECTION DESIGNATION
EB	- EAST BOUND		
FIN. GR.	- FINISHED GRADE		
WB	- WEST BOUND		
INV.	- INVERT		
JT.	- JOINT		
N.T.S.	- NOT TO SCALE		



E 3 - Denotes changes to C.O. 332 - 2/29/80.

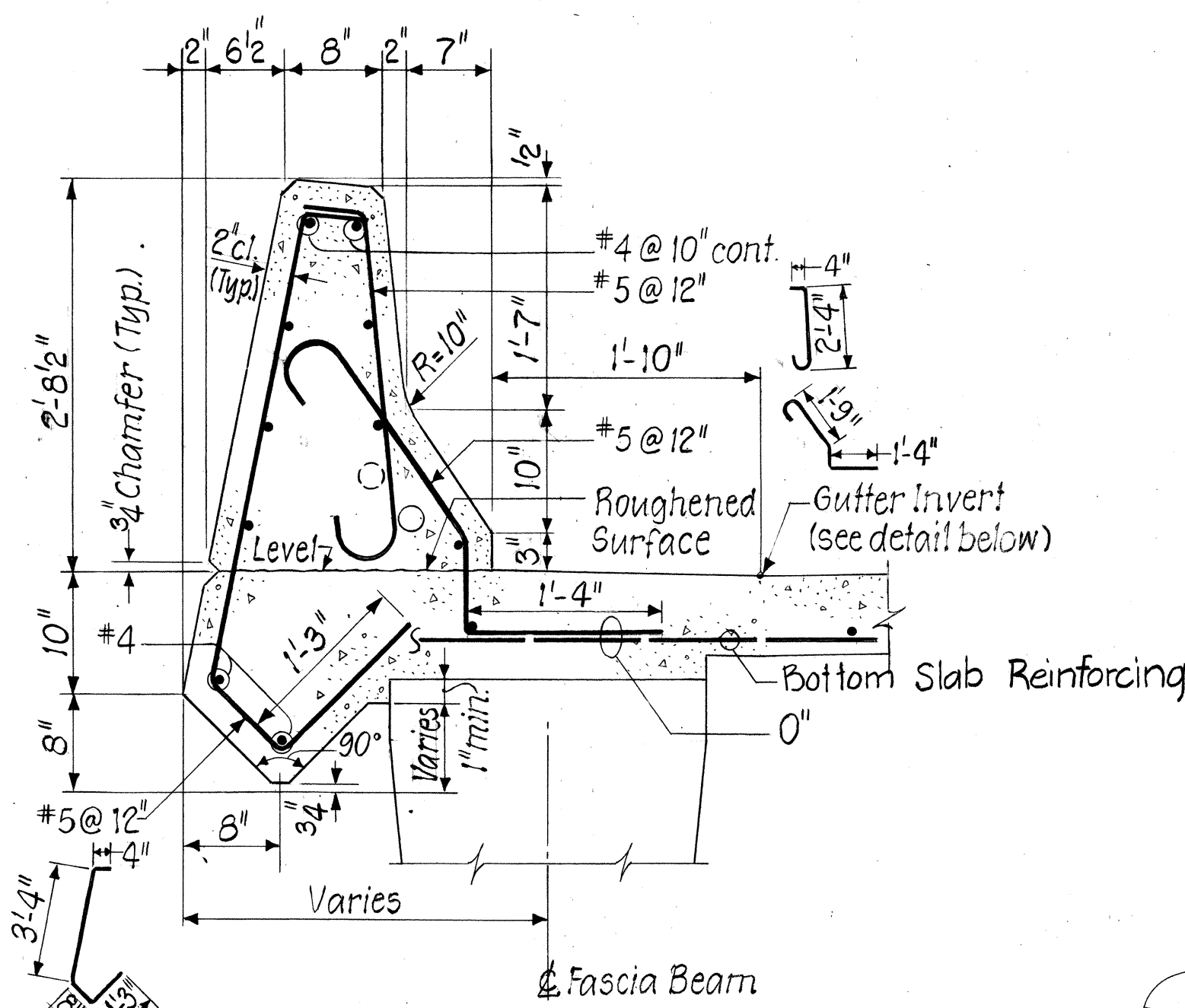


THIS WORK WAS PREPARED BY
ME OR UNDER MY SUPERVISION.
Vijay chandra
Signature

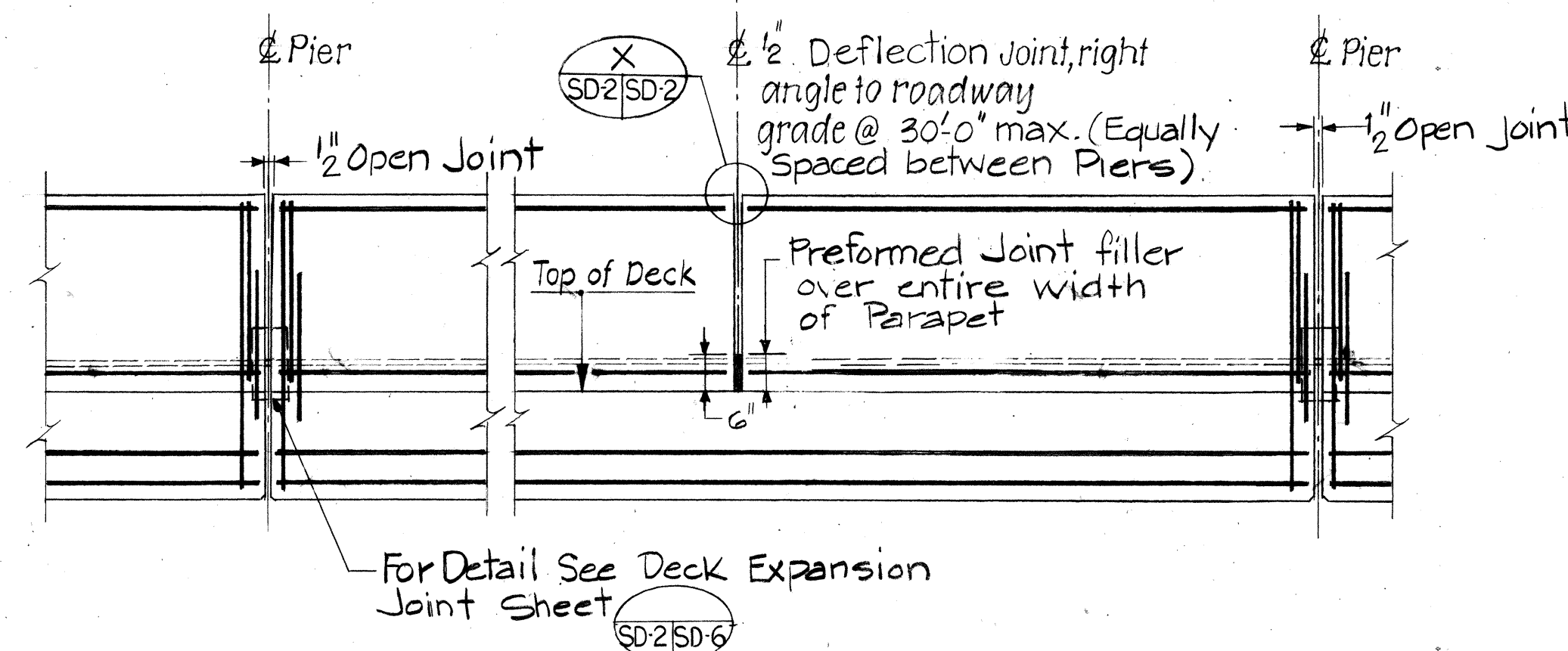
2/29/80	Added notes (F) & (G) to item 10. Foundation
DATE	REVISION
STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION	
GENERAL NOTES	
KEEHI INTERCHANGE F.A.I. PROJECT NO. I-HI-1 (131):17 SCALE: DATE: AUG. 31, 1977	
SHEET NoSD-1 OFSD-17 SHEETS	

C.O. 332

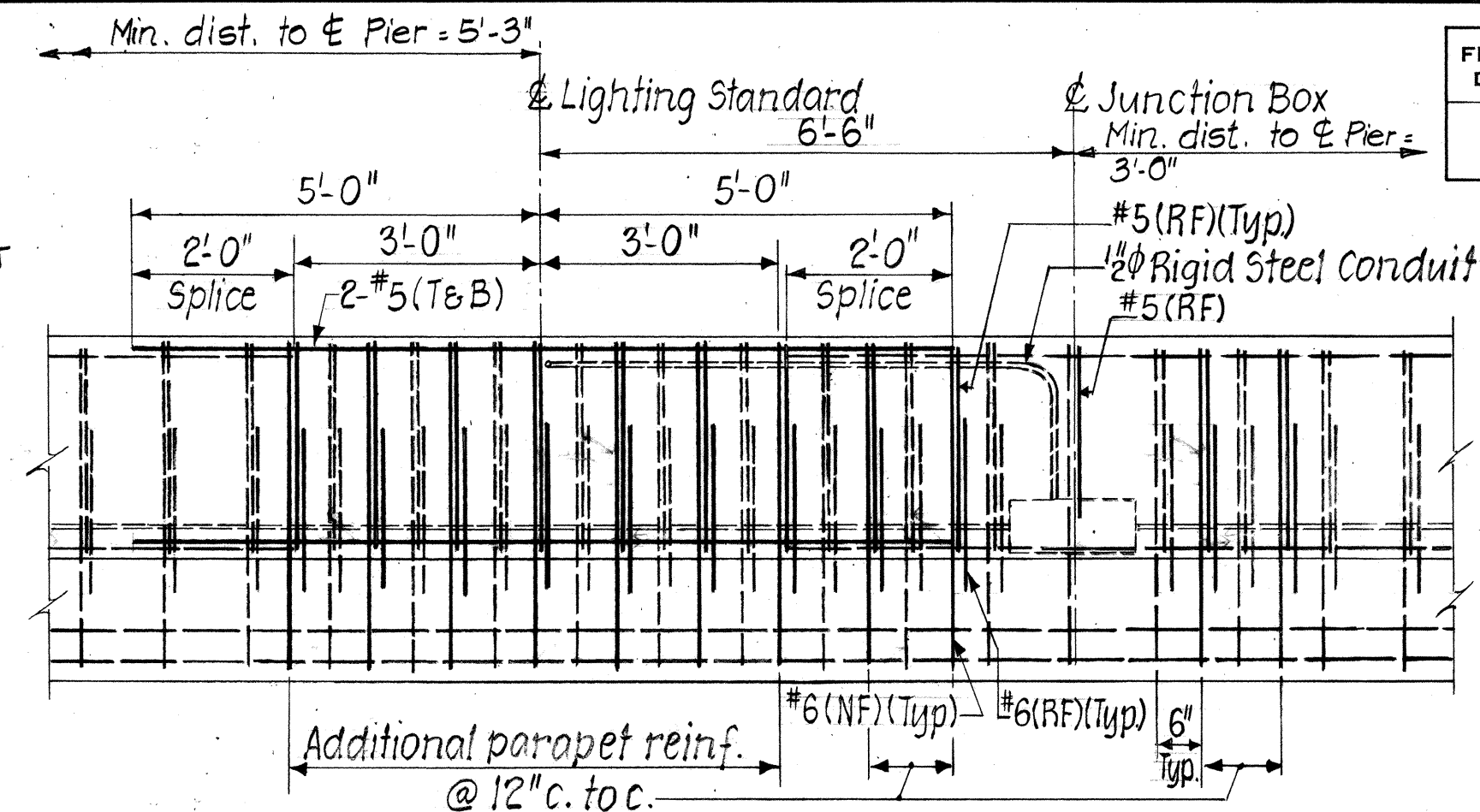
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HI-11(131)17	1977	333	394



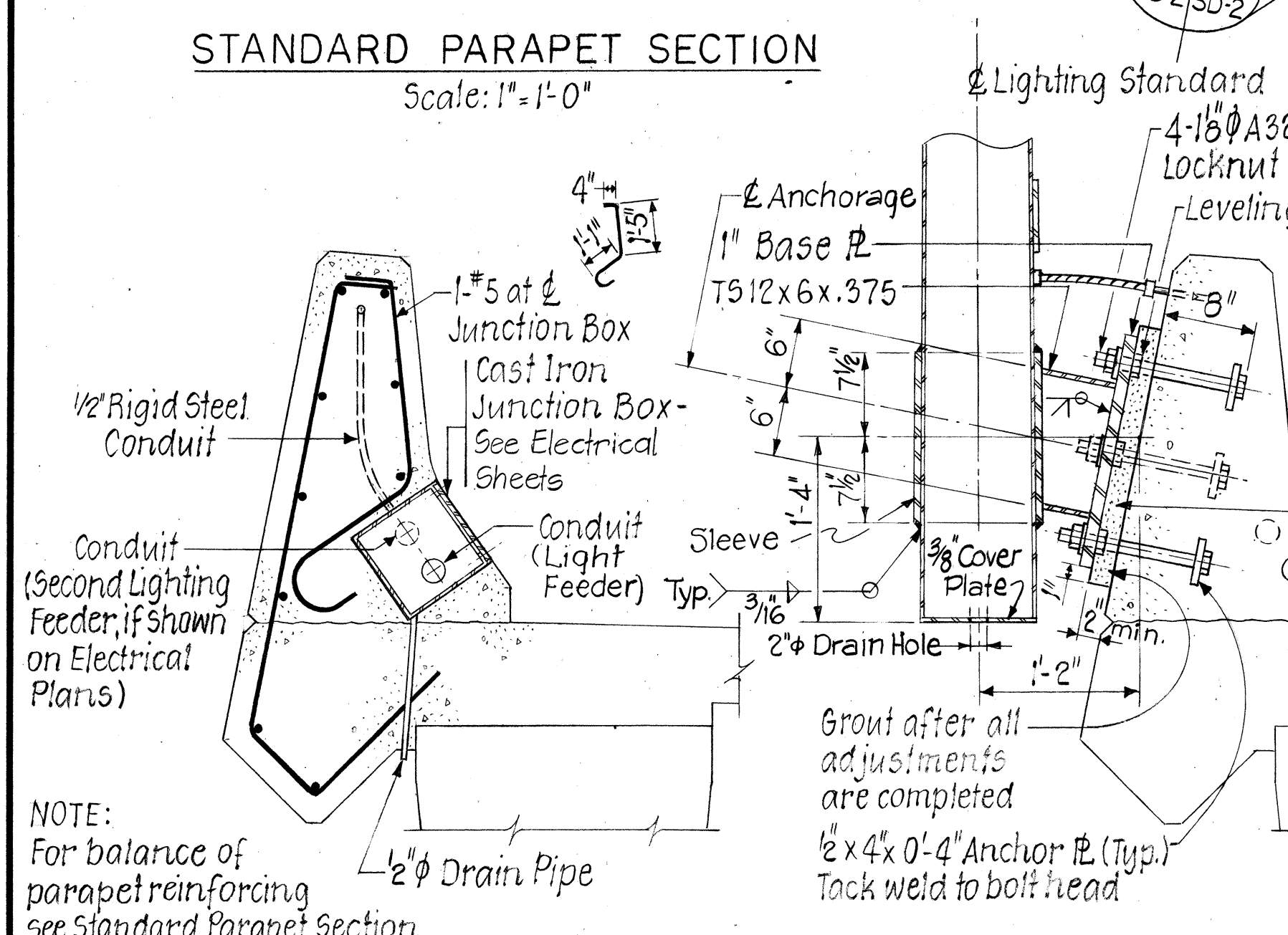
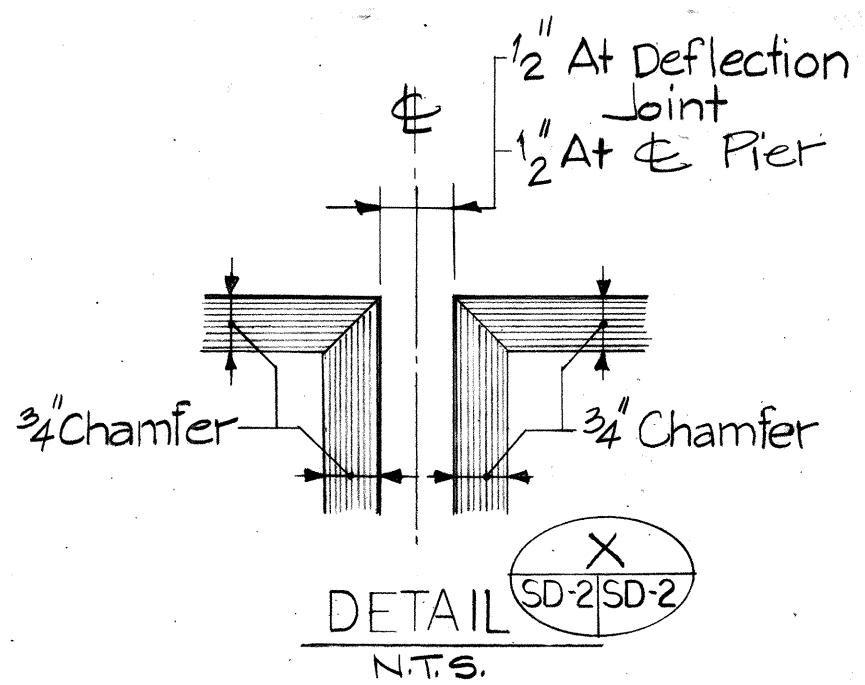
STANDARD PARAPET SECTION
Scale: 1" = 1'-0"



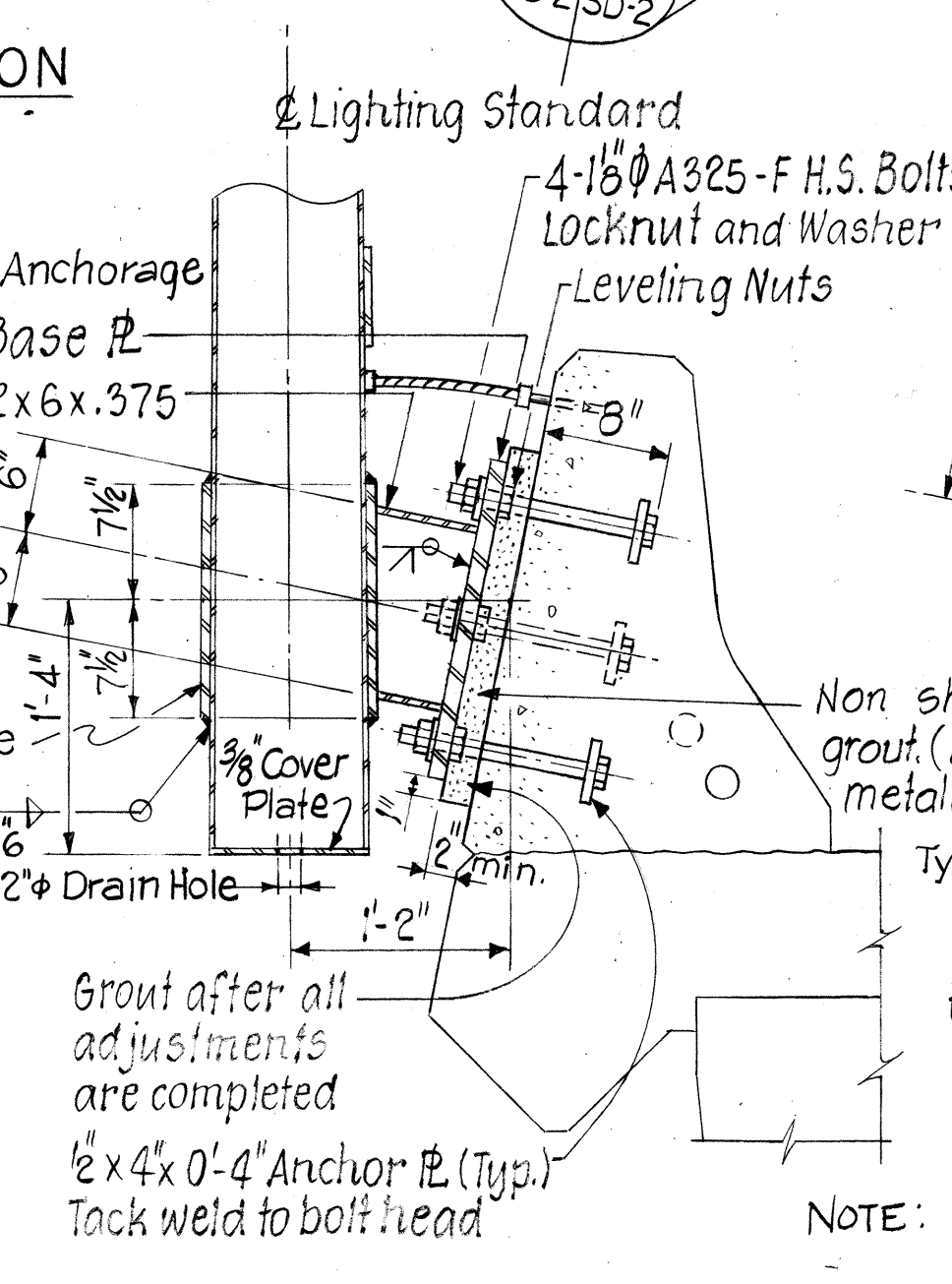
STANDARD PARAPET TYPICAL LONGITUDINAL SECTION
Scale: 1/2" = 1'-0"



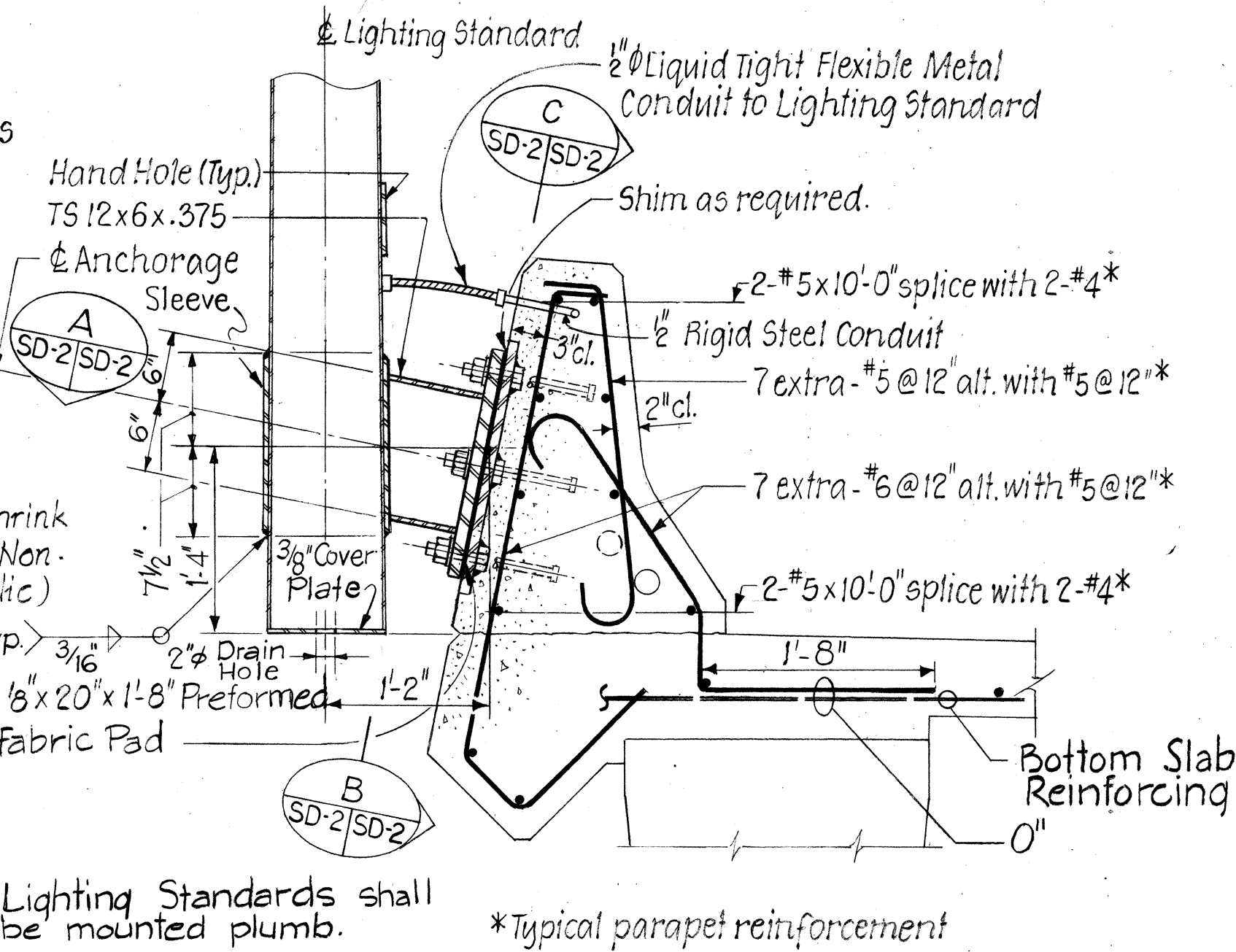
PARAPET REINF. AT LIGHTING STANDARD & JUNCTION BOX TYPICAL LONGITUDINAL SECTION
Scale: 1/2" = 1'-0"



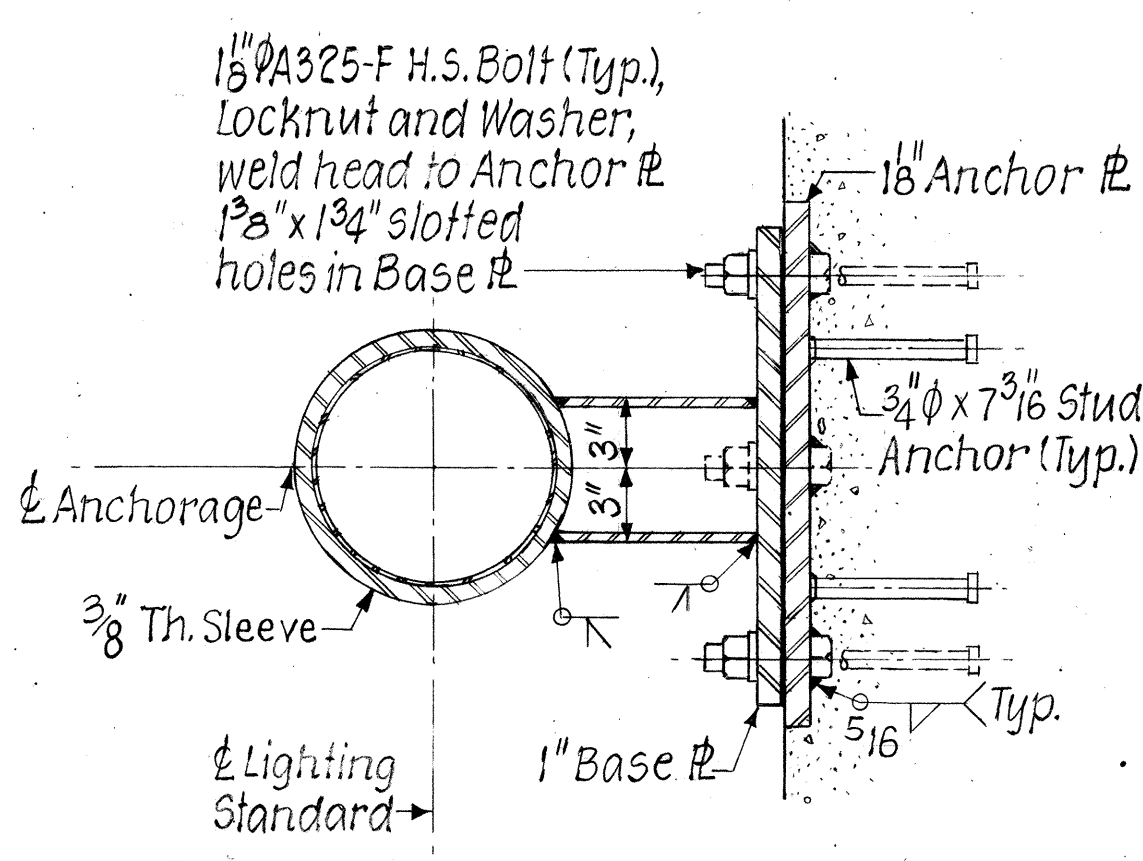
PARAPET DETAIL AT JUNCTION BOXES
Scale: 1" = 1'-0"



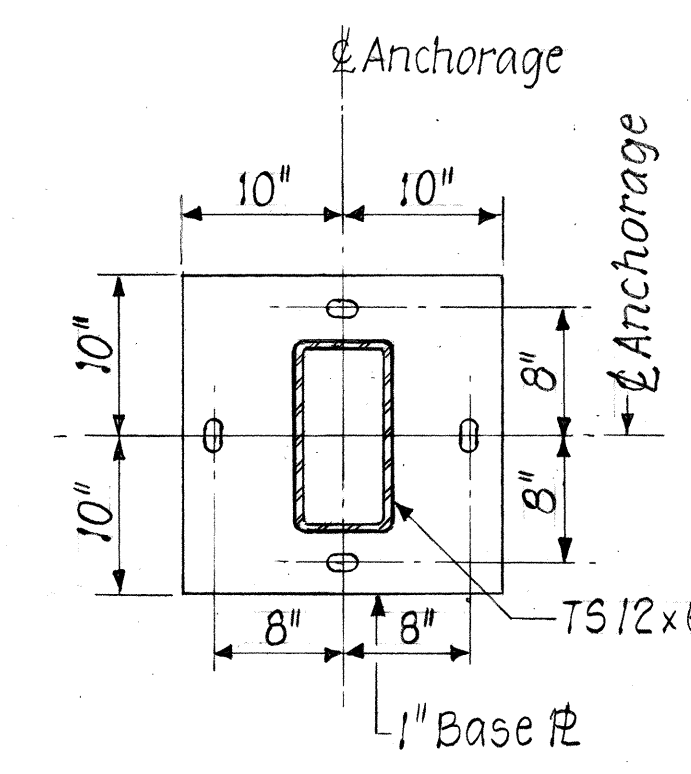
ALTERNATE METHOD LIGHTING STANDARD ATTACHMENT
Scale: 1" = 1'-0"



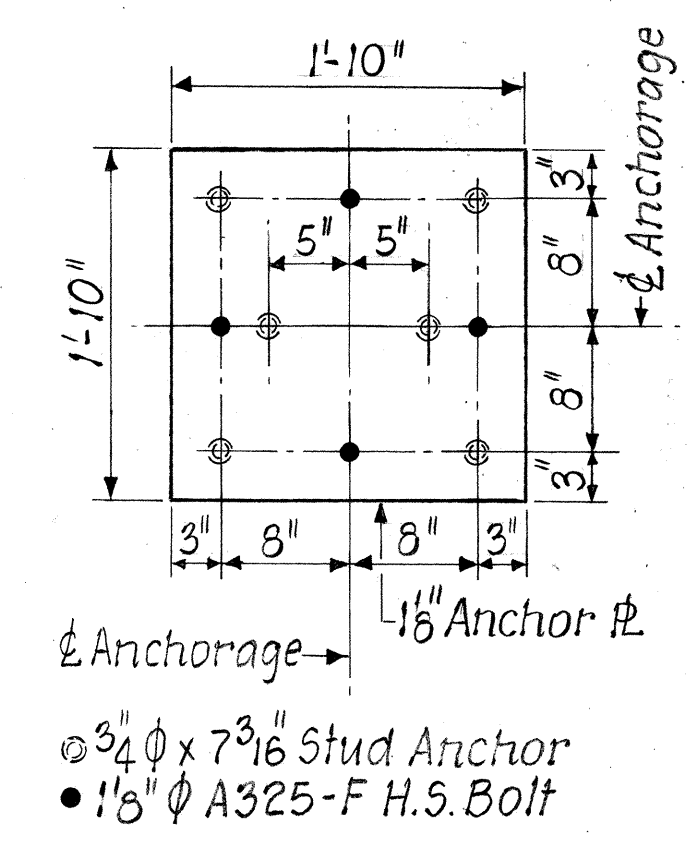
PARAPET DETAIL AT LIGHTING STANDARD
Scale: 1" = 1'-0"



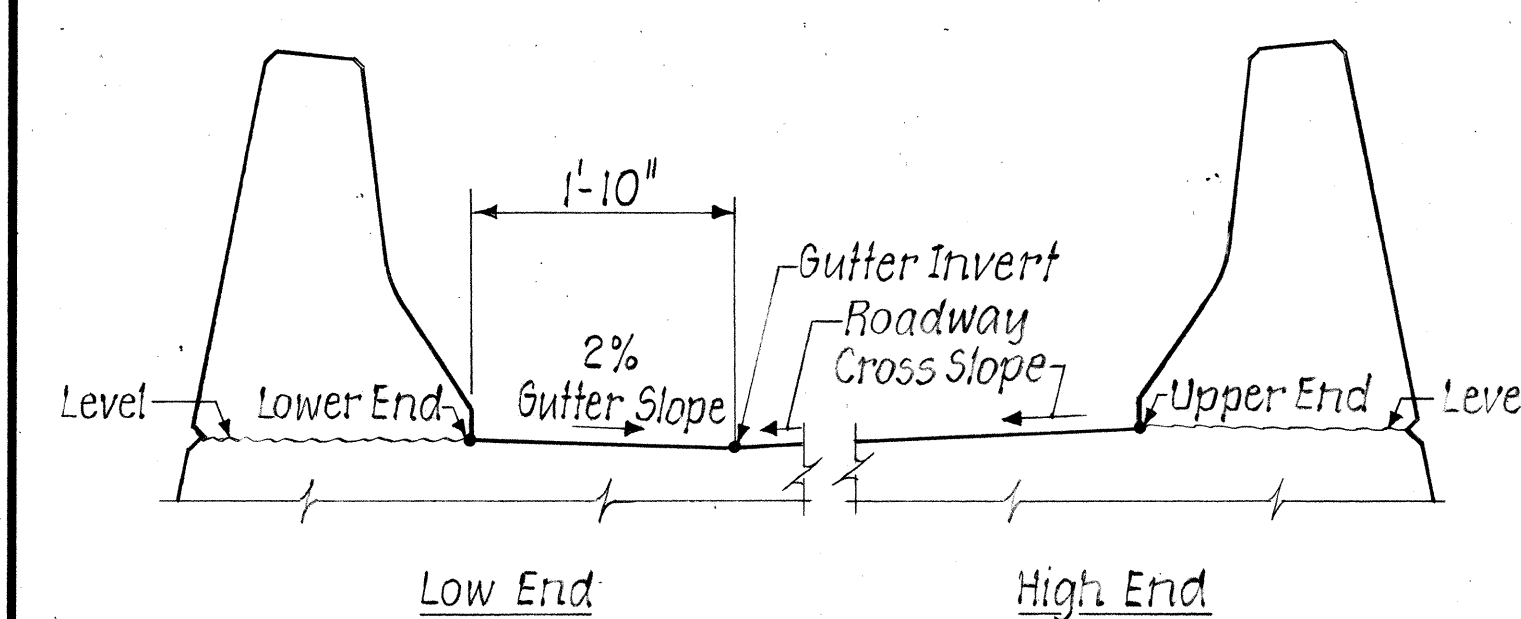
SECTION A
Scale: 1/2" = 1'-0"



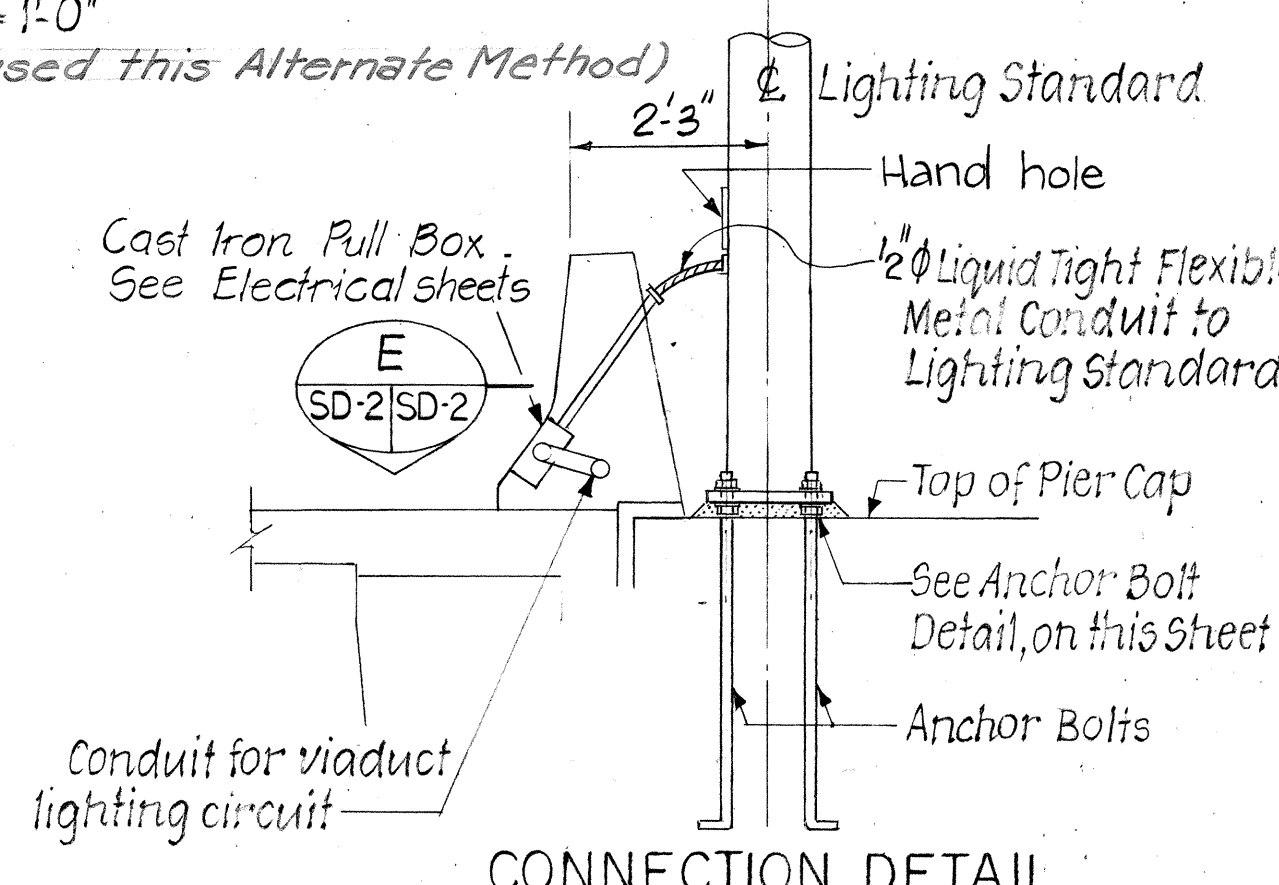
SECTION B
Scale: 1" = 1'-0"



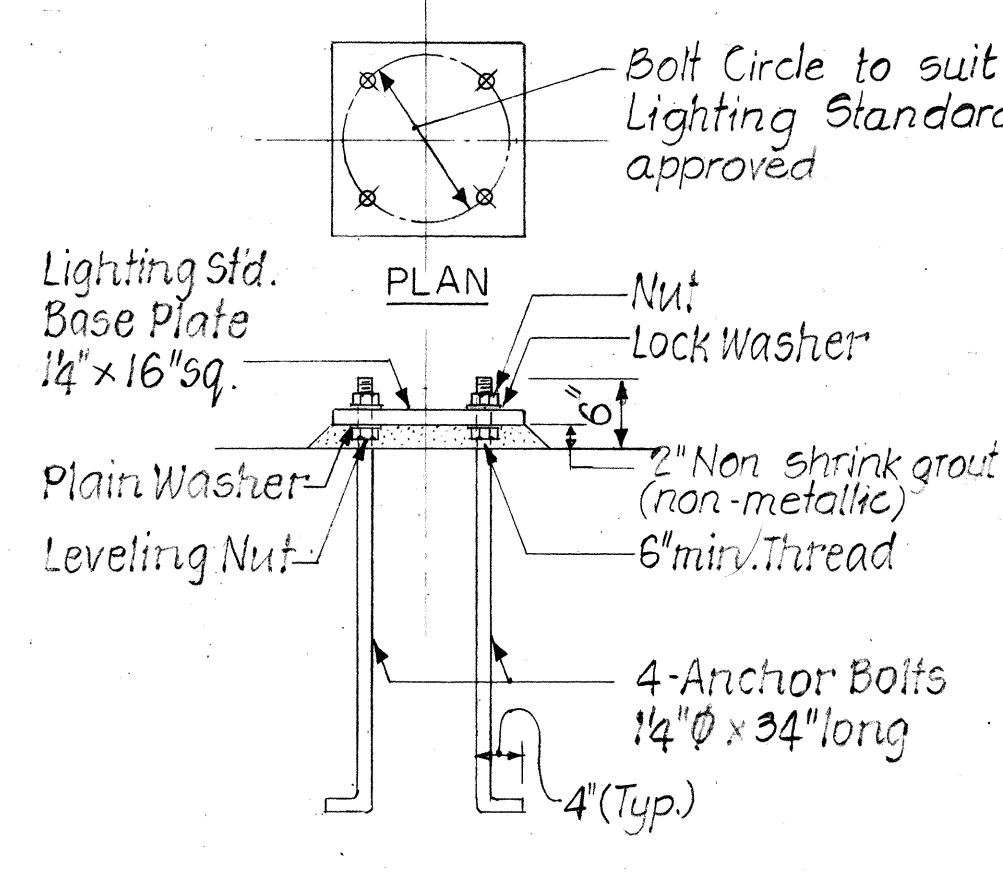
SECTION C
Scale: 1" = 1'-0"



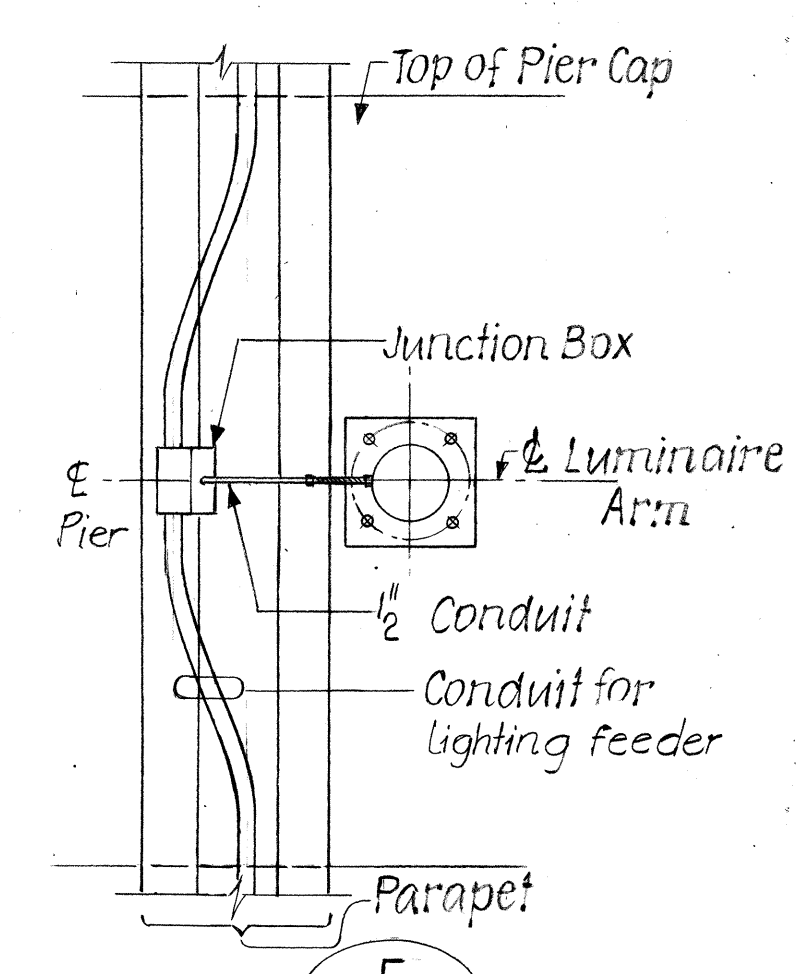
TYPICAL CROSS SLOPE DETAIL FOR DRAINAGE
Scale: 3/4" = 1'-0"



CONNECTION DETAIL
Scale: 1/2" = 1'-0"



ANCHOR BOLT DETAIL
Scale: 3/4" = 1'-0"



SECTION E
Scale: 1/2" = 1'-0"

NOTE:
Anchorage system for Lighting Standard shall be hot-dip galvanized.

VIVAY CHANDRA
REGISTERED PROFESSIONAL ENGINEER
No. 4120
HAWAII, U.S.A.
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.
Signature

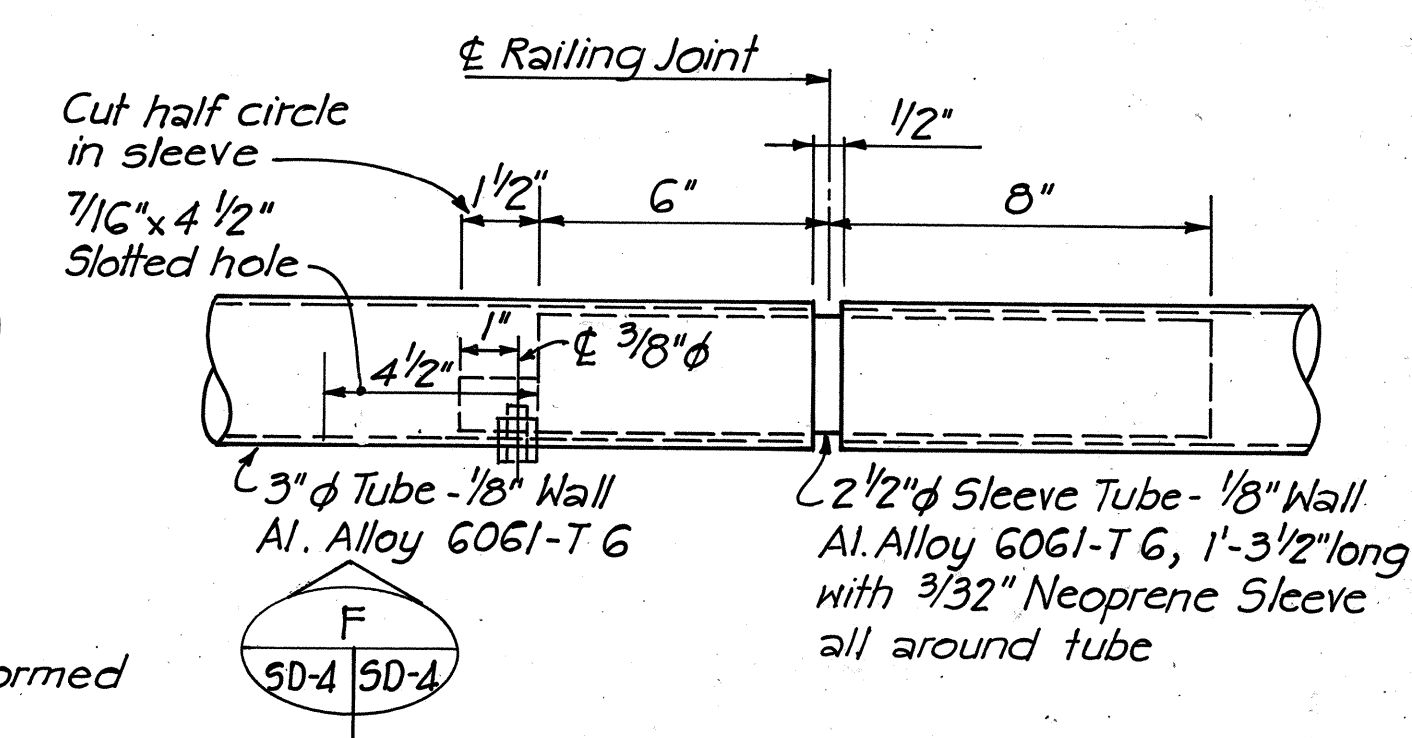
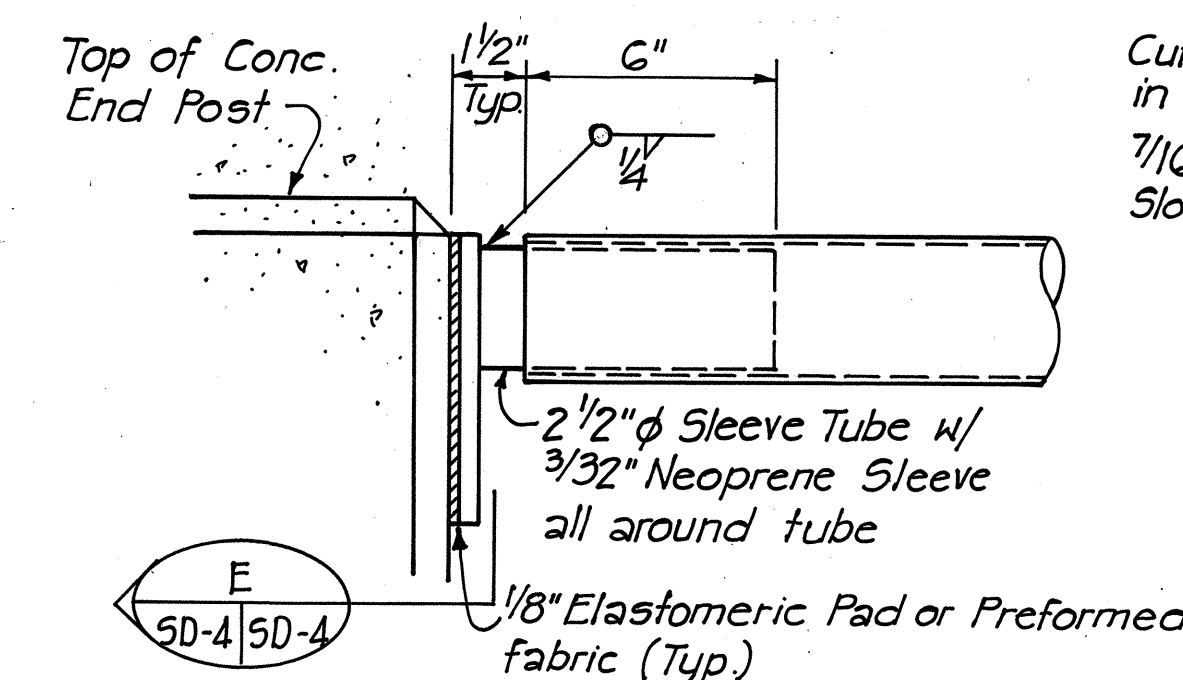
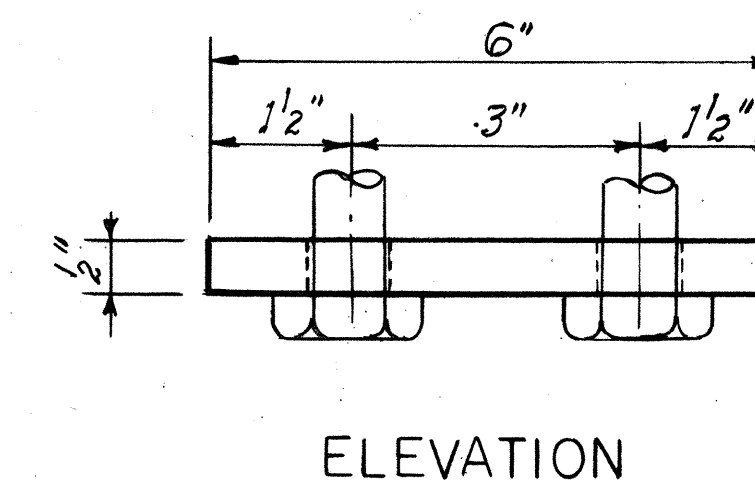
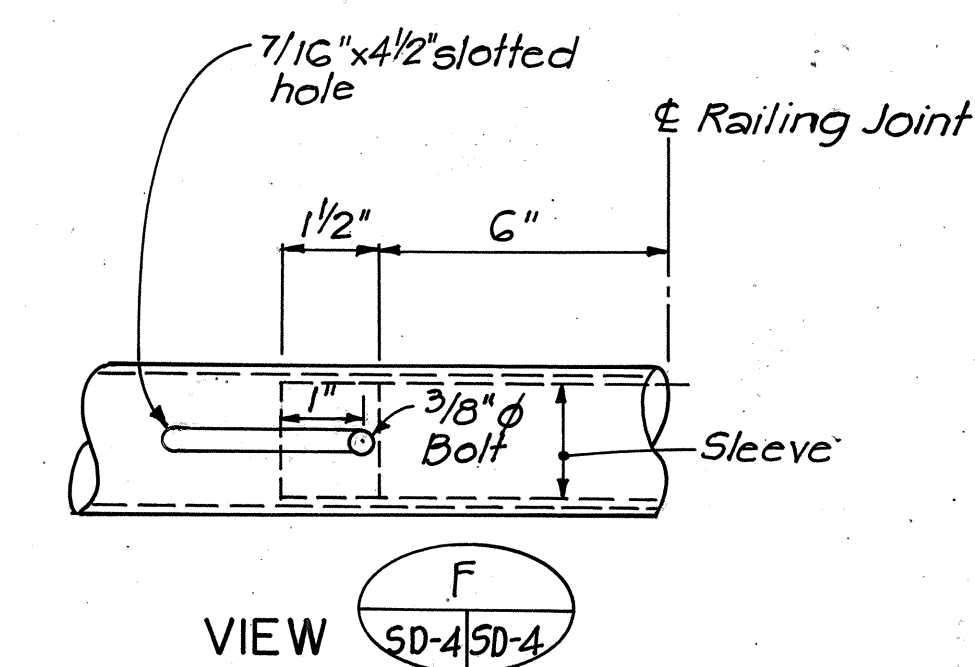
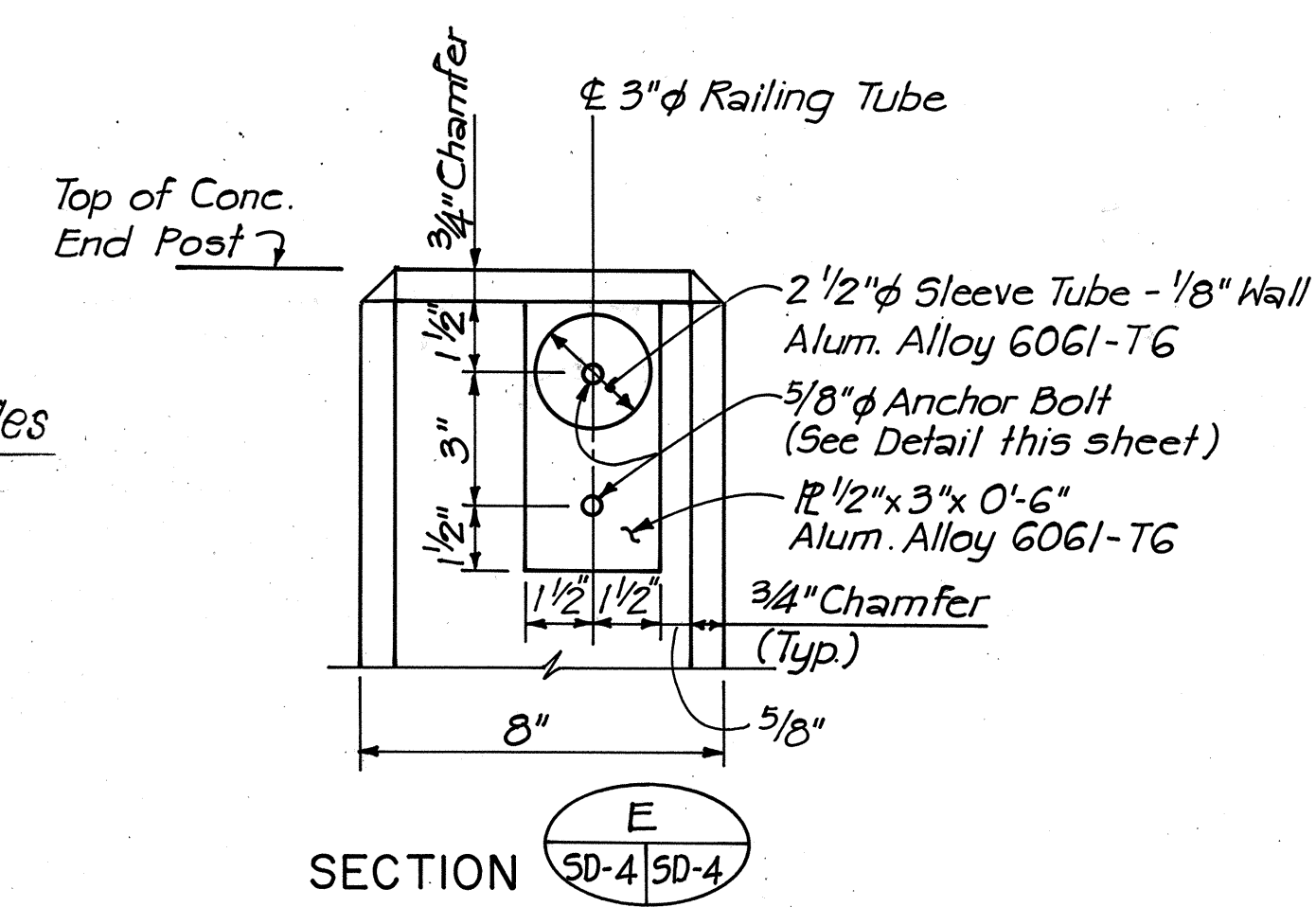
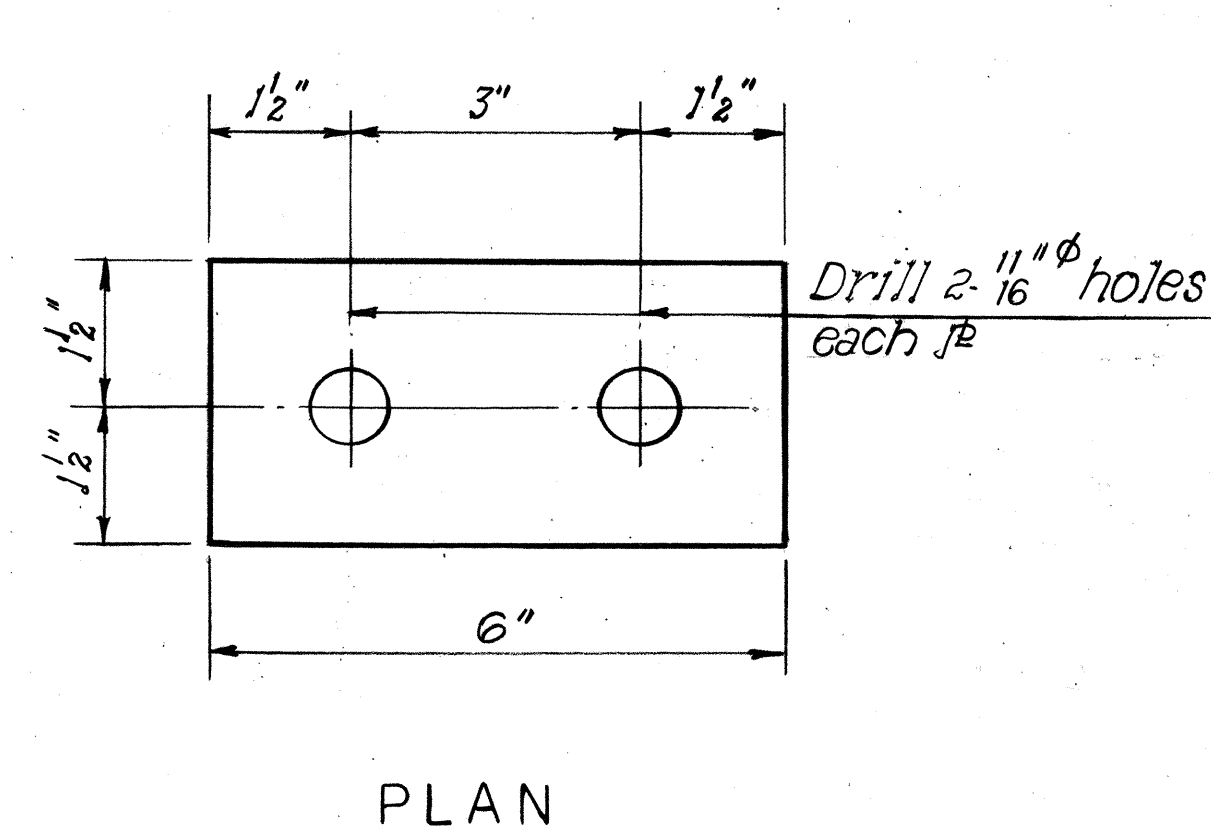
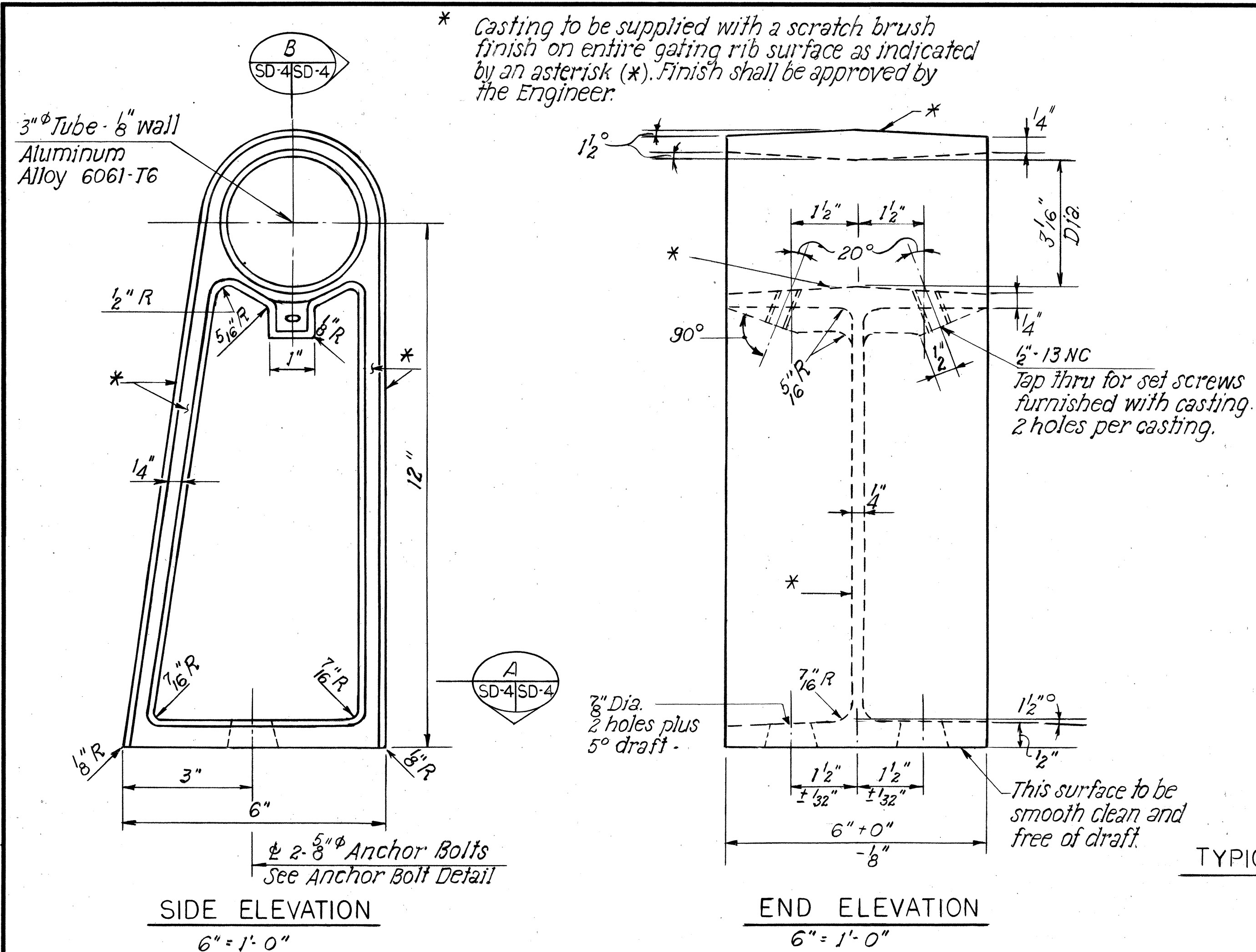
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STANDARD PARAPET DETAILS

KEEHI INTERCHANGE
F.A.I. PROJECT NO. 1-HI-1(131)17
SCALE: AS SHOWN
DATE: AUG. 31, 77

SHEET No. SD-2 OF SD-17 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HM(131):17	1977	335	394

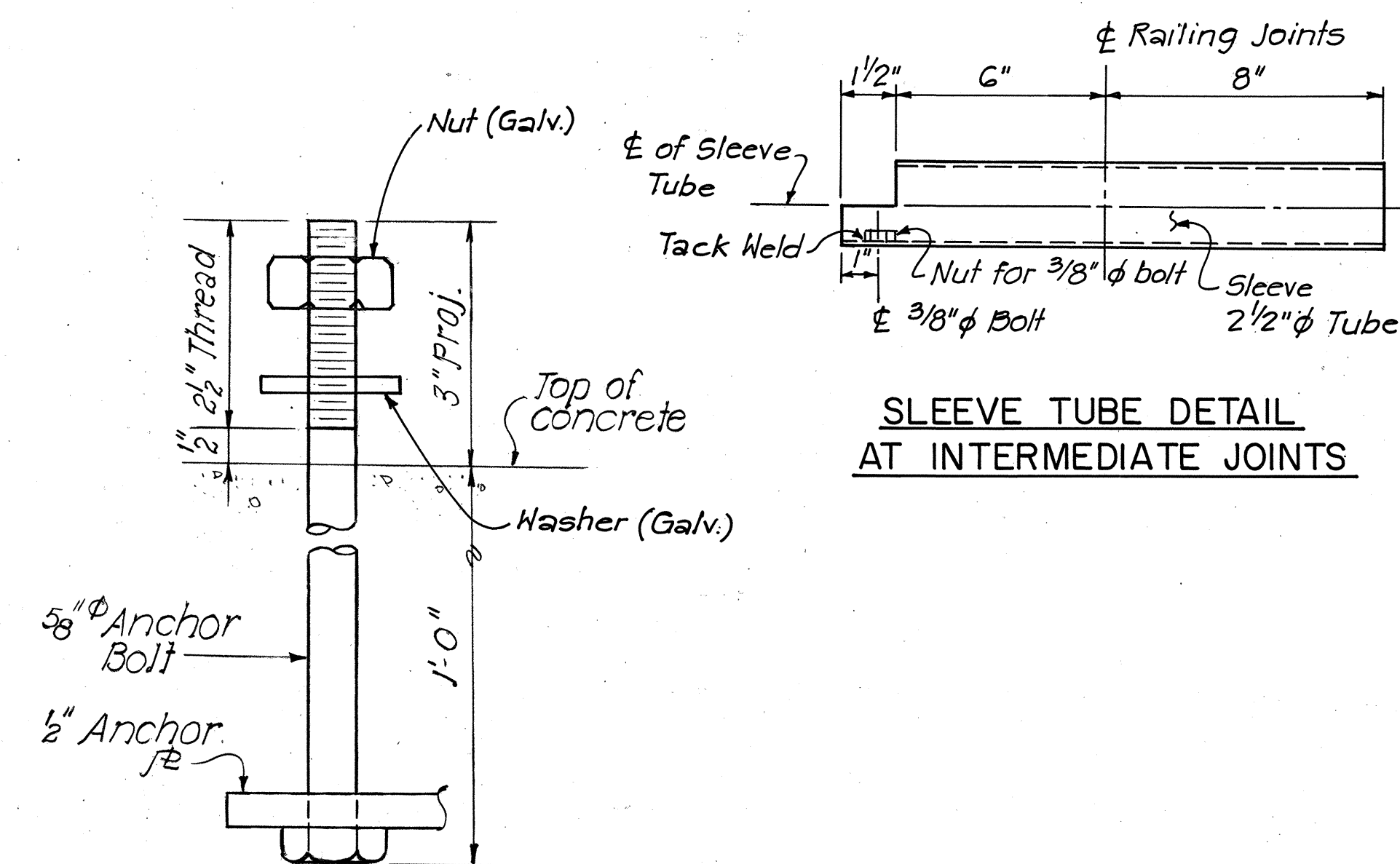
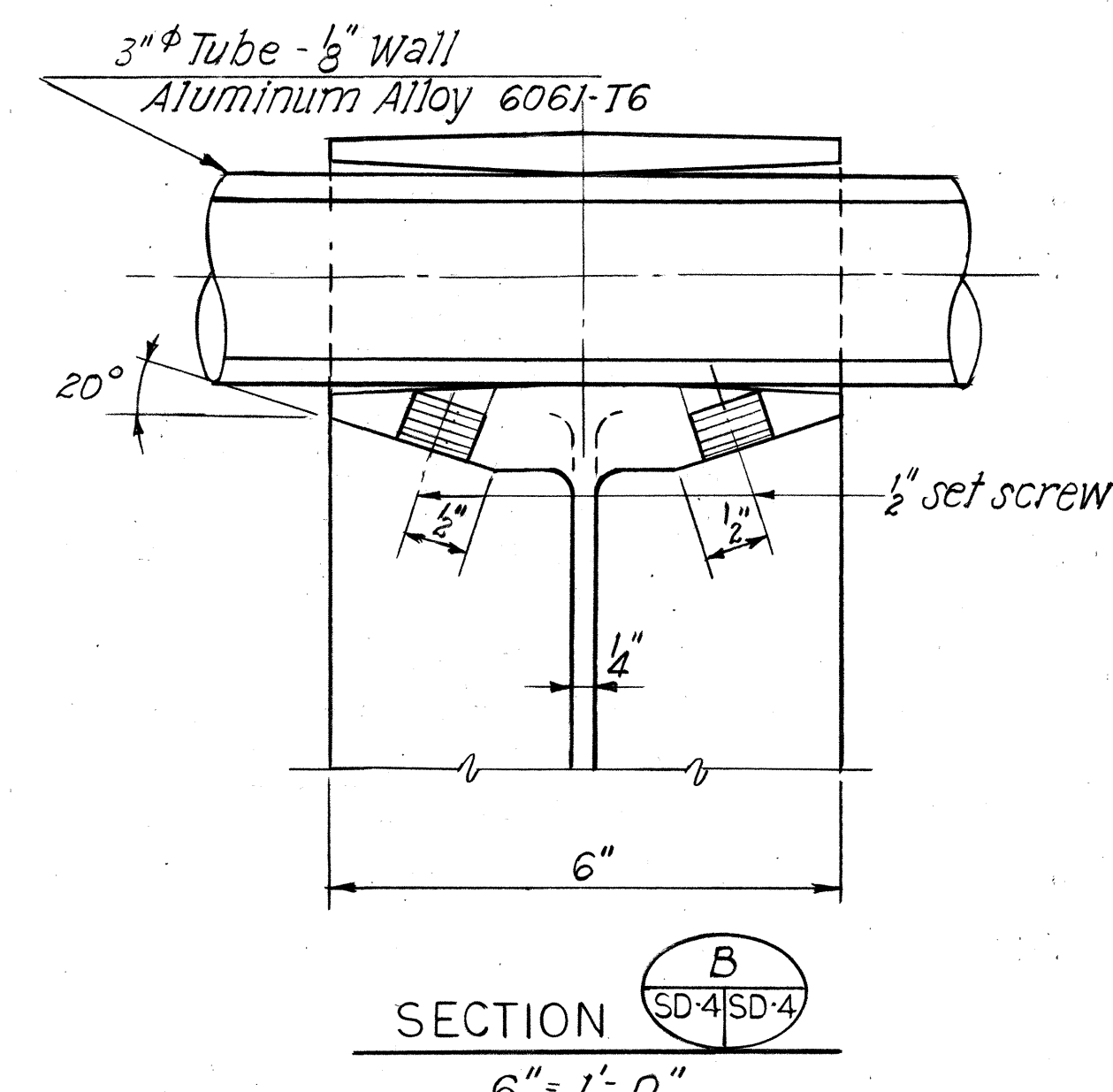
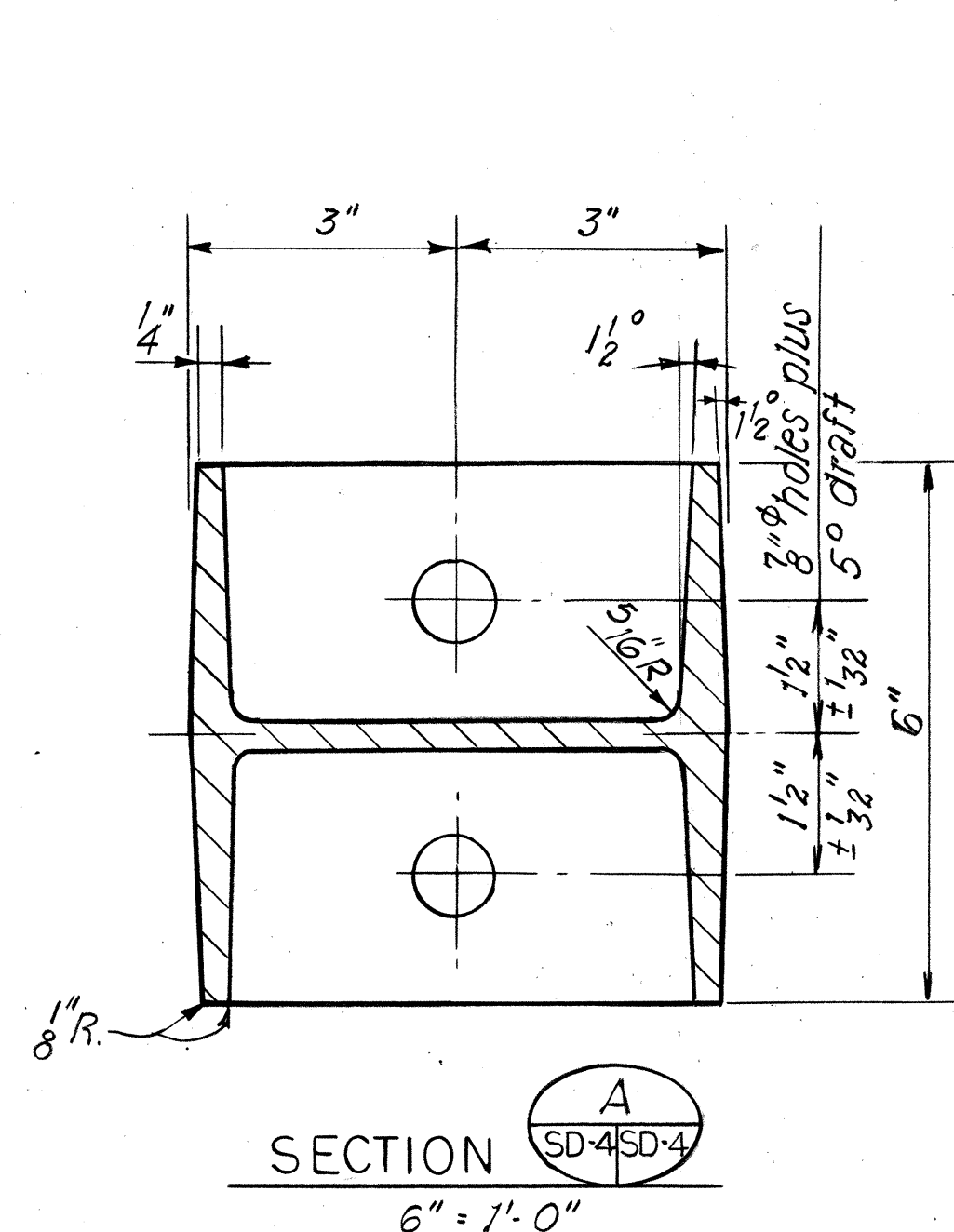


TYPICAL ANCHOR PLATE DETAILS (A36 STEEL)
Scale: 6" = 1'-0"

ELEVATION AT CONCRETE END POST ELEVATION AT INTERMEDIATE JOINTS

DETAIL 

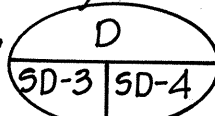
Scale: 3" = 1'-0"



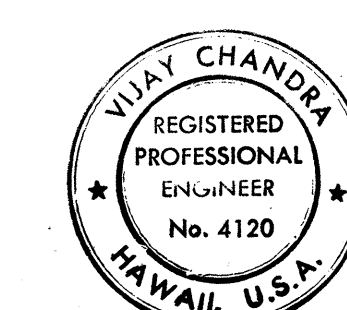
ANCHOR BOLT (Galv. Steel ASTM A325)
(Incidental to Railing)
6" = 1'-0"

SLEEVE TUBE DETAIL AT INTERMEDIATE JOINTS

- NOTES:
- For cast post and rail, see specifications.
 - Unless otherwise specified.

Draft angle	= 3°	Casting tolerance	= $\pm \frac{1}{16}$ "
Corner radii	= $\frac{1}{8}$ " R	Wall thickness	= $\pm \frac{1}{32}$ "
Fillet radii	= $\frac{3}{8}$ " R		
 - Cast post shall be of Aluminum Alloy A344-T4.
 - Elastomeric or Fabric Pad between concrete parapet and bottom of each post shall be incidental to "Metal Bridge Railing".
 - Material shown on Detail  shall be incidental to "Metal Bridge Railing".

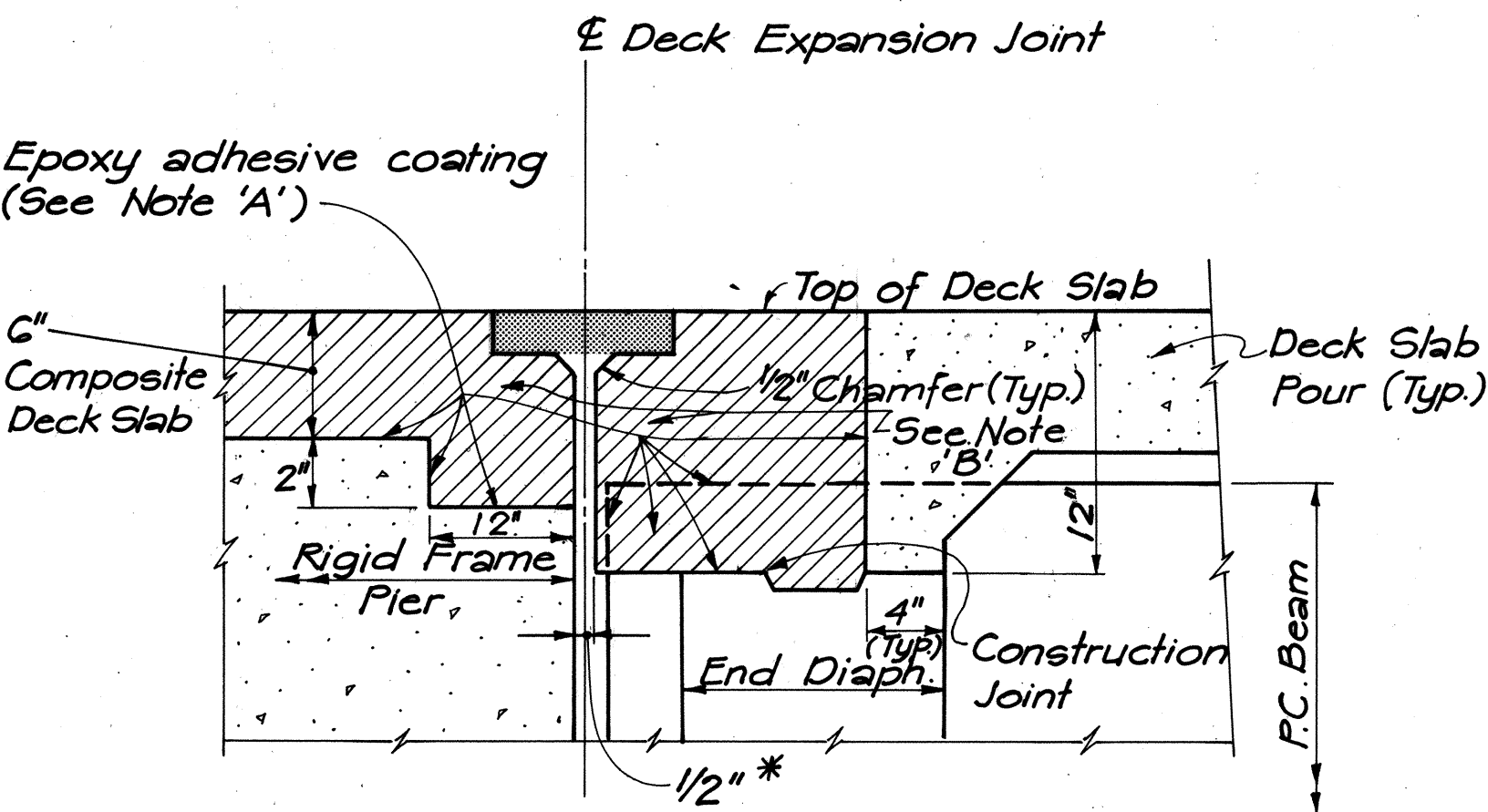
D Detail added. Nut & Washer added in Anchor Bolt. Note 5 added.	
DATE	REVISION
STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION STANDARD RAILING DETAILS KEEHI INTERCHANGE F.A.I. PROJECT NO. 1-HI-1(131):17 SCALE: AS SHOWN DATE: AUG. 3, 77 SHEET NO SD-4 OF SD-17 SHEETS	



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Vijay chandra
Signature

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-HI-1(131)17	1977	337	394

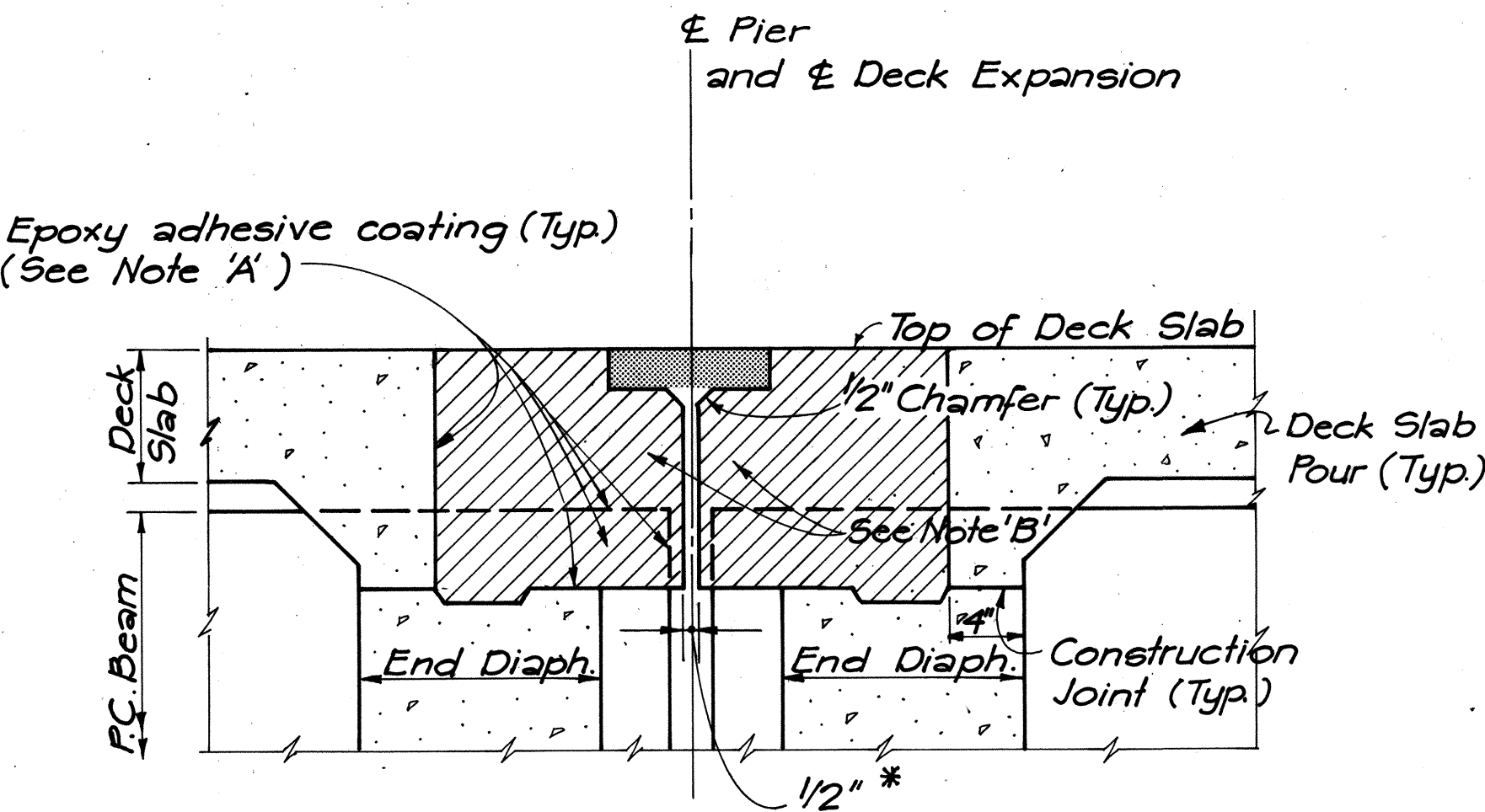


TYPICAL DECK SECTION AT RIGID FRAME
N.T.S.

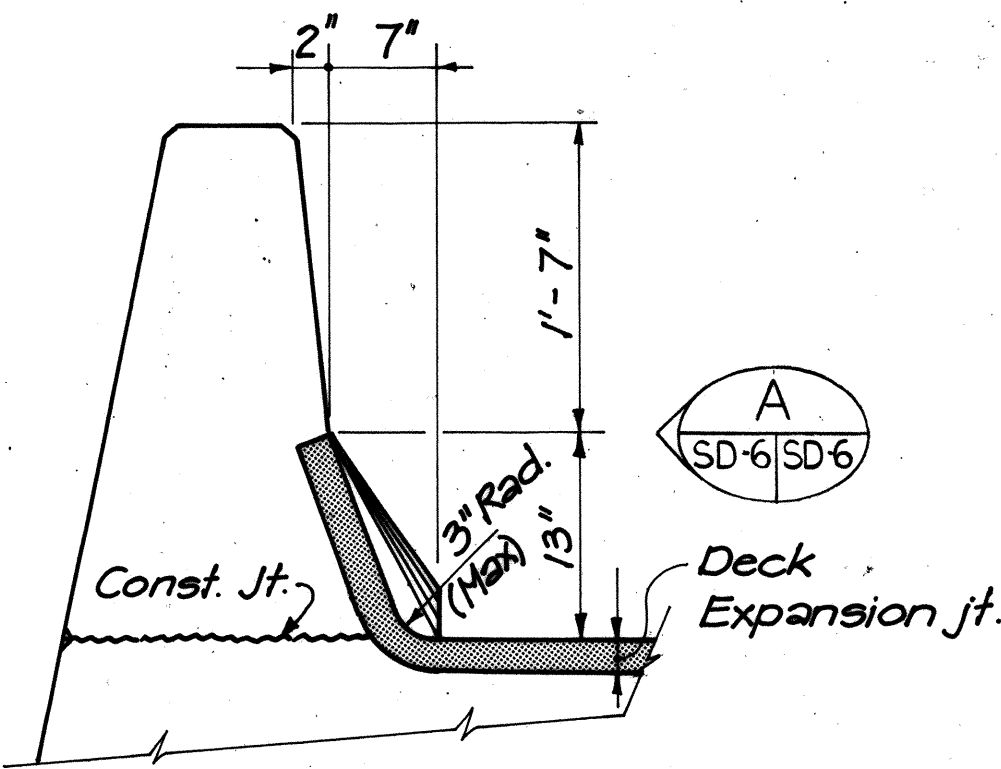
NOTE 'A': Just prior to concreting of the hatched areas, coat the indicated surfaces with an epoxy adhesive. (See Special provision Sec.705.13)

NOTE 'B': Hatched Areas shall be concreted after 28 days of deck slab pour. Anchor devices shall be located and held in their final position, as per Manufacturer's requirement, during the concreting of the hatched areas.

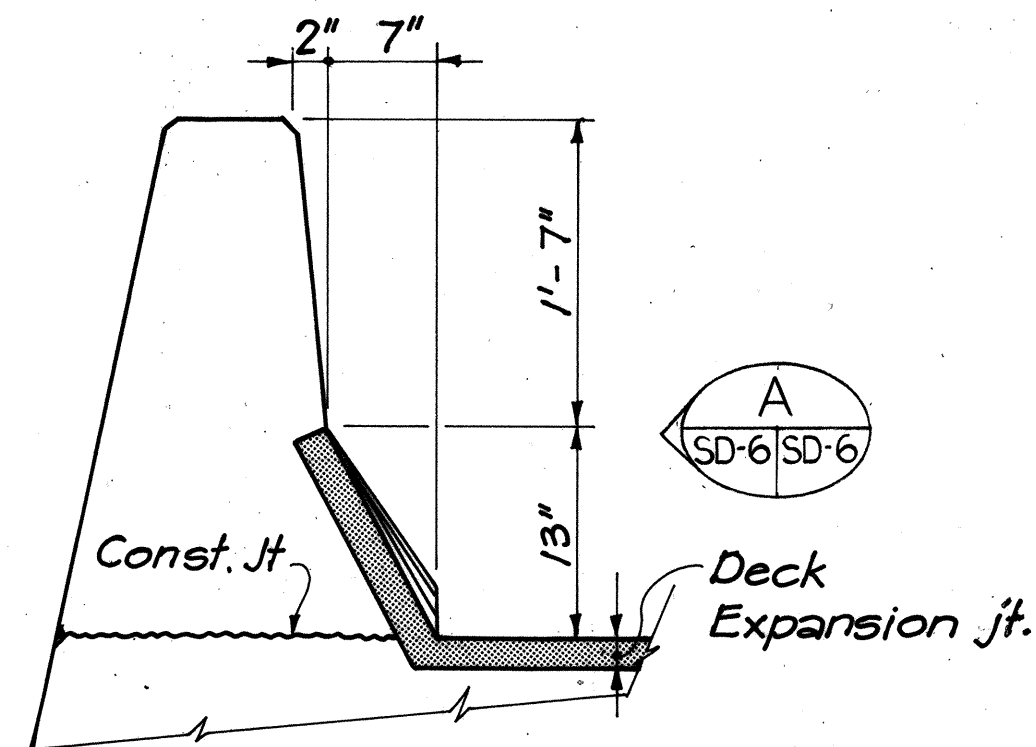
* Joint opening at time of Installation of joint
Joint to be formed by Premolded filler.



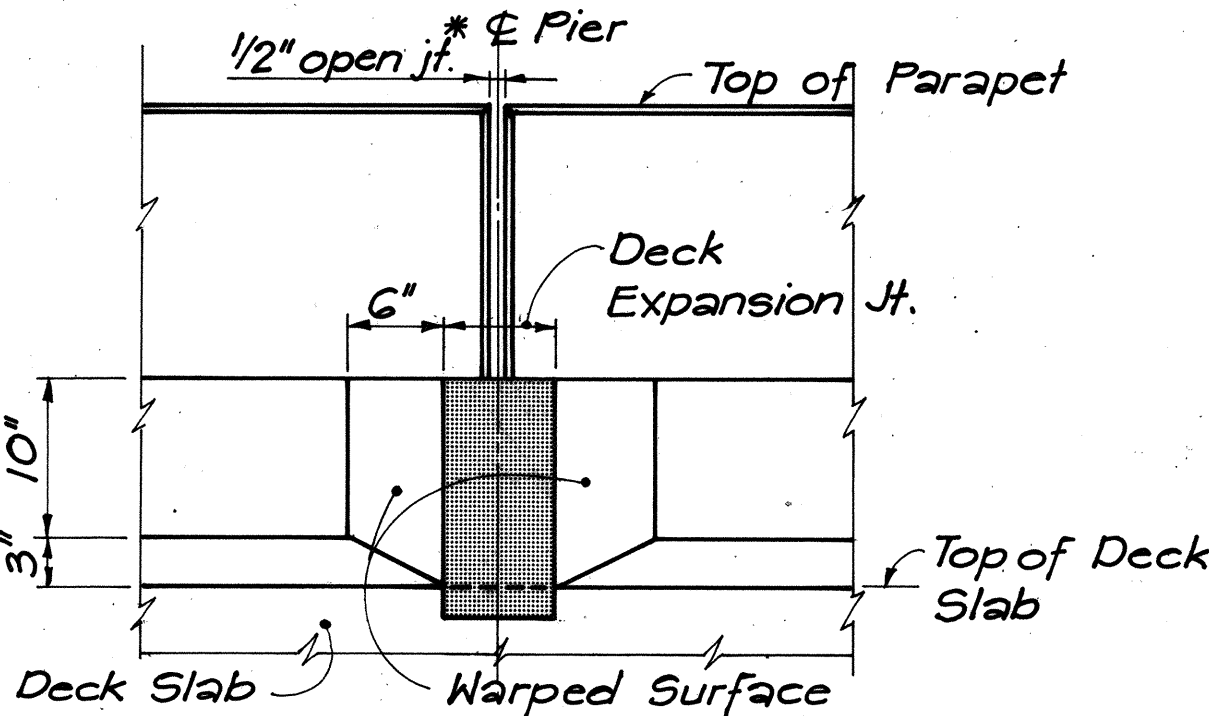
TYPICAL DECK SECTION
(AT ALL PIERS EXCEPT RIGID FRAMES)
N.T.S.



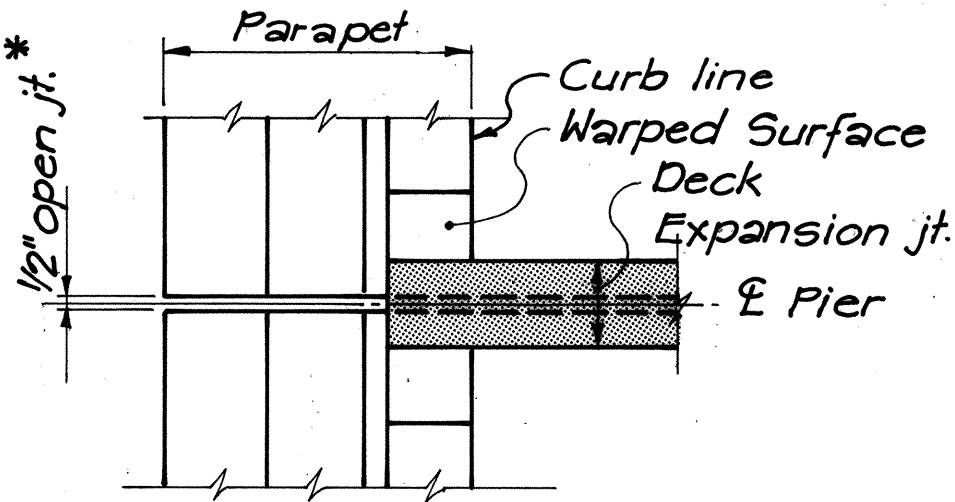
TYPICAL SECTION AT STANDARD PARAPET



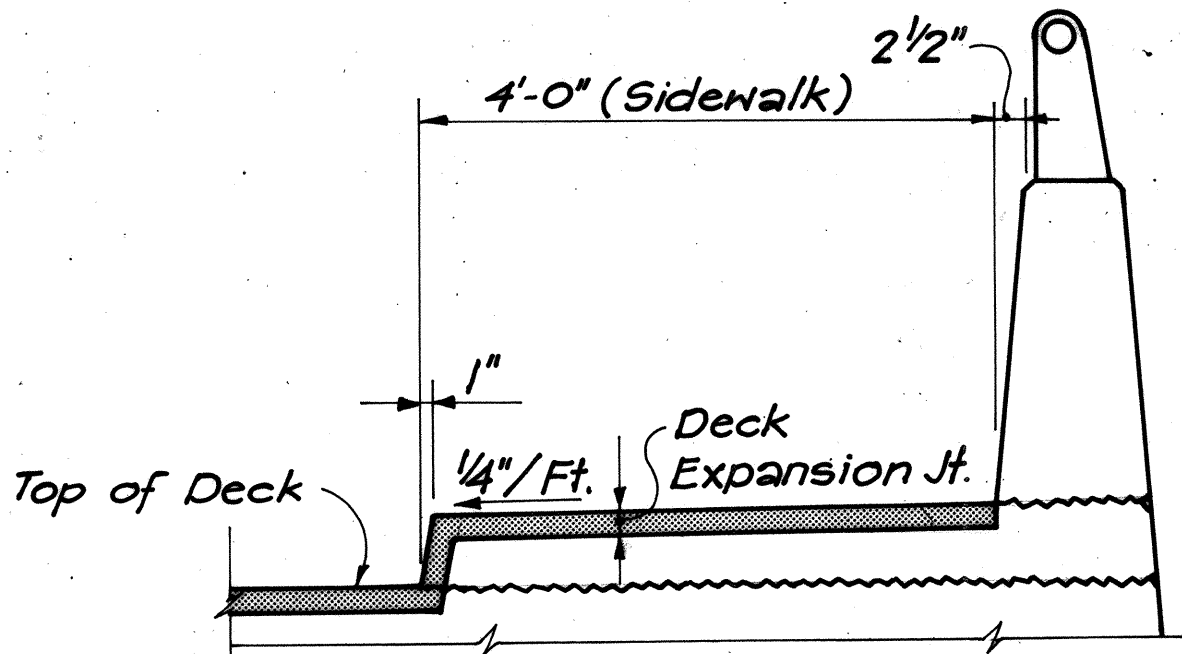
ALTERNATE DETAIL AT PARAPET
N.T.S.



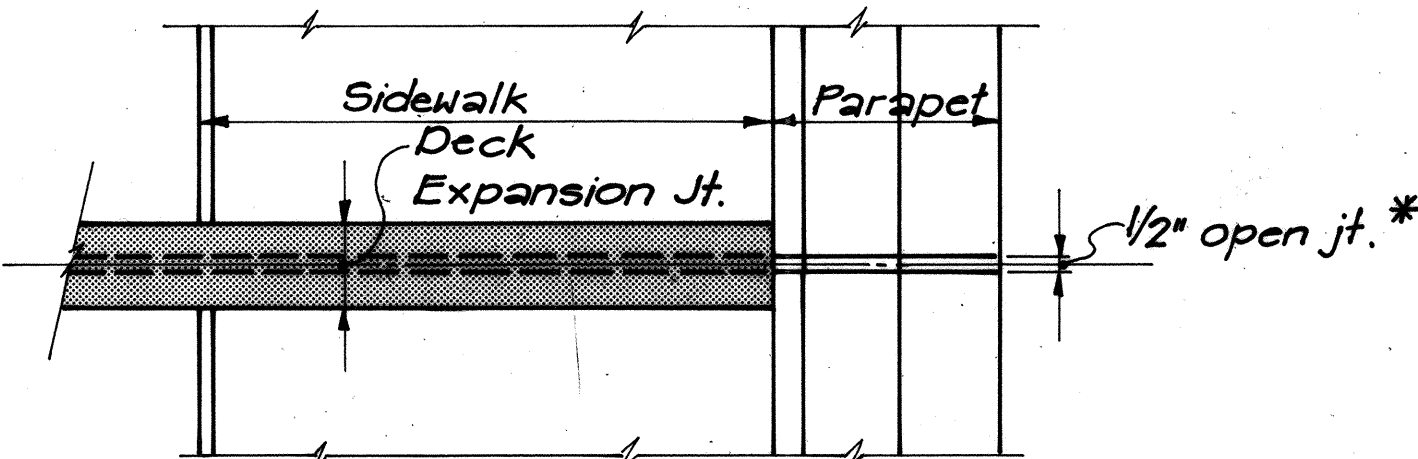
VIEW A
N.T.S. SD-6/SD-6



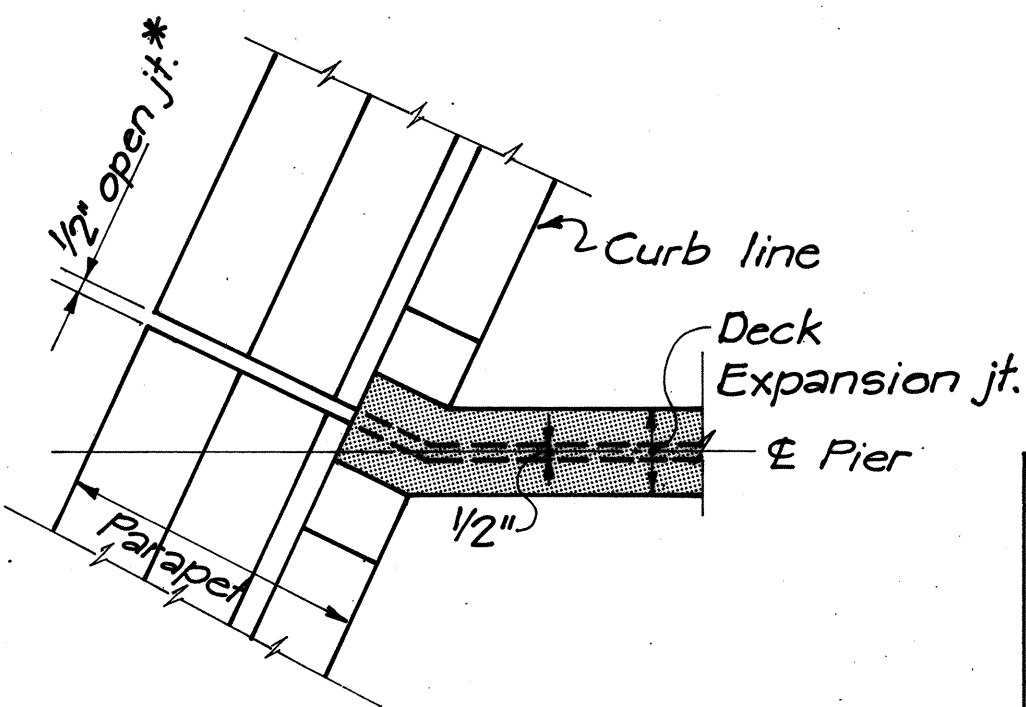
TYPICAL PLAN _ SKEW ANGLE = 0°- 15°
N.T.S.



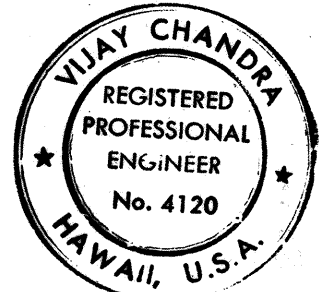
TYPICAL SECTION AT SIDEWALK PARAPET
N.T.S.



TYPICAL PLAN
N.T.S.



TYPICAL PLAN _ SKEW ANGLE = OVER 15°
N.T.S.



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ME OR UNDER MY SUPERVISION.

Vijay Chandra
Signature

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

DECK EXPANSION JOINT
SHEET 1 OF 2

KEEHI INTERCHANGE
F.A.I. PROJECT NO. 1-HI-1 (131) 17
SCALE: N.T.S. DATE: AUG 31, 77

SHEET NoSD-6 OFSD-17 SHEETS

Diagram illustrating the cross-section of a bridge deck slab and its connection to the support structure. The diagram shows the following components and dimensions:

- Aluminum frame (6061-T6):** The main structural member supporting the deck slab.
- Neoprene seal:**:** A sealant used to prevent water ingress at the joint.
- Top of deck slab:** The upper surface of the concrete deck.
- Aluminum frame contact surface w/ conc. to be coated w/ bitumastic compound:** The surface of the aluminum frame in contact with the concrete, which should be coated with a bitumastic compound.
- 2 1/2" ϕ x 6" long tie bolt @ 6" o.c. (Min.):** Reinforcing bars (tie bolts) used to secure the concrete deck to the aluminum frame.
- Dimensions:**
 - Overall width: 8 1/8" *
 - Flange width (each side): 2 1/8"
 - Web thickness: 1/4" *
 - Deck slab thickness: 5 11/16"
 - Frame height: 2"

Technical drawing illustrating the cross-section of a deck slab assembly, showing the installation of a neoprene block and a bendoflex gland.

Dimensions:

- Top of Deck Slab: 5" (left section), 1 1/2" (gland area), 5" (right section).
- Neoprene Block: 1 1/2" x 5" x 6'-0" Standard Section.
- Top of Deck Slab: 1 1/2" (left section), 1/8" (right section).

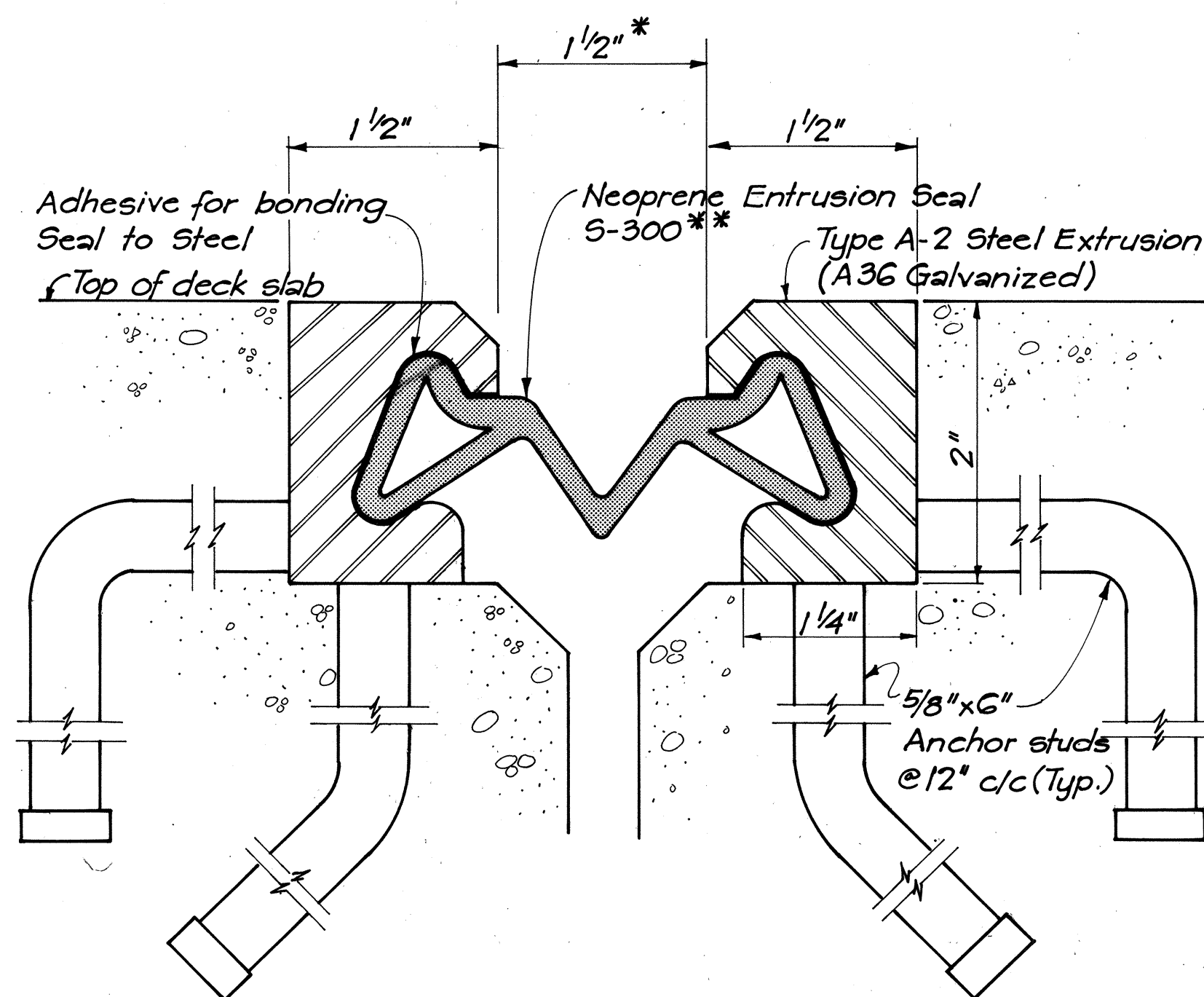
Components and Materials:

- Neoprene Block (1 1/2" x 5" x 6'-0" Standard Section, 75 Durometer).
- Bolt w/ lock washer (as per Manufacturer's requirement).
- Bedding Tape.
- Cast in Place high strength insert @ 12" o.c.
- Bendoflex gland **.
- Fabric.

Notes:

- Surface to be treated with a Sealant before placing the gland.
- Blocks to be connected to each other w/ a tongue and groove connection.
- Bolt hole @ 12" o.c.

* = Installation width @ 75°F
** = Seal/Gland shall be continuous across the full width of the Structure



Technical drawing of a cross-section of a bridge deck showing the assembly of a bolt hole. The drawing includes dimensions and labels for various components:

- Dimensions:**
 - Overall width: $4\frac{1}{4}"$
 - Distance from centerline to edge: $2\frac{7}{8}"$
 - Distance from centerline to bolt hole center: $1\frac{1}{8}"$
 - Distance from bolt hole center to edge: $2\frac{7}{8}"$
 - Distance from centerline to steel channel: $1\frac{1}{4}"$
 - Distance from steel channel to edge: $1\frac{1}{4}"$
 - Distance from steel channel to bolt hole center: $1\frac{5}{8}"$ (Typ.)
 - Distance from steel channel to edge: $2\frac{1}{2}" \times 5\frac{1}{8}" \times \frac{3}{16}"$
- Labels:**
 - Tread Pattern Slots** $\frac{1}{4}"$ wide x $\frac{1}{8}"$ deep
 - Neoprene dam Pad**
 - Bolt w/ lockwasher** (as per Manufacturer's requirement)
 - Top of Deck Slab**
 - 90 Durometer Neoprene Rubber** surrounding bolt holes
 - Neoprene dam pad including Convolution, Flap and Steel Channel** to be an integrally molded Unit
 - Steel channel bonded to rubber** $2\frac{1}{2}" \times 5\frac{1}{8}" \times \frac{3}{16}"$

FEL-SPAN TYPE T-20
 Manufacturer : FEL-PRO Incorporated)

NOTE: The entire assembly to be factory vulcanized into 8'-0" (Min.) long units before shipping. The convolution flap between units to be lapped and field vulcanized.

Four alternative Deck Expansion Joint Systems are shown. Contractor has the option of selecting any one of the systems. The Contractor shall submit detailed shop drawings of the selected joint to the Engineer for approval.

Contractor used On-Flex 25 deck expansion joint system.



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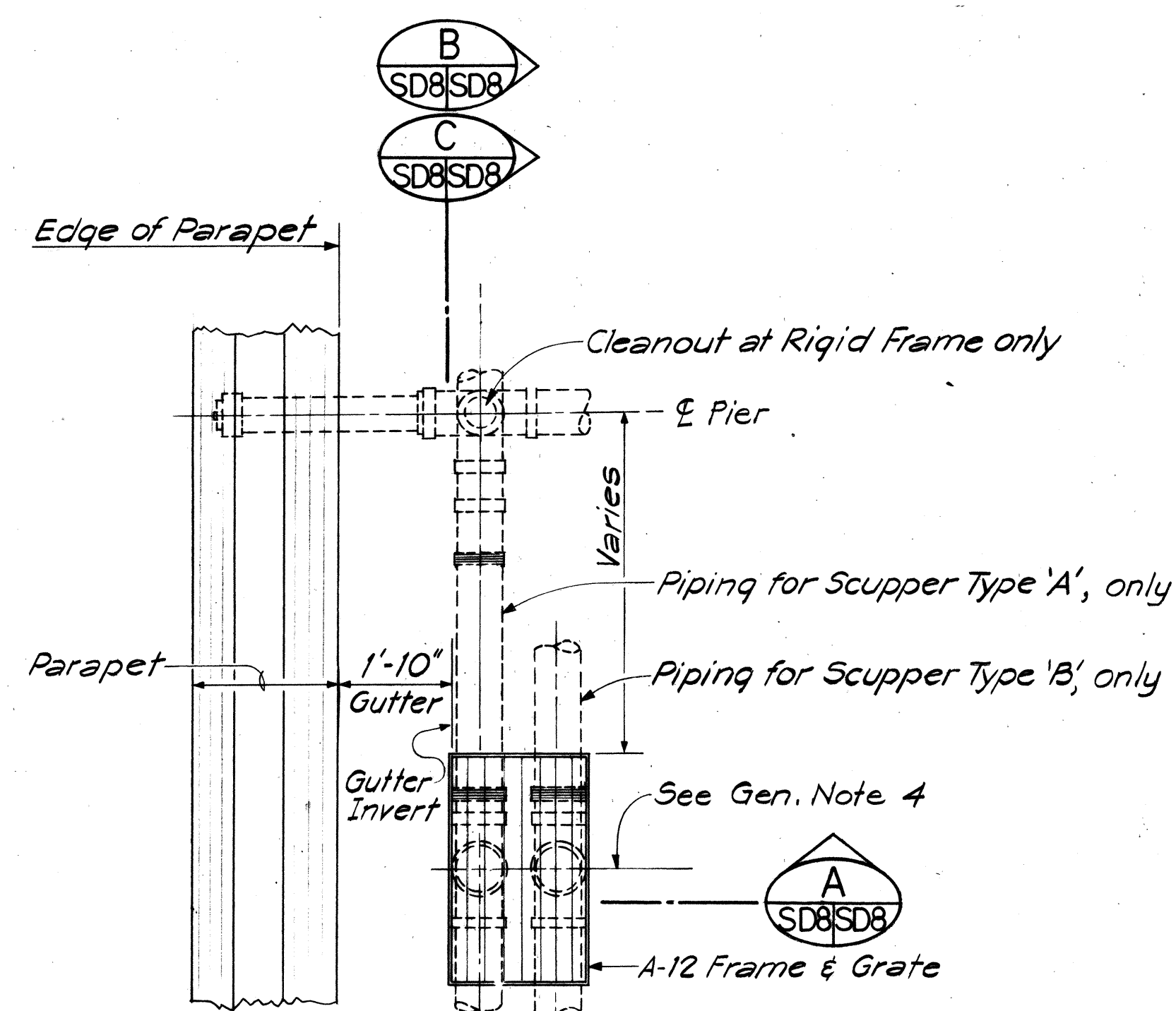
Vijay chand
Signature

DECK EXPANSION JOINT
SHEET 2 OF 2

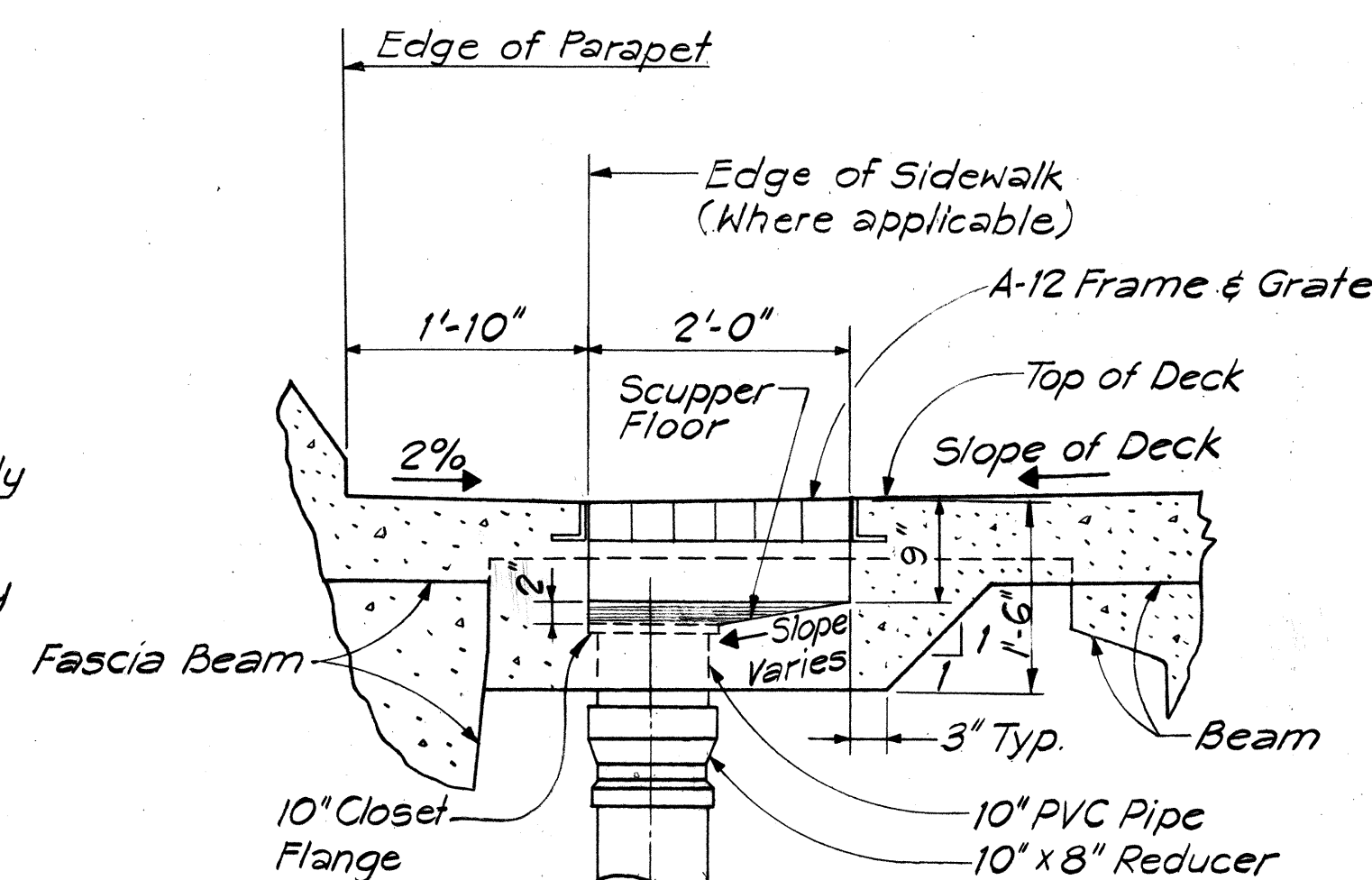
KEEHI INTERCHANGE
F.A.I. PROJECT NO. 1-HI-1 (131) : 17
SCALE : N.T.S. DATE : AUG. 31. 77

SHEET No.SD-7 OFSD-17 SHEETS

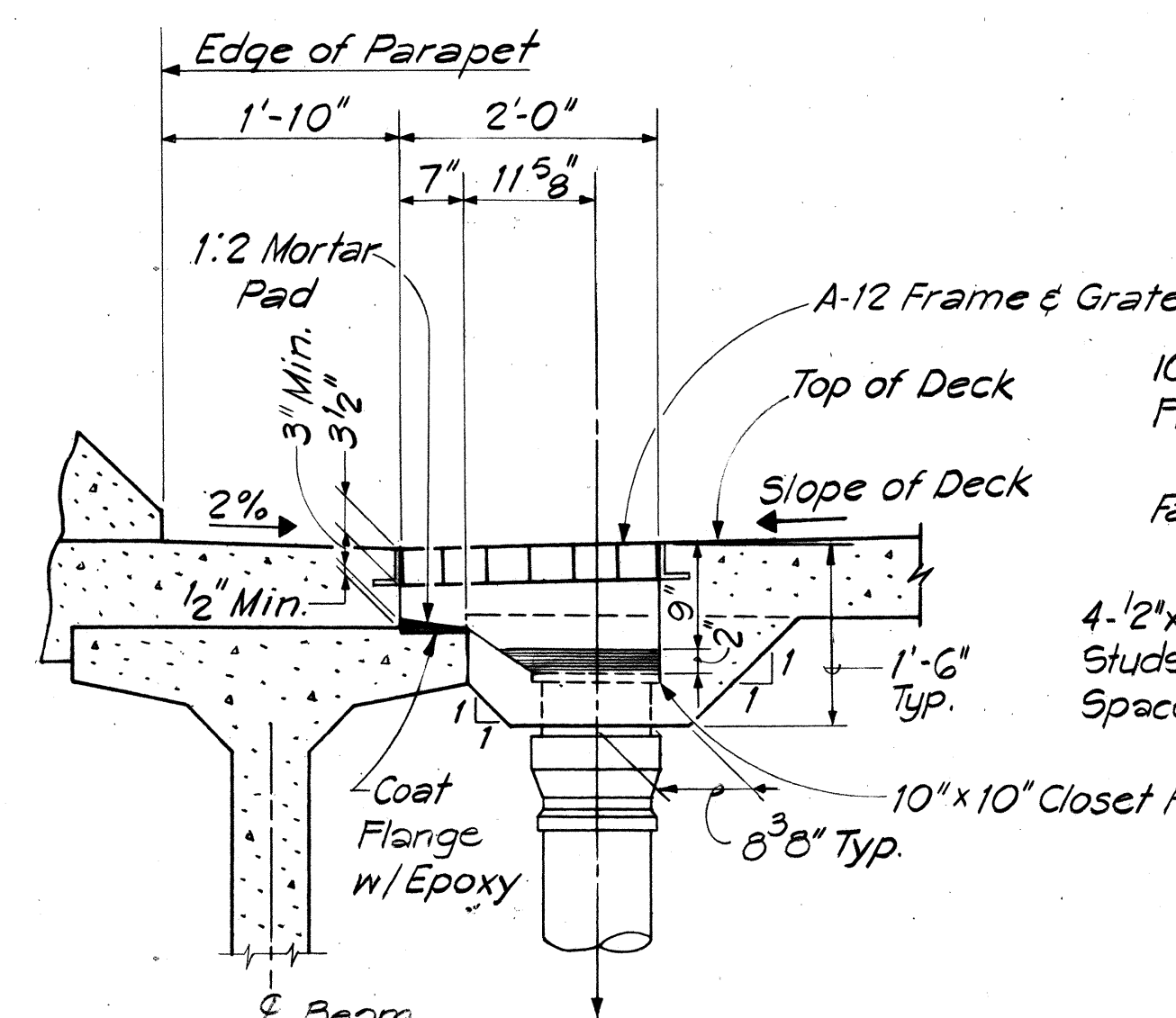
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-H11(131)17	1977	339	394



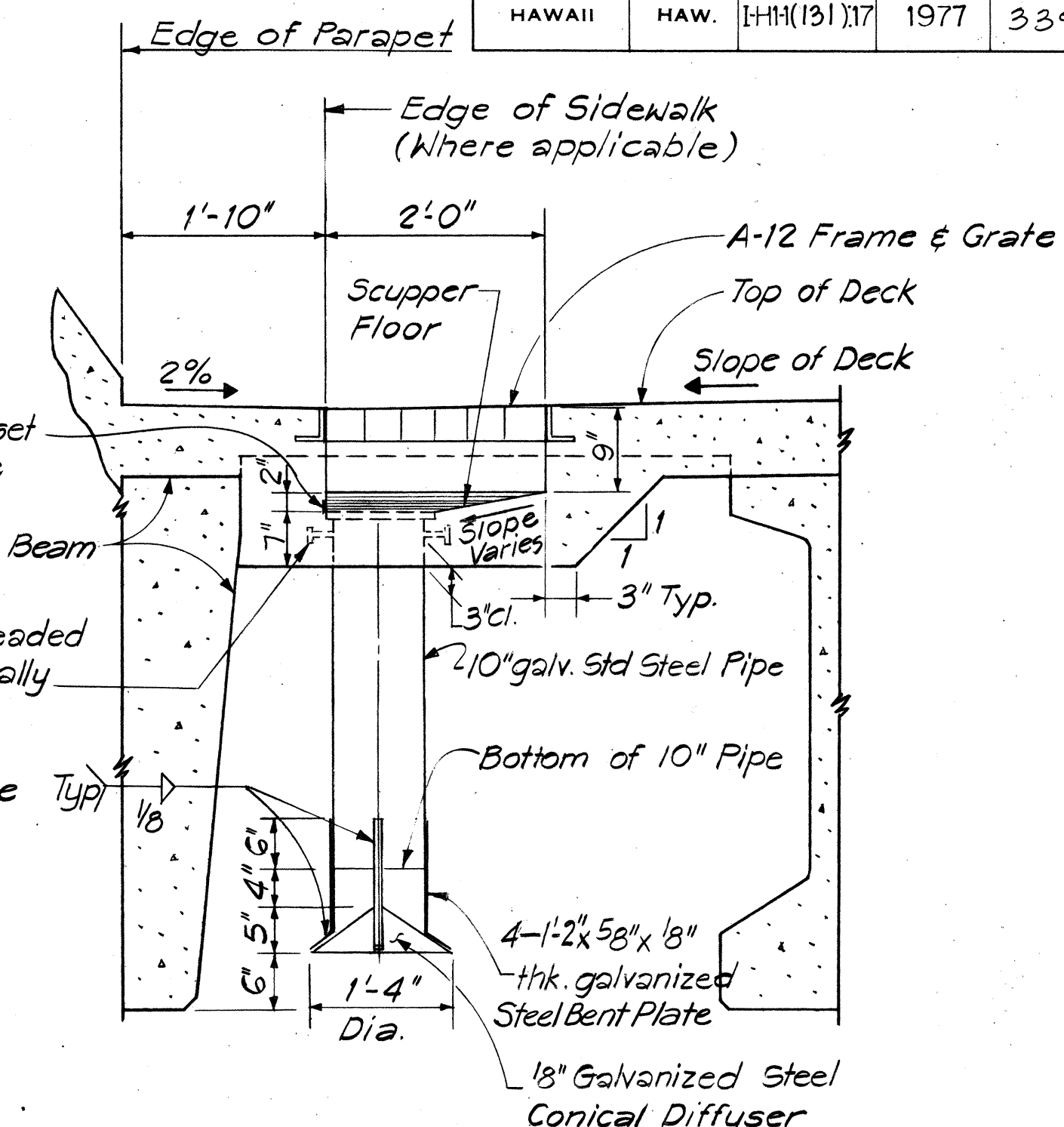
PLAN - SCUPPER
SCALE: 1/2"=1'-0"



SECTION 
SCUPPER TYPE 'A'
SCALE: 3/4"=1'-0"

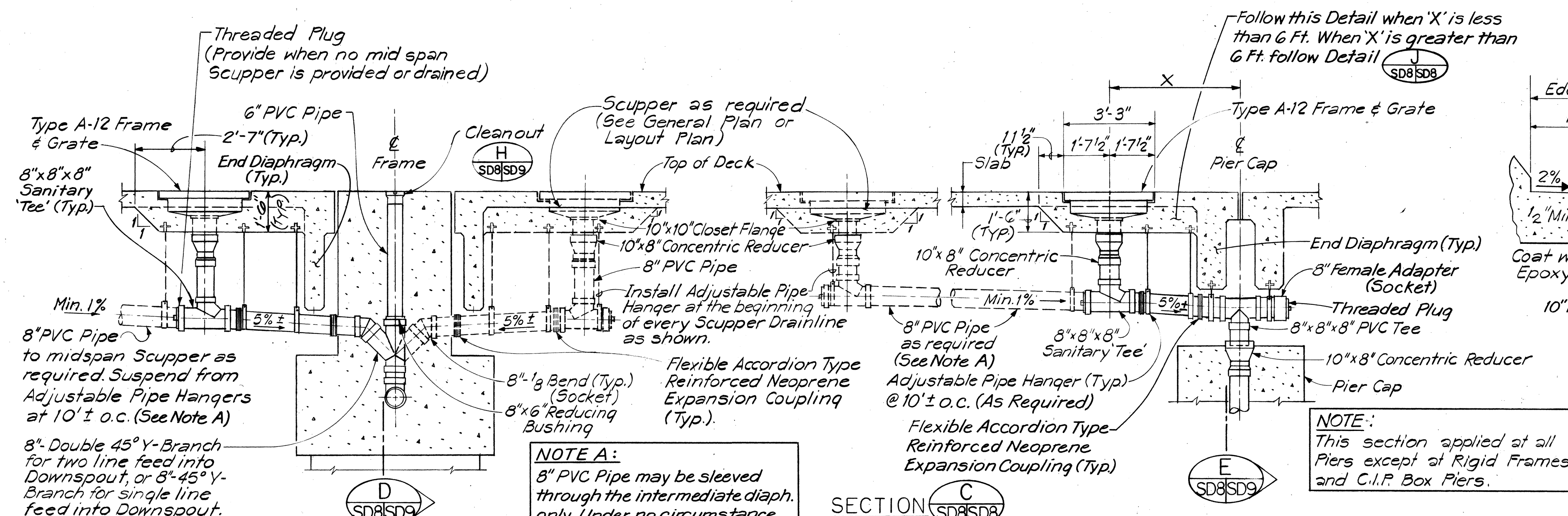


SECTION  SCUPPER TYPE 'B'
SCALE 3/4"=1'-0"

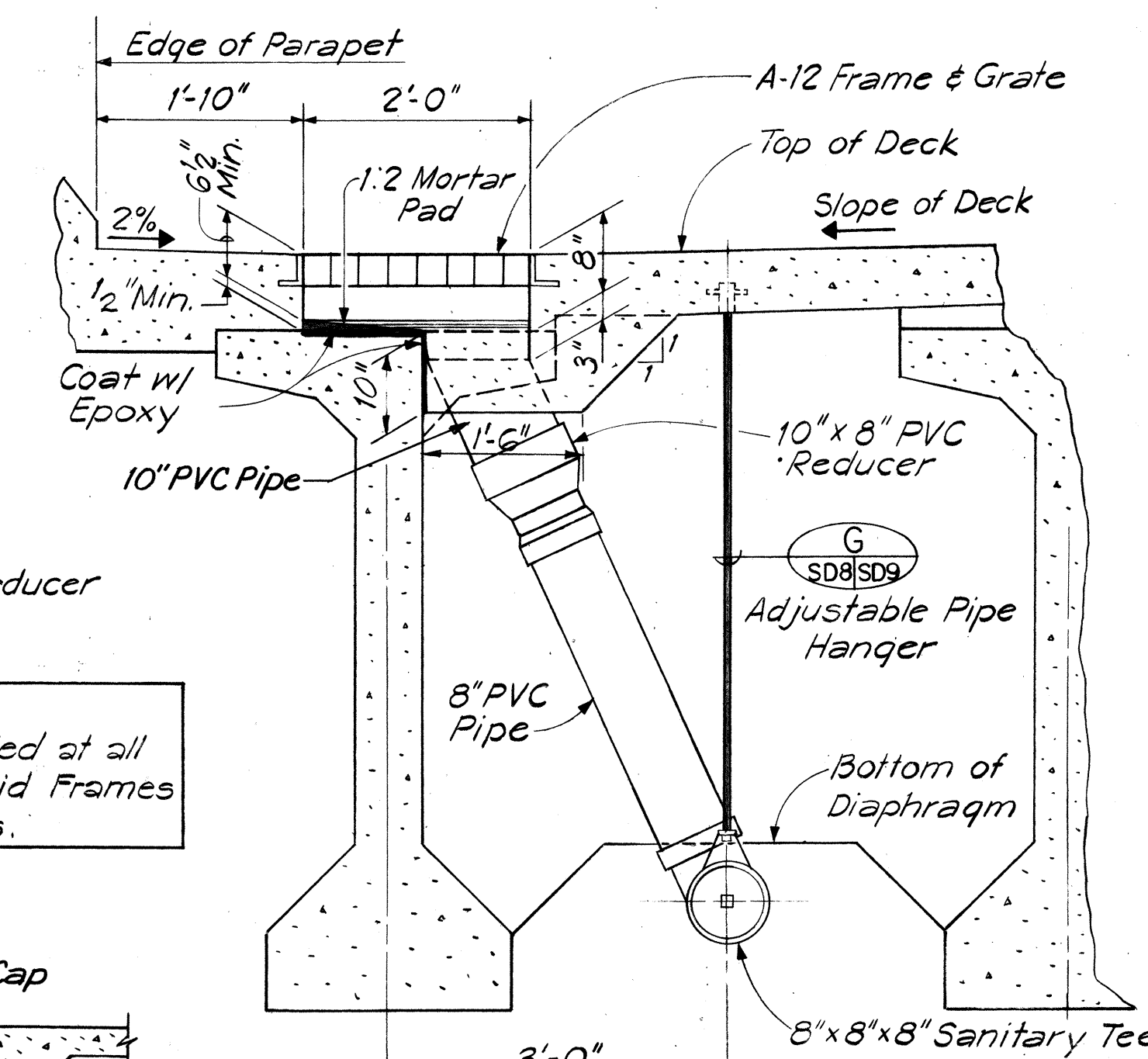
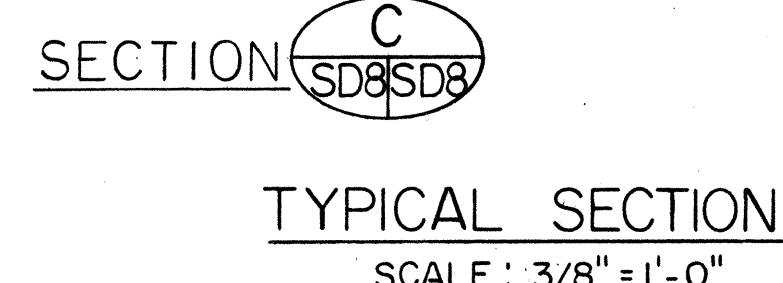


SCUPPER TYPE 'C'

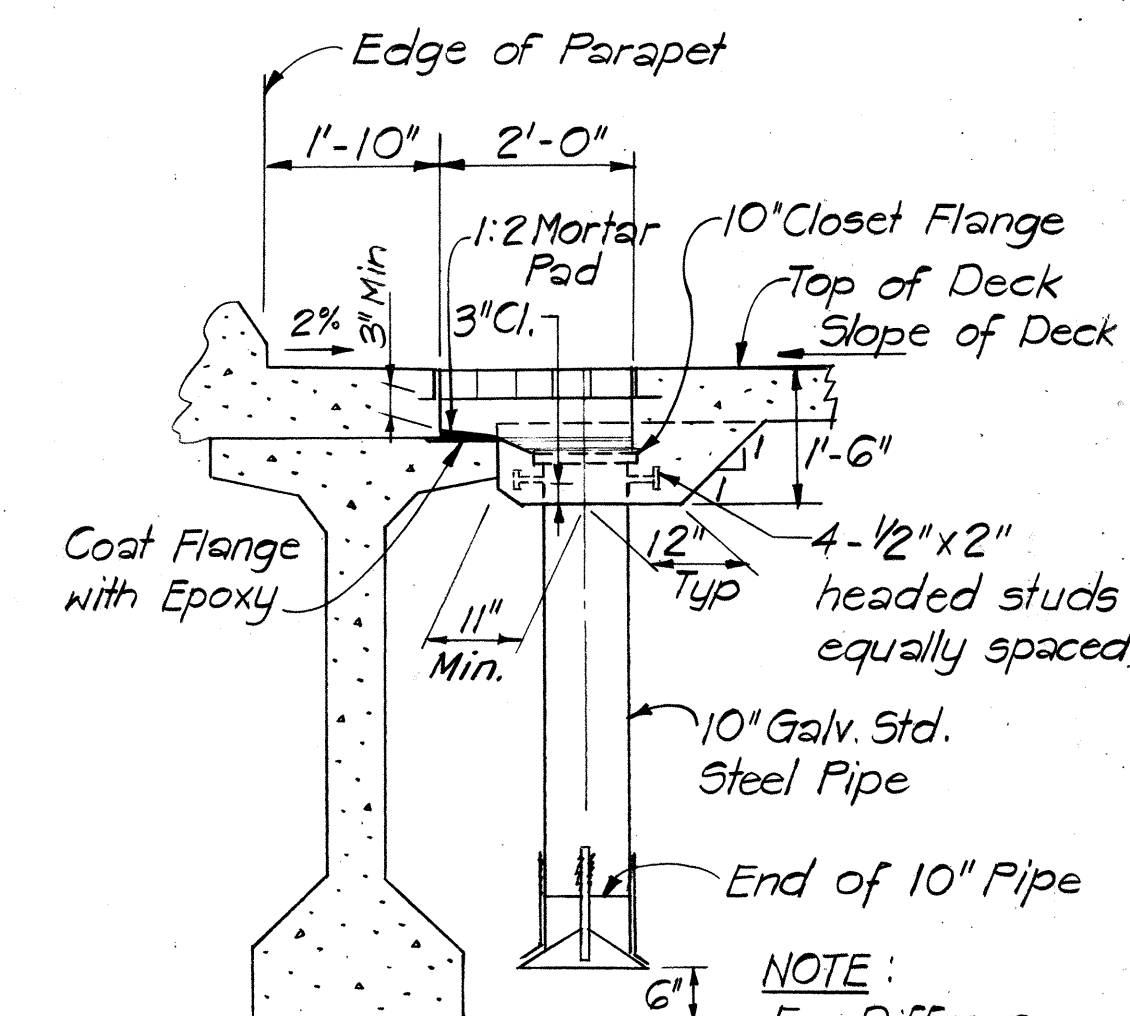
SCALE: 3/4" = 1' - 0"



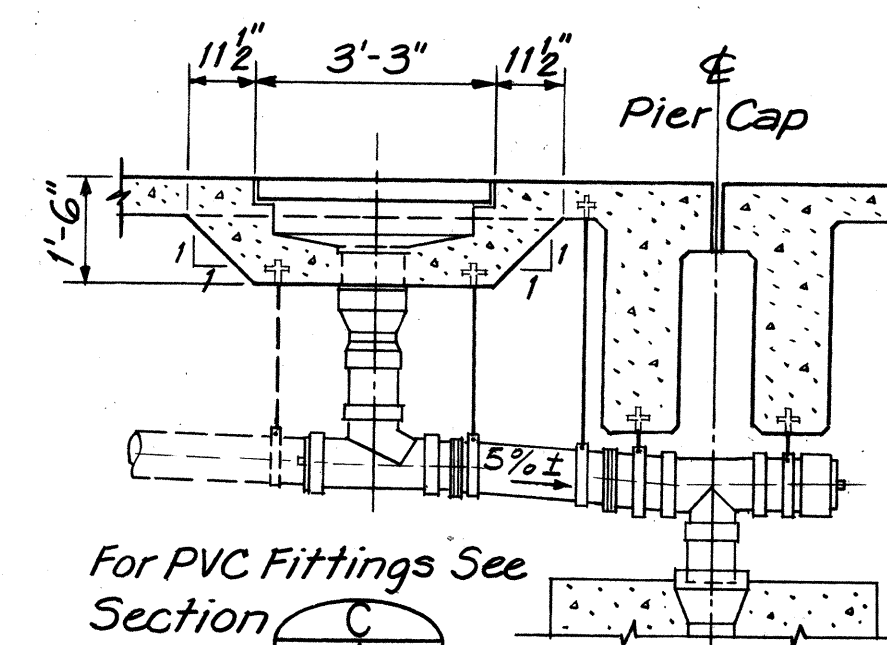
SECTION  shall the 8'
through the
TYPICAL SECTION (AT RIGID FRAME PIERS)
SCALE : 3/8" = 1'-0"



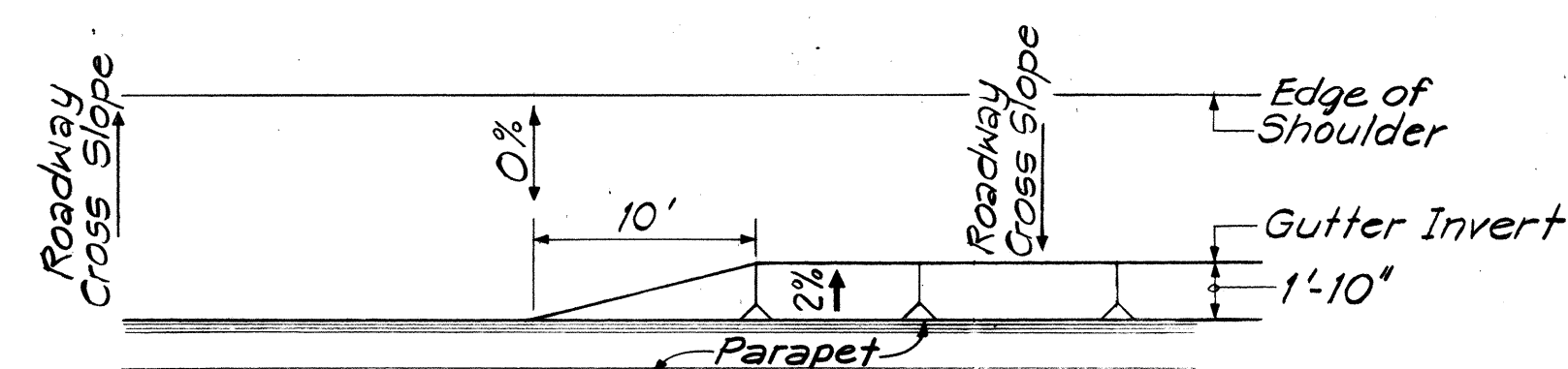
SCUPPER TYPE 'D'



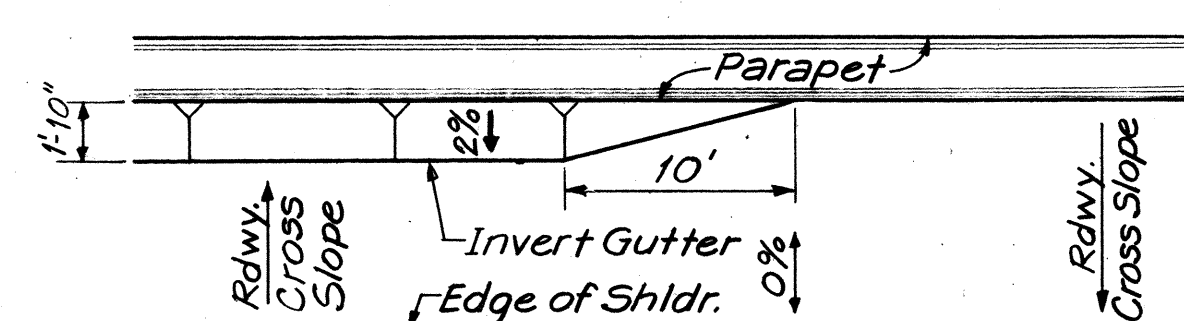
SCUPPER TYPE 'E'
SCALE: 1/2" = 1'-0"



DETAIL 
SCALE: 3/8"=1'-0"



TYPICAL GUTTER TRANSITION AT ROLLOVER
N.T.S.



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE DRAINAGE DETAILS
SHEET 1 OF 2

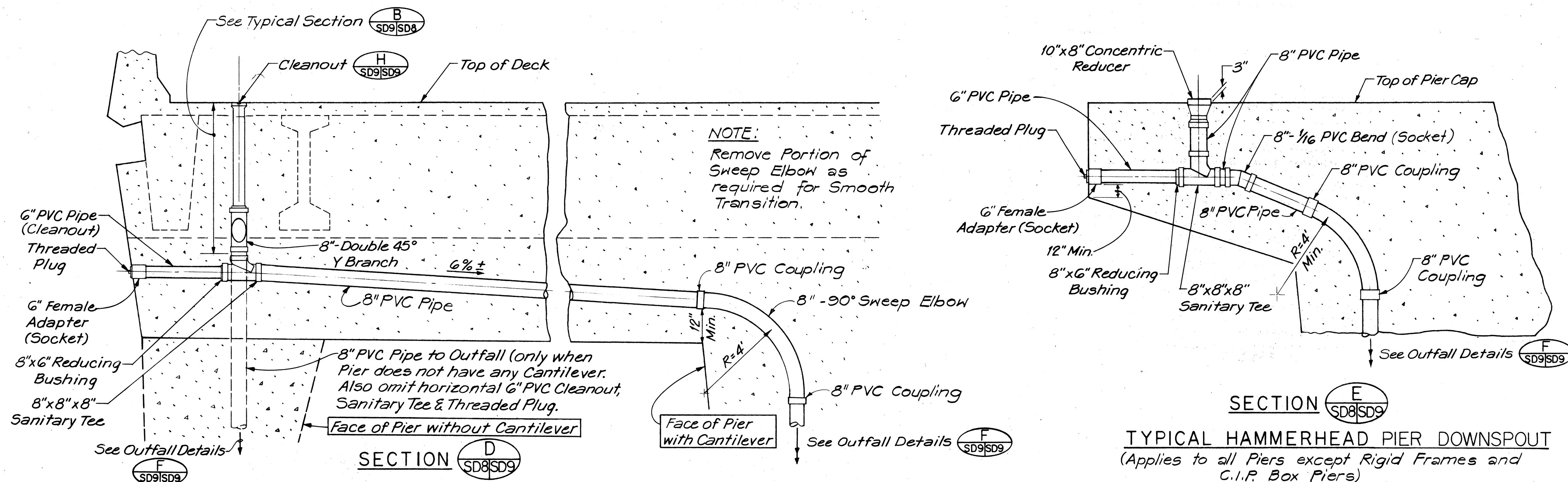
KEEHI INTERCHANGE
F.A.I. PROJECT NO.1-HI-1 (131) :17
SCALE: AS SHOWN DATE :AUG31,77
SHEET NoSD-8 OFSD-17 SHEETS



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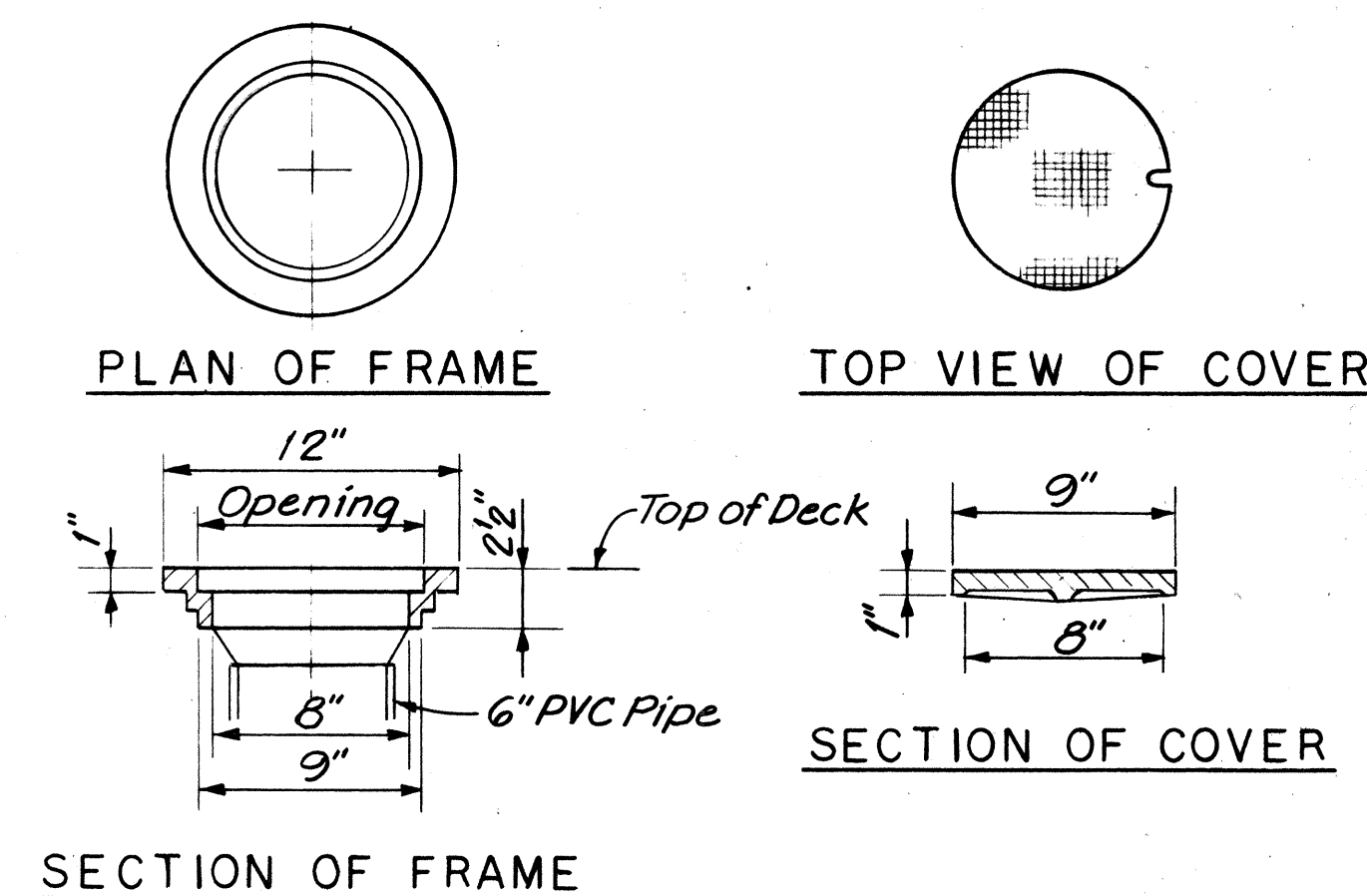
Vijay chandra
Signature

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-H14(131)17	1977	340	394

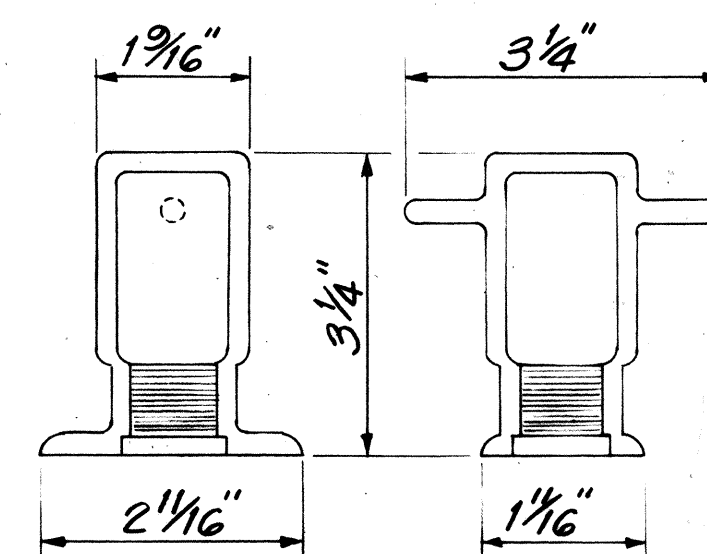
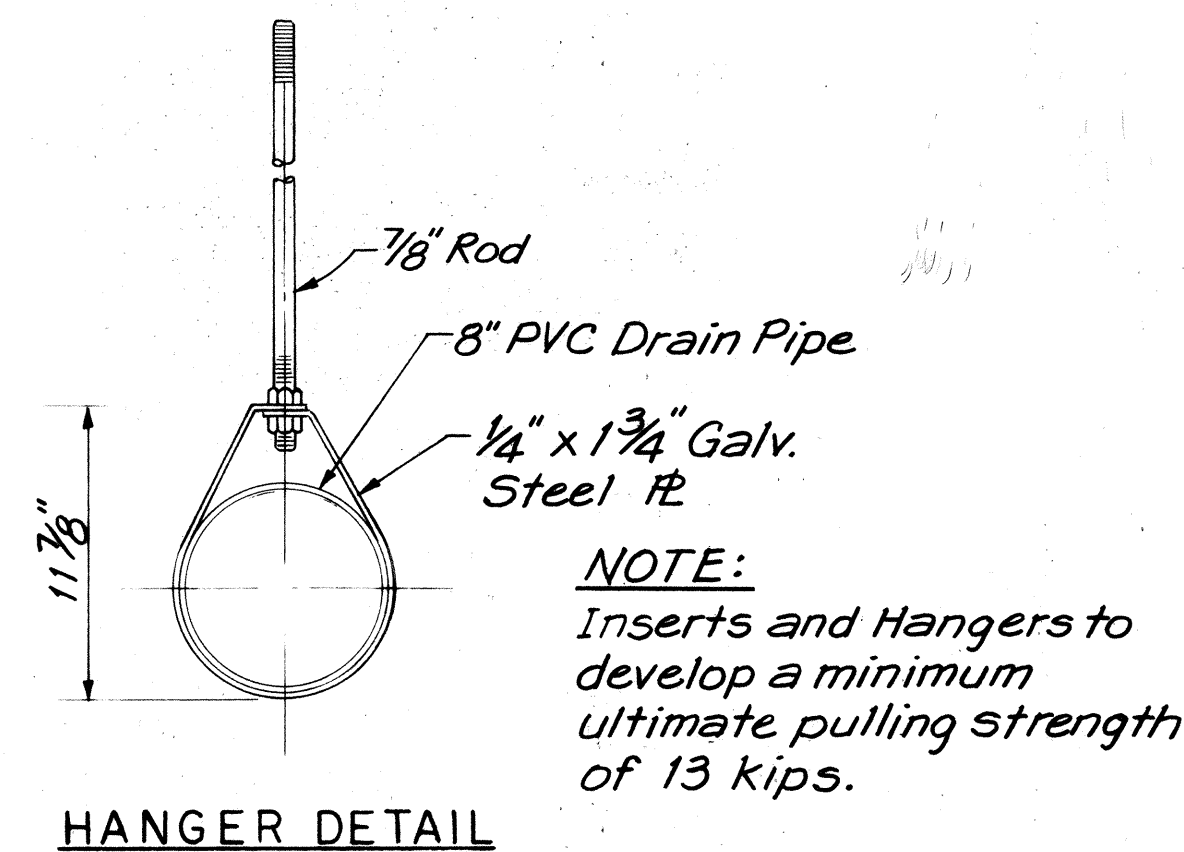
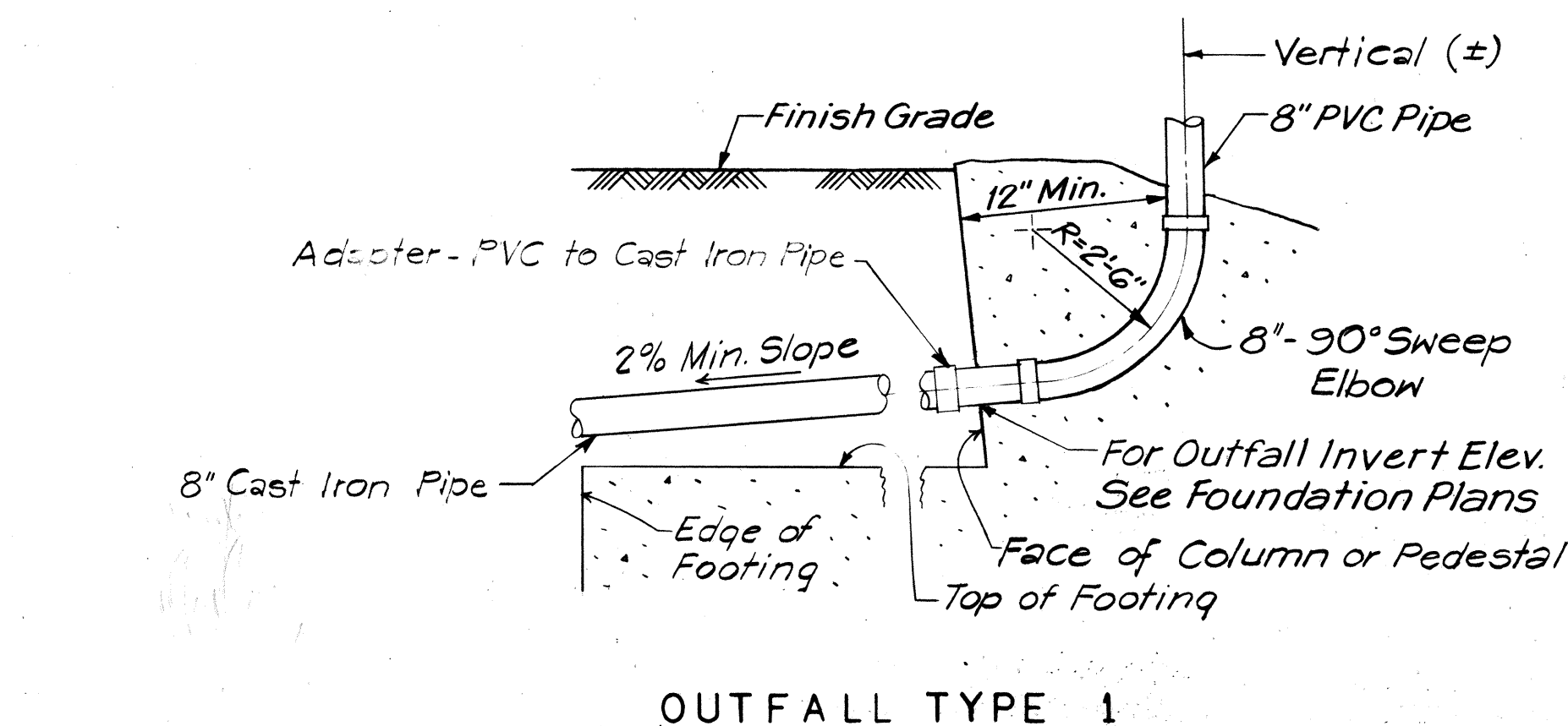
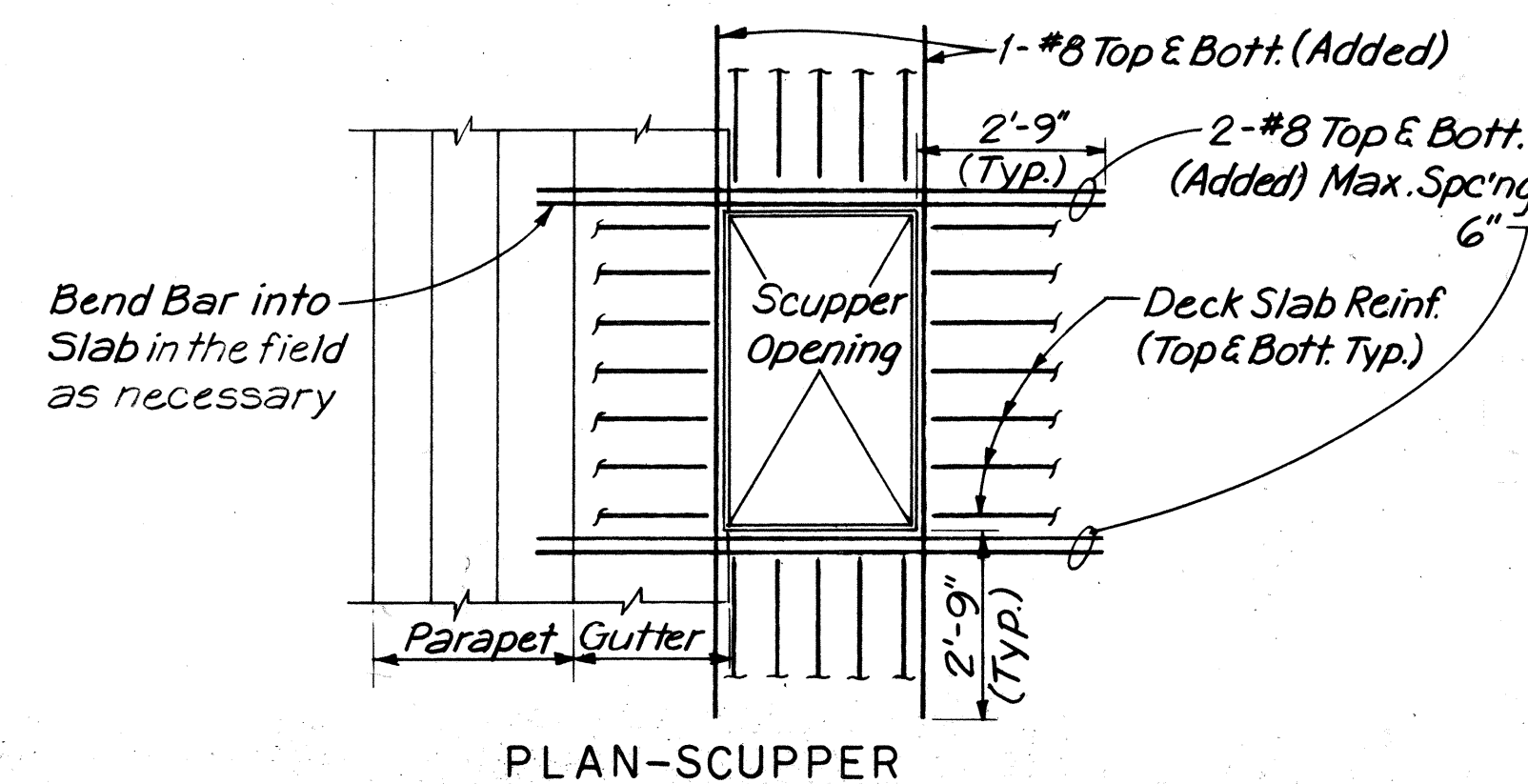


GENERAL NOTES:

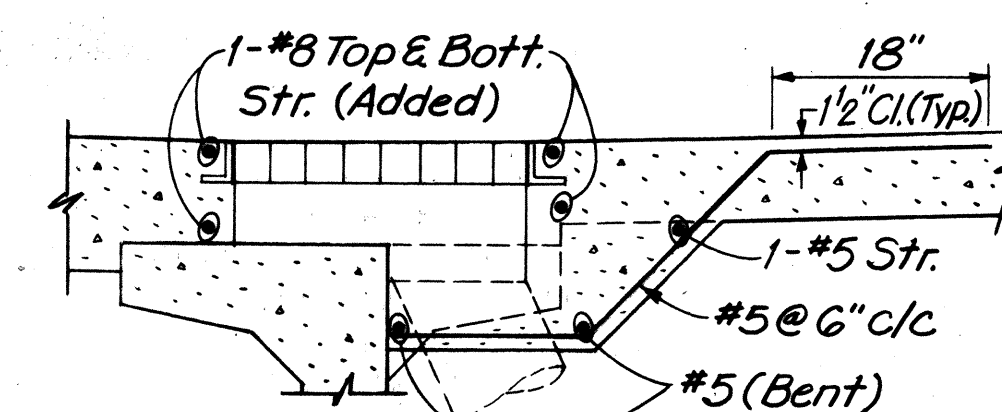
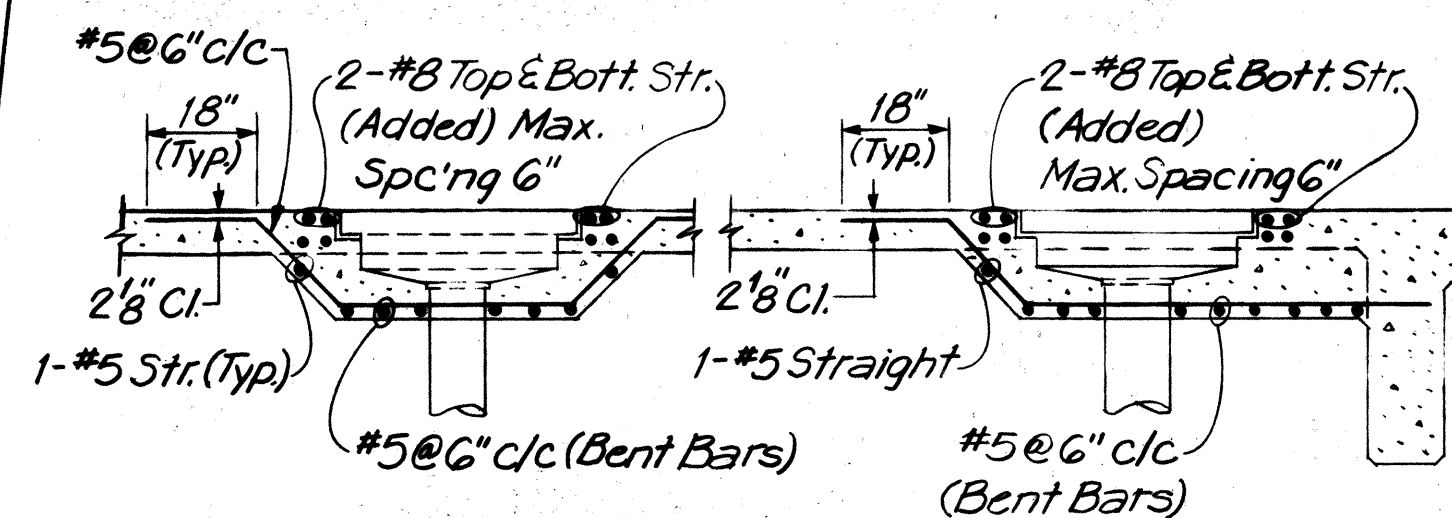
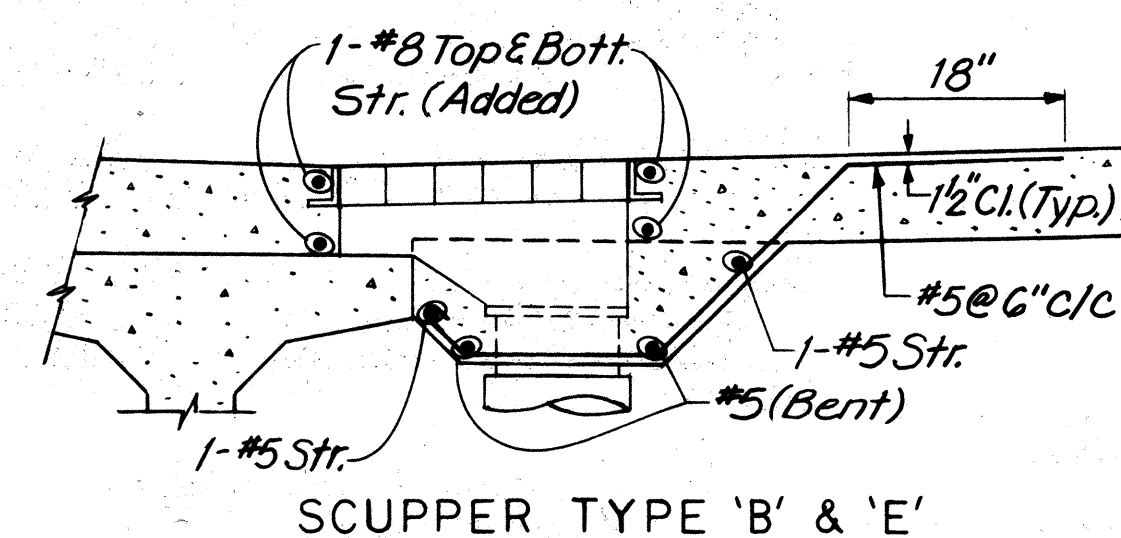
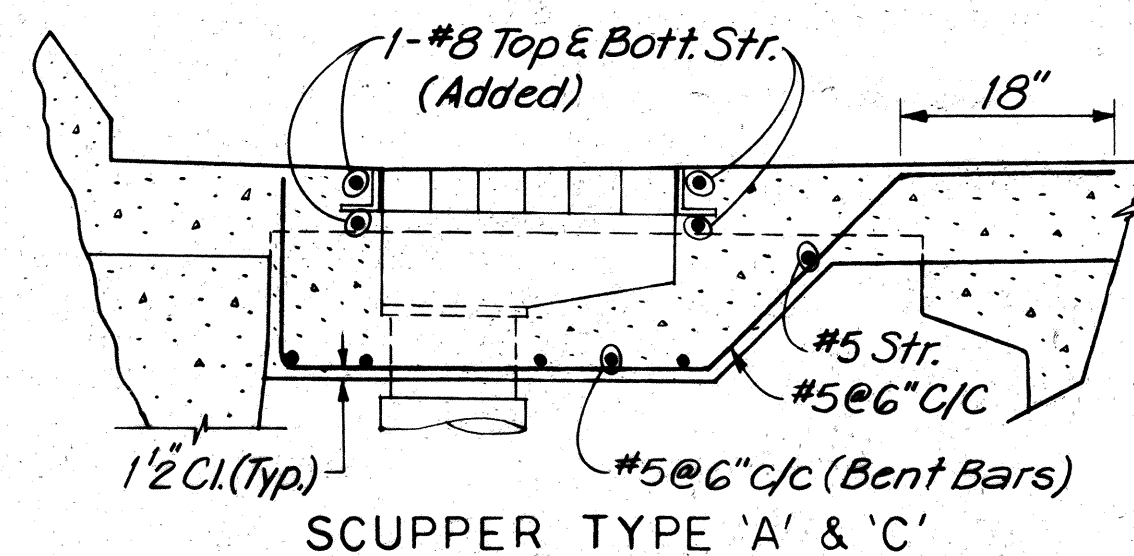
- All Pipe Slopes shall be maximized to the greatest Extent possible without extending below the bottom of the Beams.
- All PVC Pipes shall be Type I Schedule 40.
- Deck Drain Cleanout Metal Frame and Cover Casting shall be Ductile Iron. Designed for Highway Traffic Wheel Loads of 16,000Lbs And shall conform with ASTM Designation A 536.
- For Location of Scuppers and Outfall Type, See General Plan (or Layout Plan) of Structure.



TYPICAL DECK DRAIN CLEANOUT FRAME AND COVER



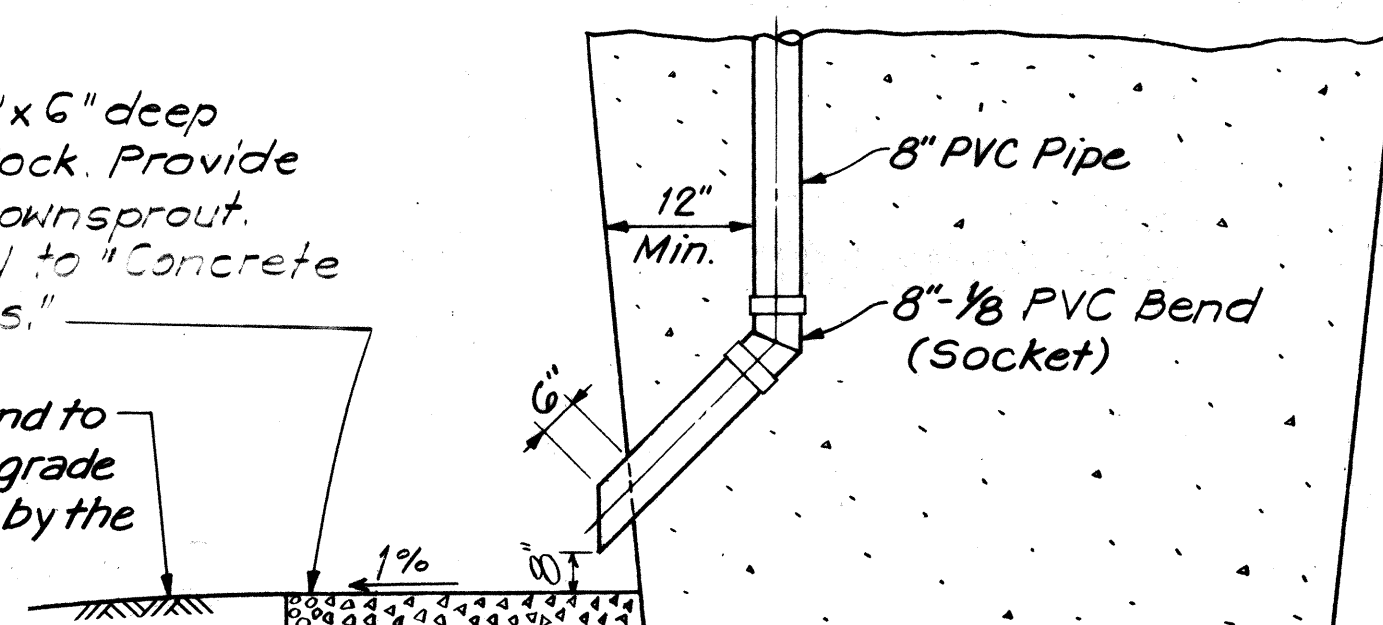
DETAIL OF PIPE HANGERS AND INSERTS



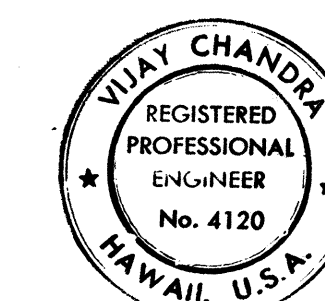
REINFORCEMENT DETAILS AT SCUPPERS

5'-0" x 5'-0" x 6" deep crushed Rock. Provide at each Downspout. (Incidental to "Concrete in Bridges."

Earth mound to meet final grade as directed by the Engineer.



OUTFALL DETAILS



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Vijay Chandra
Signature

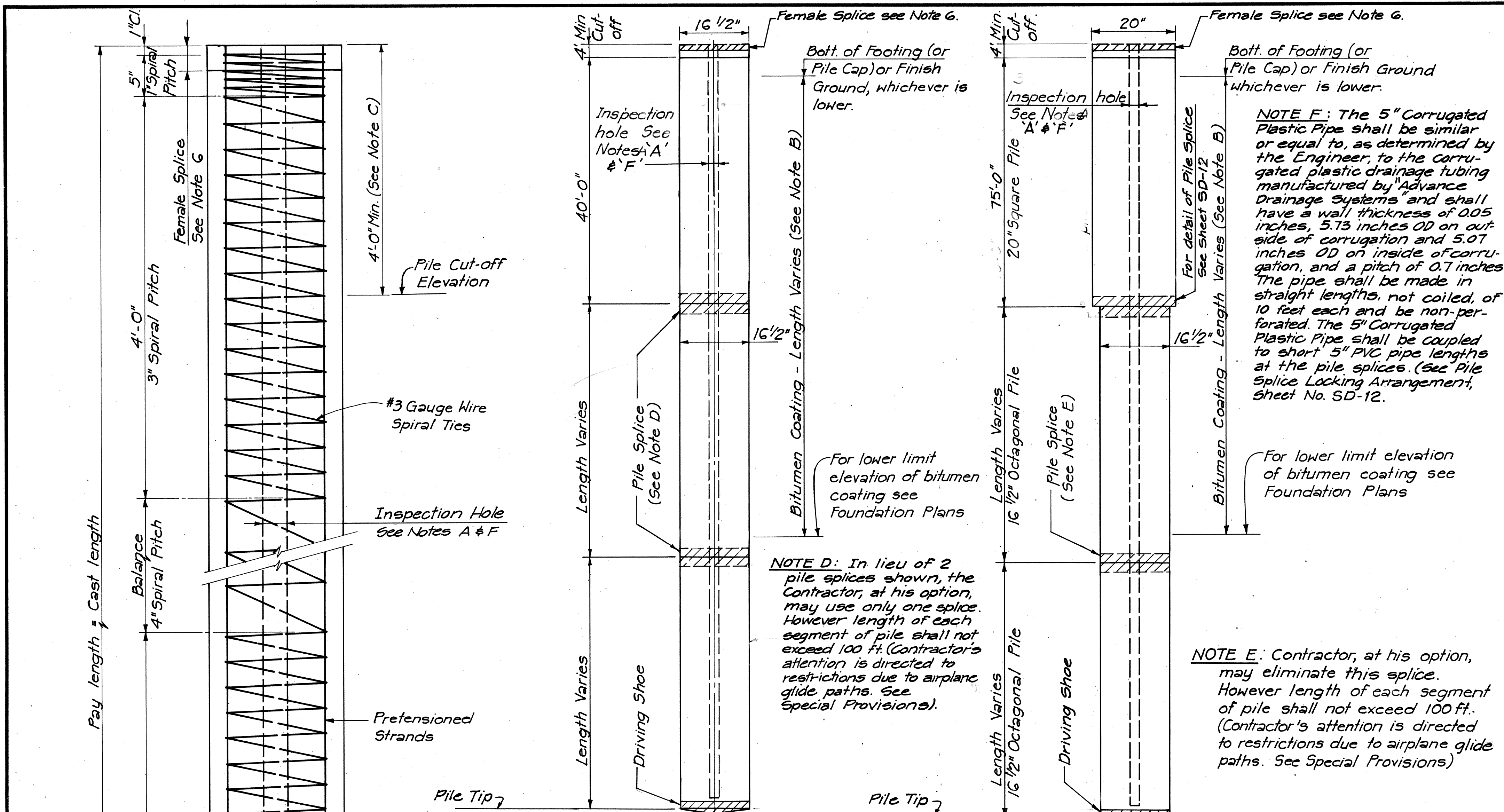
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE DRAINAGE DETAILS
SHEET 2 OF 2
KEEHI INTERCHANGE
F.A.I. PROJECT NO. 1-H1-1 (131) :17
SCALE: N.T.S.
DATE: AUG. 31, 77
SHEET NoSD-9 OF SD-17 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HI-1(131):17	1977	341	394

PILE NOTES :

- Concrete shall have a minimum ultimate 28-day strength of 7000 psi. Required minimum ultimate concrete strength before release of strands = 4000 psi.
- Prestressed strands shall be 7 wire stress-relieved strands with an ultimate strength of 270 ksi. Diameter of strands shall be as shown on the Typical Pile Sections. Total loss due to creep, shrinkage, elastic shortening and relaxation of steel = 37,000 psi. Total working force shown on the Typical Pile Sections is the net force after all losses.
- Non-prestressed reinforcing steel for dowels of splices and pile build-up shall be A.S.T.M. A615 Grade 60. Spiral ties shall be cold drawn A.S.T.M. A02.
- For location of Pile Types A and B see foundation plans.
- Contractor has the option to use either of the Driving Shoes shown. All piles shall have driving shoes.
- At the top of the top segment of the pile, a female splice shall be cast along with the pile. No pile shall be driven without a female part of the splice at the top of the pile. Cost of all splices in a pile shall be incidental to furnishing of Pile and will not be paid for separately.
- Contractor shall submit shop drawings and calculations of the splice for approval by the Engineer at least 3 months prior to casting the piles. This is to provide sufficient time for review of shop drawings and calculations and for delivery time from the Supplier.
- The splice shall be designed for an ultimate moment of 120 ft. Kips.
- Portion of pile embedded into footing (or pile cap) shall be of sound concrete. Damaged portions shall be removed and pile shall be built-up to the cut-off elevation shown on the Plan.
- The plumb of pile shall be checked for conformance to the allowable tolerances, as specified in the special provisions, before constructing the build-ups.
- Work of cutting off Prestressed Concrete Piles or Concrete Build-ups shall be performed in such a manner as to avoid spalling or damaging the pile below cut-off. In case of such damage the pile shall be replaced or repaired by the Contractor at his own expense as directed by the Engineer.
- Piles are to be manufactured & installed in accordance with the tolerances referred to in the Special Provisions. Piles not conforming to the tolerance shall be rejected and will not be paid for.
- The Contractor shall furnish necessary labor and/or equipment to aid the Engineer in lowering and raising the plumb bob in the central inspection hole immediately after driving each pile. He shall also clean out any debris and/or soil in the central hole that interferes with the inspection. Such assistance shall be incidental to pile driving and will not be paid for separately.
- Driving on pile build-ups shall not be permitted.



PILE TYPE A

N.T.S.

NOTE A: Provide inspection holes in all piles as follows:

- All piles, except Load Test Piles, shall have a 3" PVC Pipe cast in the center of the pile. The pipe shall have a radial tolerance of 1/8" off-set from center of pile except that within 3 feet of the ends of pile segments the radial tolerance shall be 1/16" off-set from the center of the pile. The 3" PVC Pipe shall be Schedule 40 (3.500" OD, 3.068" ID) rigid pipe with socket couplings. The allowable deviation from straight for the pipe is 1/8" in any 10 feet of length.
- Load Test Piles shall have a 5" Corrugated Plastic Pipe cast in the center of the pile instead of the 3" PVC pipe. The pipe shall be placed to the same tolerances specified from the 3" PVC pipe. For additional details see Note F.

NOTE B: The Bitumen Coating shall not protrude into footing or Pile Cap. When a pile is exposed above ground, as in the case of certain pile bents, the bitumen coating shall be up to the finished grade (or stream bed) only. If the Pile Tip ends higher than the estimated Pile Tip Elevation, the Contractor shall remove the bitumen coating from the pile head that is to be embedded into the footing (or Pile Cap), or the portion above the finished grade, as applicable.

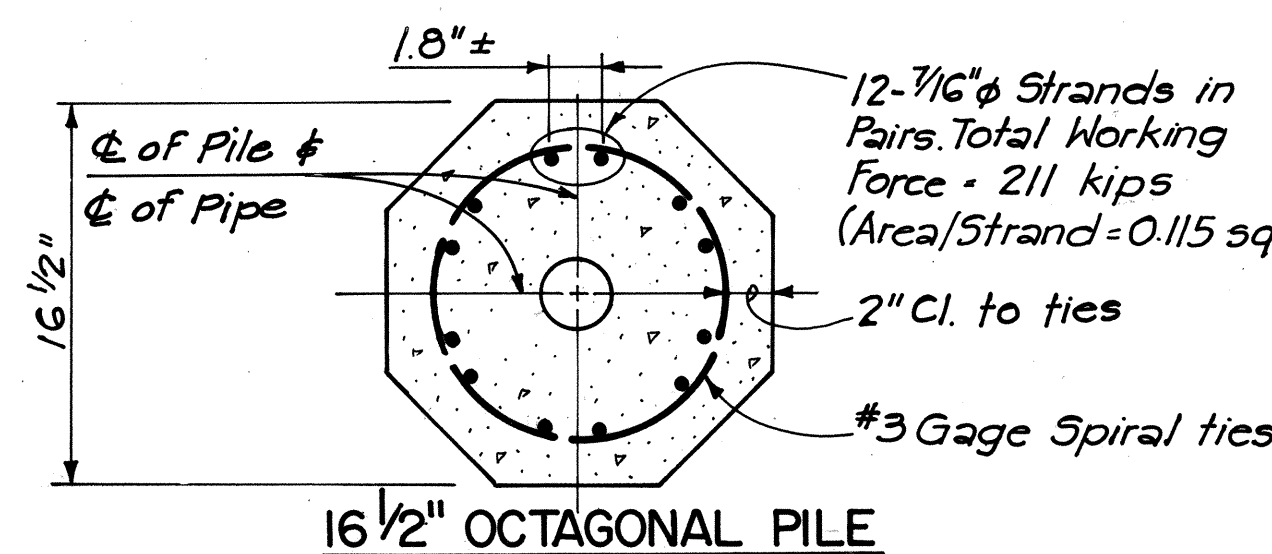
NOTE C: Remove concrete over this length from top of top segment of pile. Leave strands exposed, for embedment into footing (or pile cap). Cut-off of concrete shall be normal to the axis of pile. For pile cut-off elevations see Foundation Plans.

PILE TYPE B

N.T.S.

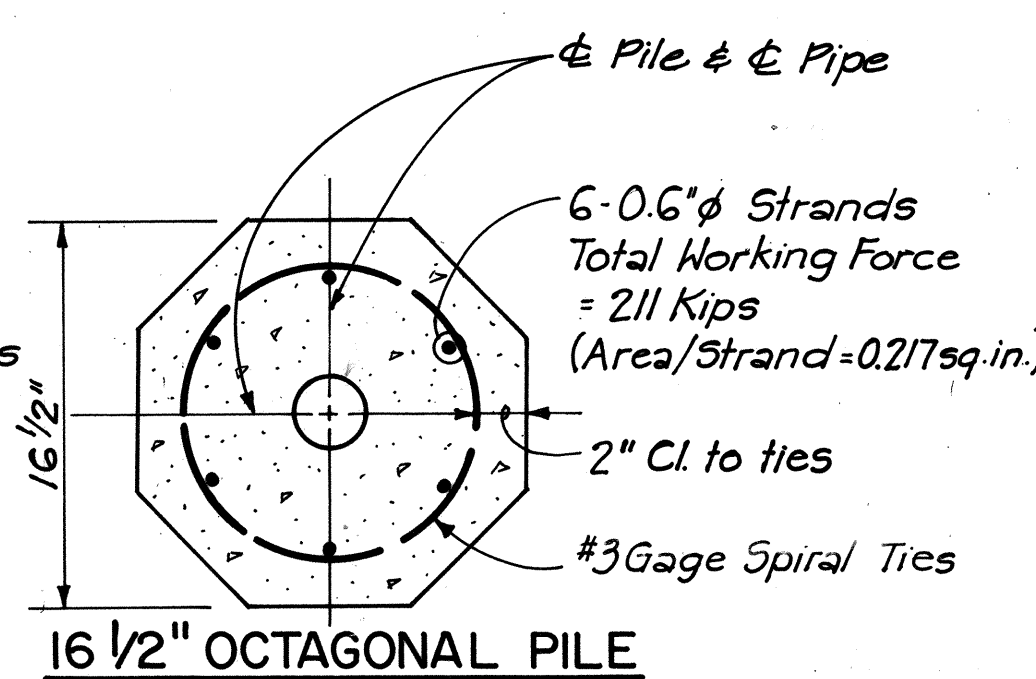
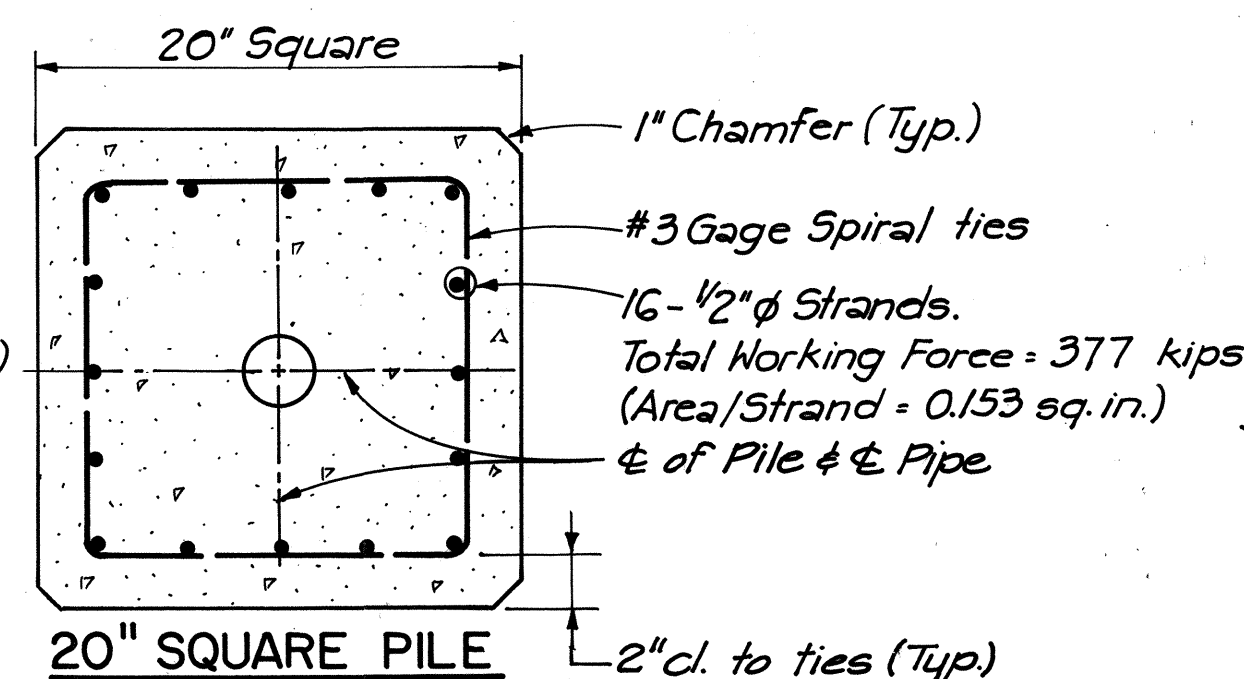
TYPICAL PILE DETAIL

N.T.S.



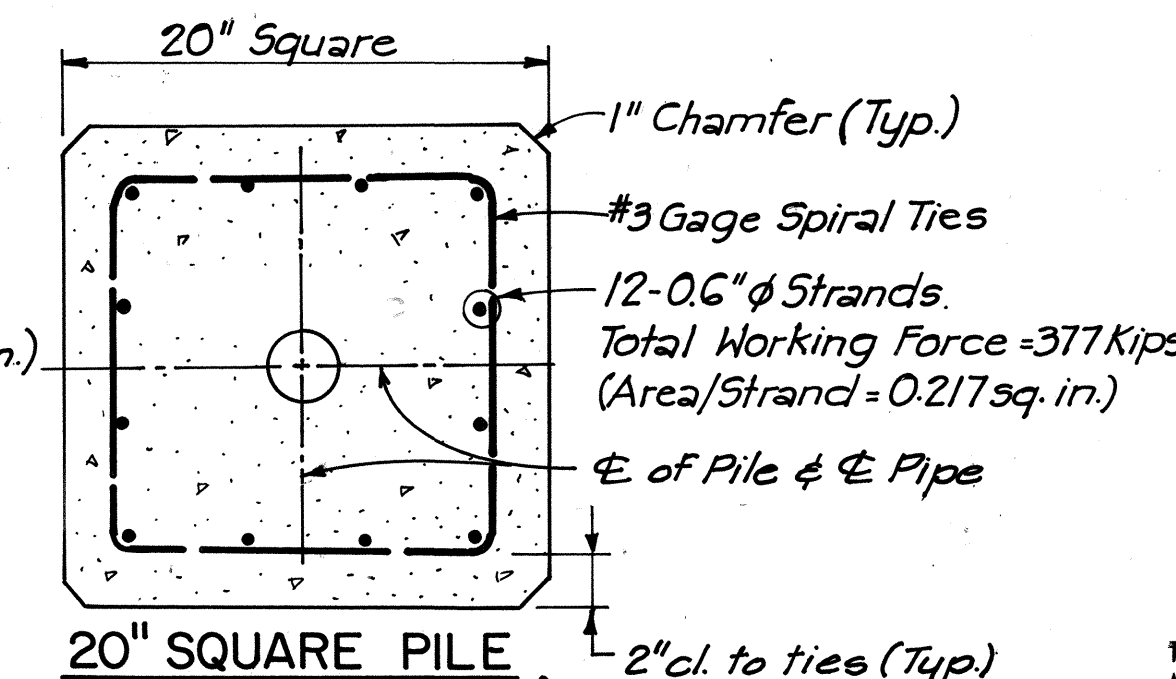
TYPICAL PILE SECTION

Scale: 1/2" = 1'-0"



OPTIONAL PILE SECTION

Scale: 1/2" = 1'-0"



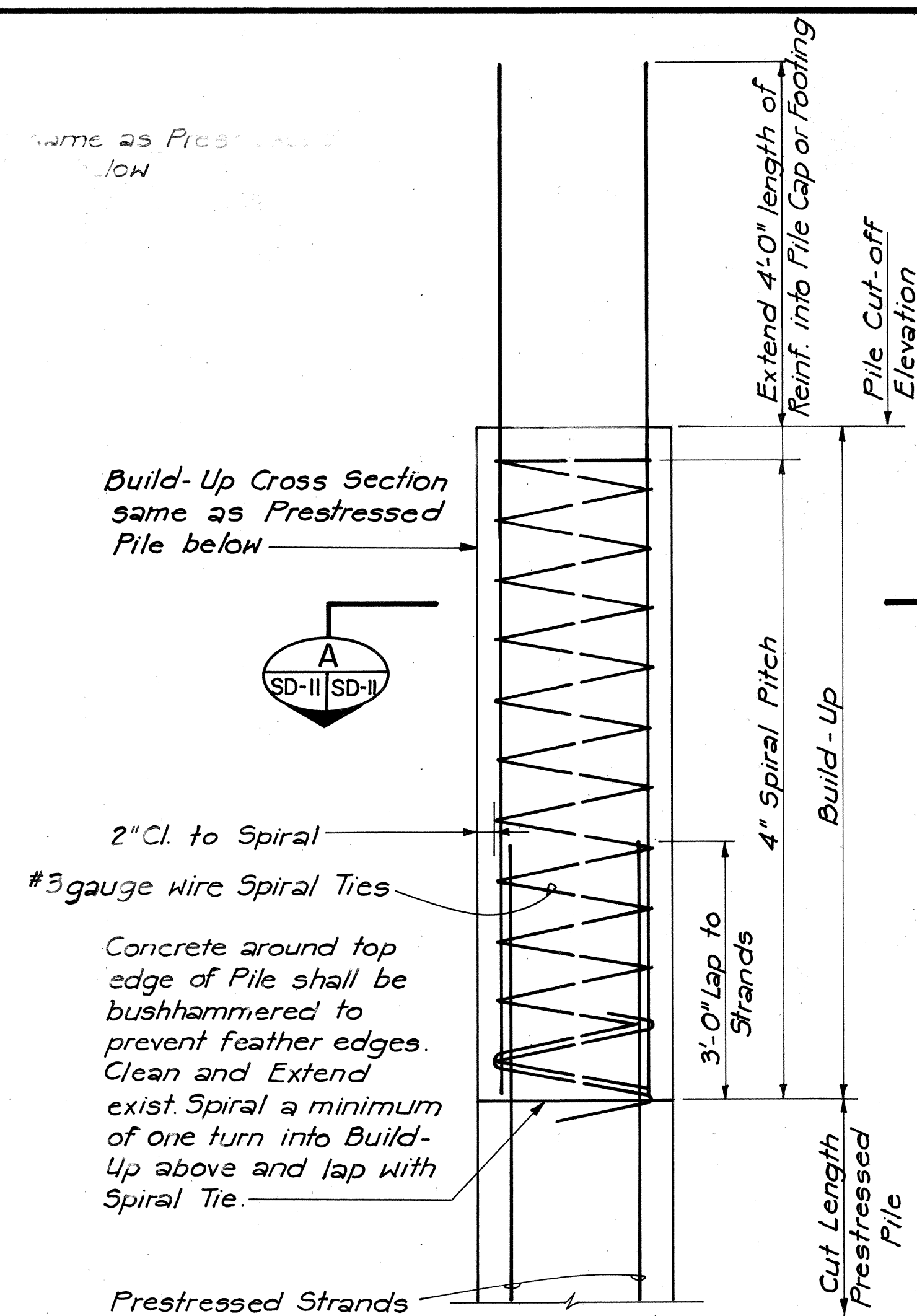
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.
Joy Chandra
Signature

3/23/78	Revised Typical Pile Details, Sections, Pile Type A & B. Revised Notes A, B, 5, 6 & 13. Added Note F.
DATE	REVISION
STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION	
PRESTRESSED CONCRETE PILES DETAILS	
SHEET 1 OF 3	
KEEHI INTERCHANGE F.A.I. PROJECT NO. 1-HI-1(131):17	
SCALE: AS SHOWN	DATE: AUG. 31, 77
SHEET No. SD-100FSD-17 SHEETS	

ADD. 341

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-HI-1(131):17	1977	342	394

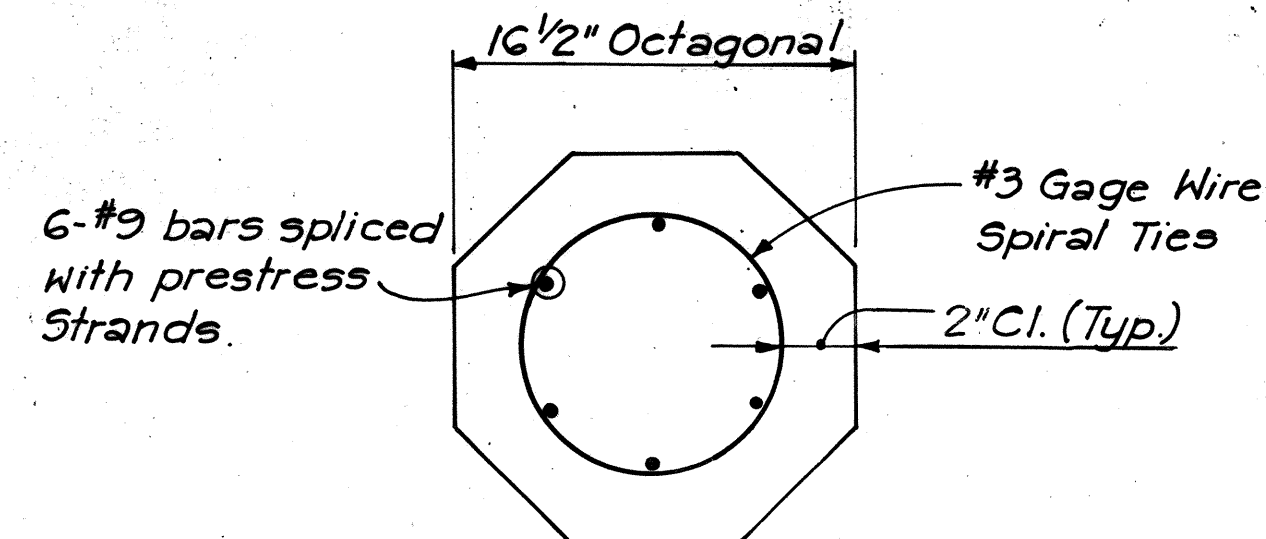
Build-up same as Prestressed Pile below



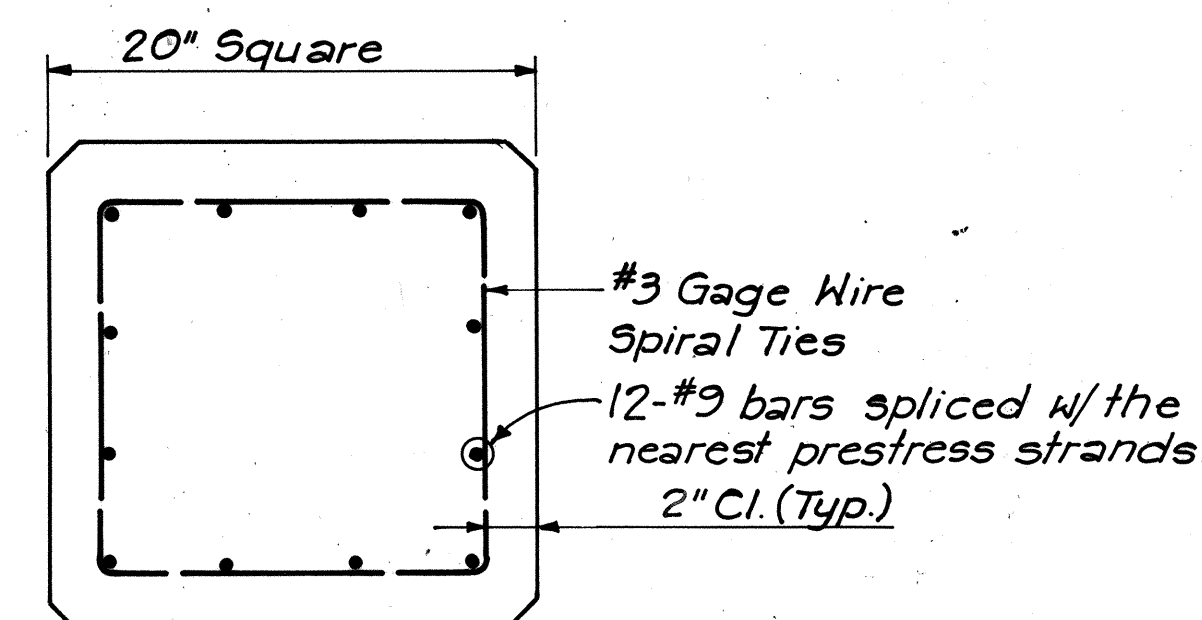
WITHOUT DRIVING

TYPICAL PILE BUILD-UP DETAILS

Not to Scale



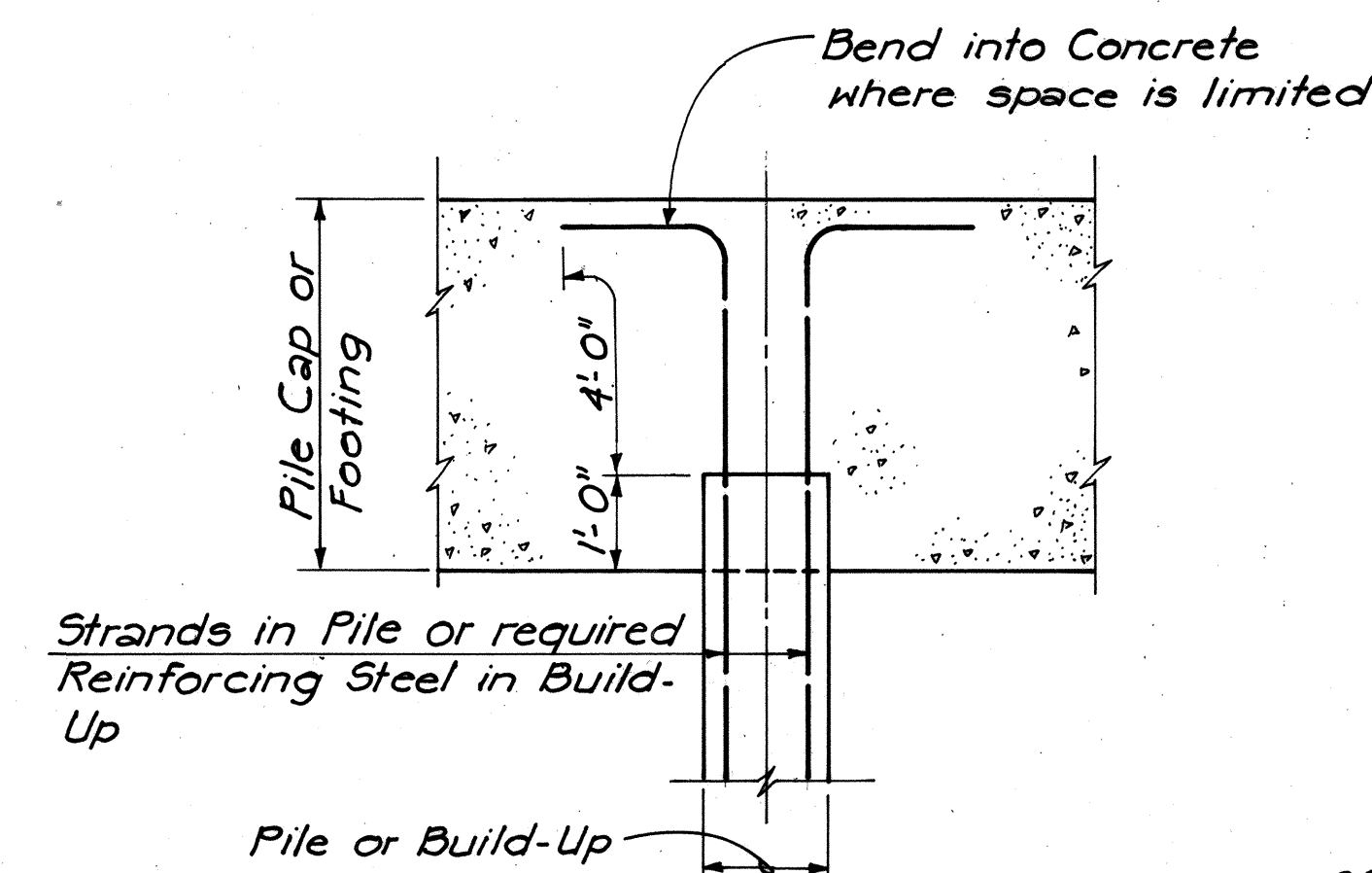
SECTION A
Scale: 1/2" = 1'-0"



SECTION A
Scale: 1/2" = 1'-0"

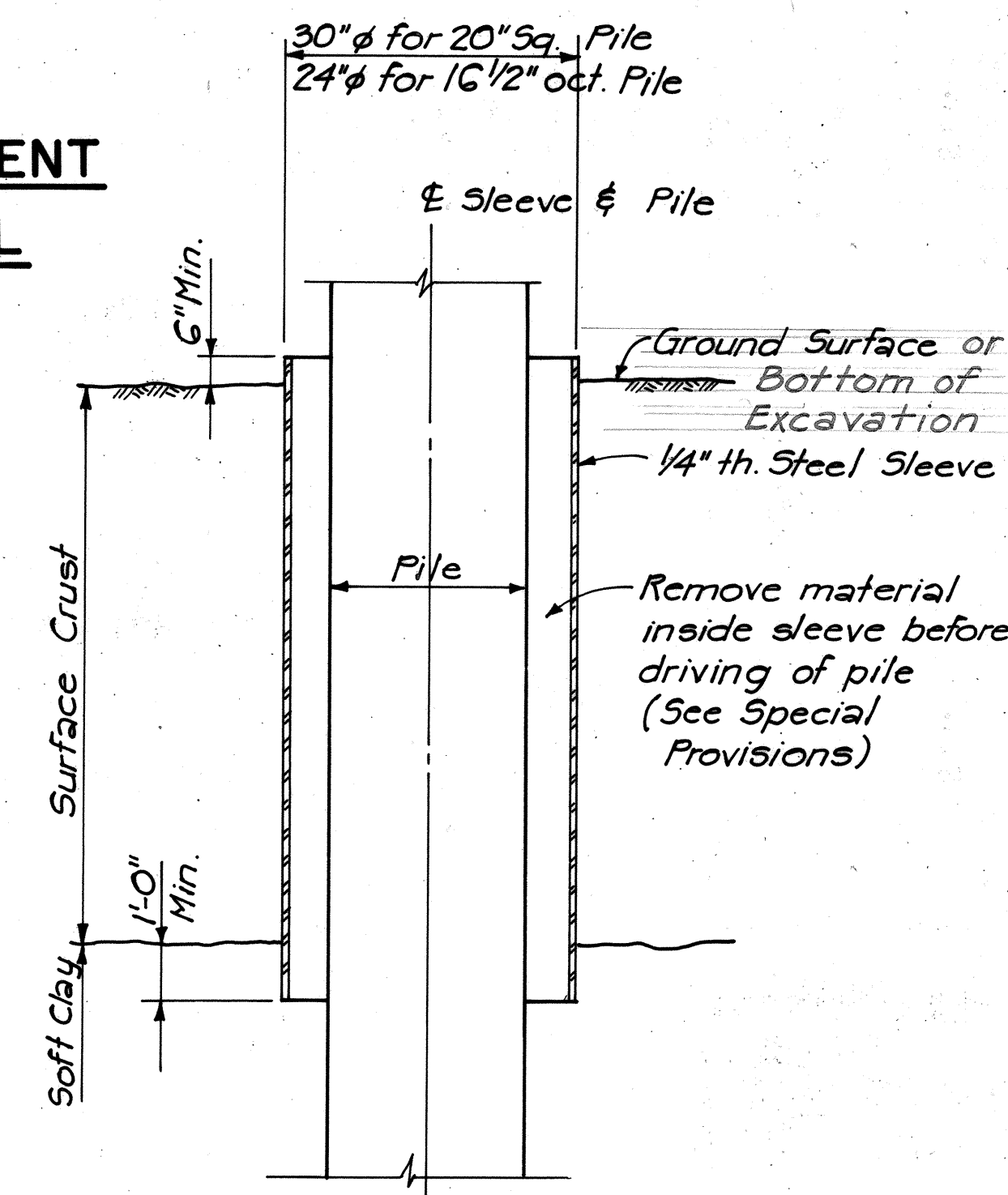
NOTE:

Concrete in pile build-ups shall have a minimum comp. cyl. strength f_c' of 5,000 psi at 28 days.



PILE REINFORCEMENT EXTENSION DETAIL

Scale: 1/2" = 1'-0"

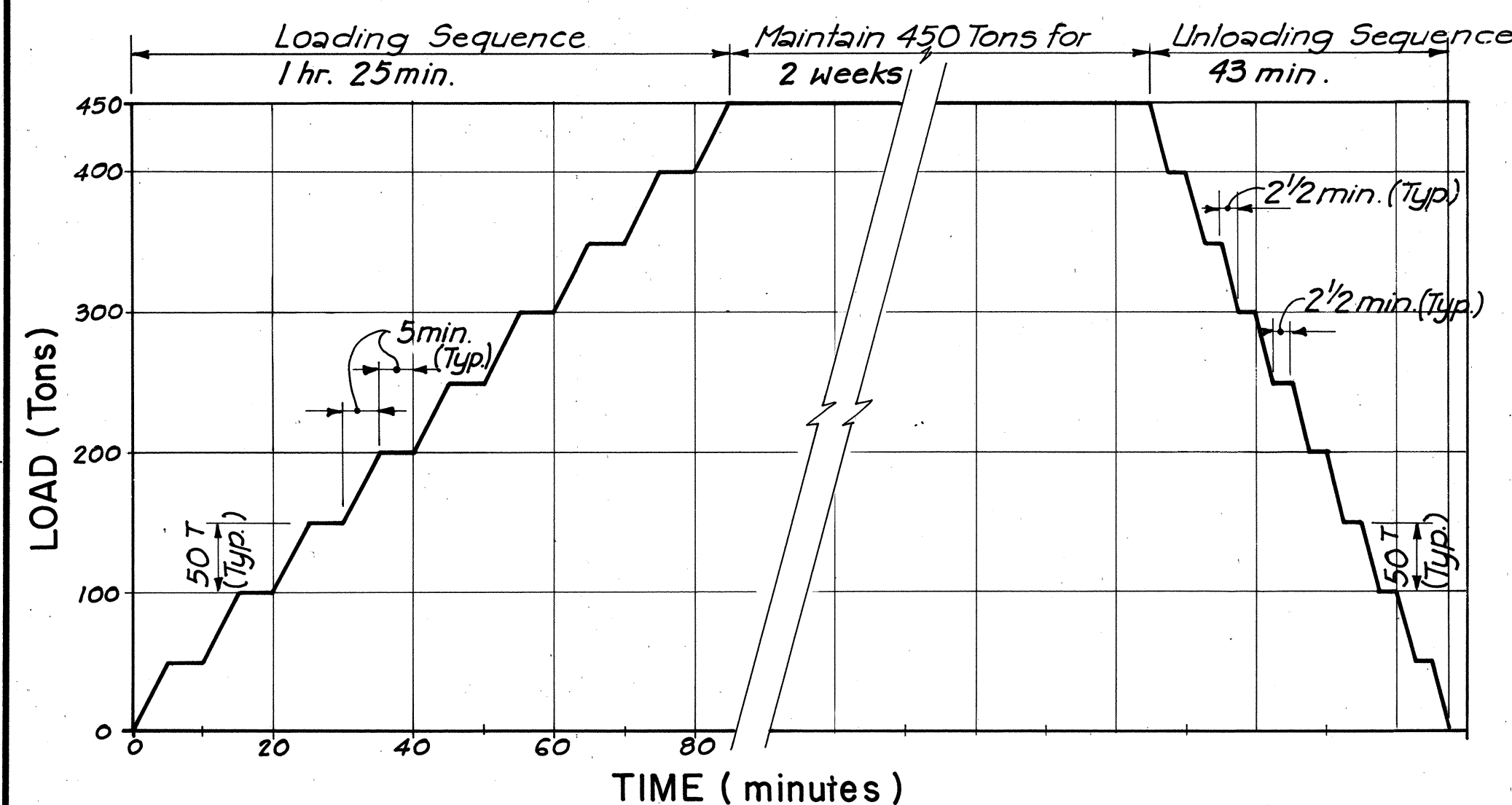


STEEL SLEEVE DETAIL

Not to Scale

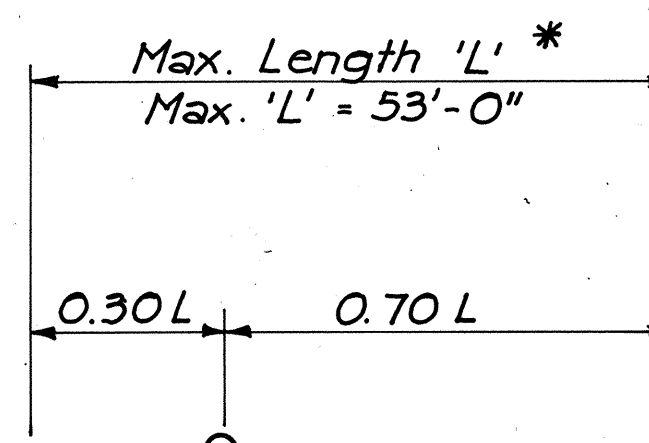
NOTE: No steel sleeve required when there is no bitumen coating on the pile.

Surface Crust is defined as the material between the ground line, or bottom of excavation, and the uniform soft clays (without the presence of granular materials).

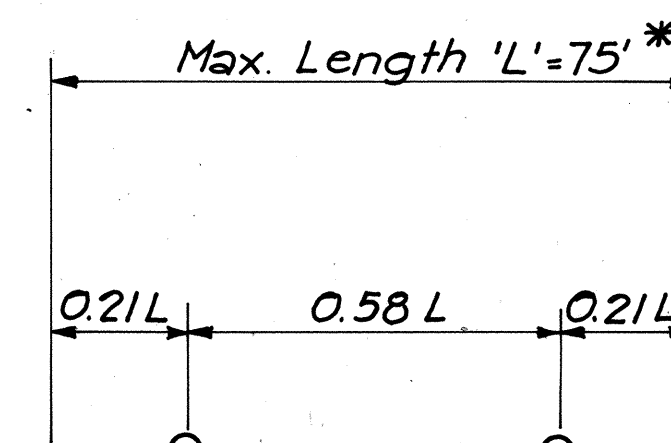


PILE LOAD TEST SEQUENCE DIAGRAM

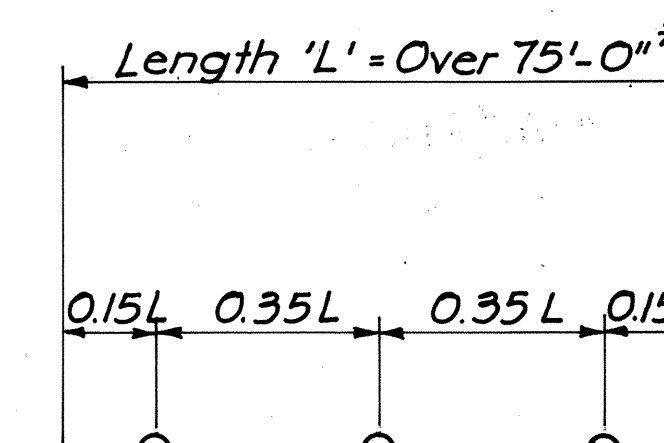
Scale: Horizontal - 1" = 20 Minutes
Vertical - 1" = 100 Tons



1- POINT PICK-UP



2- POINT PICK-UP

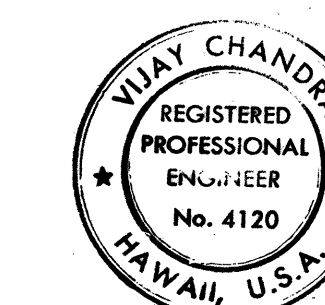


3- POINT PICK-UP

* The Length 'L' is the Distance, End-to-End of Pile before Cut-off

NOTE:

Unless special lifting devices are attached for pick-up points, pick-up points shall be plainly marked on all piles after removal of the forms, and all lifting shall be done at these points. The use of special embedded or attached lifting devices, the employment of pick-up points, or any other method of pick-up shall be subject to approval by Engineer.



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Vijay Chandra
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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

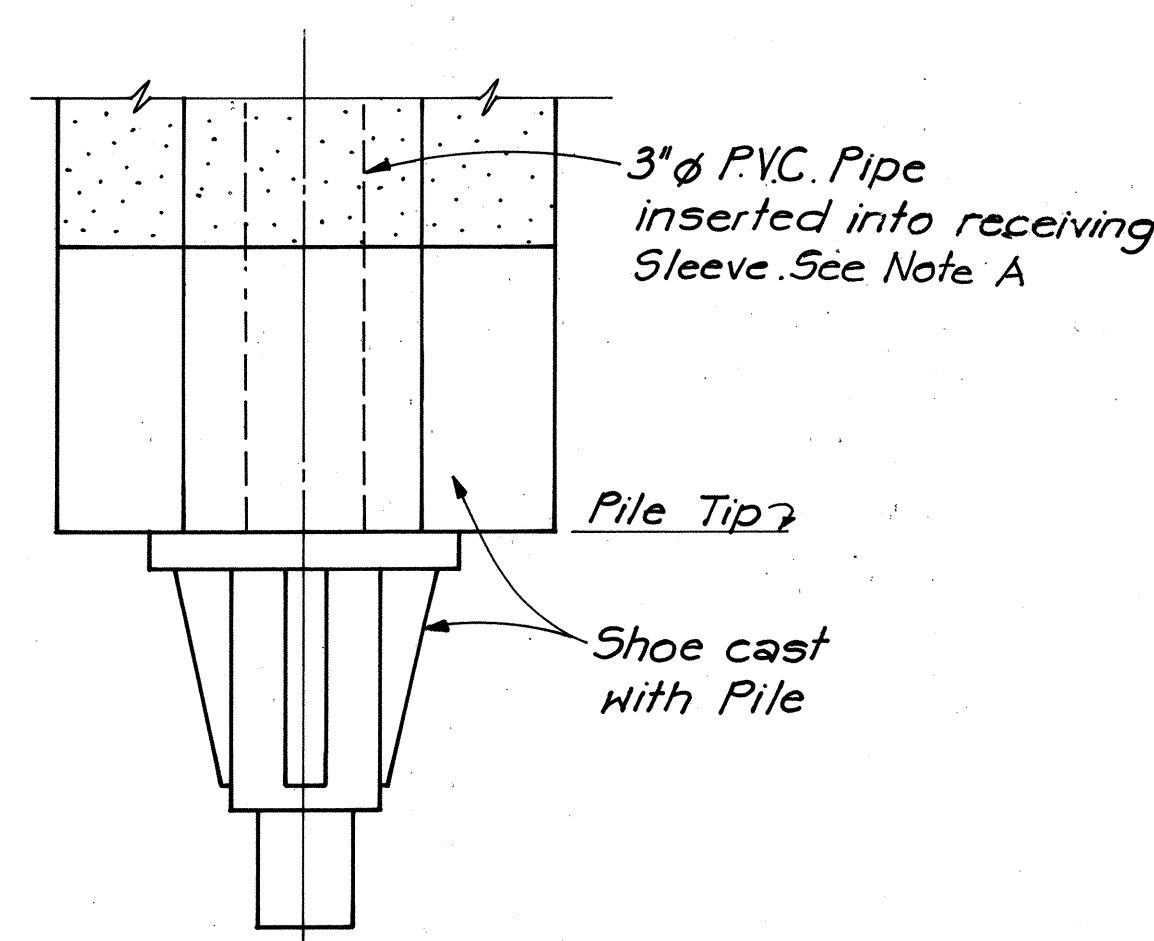
PRESTRESSED CONCRETE PILES DETAILS

SHEET 2 OF 3

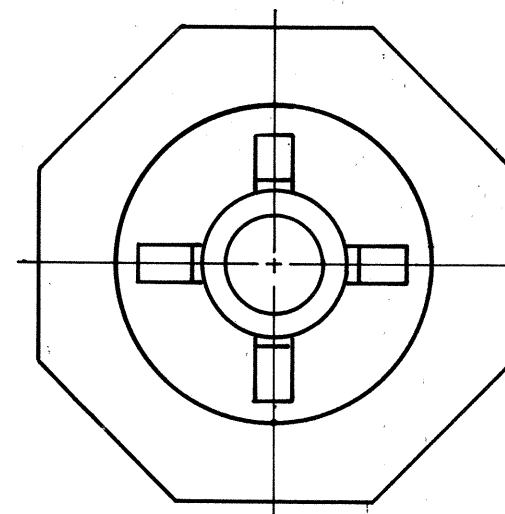
KEEHI INTERCHANGE
F.A.I. PROJECT NO. 1-HI-1(131):17
SCALE: AS SHOWN DATE: AUG. 31, 77

SHEET NO SD-II OF SD-17 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(131):17	1977	343	394



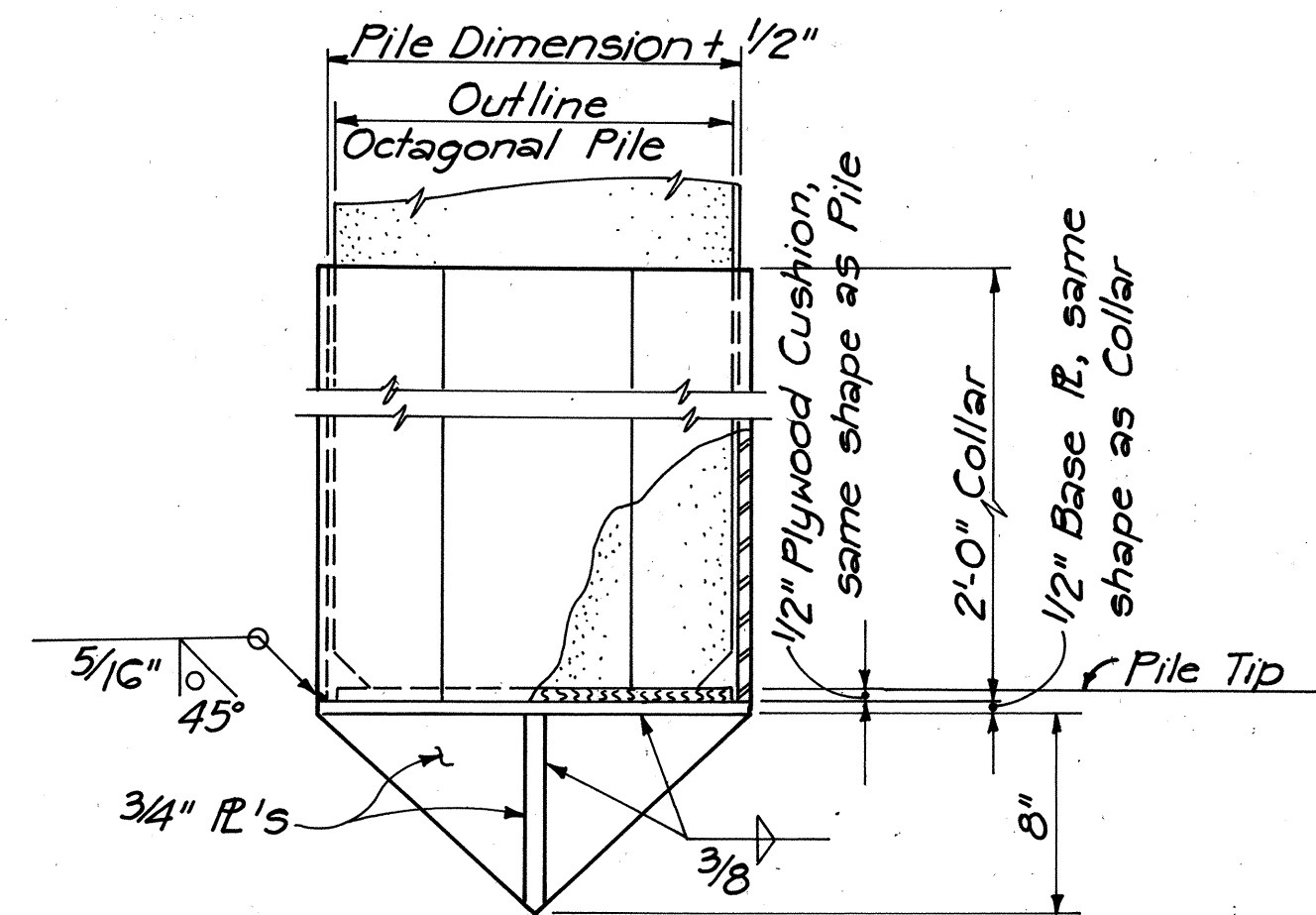
ELEVATION



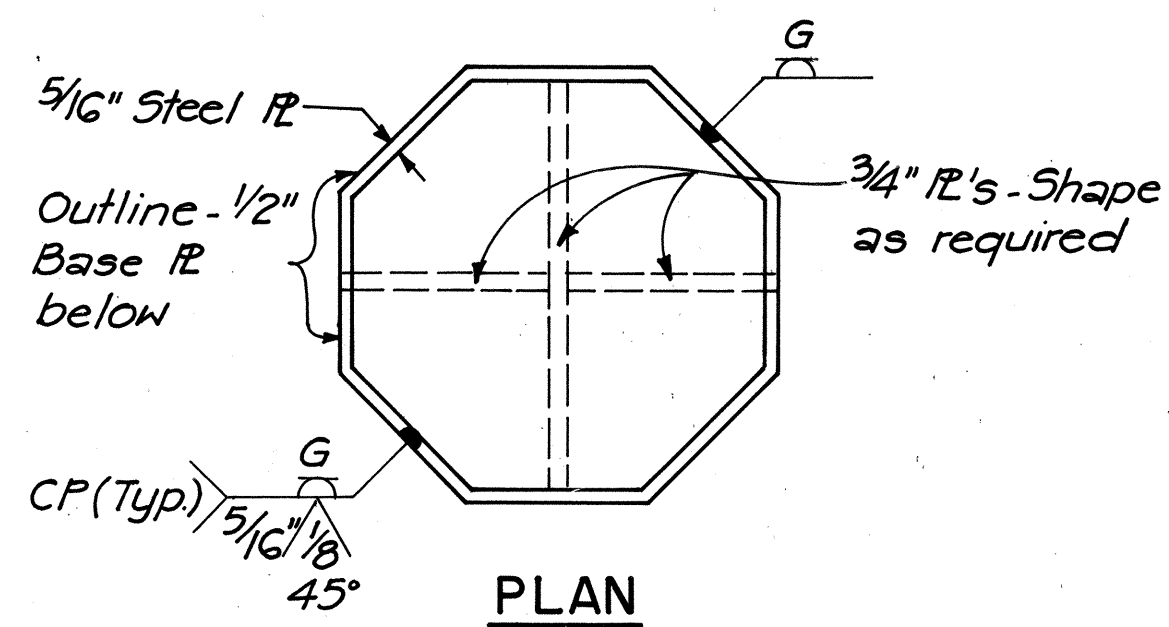
PLAN

ROCK DRIVING SHOE

Not to Scale



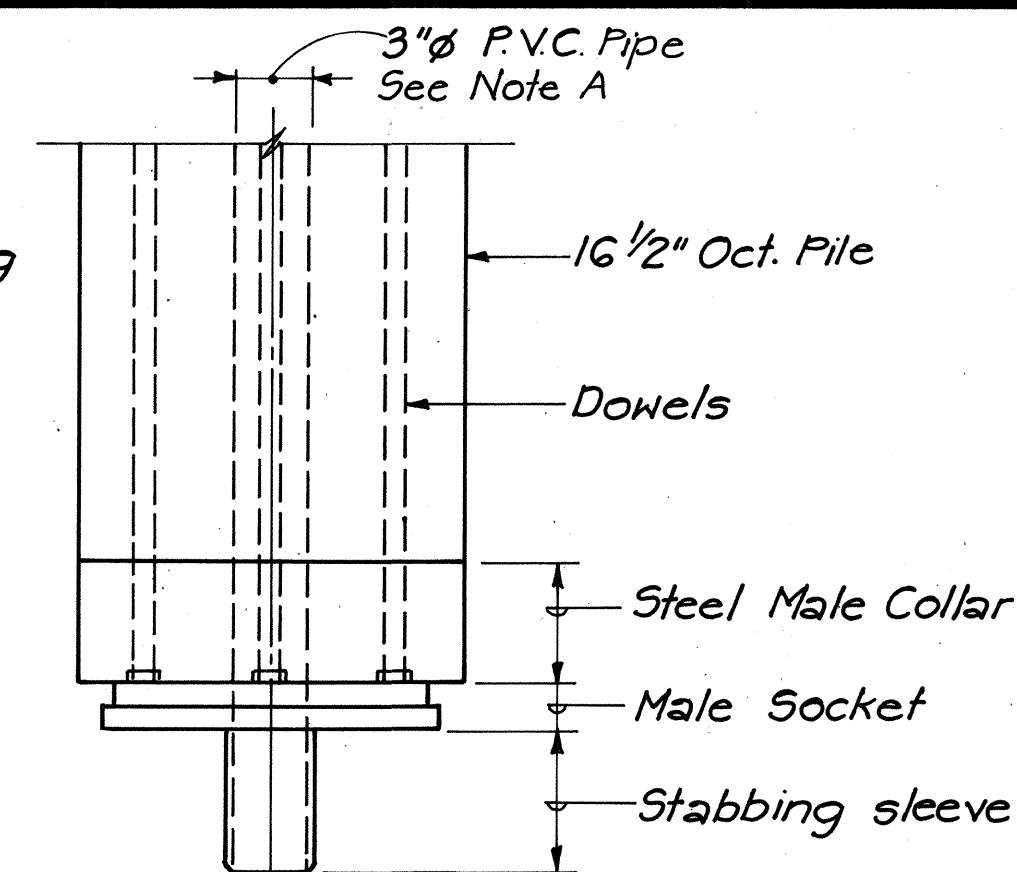
ELEVATION



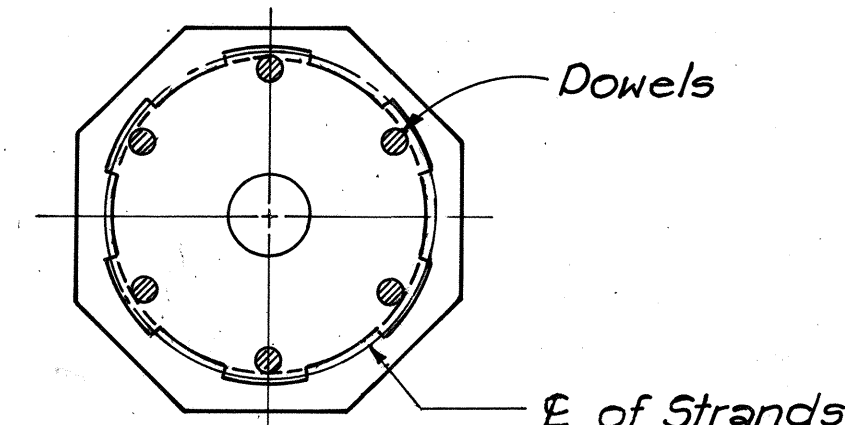
PLAN

CROSS DRIVING SHOE

Not to Scale

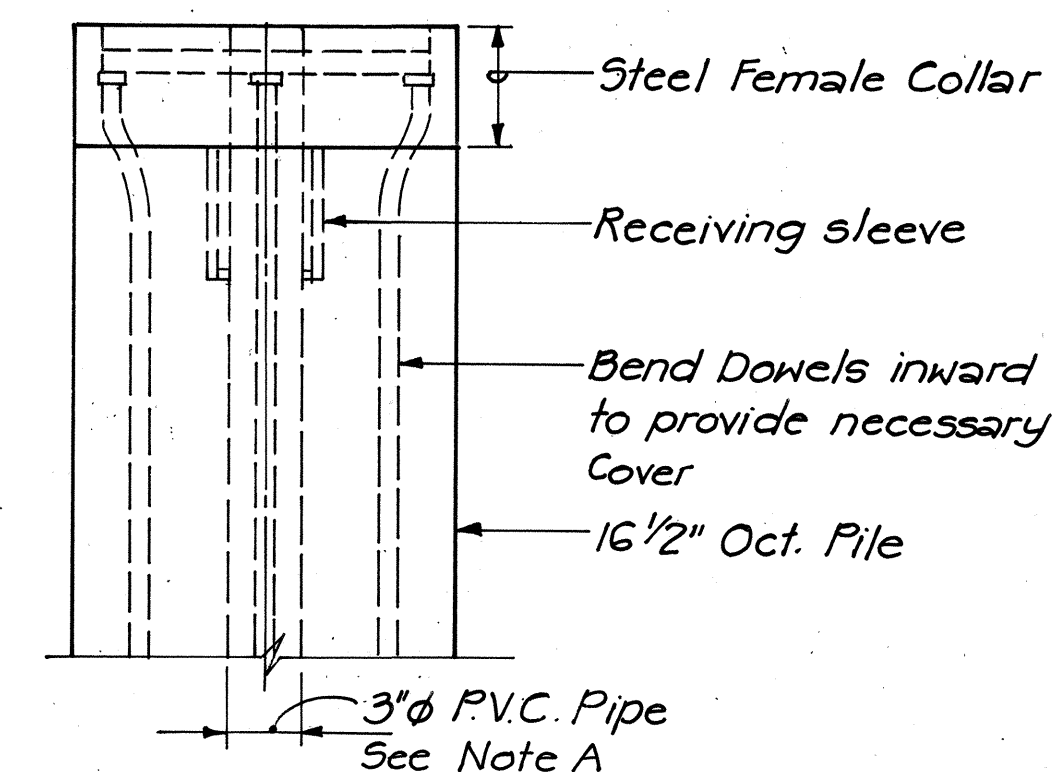


ELEVATION

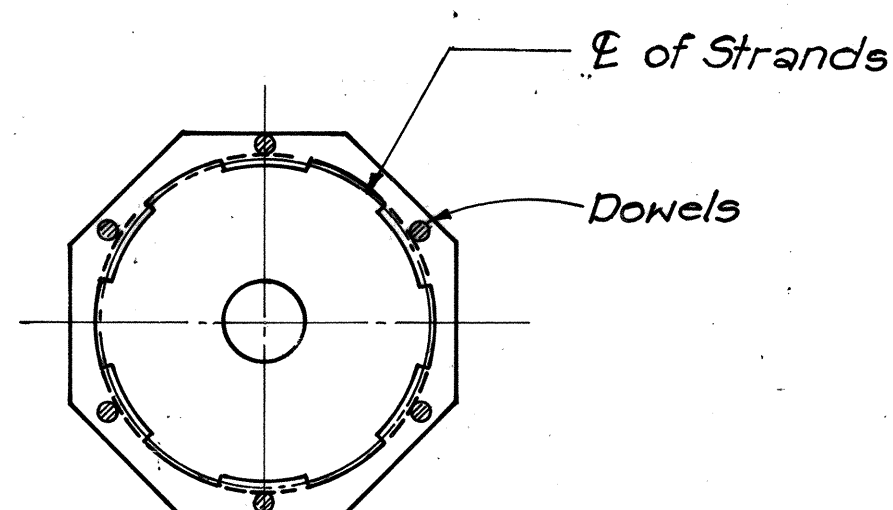


PLAN

MALE END



ELEVATION

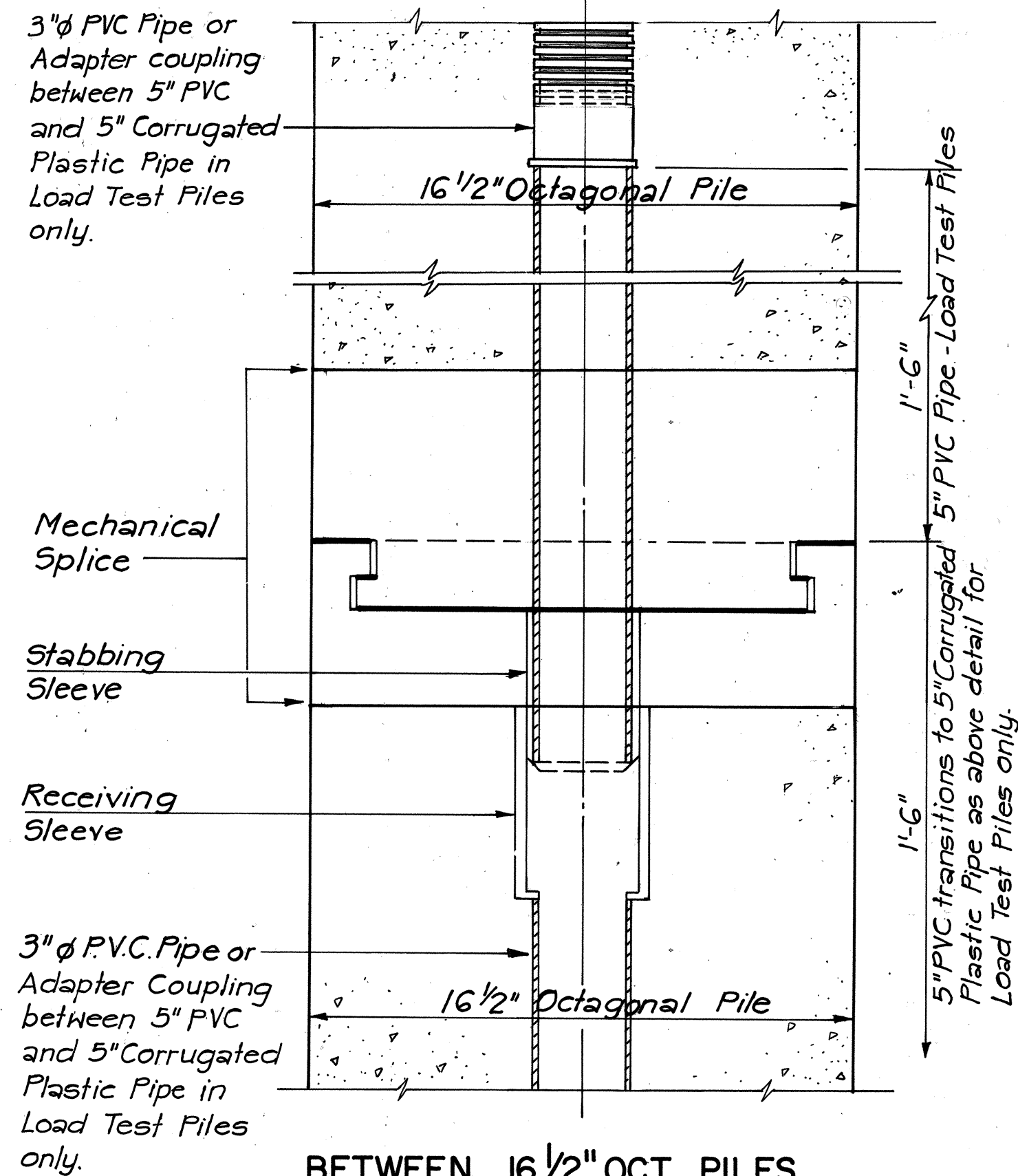


PLAN

FEMALE END

16 1/2" OCTAGONAL PILE SPLICE

Scale: 1 1/2" = 1'-0"

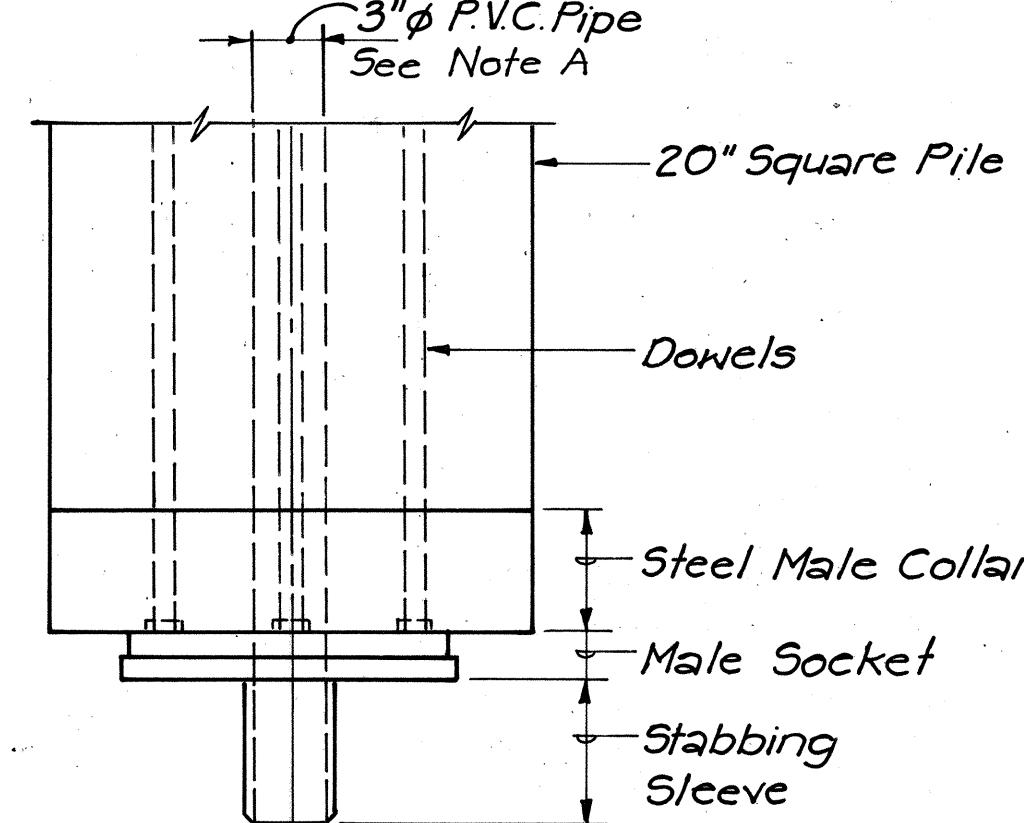


BETWEEN 16 1/2" OCT. PILES

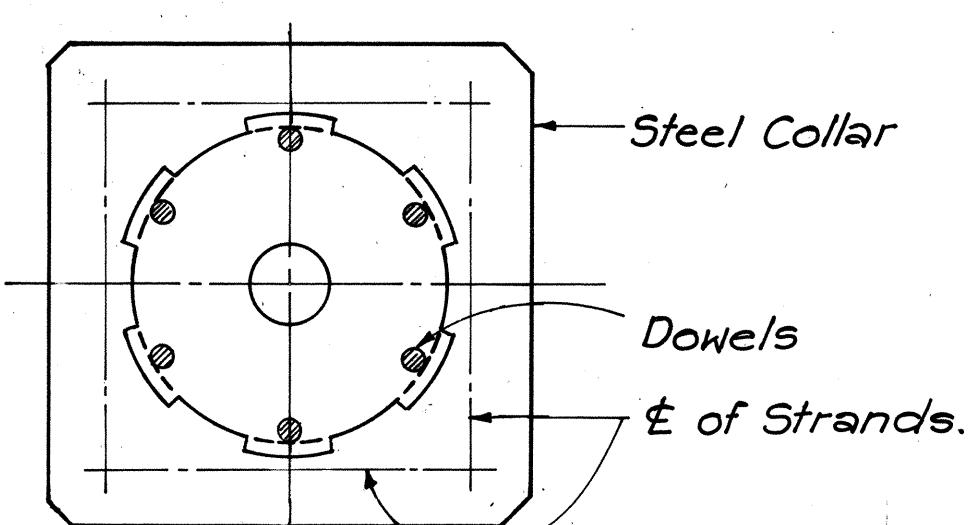
PILE SPLICE LOCKING ARRANGEMENT

Scale: 3" = 1'-0"

NOTE A: 3" PVC Pipe used throughout pile length except on Load Test Piles. Load Test Piles shall have a 5" central hole made by 5" Corrugated Plastic Pipe & short 5" PVC Segments near splices as shown above



ELEVATION

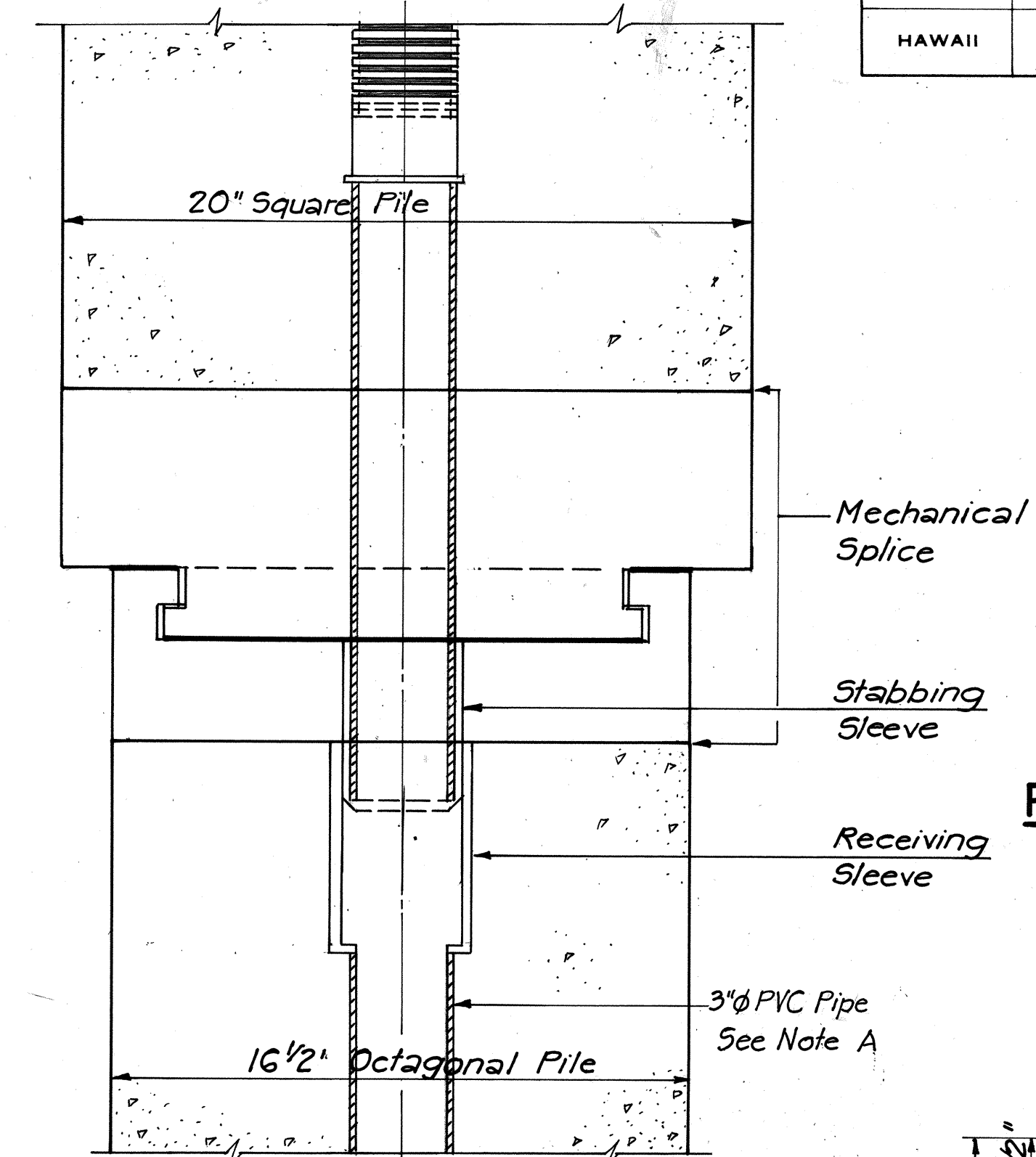


PLAN

MALE END

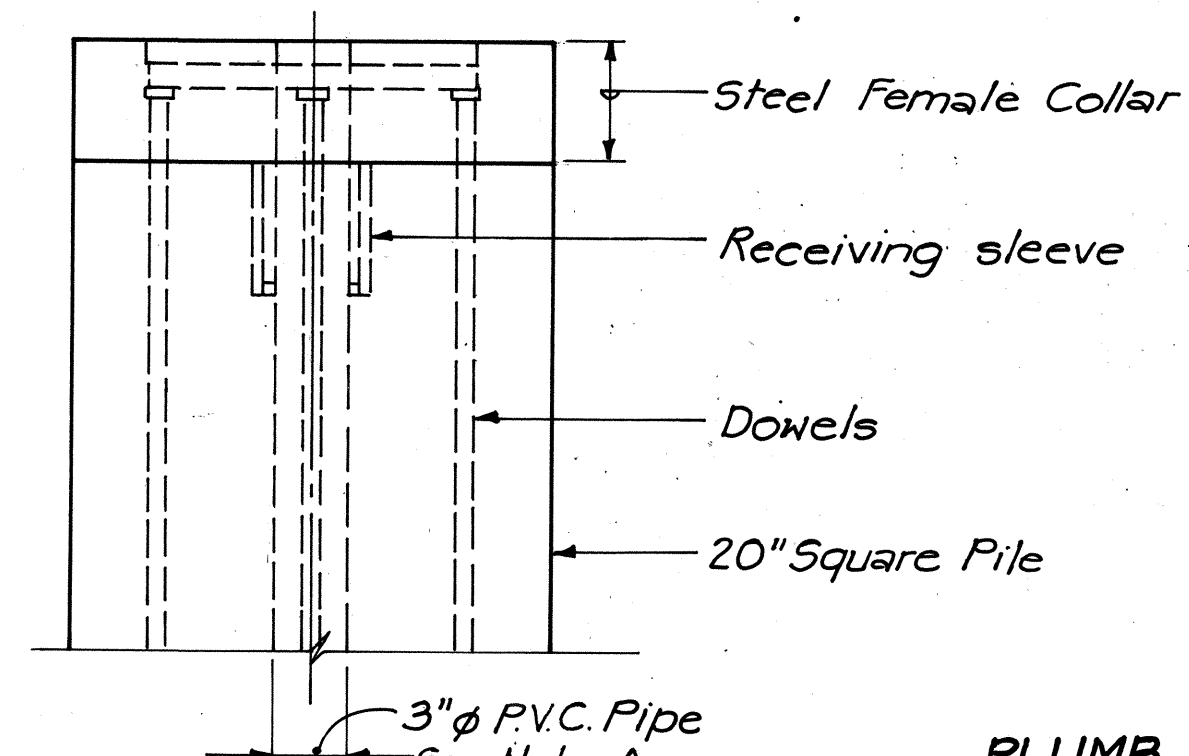
20" SQUARE PILE SPLICE

Scale: 1 1/2" = 1'-0"

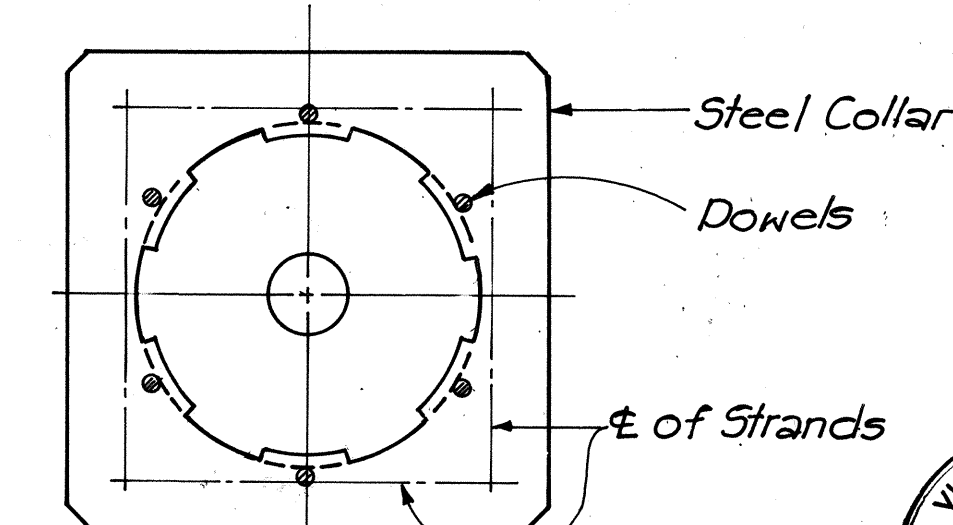


BETWEEN 20" SQ. PILE & 16 1/2" OCT. PILE

NOTE: 5" PVC Pipe & 5" Corrugated Plastic Pipe similar to Details shown on 16 1/2" Oct. Piles



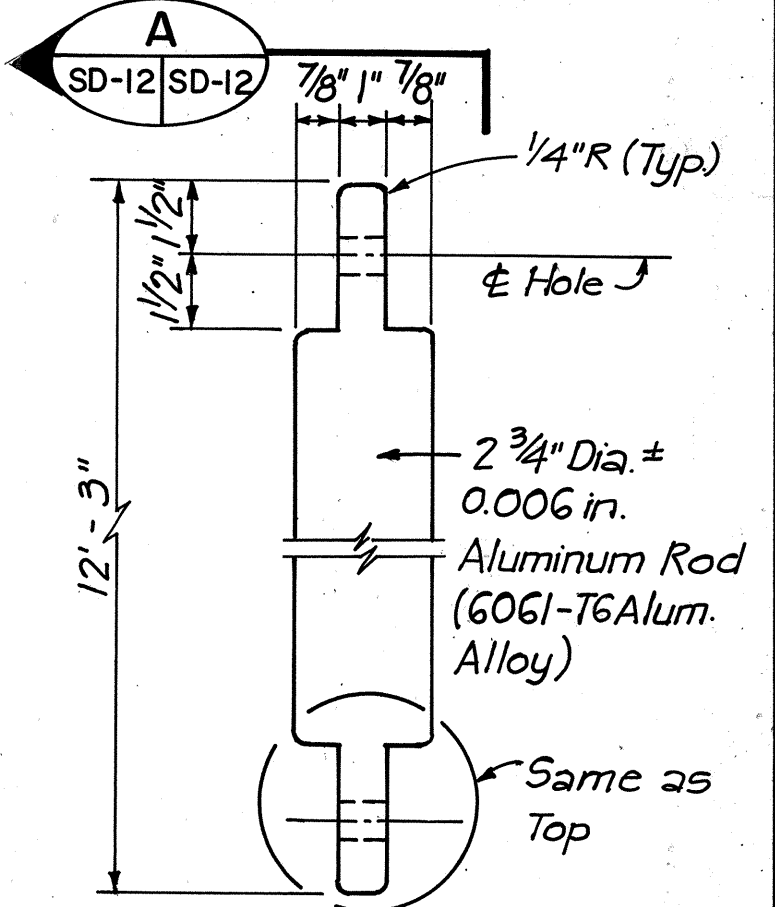
ELEVATION



PLAN

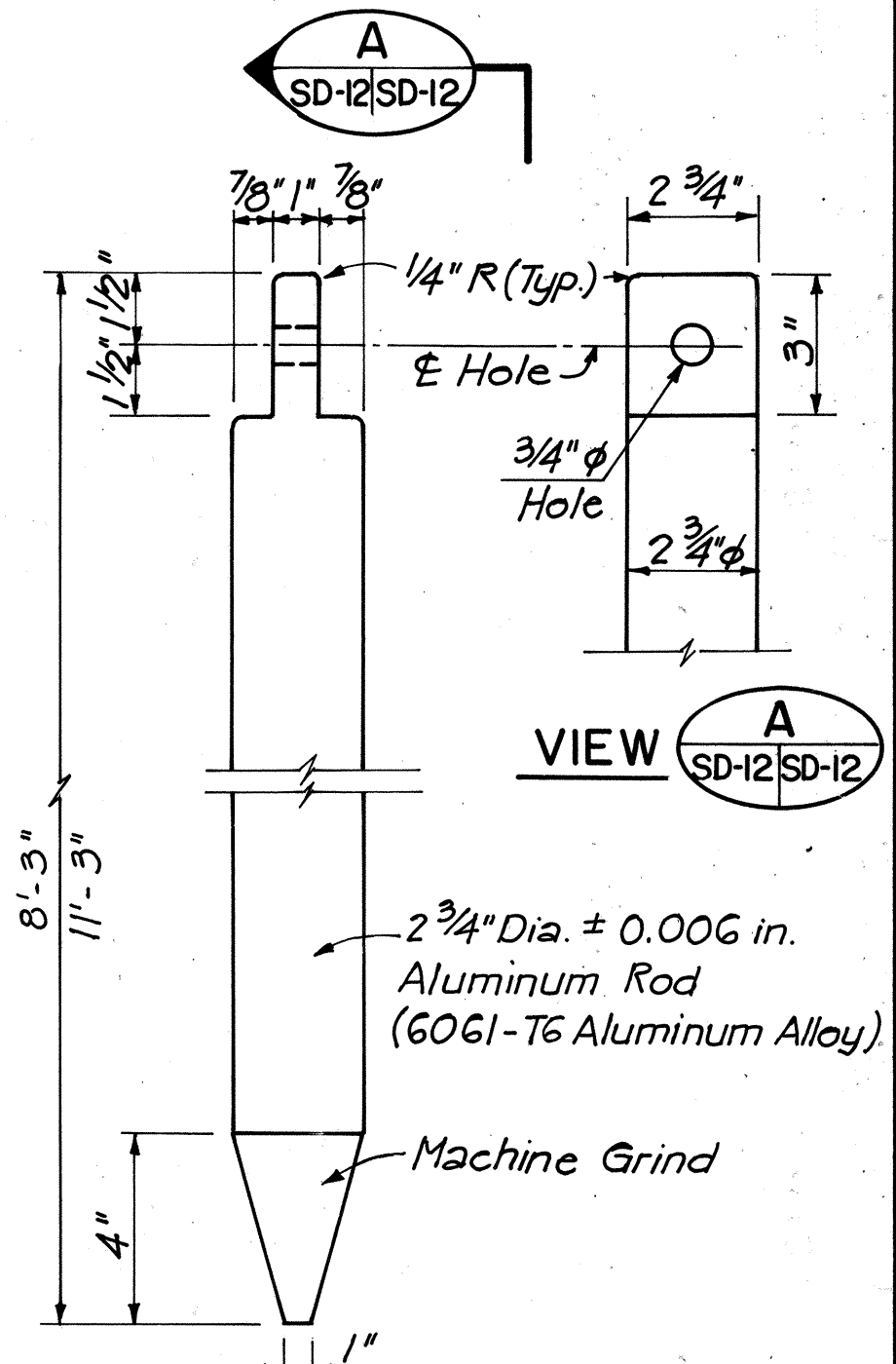
FEMALE END

NOTE: Cost of splice incidental to 20" sq. pile



PLUMB BOB DETAIL B

Scale: 3" = 1'-0"



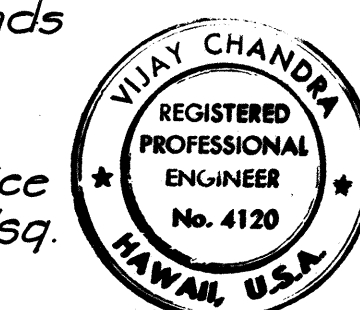
PLUMB BOB DETAIL A

(2 required)
Scale: 3" = 1'-0"

PLUMB BOB NOTES:

1. All sharp edges shall be rounded.
2. Allowable deviation from straight = 1/8"

3/23/78	Added Plumb Bob Detail B, Revised Detail A, Added 5" Cor. Plastic Pipe to Load Test Piles. Added Note A.
DATE	REVISION
STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION	
PRESTRESSED CONCRETE PILES DETAILS	
SHEET 3 OF 3	
KEEHI INTERCHANGE	
F.A.I. PROJECT NO. I-HI-1 (131):17	
SCALE: AS SHOWN	
DATE: AUG. 31, 77	
SHEET NO. SD-12 OF SD-17 SHEETS	

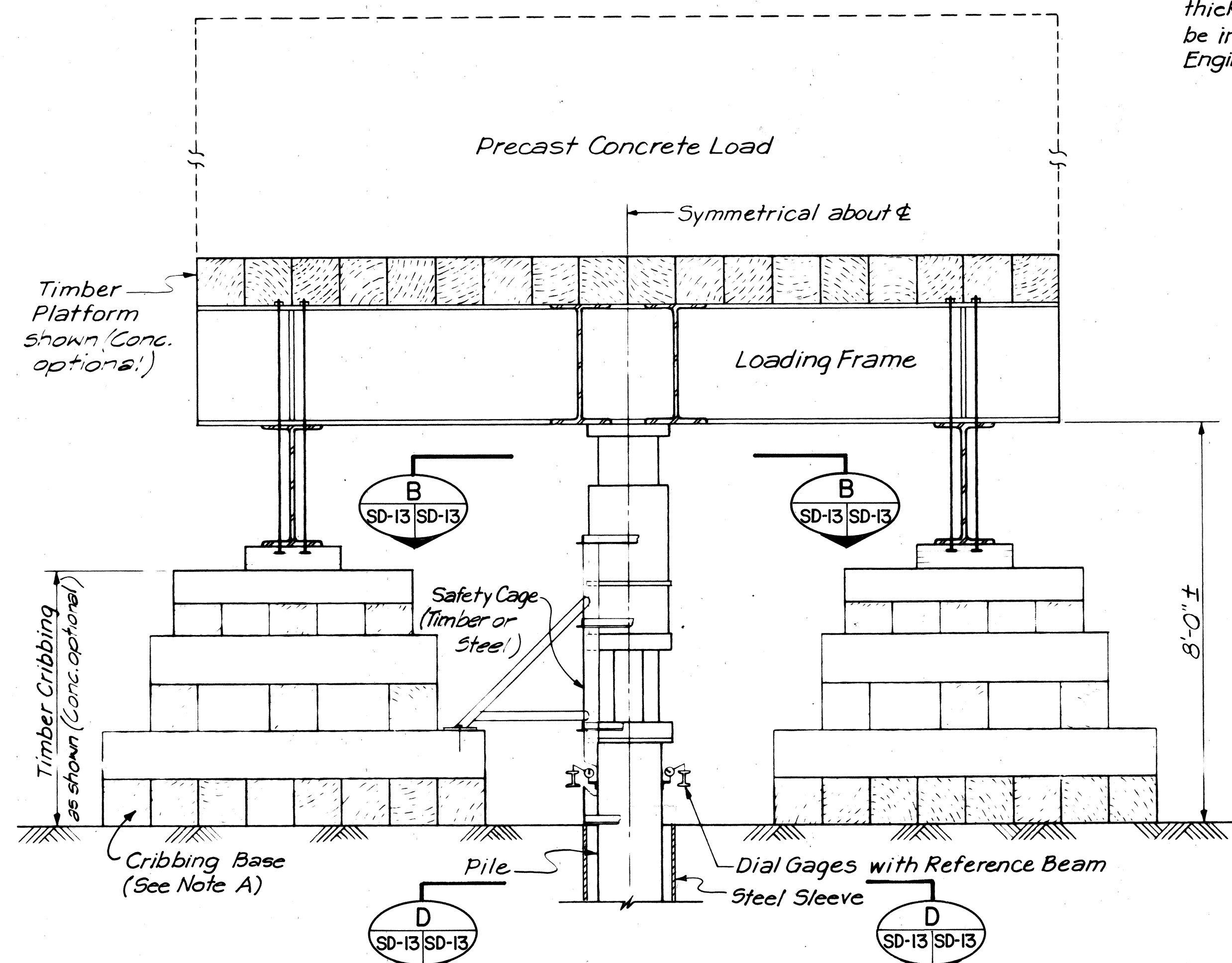


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Vijay Chandra
Signature

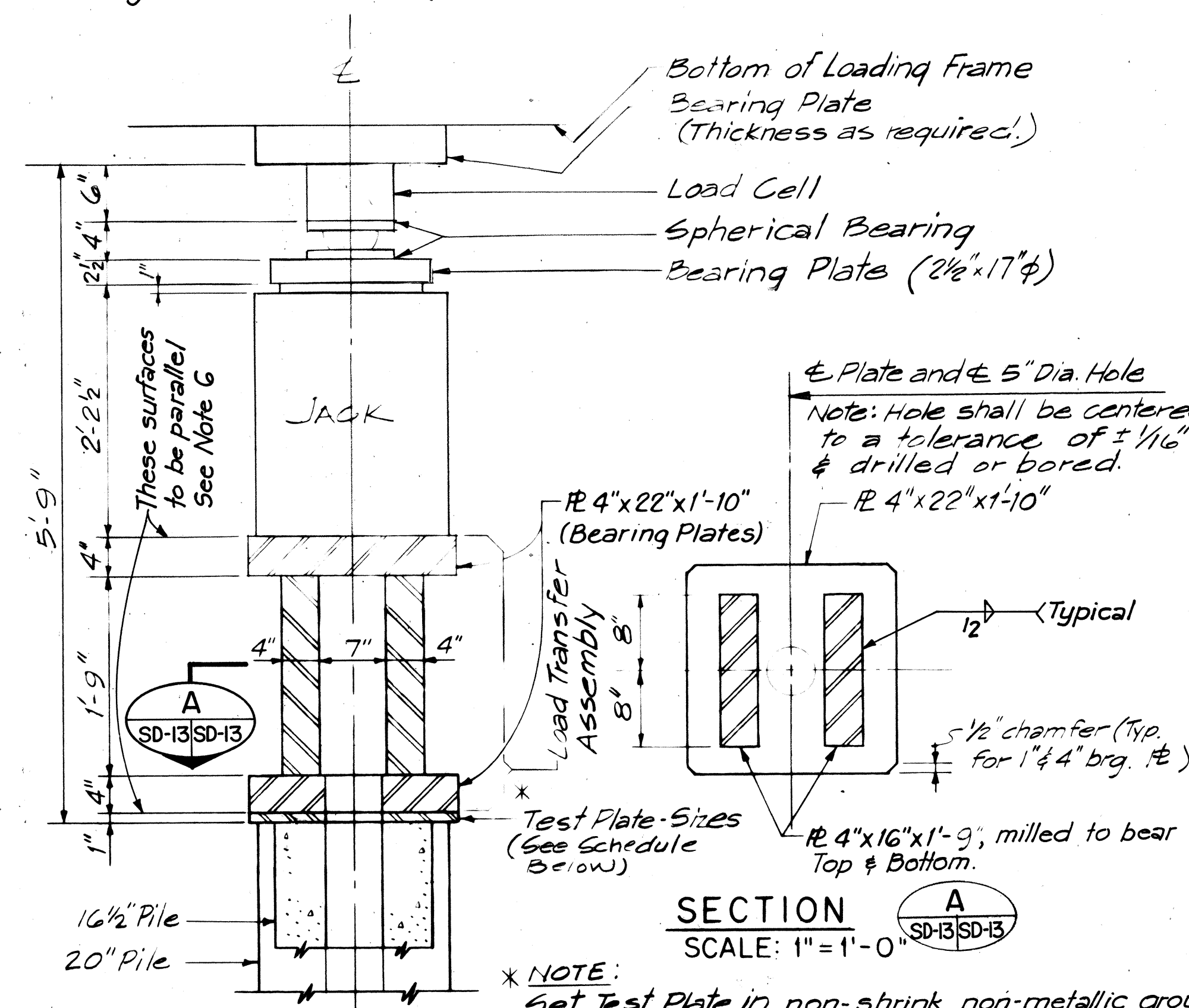
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-HI-1(131):17	1977	344	394

NOTE 'A'
The Contractor shall determine the thickness of the crust at the locations of load tests before proceeding with the load tests. The cribbing base dimensions are 10' x 24' for a crust of 6' thickness or more. In case the thickness of crust is found to be between 4' to 6', the cribbing base shall be increased to 10' x 28'. If the thickness of crust is less than 4 feet, the Engineer shall designate a new location(s) for the load test(s).



SCHEMATIC SET-UP FOR APPLYING LOADS TO TEST PILE

SCALE: 1/2" = 1'-0"

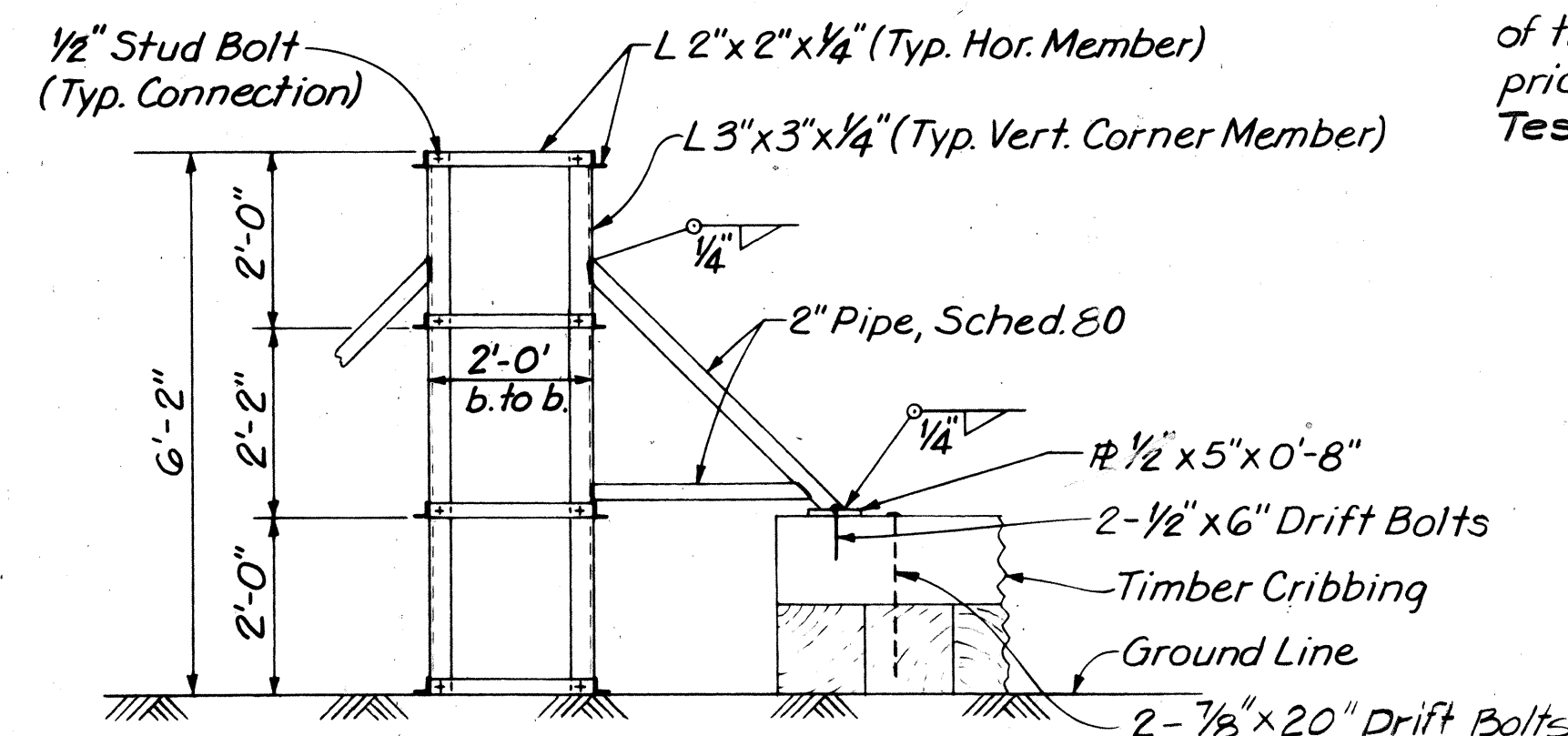


DETAIL FOR JACKING SET-UP

SCALE: 1" = 1'-0"

Schedule			
Test Plate - Size	Pile Size	Plate Size	Hole Size
16 1/2"	16 1/2"	16 1/2"	16 1/2"
20"	20"	20"	20"
16 1/2"	16 1/2"	16 1/2"	16 1/2"

* NOTE:
Set Test Plate in non-shrink non-metallic grout. Set on Pile so as to obtain full bearing. Plate surfaces receiving grout need not be planed.



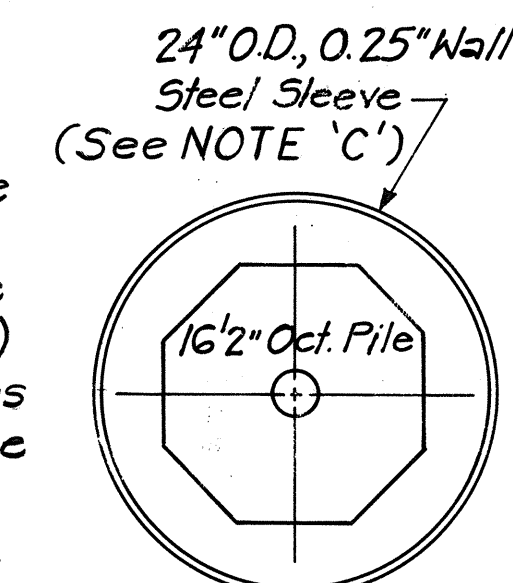
TYPICAL ELEVATION

NOTE 'B'
Loading set-up of Load Test For Trestle Structure shall provide similar detail shown on Schematic Set-up For Applying Loads To Test Pile on this sheet, including safety cage

LOAD FRAME NOTES :

1. The Loading Frame shall be designed to support load required to permit the Test Pile to be loaded to 500 Tons. Loading blocks to be provided by Contractor.
2. Design of Frame shall conform to the latest Edition of the Manual of Steel Construction by the American Institute of Steel Construction. Allowable stresses in this manual may be increased by 20% for main structural members. This increase in allowable stresses will not be allowed for Connections, Bolts and Welds.
3. At the option of the Contractor, the Loading Frame may be fabricated from new or used structural steel shapes; or loading frames previously used in other projects may be used provided the design and construction of the frame to be furnished conforms to the requirements in Notes 1 and 2 above.
4. Timber Cribbing shall have a soil bearing area of not less than 160 square feet at each side of loading Jack.
5. Timber Cribbing and Platform may be of new or used sound timber and shall be of adequate size and grade to safely carry the loads imposed upon them resulting from the Load Tests.
6. Surfaces of all bearing plates shall be planed or milled smooth to provide full surface contact, Top & Bottom of Load Transfer Assembly to be planed after welding.

NOTE 'C'
Load Test Piles shall be backfilled and Sleeve removed in accordance with Section 505.03(L) of the Special Provisions prior to performing the Test.

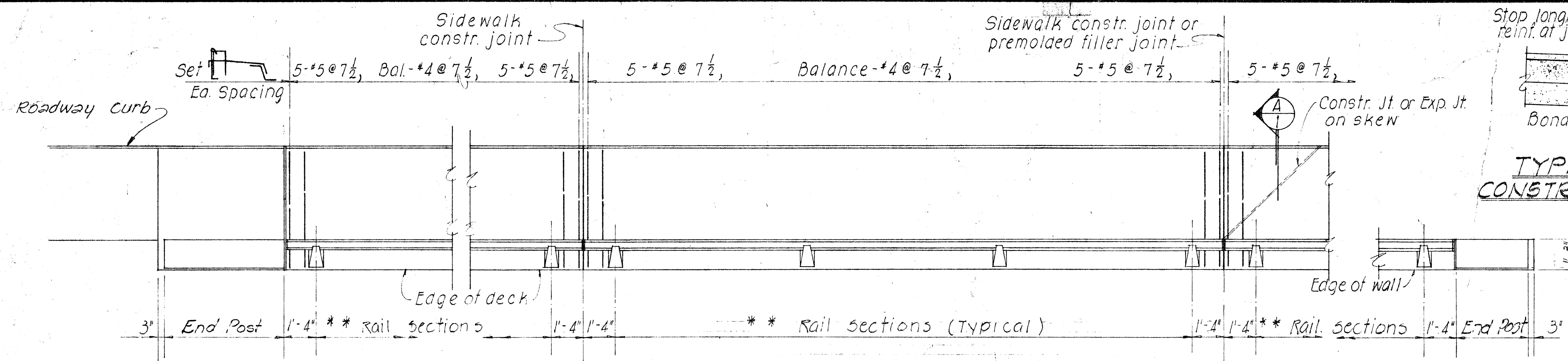


16 1/2" OCT. PILE

SECTION D SD-13/SD-13

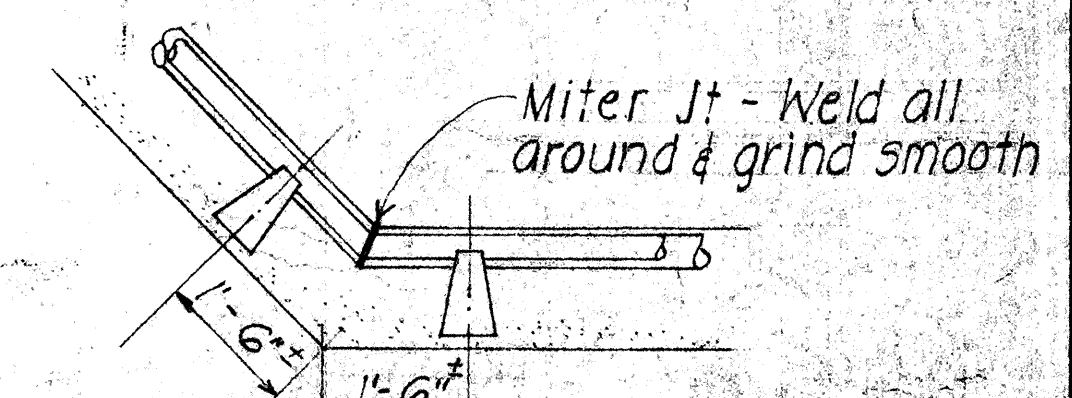
11/16/78	Deleted "LOAD TEST FOR TRESTLE STRUCTURE", SECTION 'E' & DETAIL 'X'. Revised SECTION 'D'. Added NOTE 'C'.
DATE	REVISION
STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION	
LOAD TEST SET-UP	
KEEHI INTERCHANGE F.A.I. PROJECT NO. 1-HI-1 (131):17	
SCALE: AS SHOWN	DATE: AUG. 31, '77
SHEET NO. SD-13 OF SD-17 SHEETS	

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TYPICAL SIDEWALK CONSTRUCTION JOINT DETAIL
Scale: 1/2" = 1'-0"

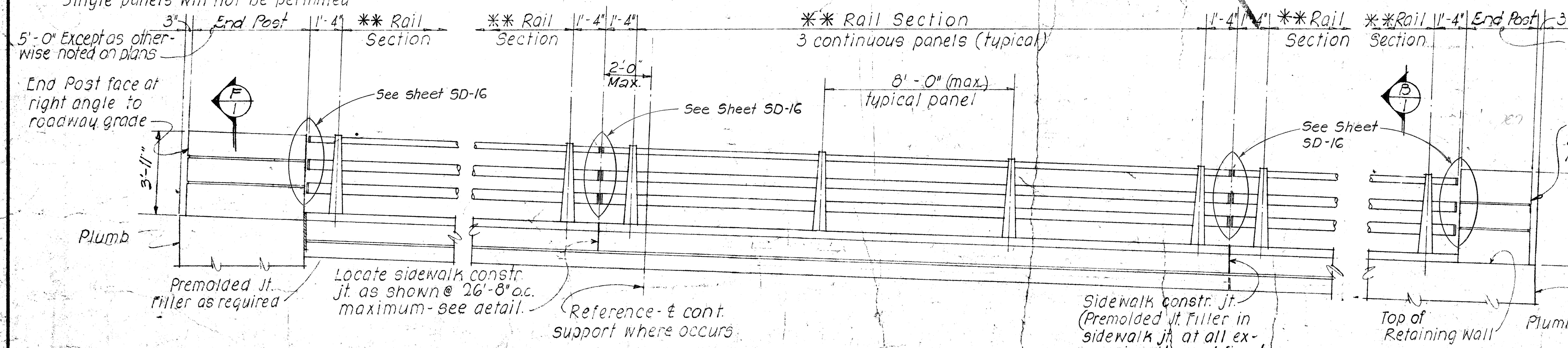
AT SKEW
Scale: 1/2" = 1'-0"



TREATMENT OF RAILING AT SKEWED ENDS
Scale: 1/2" = 1'-0"

**** NOTE!!**

The contractor shall layout the railing to provide 3 continuous panels whenever possible. 2 or 4 continuous panels will be permitted to complete the layout when 3 continuous panels cannot be made. Single panels will not be permitted.



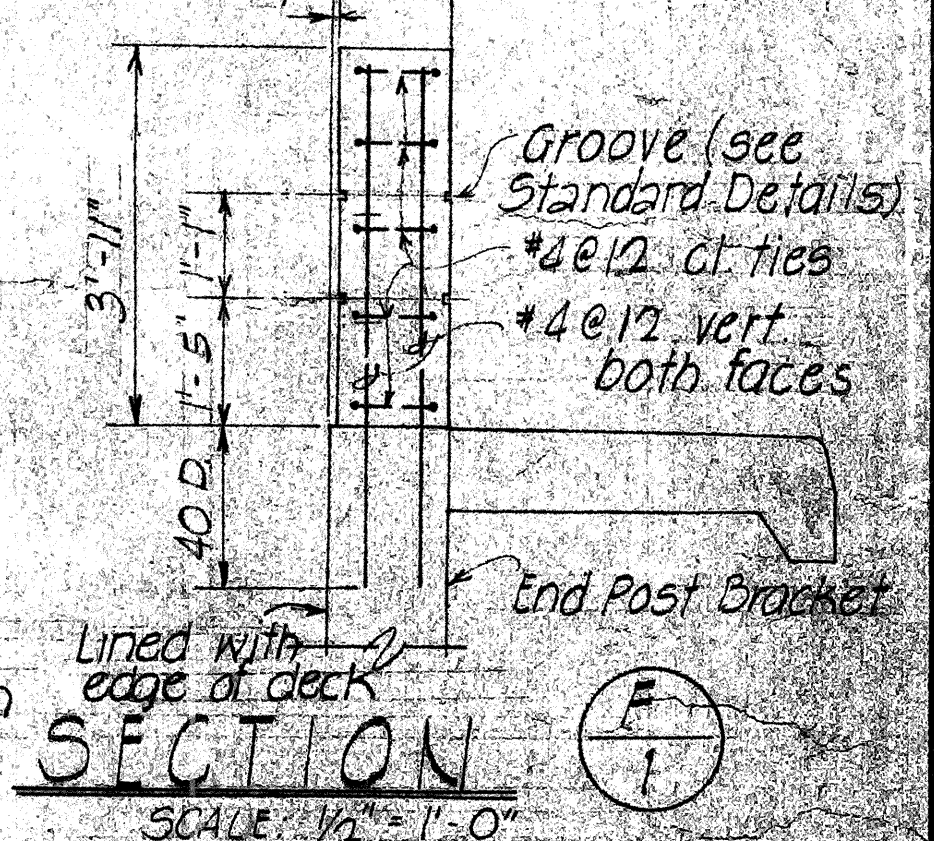
NOTES!!

1. Rail Posts shall be normal to grade.
2. Provide double posts at each expansion jt., fixed bridge jt. & as shown on detailed drawings.

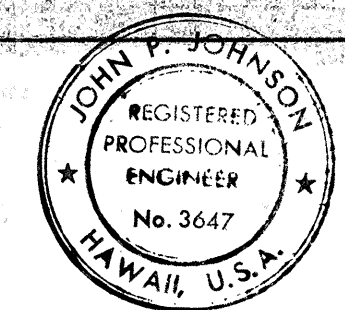
Except as otherwise noted on plans

End Post face at right angle to roadway grade.

Light Std. - To be placed between interm. post at approx. station shown on plans & 3'-0" min. from all joints



NO.	REVISION	APPROVED BY	DATE
1.	Clarify sidewalk reinf	H.C.	10-2-10
2.	Change title block rail designation. Change note.	H.C.	2-2-12



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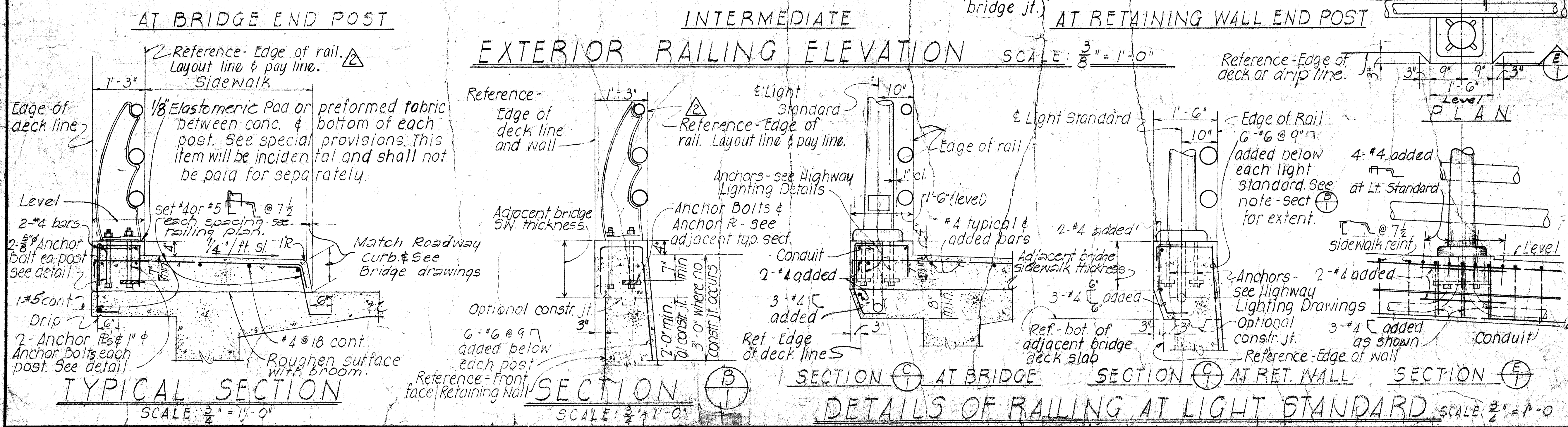
John P. Johnson

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

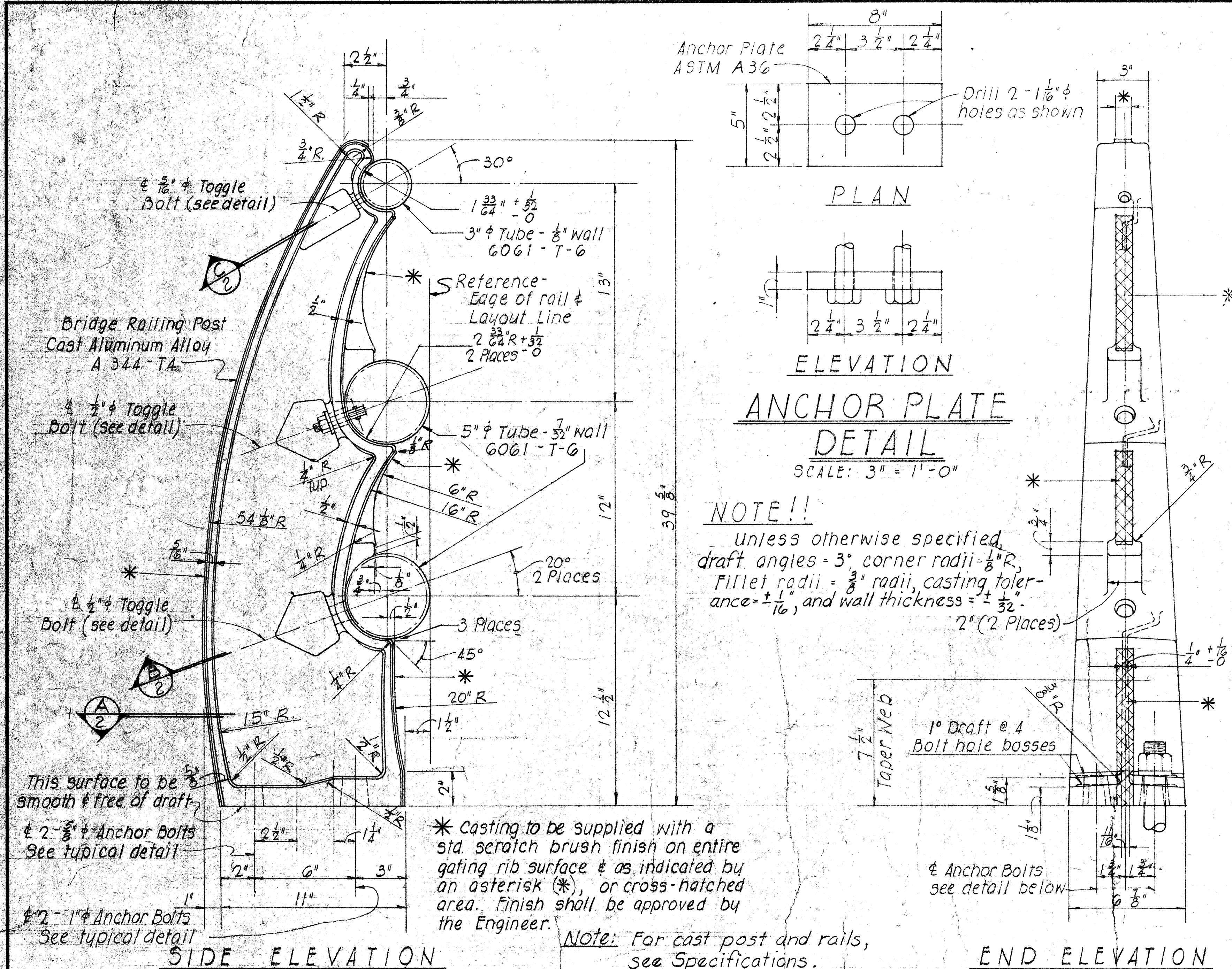
STANDARD DETAILS
ALUMINUM BRIDGE RAILING
THREE RAIL COMBINATION TYPE

KEEHI INTERCHANGE
FA.I. PROJ. NO. 1-HI-1(131) : 17
SCALE: AS SHOWN Date: AUG. 31, 1977
SHEET No. 50-14 OF 50-17 SHEETS DB-203

EXTERIOR RAILING ELEVATION
Scale: 3/8" = 1'-0"

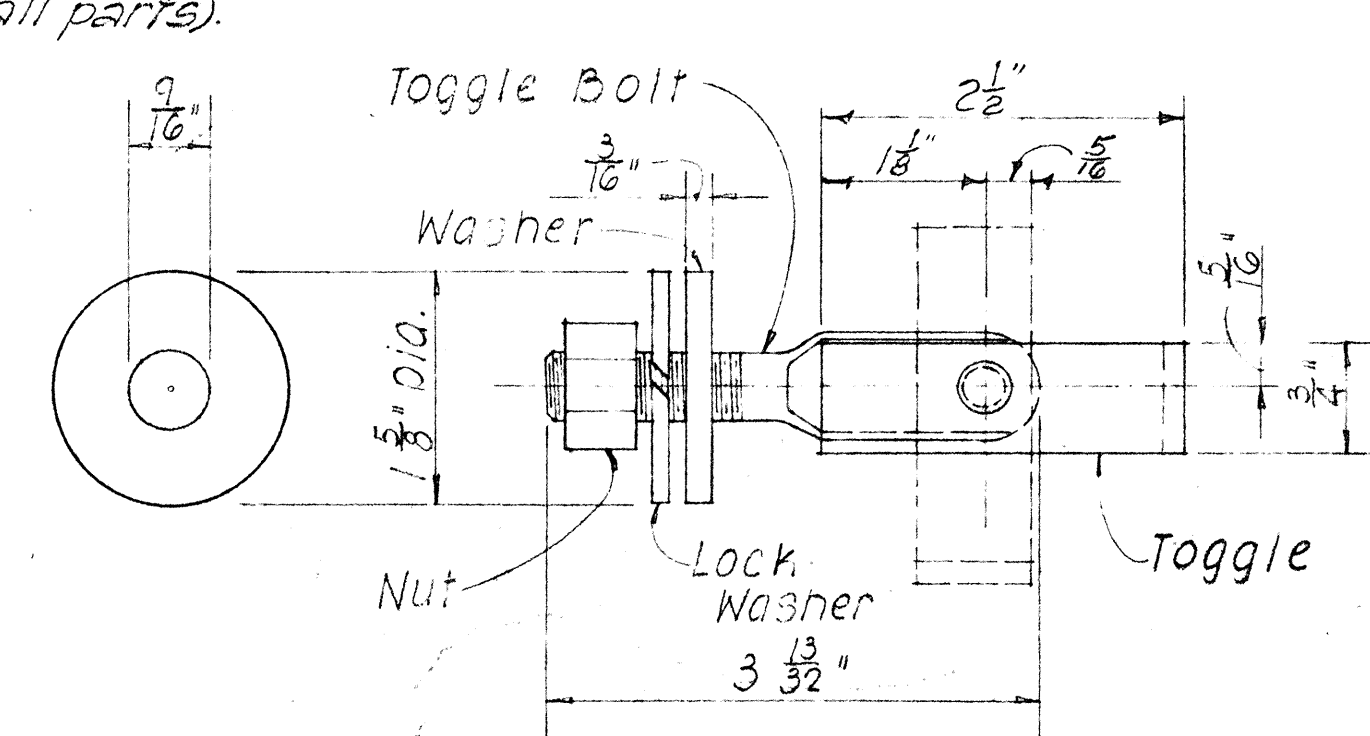


DESIGNED BY
CHECKED BY
DATE
NOTES
NO.

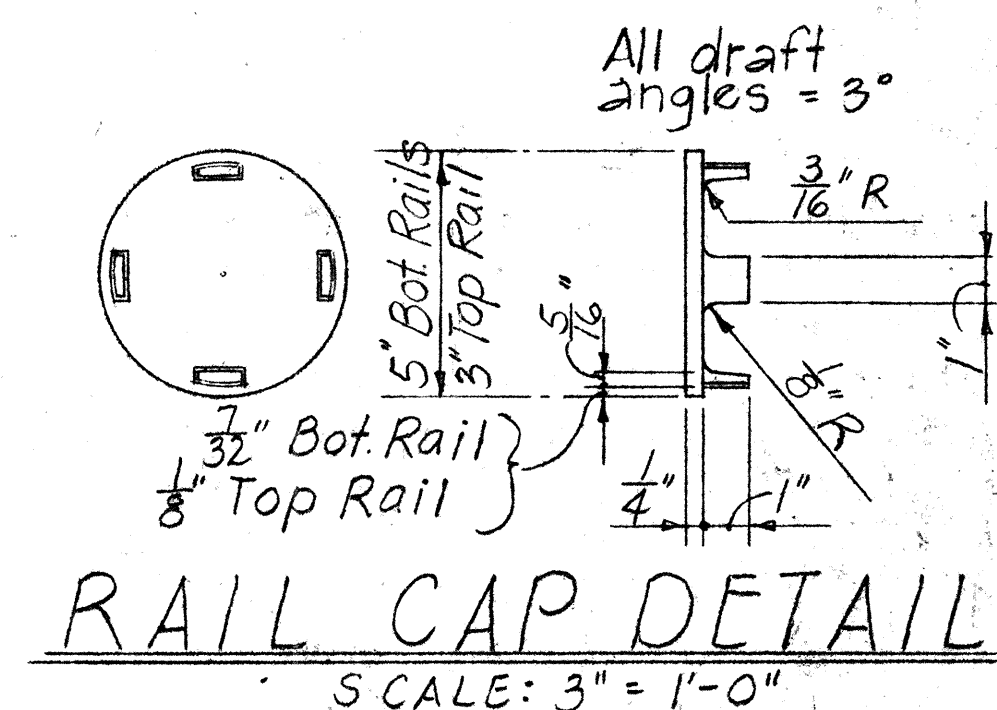


NOTE:

Toggle Bolt- Cadmium plating on steel- Type NS 0.0005" thick
ASTM- A165 (all parts).

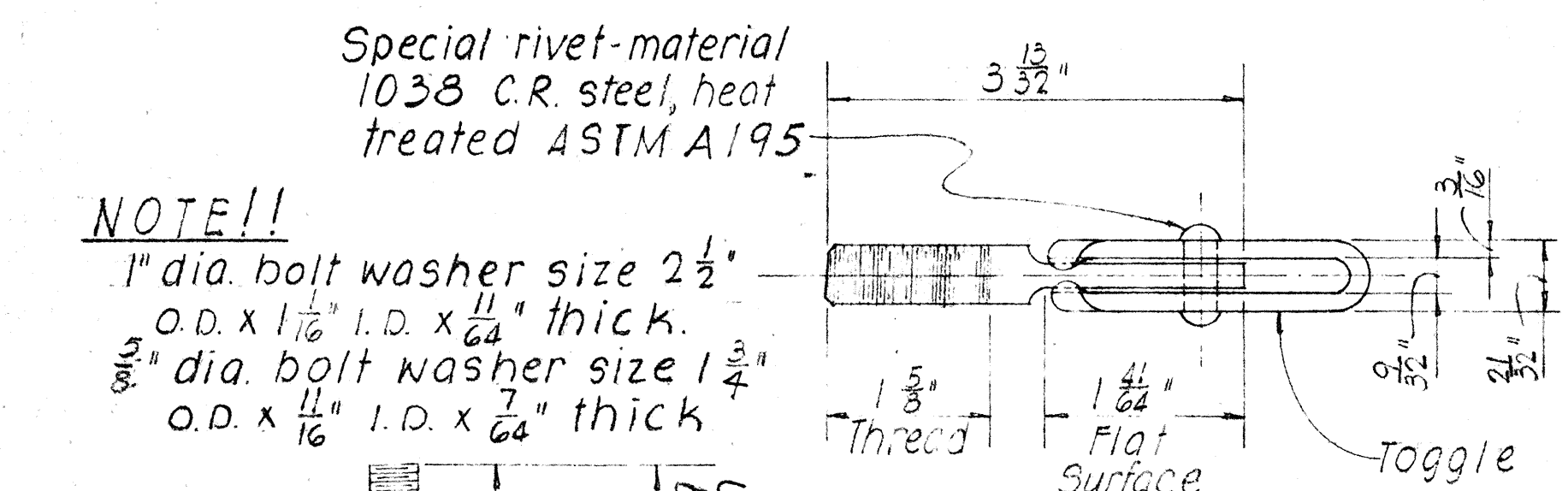


WASHER SIDE ELEVATION ASSEMBLAGE

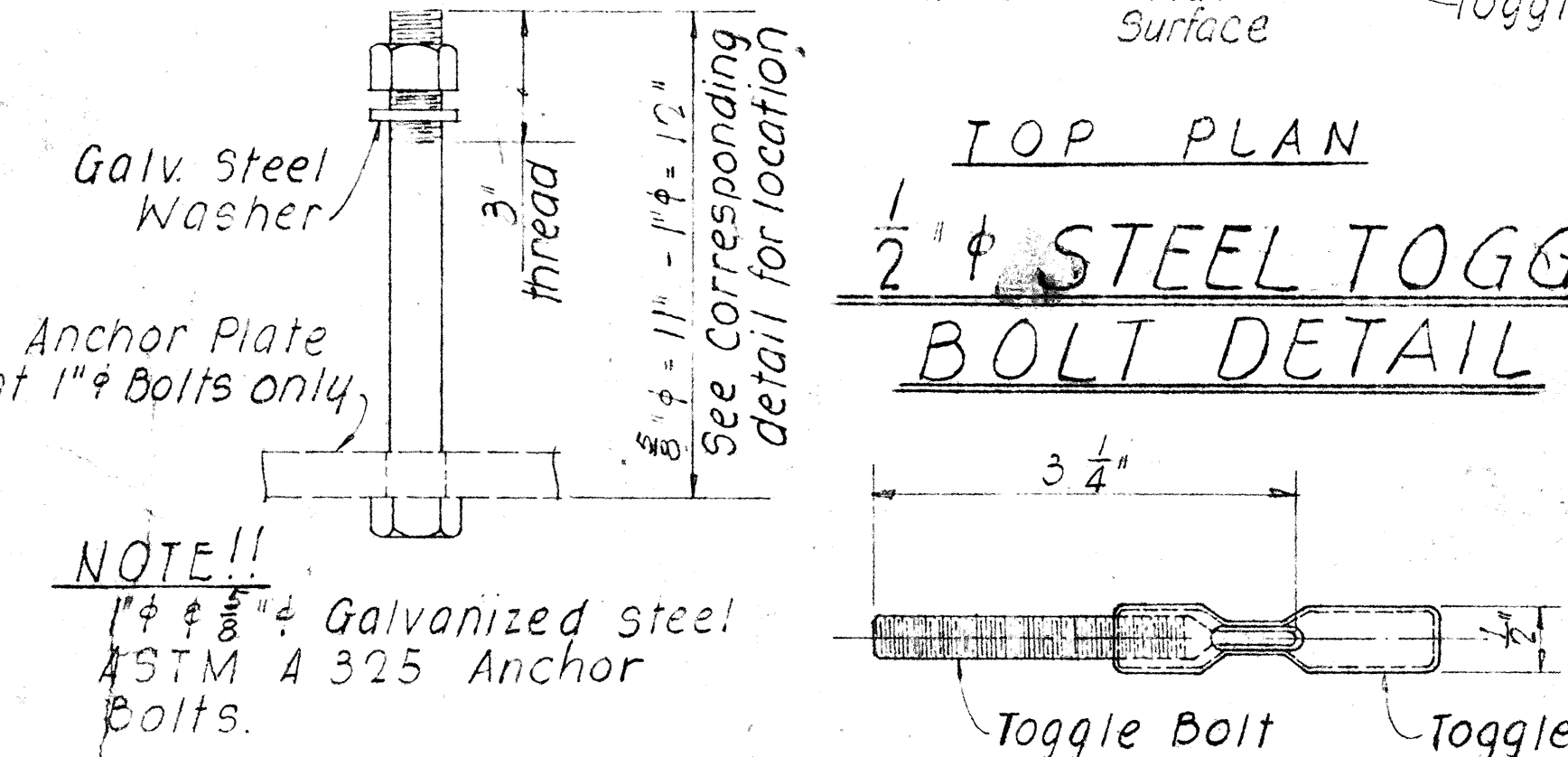


RAIL CAP DETAIL

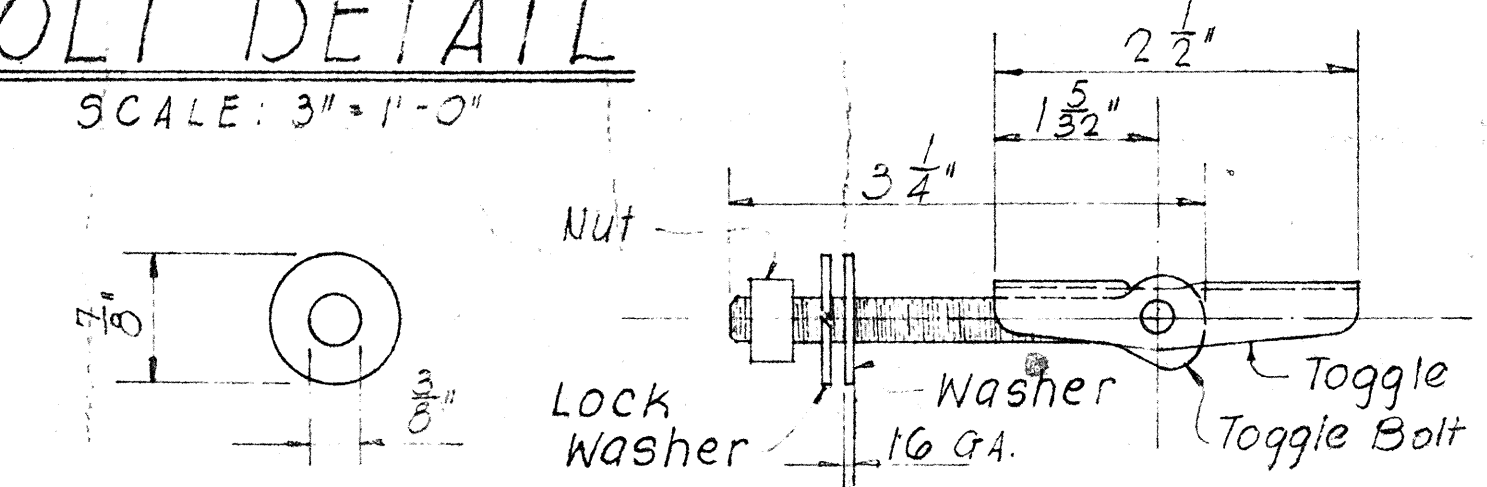
SCALE: 3" = 1'-0"



TOP PLAN
STEEL TOGGLE
BOLT DETAIL



TYPICAL ANCHOR
BOLT DETAIL
SCALE: 3" = 1'-0"



WASHER ELEVATION ASSEMBLAGE

5/16" ϕ STEEL TOGGLE BOLT DETAIL

APPROVAL RECOMMENDED:
Paul T. Yamashita 1-23-70
BRIDGE DESIGN ENGINEER DATE

APPROVED Paul T. Yamashita 1-28-70
For ASSISTANT CHIEF, ENGINEERING DATE

NO.	REVISION	APPROVED BY	DATE
	<i>Change title block rail designation. Delete note.</i>		2-4-

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

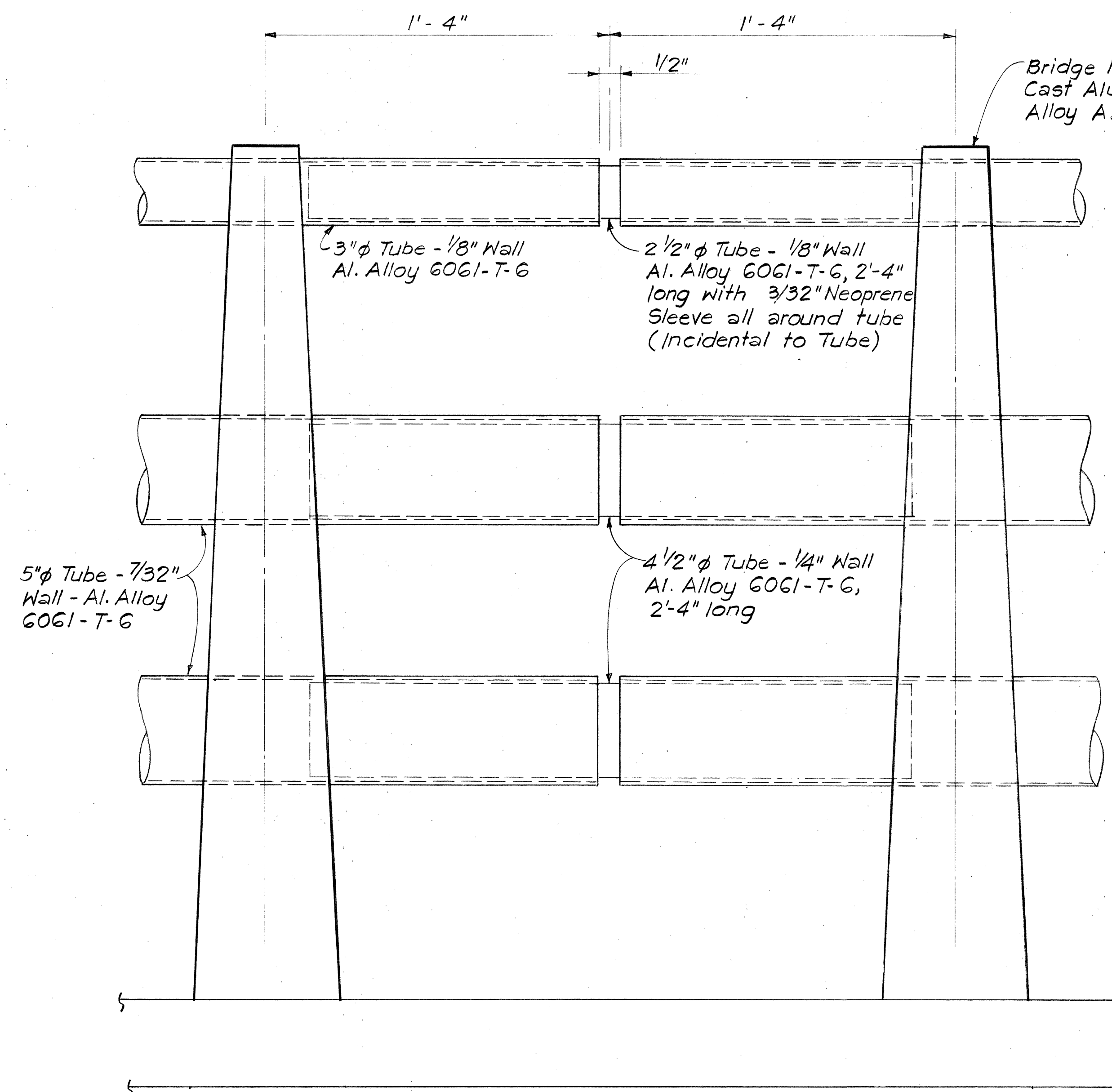
STANDARD DETAILS

ALUMINUM BRIDGE RAILING
THREE RAIL COMBINATION TYPE

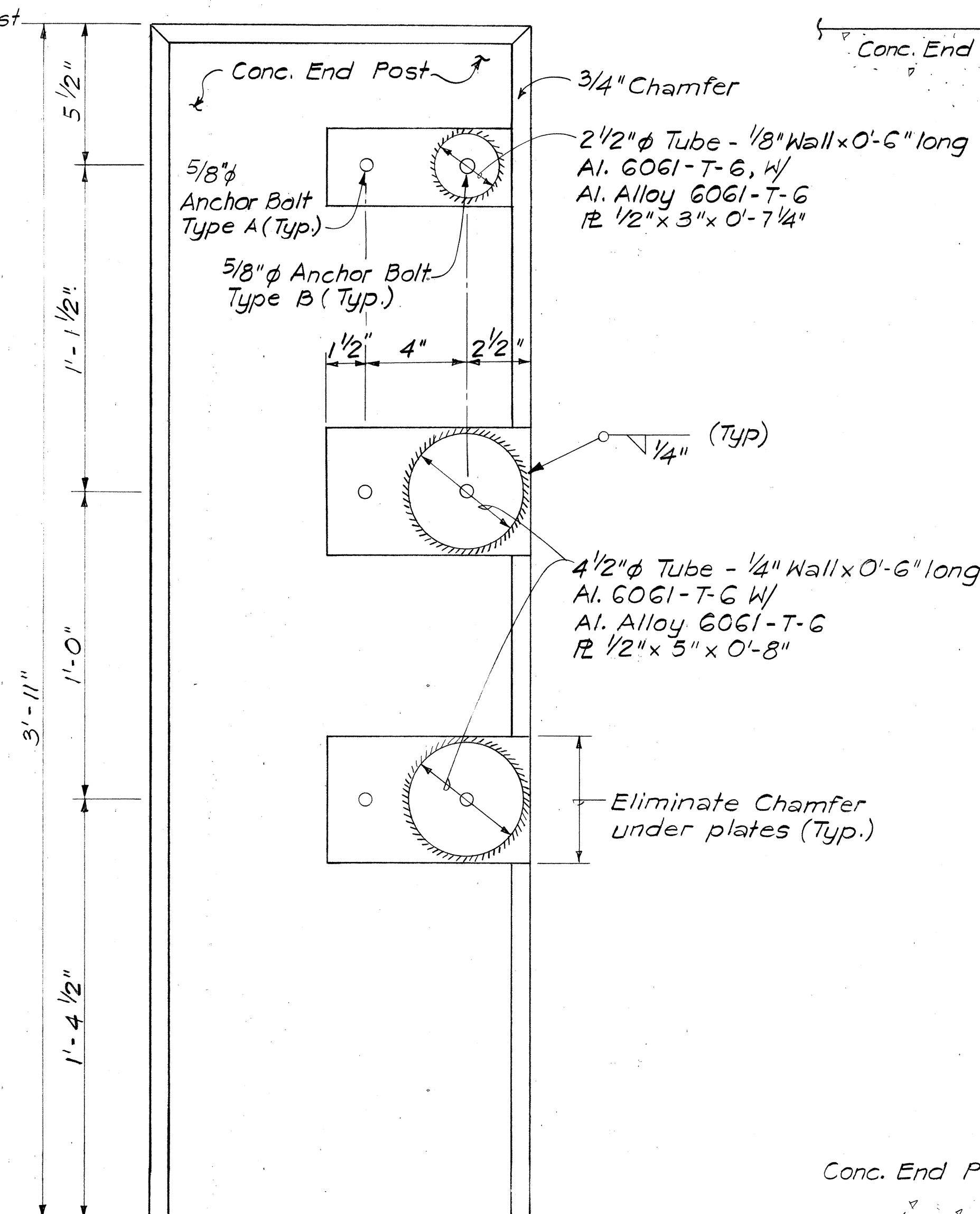
Date _____

SHEET No. 50-15 OF 50-15 SHEETS DB-208

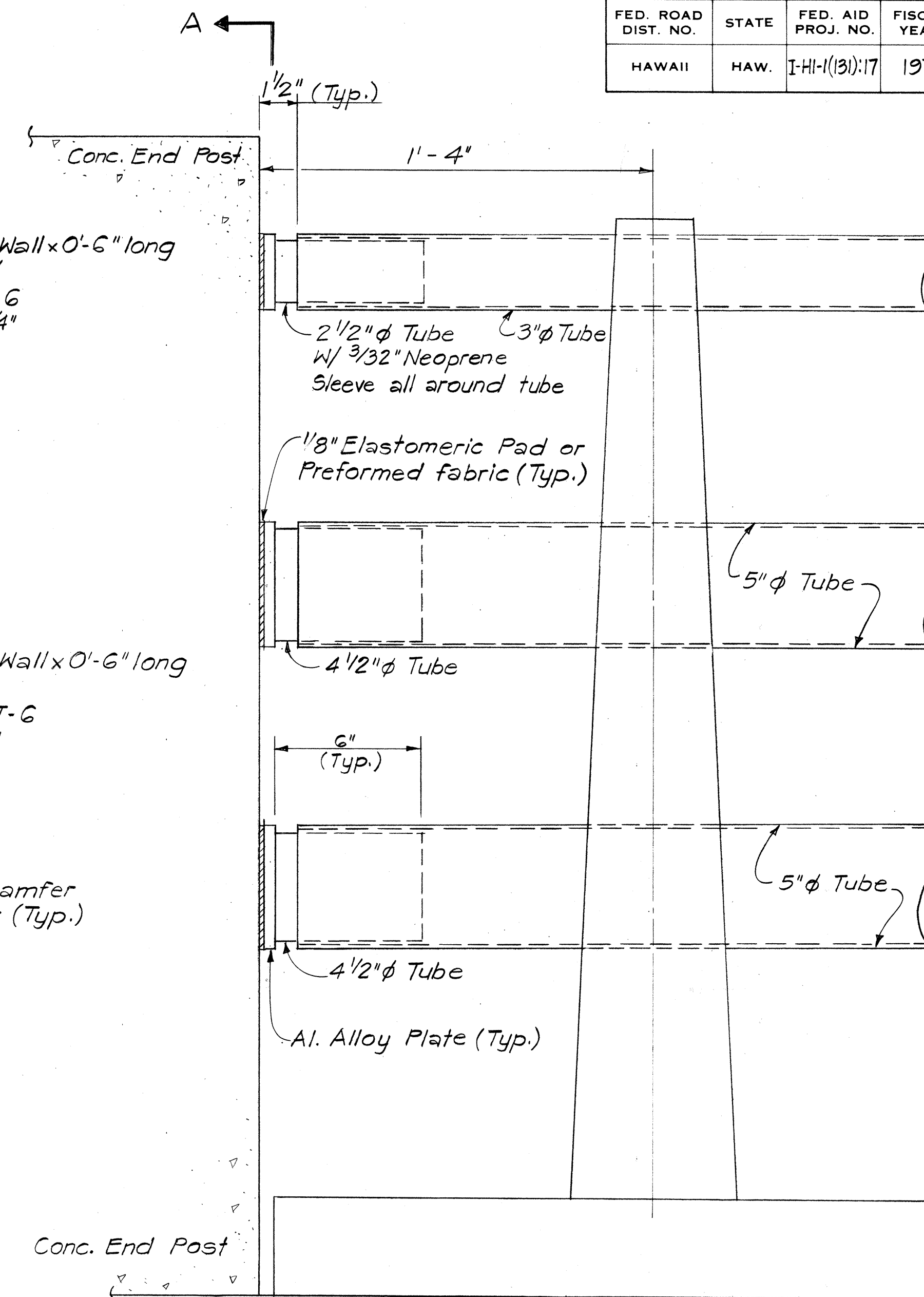
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(131):17	1977	347	394



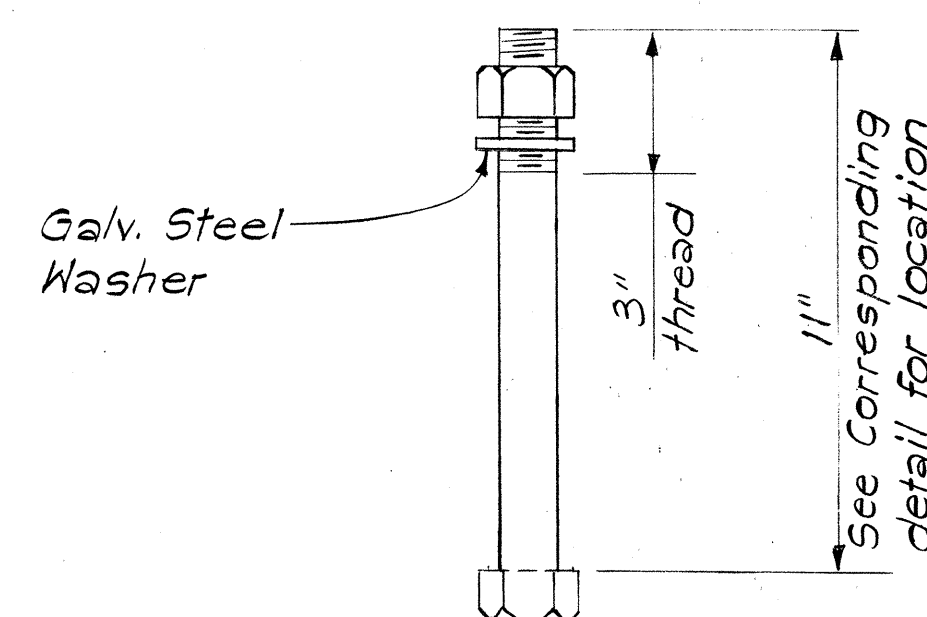
DETAIL AT INTERMEDIATE JOINTS
Scale: 3" = 1'-0"



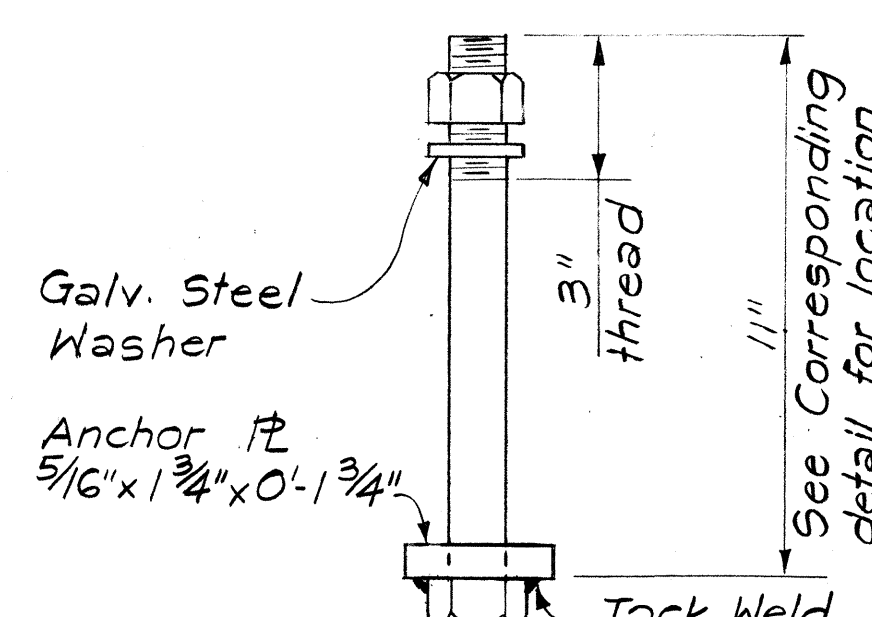
SECTION A-A
Scale: 3" = 1'-0"



DETAIL AT END POST
Scale: 3" = 1'-0"

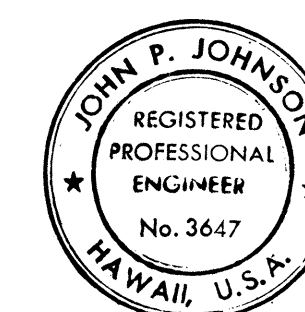


TYPICAL TYPE A ANCHOR BOLT DETAIL
N.T.S.



TYPICAL TYPE B ANCHOR BOLT DETAIL
N.T.S.

NOTE
5/8" φ dia. bolt washer size
1 3/4" O.D. x 1/16" I.D. x 7/64" thick
5/8" φ Galvanized Steel ASTM
A 325 Anchor Bolts



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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION
RAILING JOINT DETAILS
ALUMINUM BRIDGE RAILING
THREE RAIL COMBINATION TYPE
KEEHI INTERCHANGE
F.A.I. PROJ. NO.1-HI-1(131):17
SCALE: AS SHOWN DATE: Jan. 13, 1978
SHEET NO. 50 OF 50 SHEETS

