STRUCTURAL GENERAL NOTES:

General:

H : : : :

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DRAWN BY____
TRACED BY___
DESIGNED BY_
QUANTITIES BY_
CHECKED BY___

- A. Workmanship and materials shall conform to the AASHTO LRFD Bridge Design Specification, 7th Edition, Hawaii Standard Specifications for Road and Bridge Construction (2005 Edition), and HDOT Design Criteria for Bridges and Structures, August 8, 2014.
- B. The contractor shall compare all the contract documents with each other and report in writing to the engineer all inconsistencies and omissions.
- C. The contractor shall take field measurements and verify field conditions and shall compare such field measurements and conditions with the drawings before commencing work. Report in writing to the engineer all inconsistencies and omissions.
- D. The contractor shall be responsible for coordinating the work of all trades.
- E. The contractor shall be responsible for means and methods of construction, workmanship and job safety.
- F. The contractor shall provide temporary shoring and bracing as required for stability of structural members and systems.
- G. Construction loading shall not exceed design live load unless special shoring is provided. Permitted construction loads shall be properly reduced in areas where the structure has not attained full design strength.
- H. The contractor shall be responsible for protection of the adjacent properties, structures, streets and utilities during the construction period. Any damaged or deteriorated property shall be restored to the condition prior to the beginning of work or better at no cost to the State.
- I. Details noted as typical on the structural drawings shall apply in all conditions unless specifically shown or noted otherwise.
- J. Elevations and details of the existing bridges and other miscellaneous structures as shown on these plans are based on as-built drawings. The contractor shall be responsible for verifying all existing elevations and existing structure details and shall notify the engineer in writing of any discrepancies for further action.

RETAINING WALL DESIGN PARAMETERS

| Ite | em | Wall 1 | Wall 4 |
|------------------------|------------------------------------|--------|--------------|
| 0.2-second spectral re | sponse coefficient, s_{s} | 0.39 | 0. 39 |
| 1.0-second spectral re | sponse coefficient, s ₁ | 0.11 | 0.11 |
| Horizontal peak grou | und coefficient, PGA | 0.17 | 0.17 |
| Site | Class | В | В |
| Seismic De | esign Zone | 1 | 1 |
| Bearing Capacity | Extreme Event | 7,200 | 13,500 |
| (psf) | Strength | 3,240 | 6,075 |
| Passive Earth | Extreme Event | 450 | 600 |
| Pressure (pcf) | Strength | 250 | 300 |
| Lateral Earth | Active (1.5:1 Slope) | 46 | 46 |
| Pressure (pcf) | At-Rest (1.5:1 Slope) | 75 | 75 |
| Dynamic Lateral | Unrestrained | 11.1 | 11.1 |
| Earth Pressure (pcf) | Restrained | 49.3 | 49.3 |
| Coefficient | Extreme Event | 0.31 | 0.70 |
| of Friction | Strength | 0.25 | 0.56 |

Design Criteria:

- A. Dead load
 - Weight of all components of the structures, appurtenances attached thereto, and earth covers.

| - Unit weight of concrete | - 160 pcf |
|----------------------------------------------------|---------------|
| - Compacted earth | 120 pcf |
| - Future wearing surface | 25 psf |
| B. Live load | · AASHTO HL-9 |
| C. Collision load | - AASHTO TL-3 |
| D. Seismic | - See Table |
| E. Basic Wind Speed | 105 MPH |
| F. Design soil parameters for retaining structures | See Table |

Foundation:

- A. Foundation design is based upon Geotechnical Exploration and Evaluation Report by Yogi Kwong Engineers, LLC, dated June 2018.
- B. Contractor shall provide for de-watering of excavation from either surface water, ground water or seepage. NPDES permit required for discharging into state waters.
- C. Contractor shall provide for design and installation of all cribbing, sheeting, and shoring necessary for personnel safety and to preserve excavations and earth banks, and adjacent structures and property for damage. Shoring shall conform to OSHA regulations.
- D. Excavation boundaries and grade elevations for footing shall be accepted by a geotechnical engineer licensed in the State of Hawaii prior to placing the concrete and reinforcing.
- E. Footing for Wall 1 shall bear on a 6" layer of aggregate select borrow compacted to a minimum of 95% relative density in accordance with ASTM D1557. Prior to the placement of the select borrow, the exposed soils should be scarified to a depth of 6", moisture conditioned to within 2% of optimum moisture and compacted to no less than 95% relative density in accordance with ASTM D1557. Bottom of footing shall be compacted to provide a relatively firm and smooth bearing surface prior to placement of reinforcing steel and concrete. Footing for Wall 4 shall bear on a clean rock surface. If soft and/or loose materials are encountered at the bottom of excavations, they shall be over-excavated to expose the underlying firm materials. The over-excavated area shall be backfilled with the same select borrow or the footing bottom may be extended down to the underlying competent material. Contractor may substitute flowable concrete for the select borrow upon acceptance from the engineer.
- F. Footing for Wall 1 shall bear on a 6-inch layer of aggregate select borrow (min CBR=25) compacted to no less than 95% relative density in accordance with ASTM D1557. Prior to the placement of the select borrow, the exposed soils should be scarified to a depth of 6-inches, moisture conditioned to within 2% of optimum moisture content and compacted to no less than 90% relative density in accordance with ASTM D1557. Bottom of footing shall be compacted to provide a relatively firm bearing surface prior to placement of reinforcing steel and concrete. Footing for Wall 4 shall bear on a clean, rough cut rock surface free from debris and soil.
- G. If soft and/or loose materials are encountered at the bottom of excavations, they shall be over-excavated a minimum of 24-inches and replaced to proper grade with Structure Backfill Material A (JSS703.20). Contractor may substitute flowable concrete for the select borrow upon acceptance from the engineer.
- H. Engineered fill and backfill shall be in accordance with Section 703.20 of the Hawaii Standard Specifications for Road and Bridge construction, 2005 Edition.
- I. Fill should be moisture conditioned to within two percent of the optimum moisture content and placed in horizontal lifts not to exceed six inches. Fill shall be compacted to minimum 95% relative density as measured by ASTM D1557, method A or D.
- J. Backfill behind Wall 1 and 4 shall consist of either select granular fills or CLSM. Granular backfill shall be placed in uniform lifts of no more than 8 inches in loose thickness and uniformly compacted to at least 95 percent relative compaction.

| FED. ROAD DIST. NO. | STATE | FED. AID PROJ. NO. | FISCAL YEAR | SHEET NO. | TOTAL SHEETS |
|------------------------|-------|-----------------------|----------------|--------------|-----------------|
| HAWAI'I | HAW. | NH-H1-1(274) | 2018 | 83 | 126 |

Concrete:

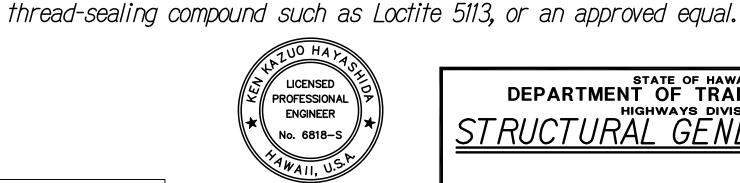
A. Concrete shall be regular weight hard rock concrete and shall have the following minimum 28-day compressive strengths and water to cement ratios: *IML*

| = a a a a y | | |
|------------------------------------|------------------------|--------------|
| | Minimum Compressive | Maxmiur |
| <u>Structural Item</u> | Strength f'c (28 days) | <u>(W/C)</u> |
| 1. Retaining Walls and Foundations | 4,000 psi | <i>0.45</i> |
| 2. Light Standard Foundations | 4,500 psi | <i>0.45</i> |
| 3. All other concrete | 4,000 psi | <i>0.45</i> |
| The was of any sale in a second | | |

- B. The use of any calcium chloride in any concrete is prohibited.
- C. Concrete delivery tickets shall record all free water in the mix at batching plant, added for consistency by driver, and any additional request by contractor up to the maximum amount allowed by the mix design.
- D. Conduits, pipes, and sleeves passing through a wall not conforming to typical details shall be located and submitted to the structural engineer for approval.
- E. Construction joints may be relocated by the contractor and submitted to the structural engineer for approval. Construction joints shall be made and relocated as not to impair the strength of the structure and to minimize shrinkage stresses. All construction joints shall be cleaned, laitance removed and wetted. See typical details for specific requirements.
- F. Unless otherwise noted, chamfer all exposed concrete edges 3/4".
- G. Reinforcing bars, anchor bolts, inserts and other items to be cast in the concrete shall be secured in position prior to placement of concrete.
- H. All inserts, anchor bolts, plates, and other structural items to be cast in the concrete shall be hot-dipped galvanized according to ASTM A153 unless otherwise noted.
- I. Non-shrink grout shall be a premixed non-metallic formula, capable of developing a minimum compressive strength of 4,000 psi in 1 day and 7,000 psi in 28 days.
- J. Stay-in-place forms shall not be allowed.

Reinforcing Steel:

- A. Reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60 unless otherwise noted. Epoxy-coated dowels shall conform to ASTM A775, Grade 60 unless otherwise noted.
- B. The contractor shall not damage the epoxy coating on the dowels and deformed bars in any way during shipment, handling, or placement. Damaged epoxy coated dowels and deformed bars shall be replaced at no cost to the State. Repair of epoxy coating as approved by the Engineer shall meet ASTM A775.
- C. Clear concrete coverage for reinforcing bars shall be as follows, unless otherwise noted:
 - 1. Footings, etc. cast against earth ----- 3" 2. Footings, walls, etc. formed and exposed to earth or weather ----- 2"
- 3. Pavement slab top bars -----C. Thru bolts for guardrail connection shall conform to AASHTO M164 (ASTM A325)
- unless otherwise noted. D. Reinforcing steel shall be spliced only where indicated on plans. Provide lap
- splice length per typical details and schedule, Sheet SO.2, unless otherwise noted. E. Mechanical splice connectors shall develop, in tension, 125 percent of the specified
- minimum yield strength of reinforcing bars. F. Bar bends and hook shall be "standard hooks" in accordance with typical details
- Sheet S0.2. G. Corrosion inhibitors for reinforcing steel threaded connections shall be a



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April 30, 2020 EXPIRATION DATE OF THE LICENSE

DEPARTMENT OF TRANSPORTATION STRUCTURAL GENERAL NOTES

INTERSTATE ROUTE H-1SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA PHASE 2 - SHOULDER WORK

SHEET No. S0.1 OF 2 SHEETS

Scale: As Shown

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Date: November 30, 2018

MINIMUM SPLICE & EMBEDMENT LENGTHS

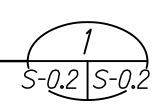
| | Lap S | Splice | | Embedment | |
|-------------|-------------|---------------|-------------|---------------|----------------------|
| | | | Stra | aight | with |
| Bar Size | Top Bars | Other Bars | Top Bars | Other Bars | Standard 90° Hook |
| #3, #4 | 29" | 21'' | 17" | 12" | 8" |
| #5 | 36" | 26" | 21'' | 15" | 10" |
| #6 | 43" | 31'' | 26" | 18" | 12" |
| #7 | 54" | 39" | 32" | 23" | 14" |
| #8 | 71'' | 51" | 42" | 30" | 16" |
| #9 | 90" | 64" | 53" | 38" | 18" |

- 1. "Top Bars" are horizontal bars with 12" or more of concrete cast below.
- 2. Splice lengths may be reduced by multiplying the tabulated values by 0.765 if the centerline of splice of adjacent bars are staggered 6'-0" o.c. for #9 bars and smaller.
- 3. Embedment lengths for straight bars may be reduced by multiplying the tabulated values by 0.80 if the bars are spaced laterally not less than 6" center-to-center, with not less than 3" clear cover measured in the direction of the spacing.
- 4. Embedment lengths for bars with 90° hook are bars with side cover, normal to plane of hook, of not less than 21/2" and cover on bar extension beyond hook not less than 2". Increase embedment length by 43% for bars not meeting these requirements.

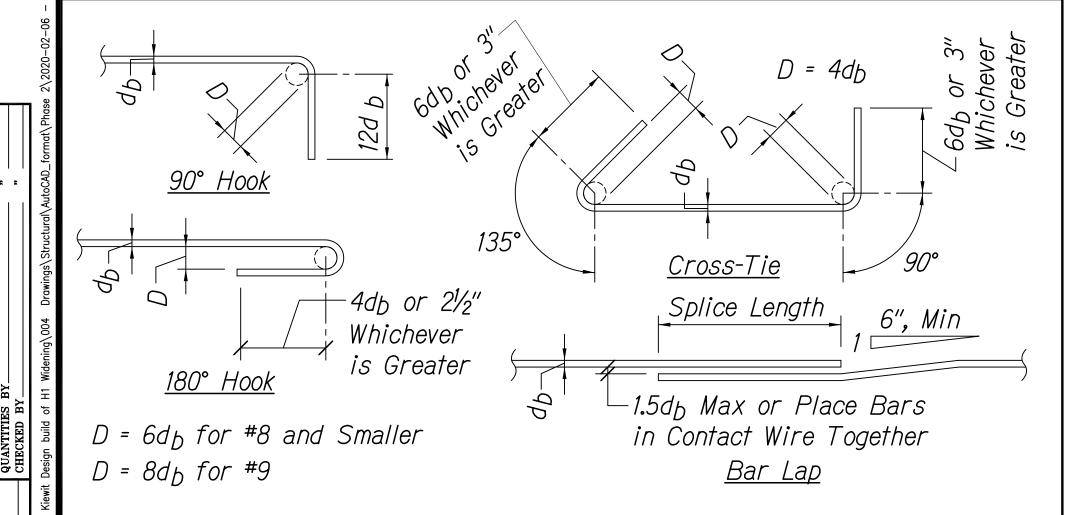
TYPICAL REBAR SPLICE AND EMBEDMENT LENGTH SCHEDULE

Not to Scale

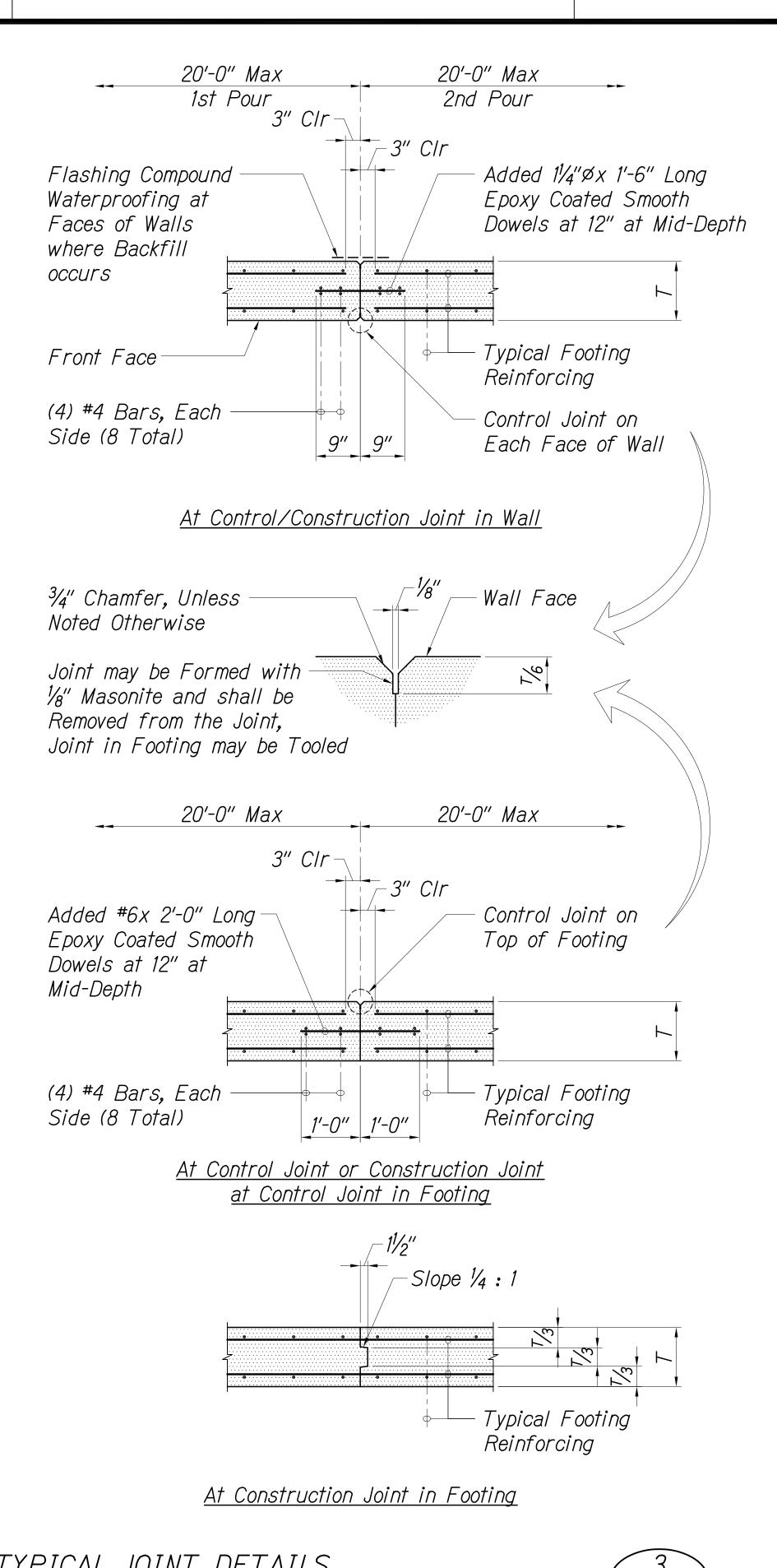
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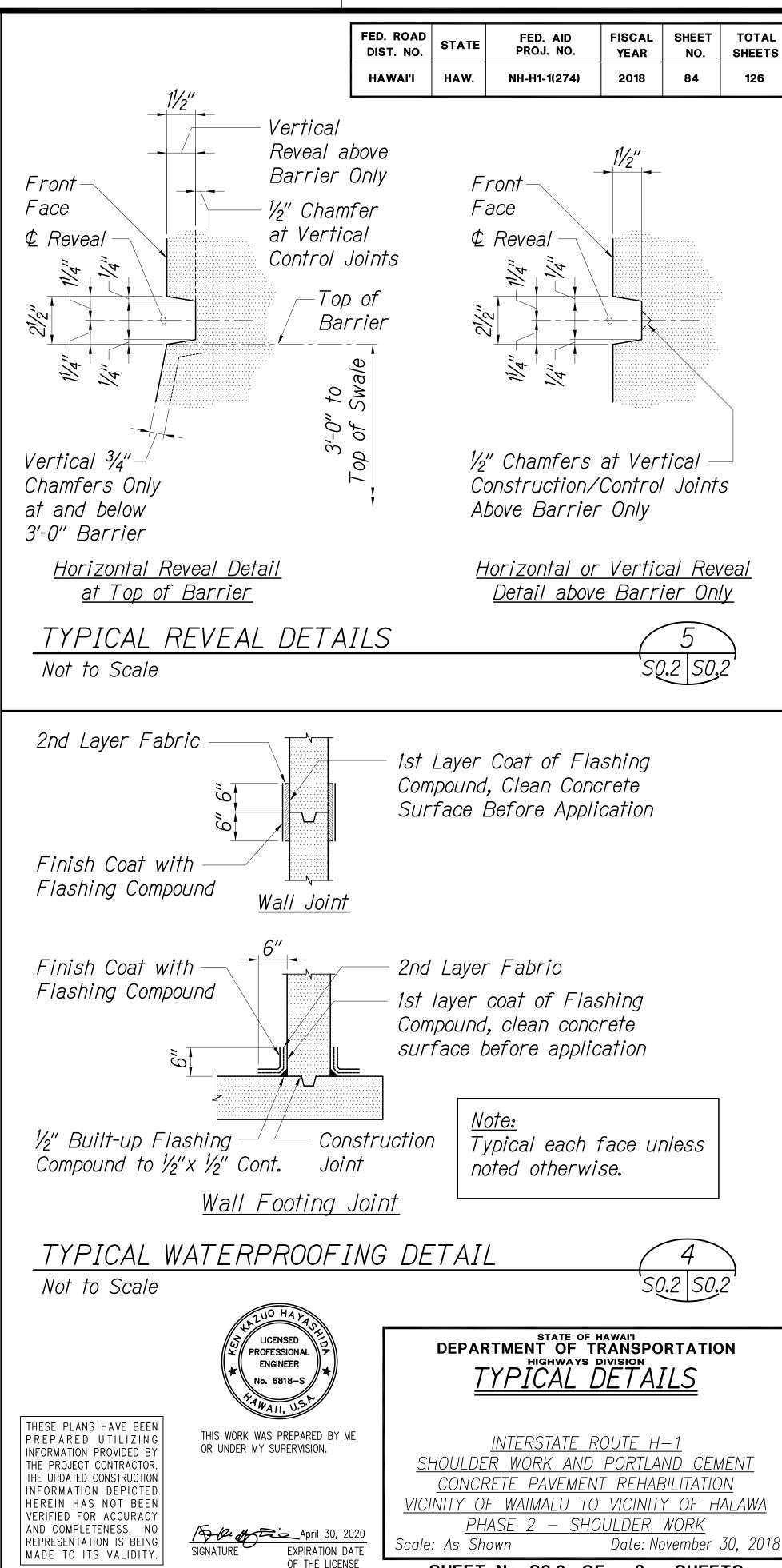


S-0.2 S-0.2



STANDARD HOOKS AND CROSS-TIE DETAIL



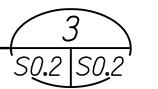


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TYPICAL JOINT DETAILS Not to Scale



Date: November 30, 2018

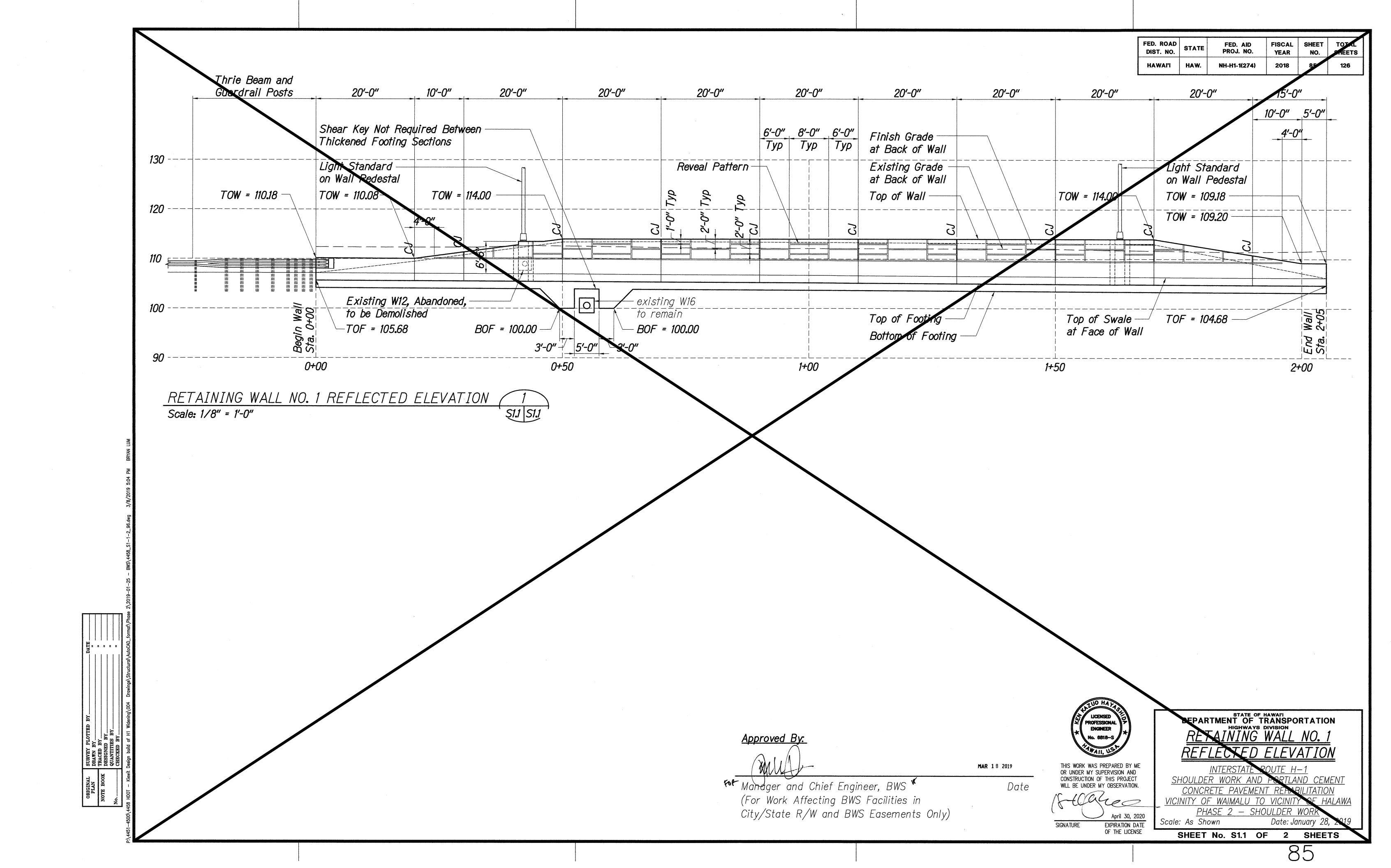
INTERSTATE ROUTE H-1

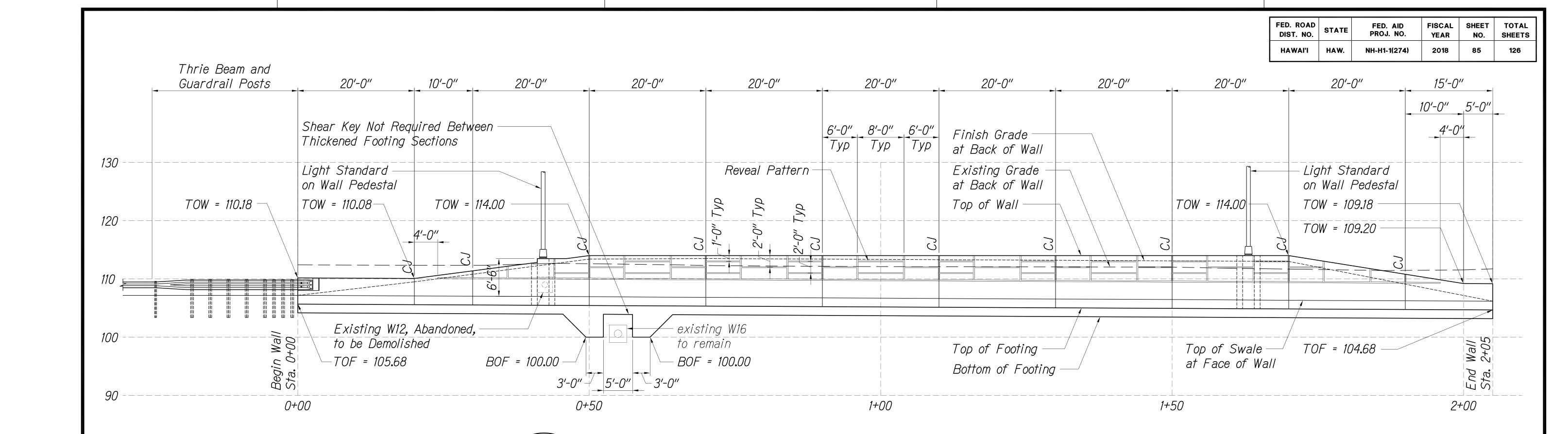
SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 - SHOULDER WORK

SHEET No. S0.2 OF 2 SHEETS

Scale: As Shown





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RETAINING WALL NO. 1 REFLECTED ELEVATION

Scale: 1/8" = 1'-0"

S1.1 S1.1

LICENSED PROFESSIONAL ENGINEER
No. 6818-S
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DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

RETAINING WALL NO. 1

REFLECTED ELEVATION

<u>INTERSTATE ROUTE H-1</u> SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION

VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 — SHOULDER WORK

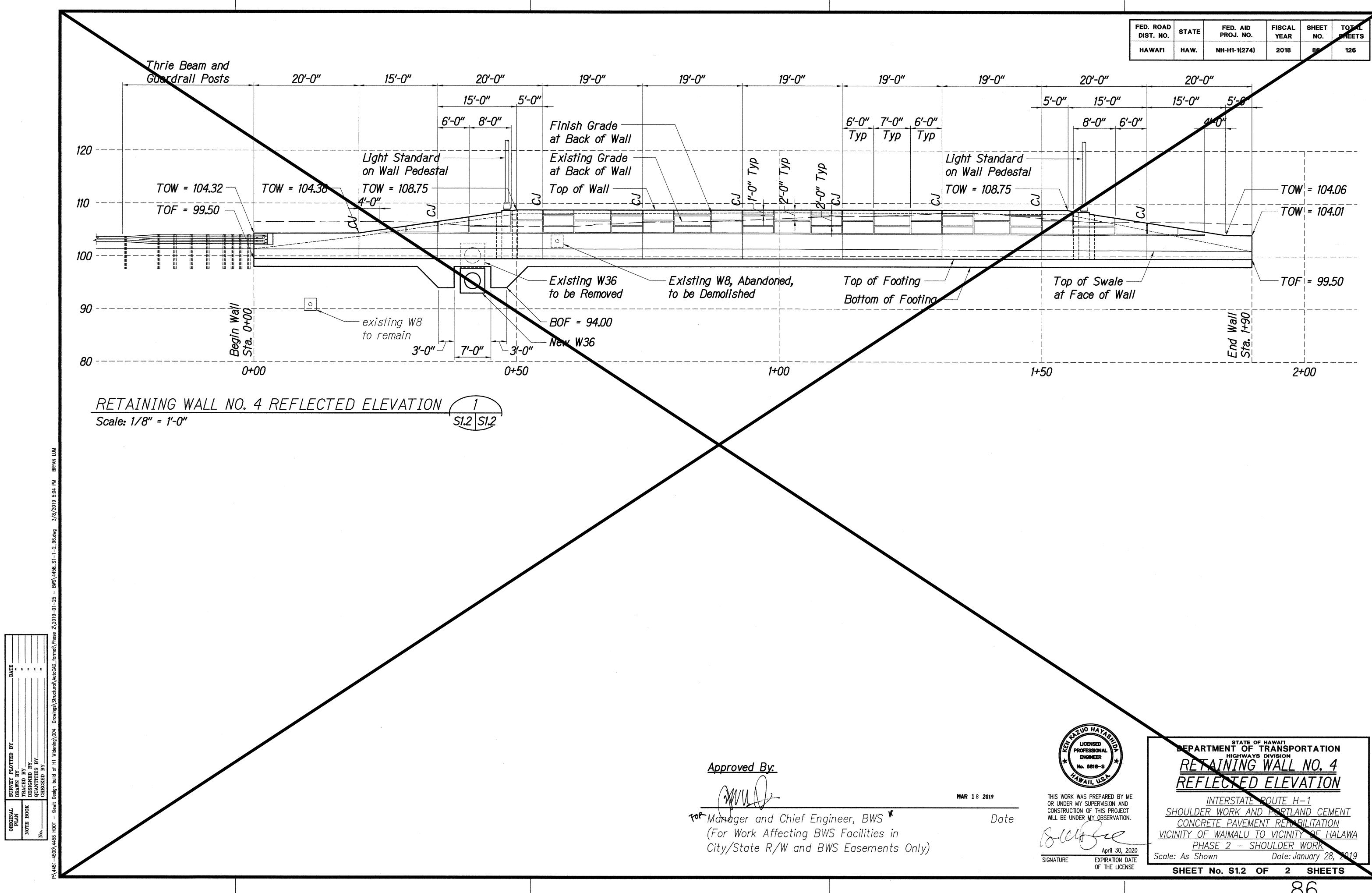
Scale: As Shown Date: November 30, 2018

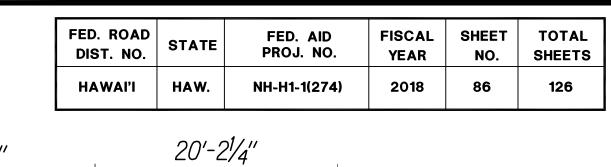
SHEET No. S1.1 OF 2 SHEETS

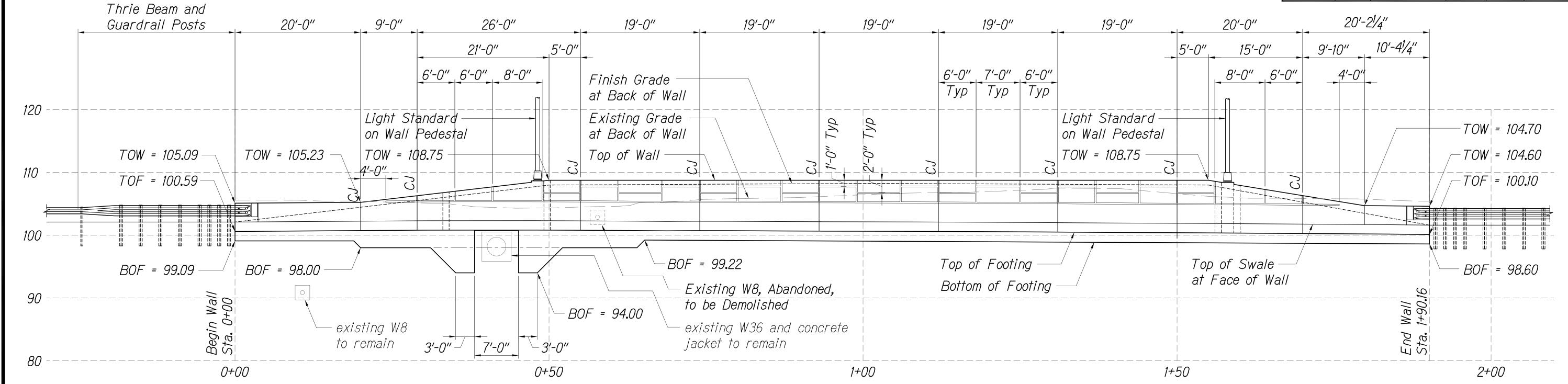
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S1.2 S1.2

RETAINING WALL NO. 4 REFLECTED ELEVATION

Scale: 1/8" = 1'-0"

E:::::

1. Concrete footing shall be placed directly on weathered basalt between Sta. 0+20 to Sta. 0+64.

2. Replace over-excavated material with Structure Backfill Material A and compacted to a minimum of 95% of its maximum dry density per the latest procedure of ASTM D1557.

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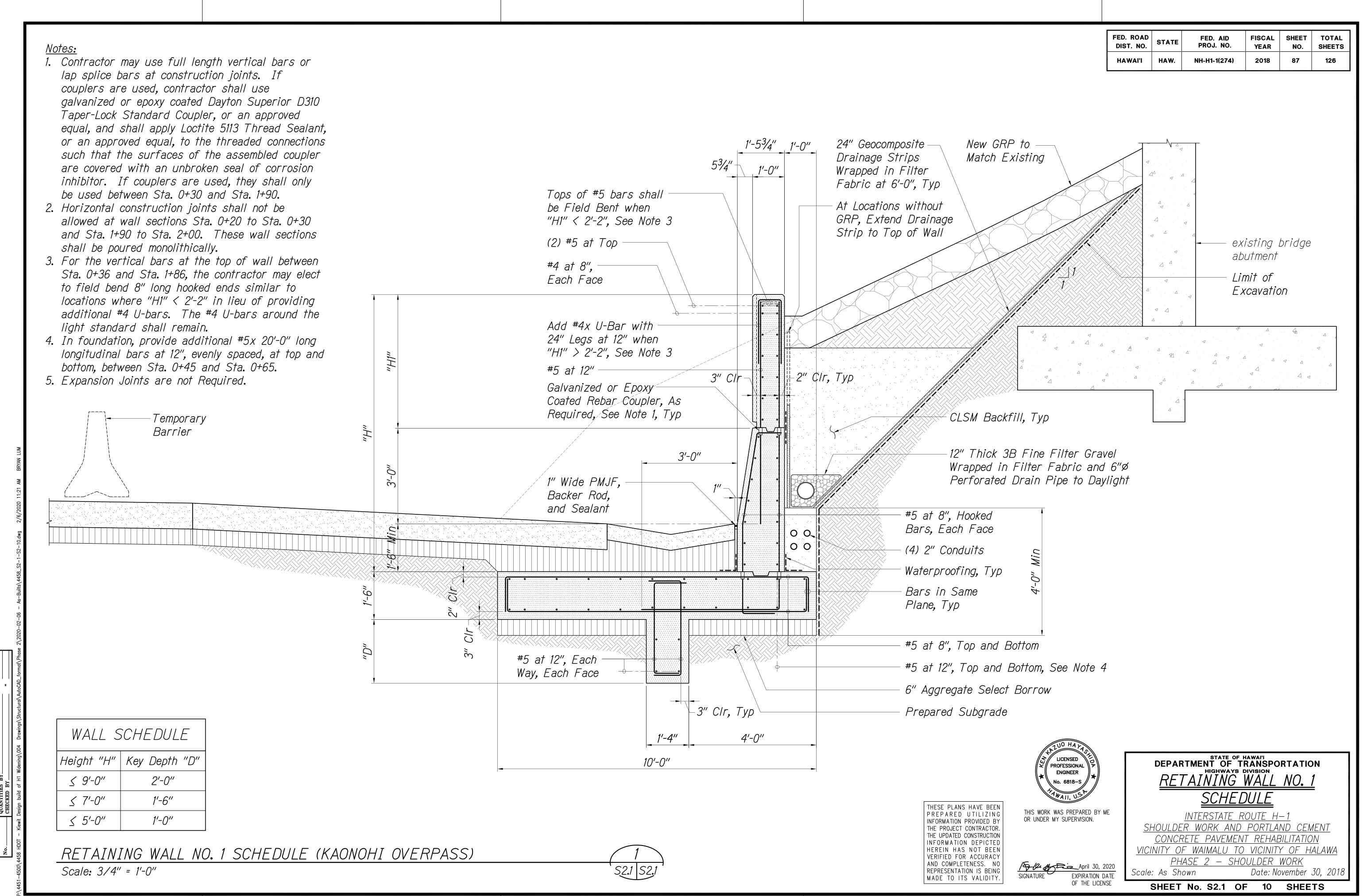
DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION RETAINING WALL NO. 4

INTERSTATE ROUTE H-1 SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 — SHOULDER WORK Date: November 30, 2018 Scale: As Shown SHEET No. S1.2 OF 2 SHEETS

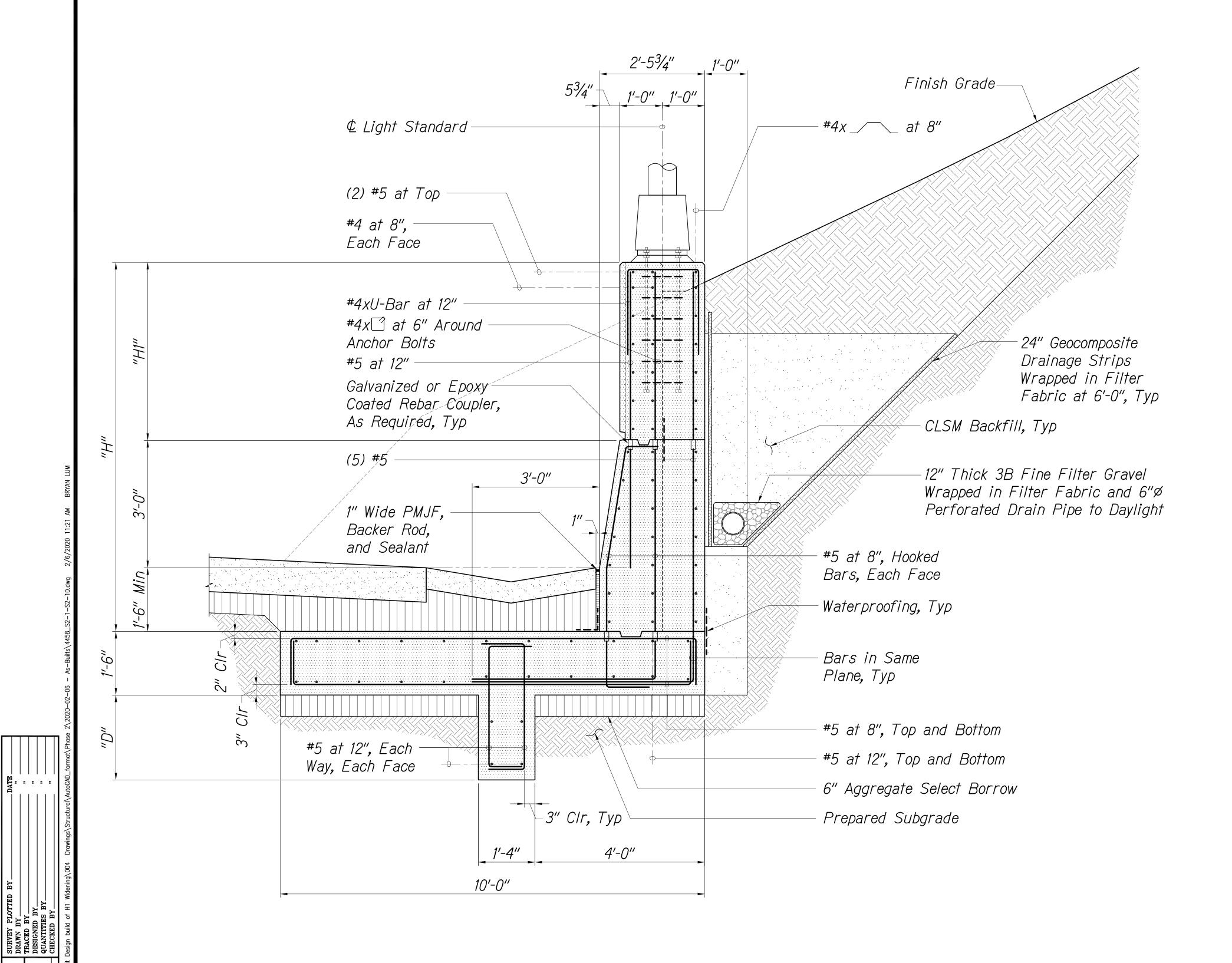
"AS-BUILT"

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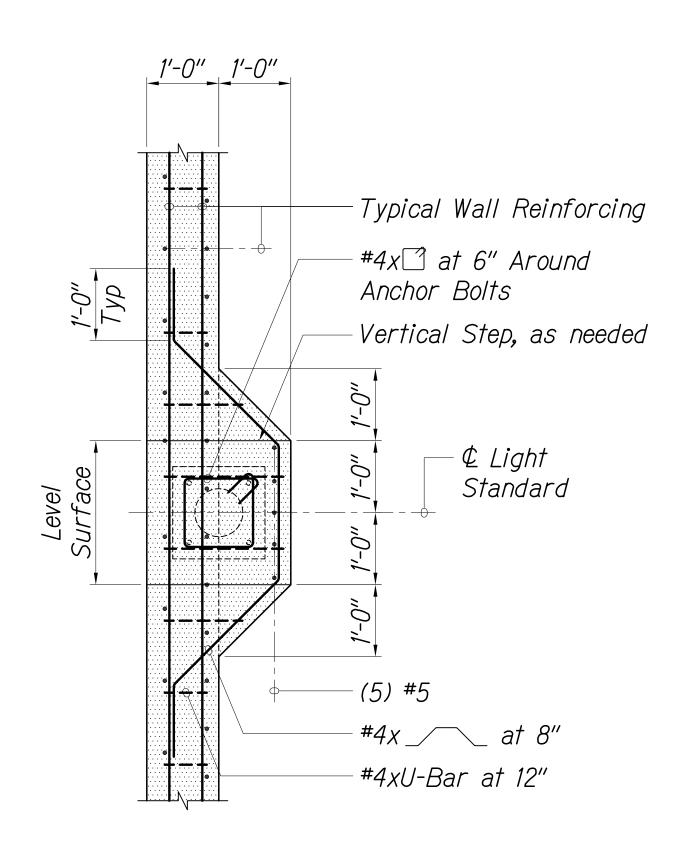
- 1. For balance of information, see Sheet S2.1.
- 2. Control joint in wall shall not be located within 7'-0" of the centerline of the light standard.

| FED. ROAD DIST. NO. | STATE | FED. AID PROJ. NO. | FISCAL YEAR | SHEET NO. | TOTAL SHEETS |
|------------------------|-------|-----------------------|----------------|--------------|-----------------|
| HAWAI'I | HAW. | NH-H1-1(274) | 2018 | 88 | 126 |



RETAINING WALL NO. 1 THICKENED SECTION AT LIGHT STANDARD

Scale: 3/4" = 1'-0"



Section - Plan View



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HIGHWAYS DIVISION

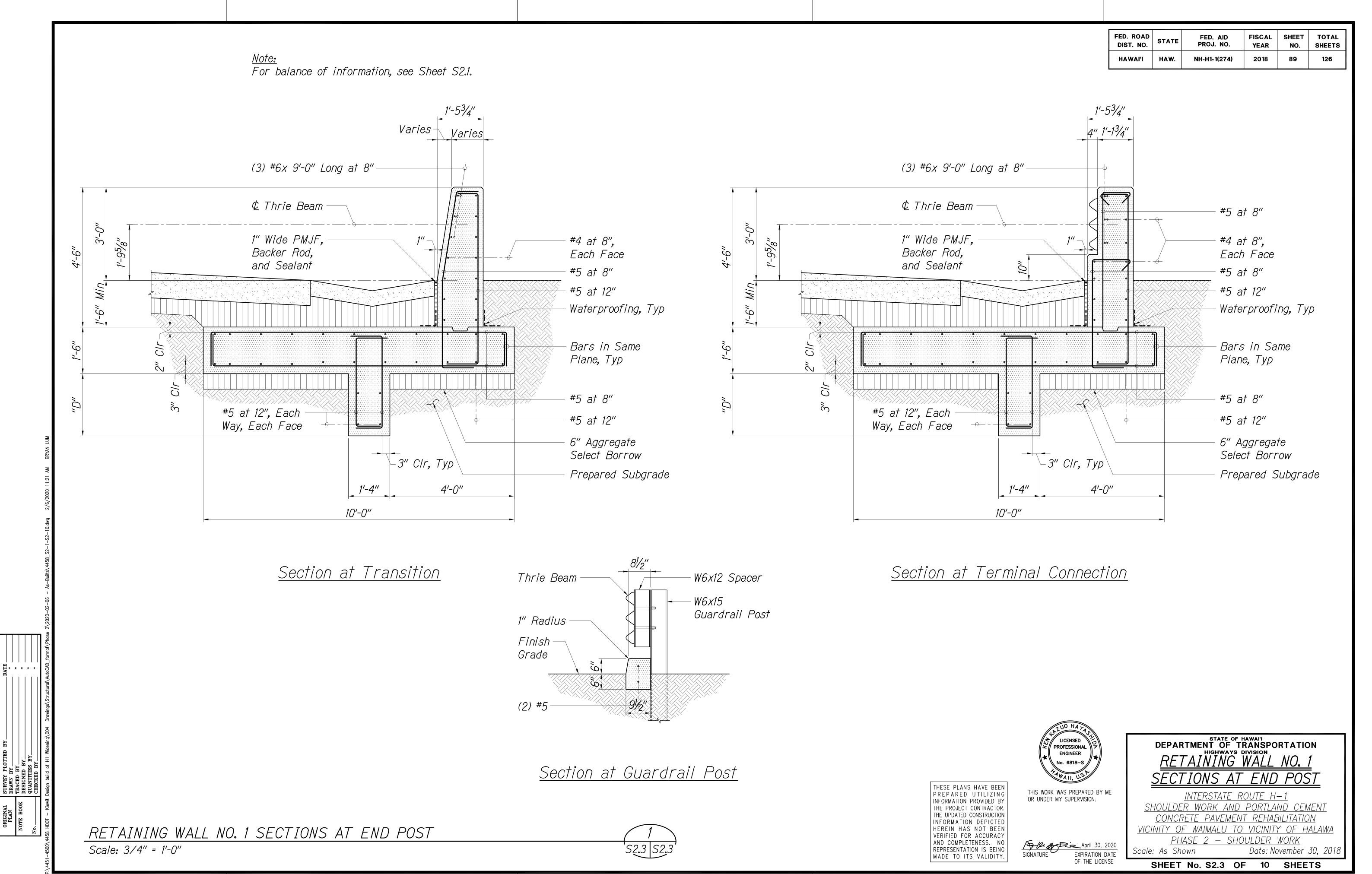
RETAINING WALL NO. 1 SECTIONS AT LIGHT STD

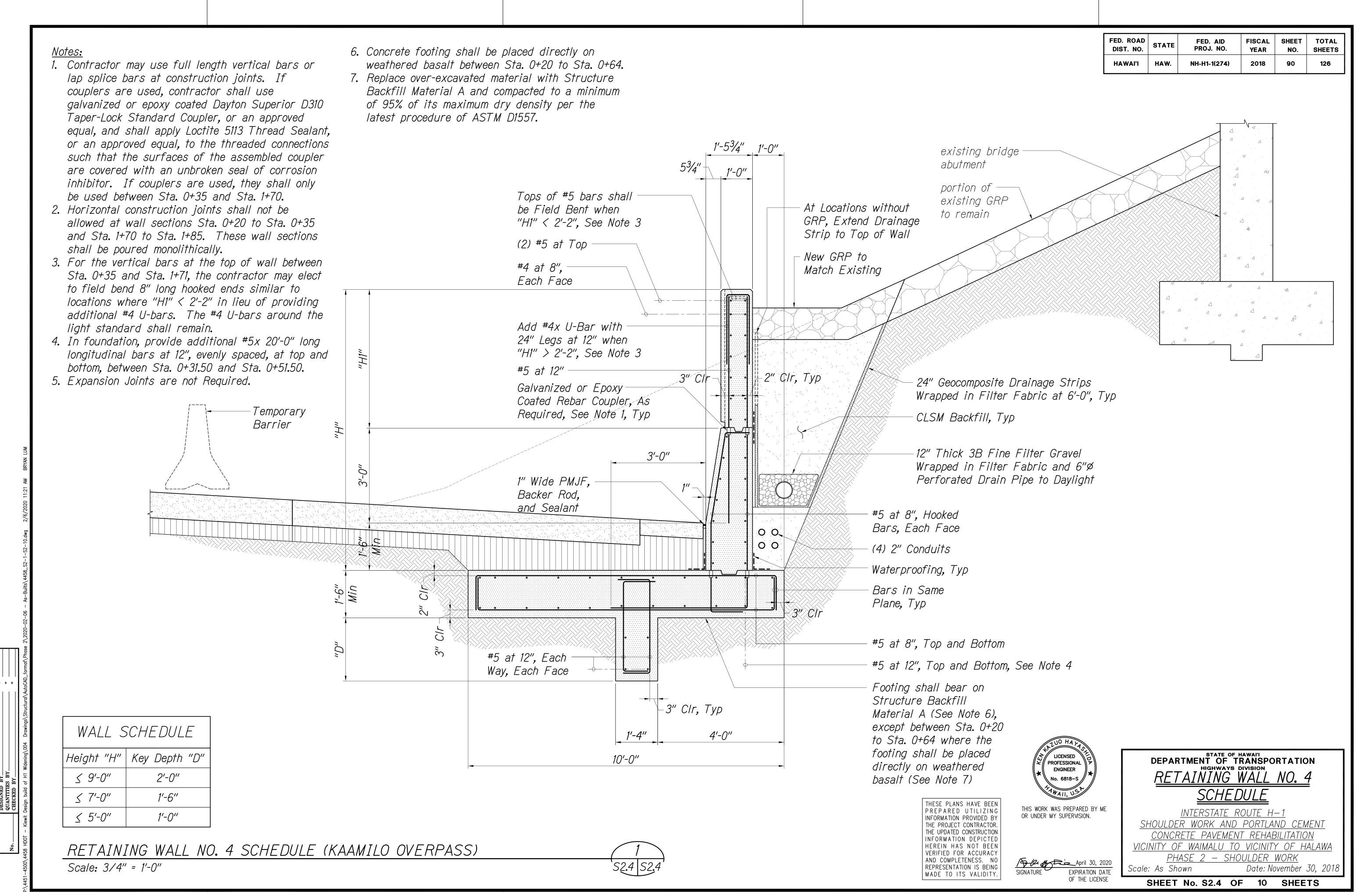
<u>INTERSTATE ROUTE H-1</u> SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 — SHOULDER WORK Date: November 30, 2018 Scale: As Shown SHEET No. S2.2 OF 10 SHEETS

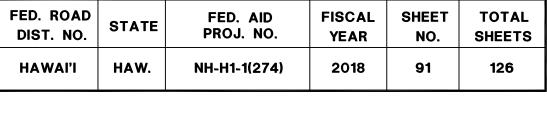
"AS-BUILT"

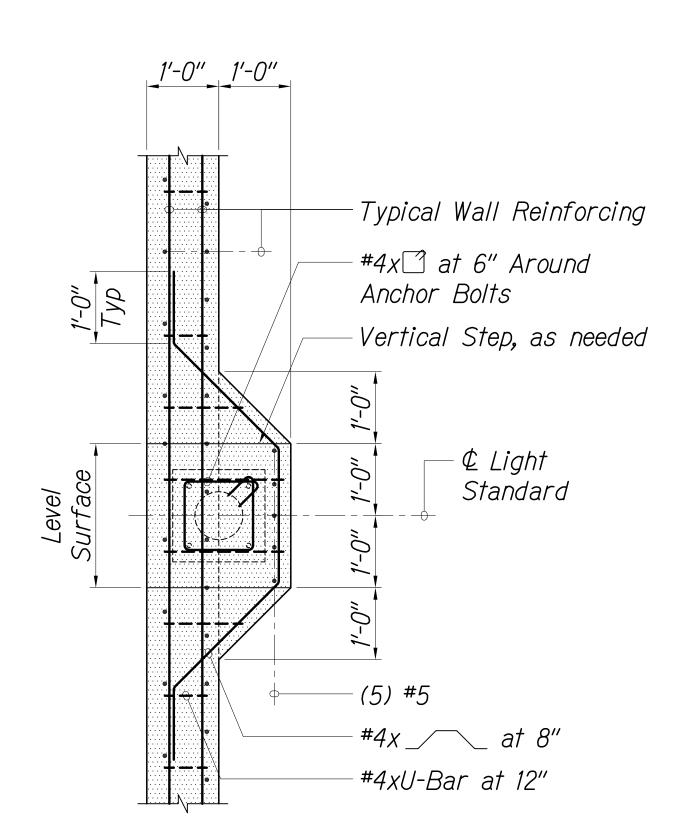
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FED. ROAD DIST. NO. FED. AID PROJ. NO. FISCAL SHEET TOTAL YEAR NO. SHEETS HAWAI'I HAW. NH-H1-1(274)





Section - Plan View



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HIGHWAYS DIVISION
RETAINING WALL NO. 4 SECTIONS AT LIGHT STD

INTERSTATE ROUTE H-1 SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 — SHOULDER WORK Date: November 30, 2018 Scale: As Shown

SHEET No. S2.5 OF 10 SHEETS

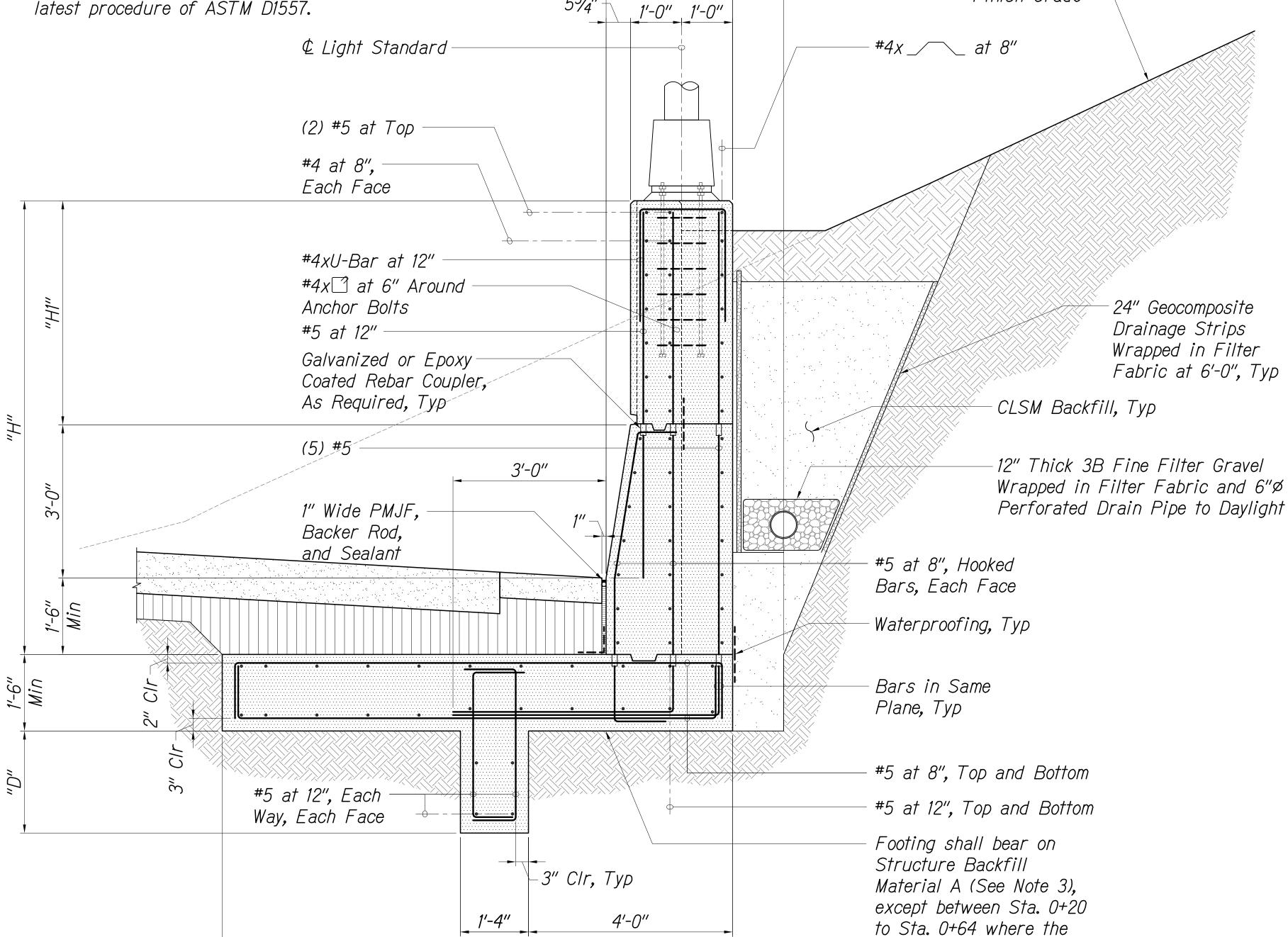
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1. For balance of information, see Sheet S2.4.

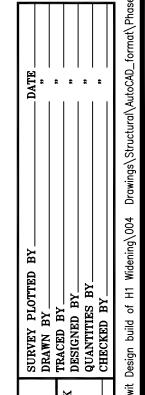
2. Control joint in wall shall not be located within 7'-0" of the centerline of the light standard.

3. Concrete footing shall be placed directly on weathered basalt between Sta. 0+20 to Sta. 0+64.

4. Replace over-excavated material with Structure Backfill Material A and compacted to a minimum of 95% of its maximum dry density per the latest procedure of ASTM D1557.



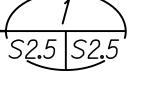
2'-53/4"



RETAINING WALL NO. 4 THICKENED SECTION AT LIGHT STANDARD

10'-0''

Scale: 3/4" = 1'-0"

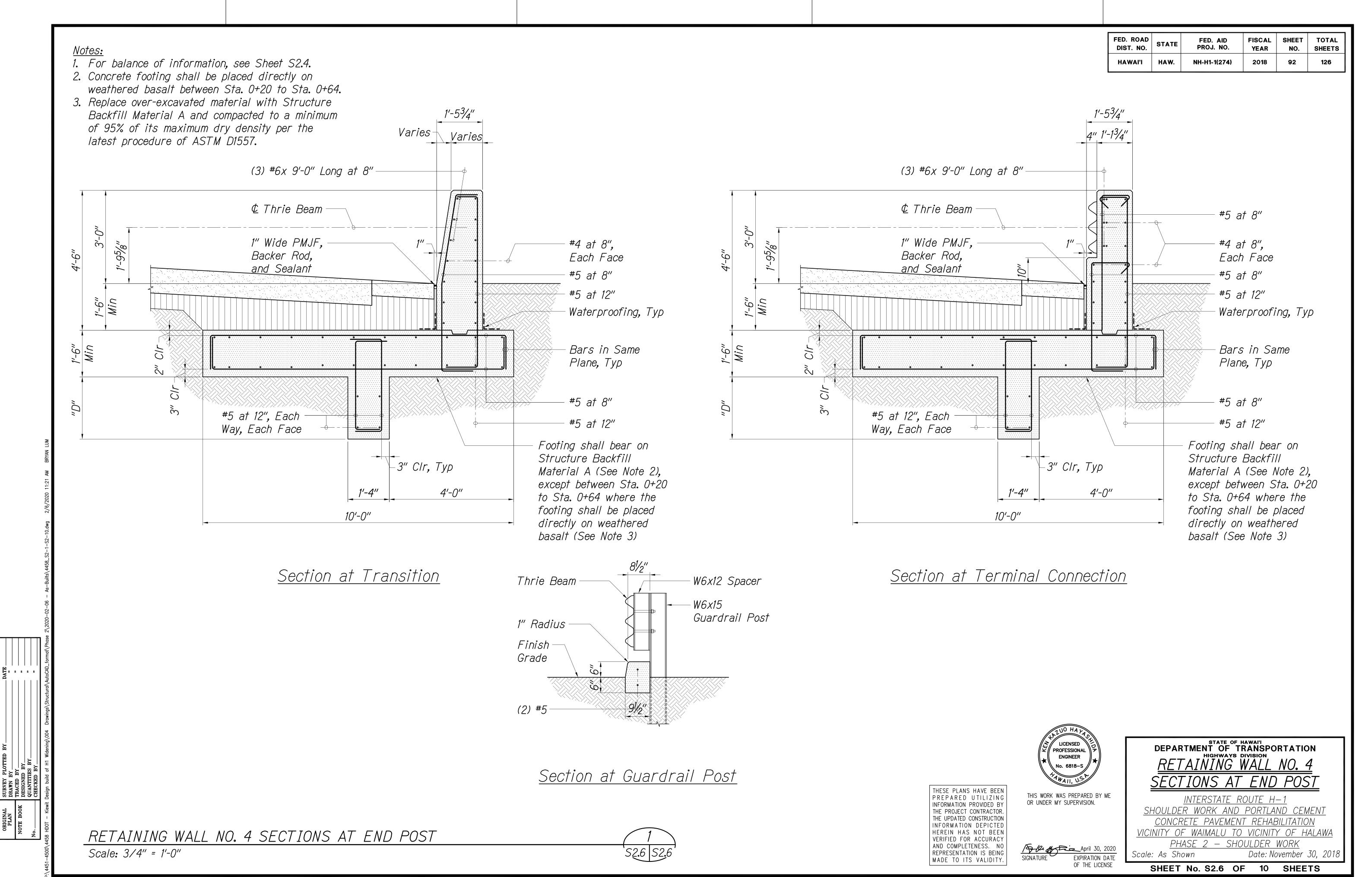


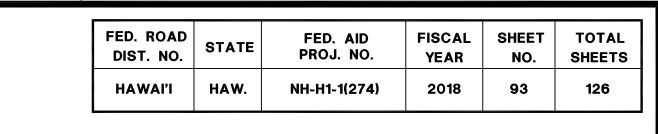
footing shall be placed

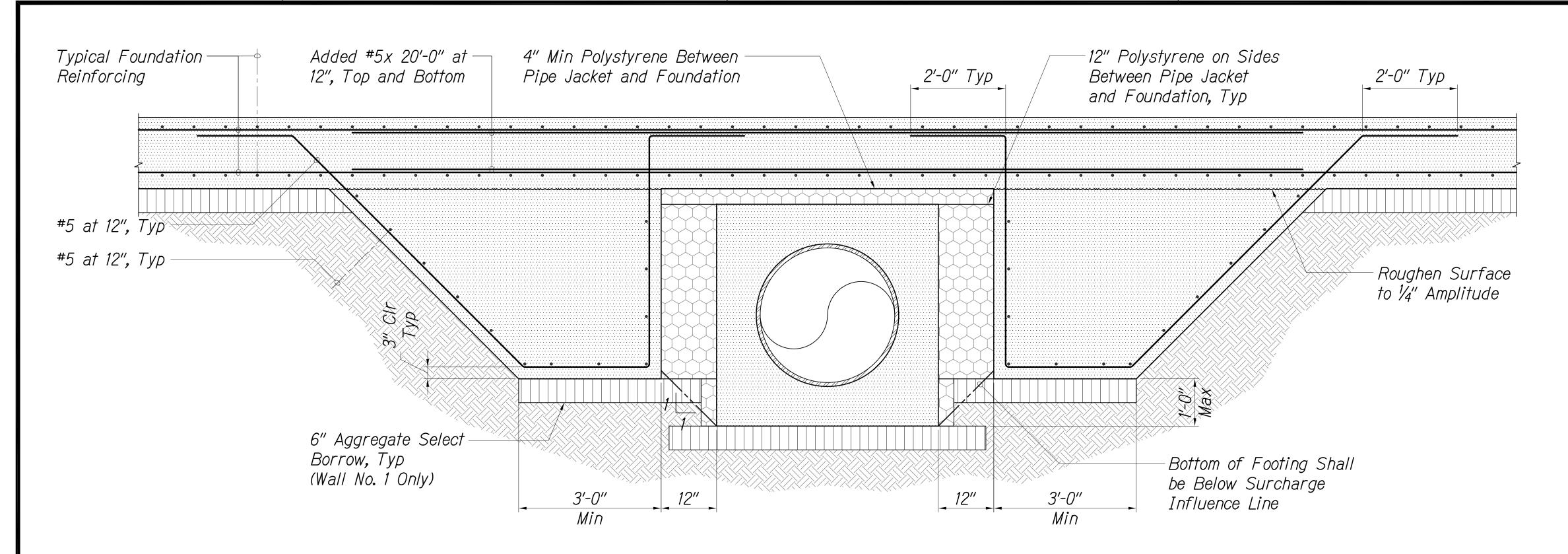
directly on weathered

basalt (See Note 4)

Finish Grade-

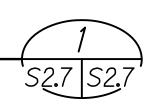


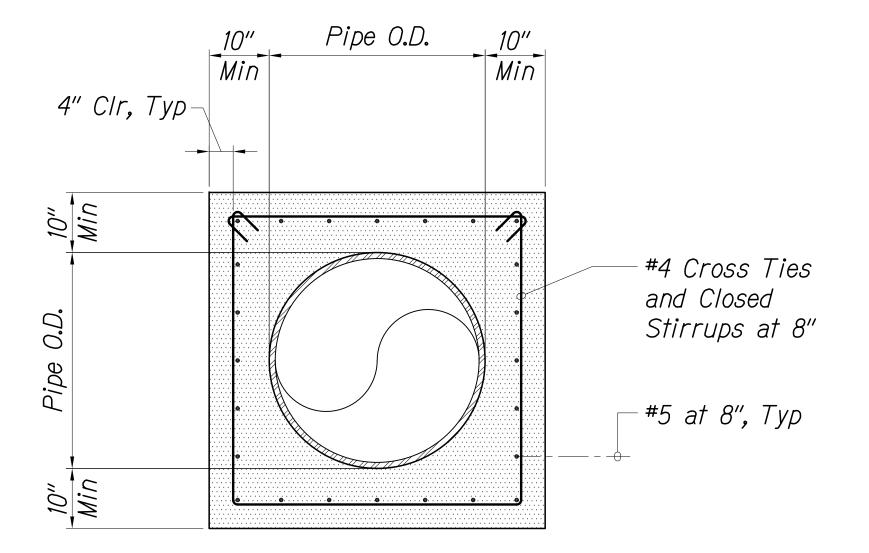




TYPICAL THICKENED FOUNDATION AT WATERLINE

Scale: 3/4" = 1'-0"





LICENSED | PROFESSIONAL **ENGINEER**

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RETAINING WALL NOS. 1 \$ 4 TYPICAL DETAILS

<u>INTERSTATE ROUTE H-1</u> SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION

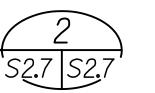
VICINITY OF WAIMALU TO VICINITY OF HALAWA PHASE 2 — SHOULDER WORK Date: November 30, 2018 Scale: As Shown

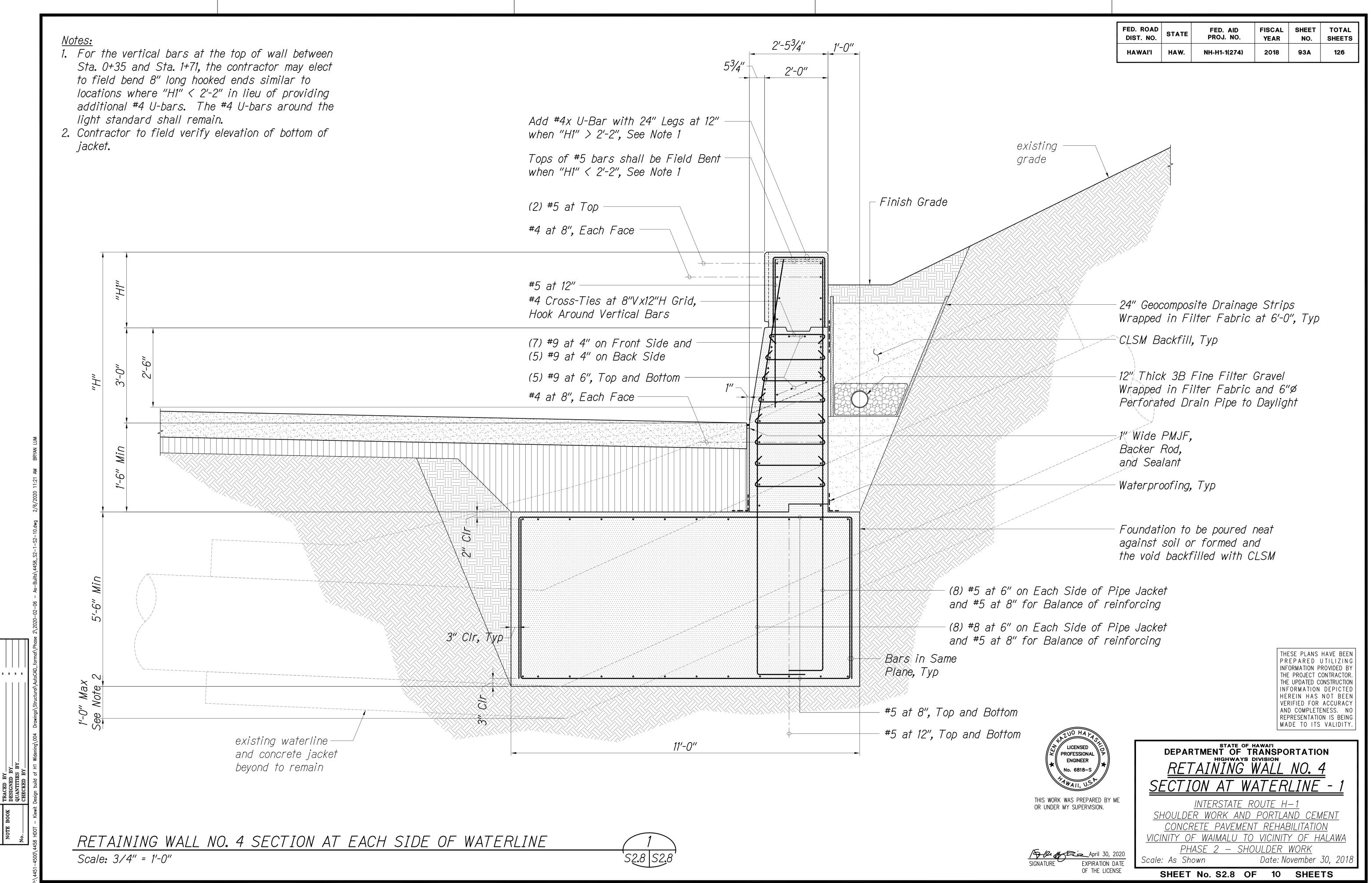
SHEET No. S2.7 OF 10 SHEETS

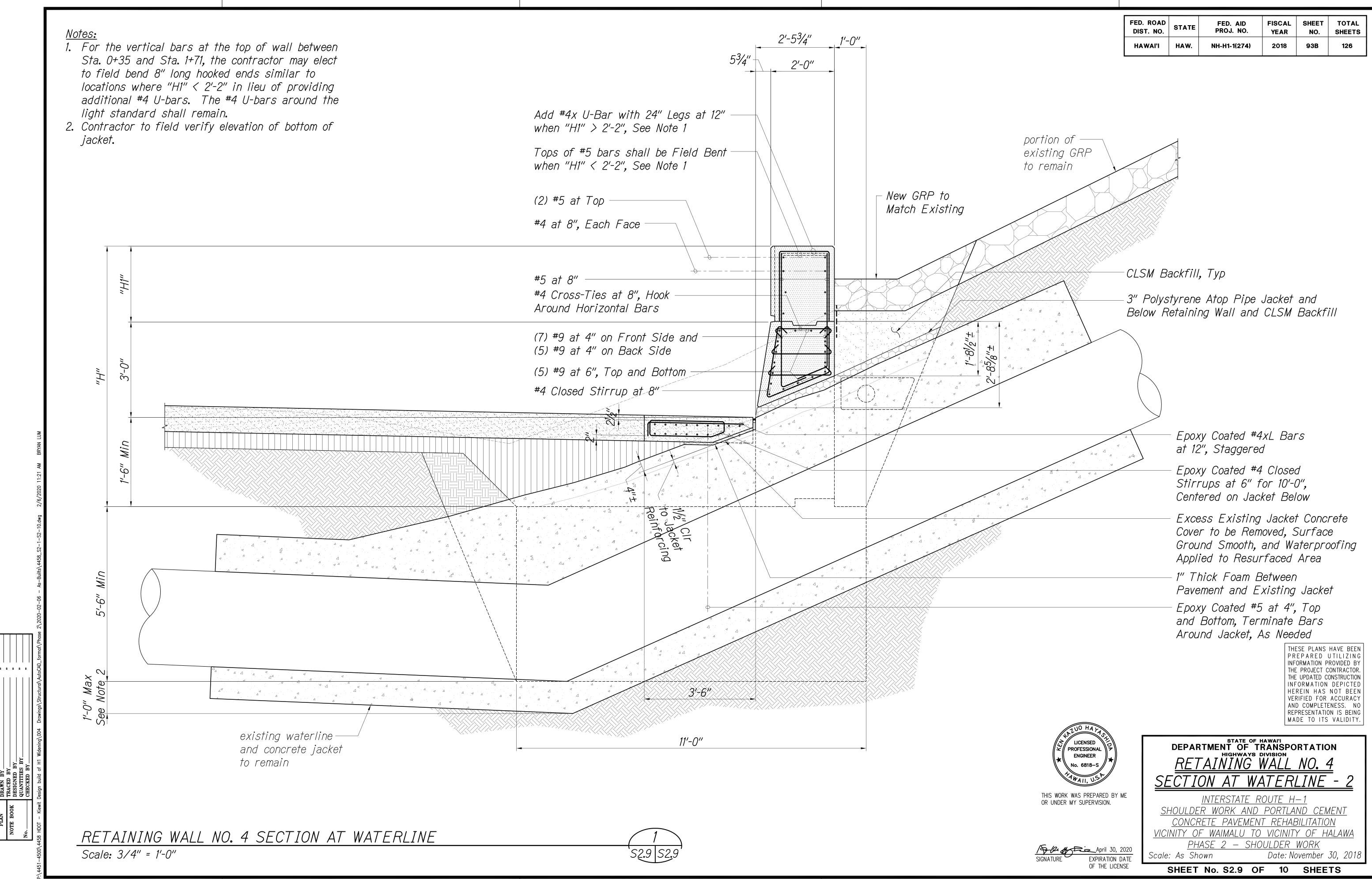
TYPICAL WATERLINE JACKET DETAIL

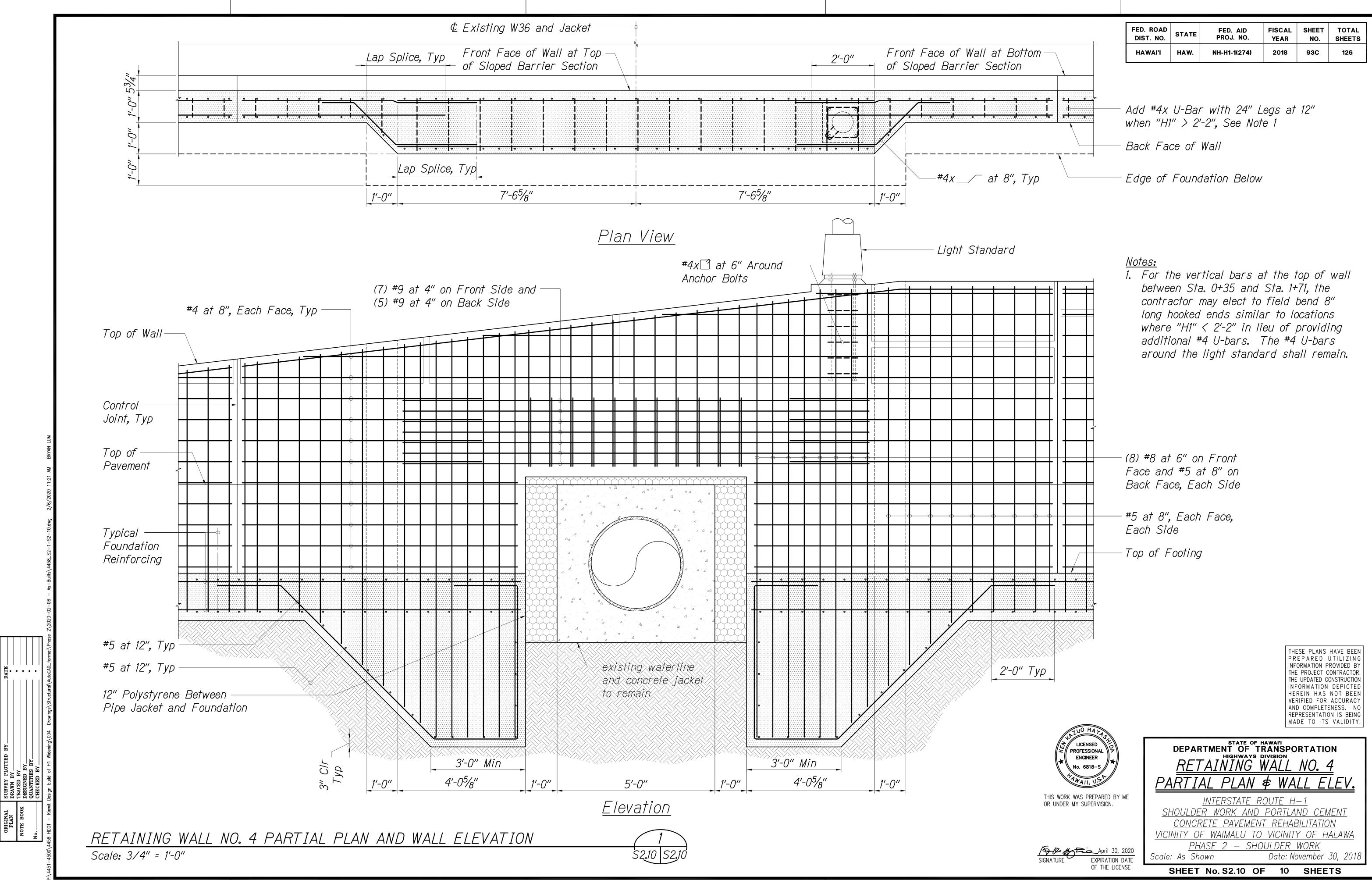
Scale: 3/4" = 1'-0"

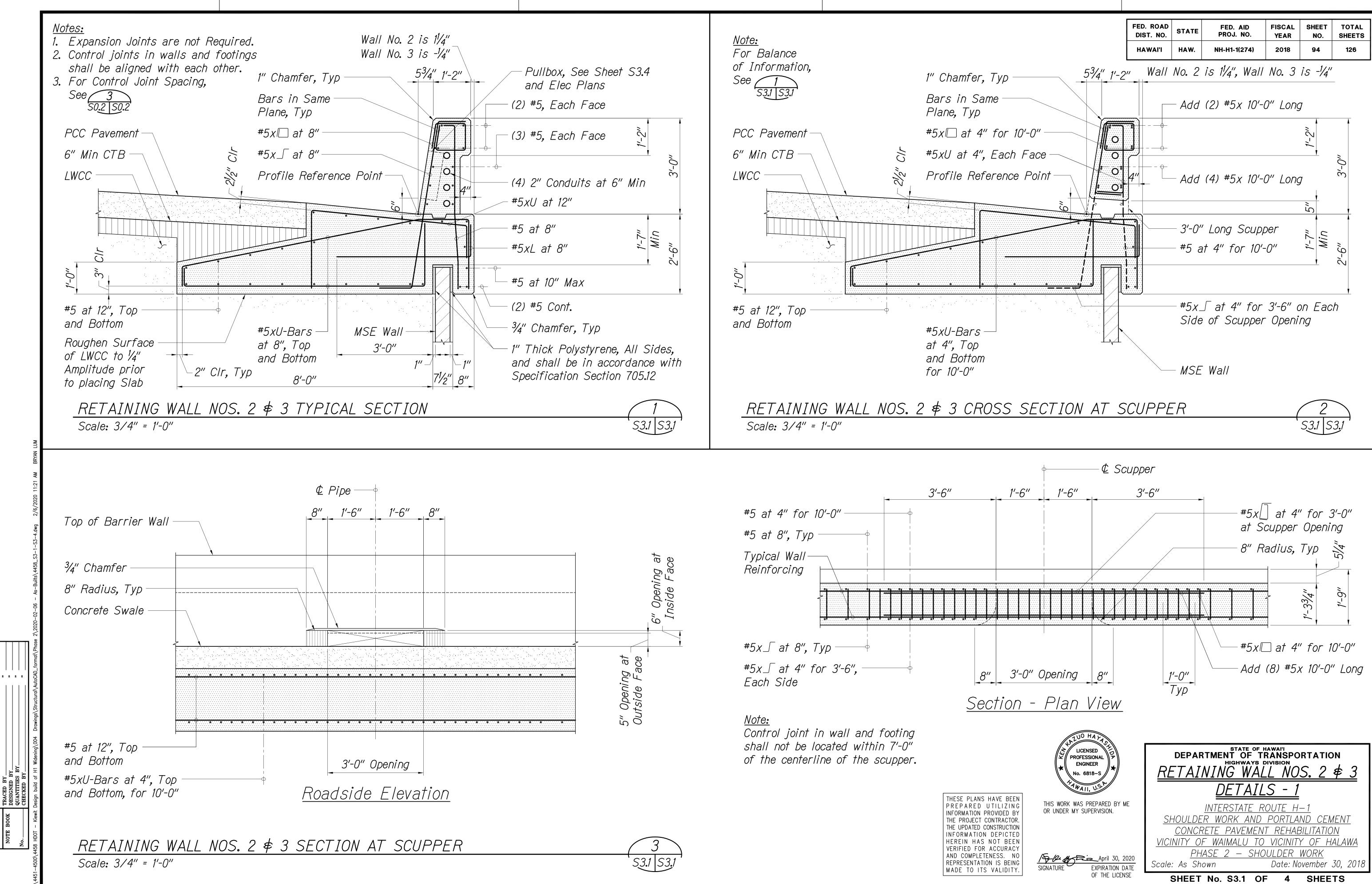
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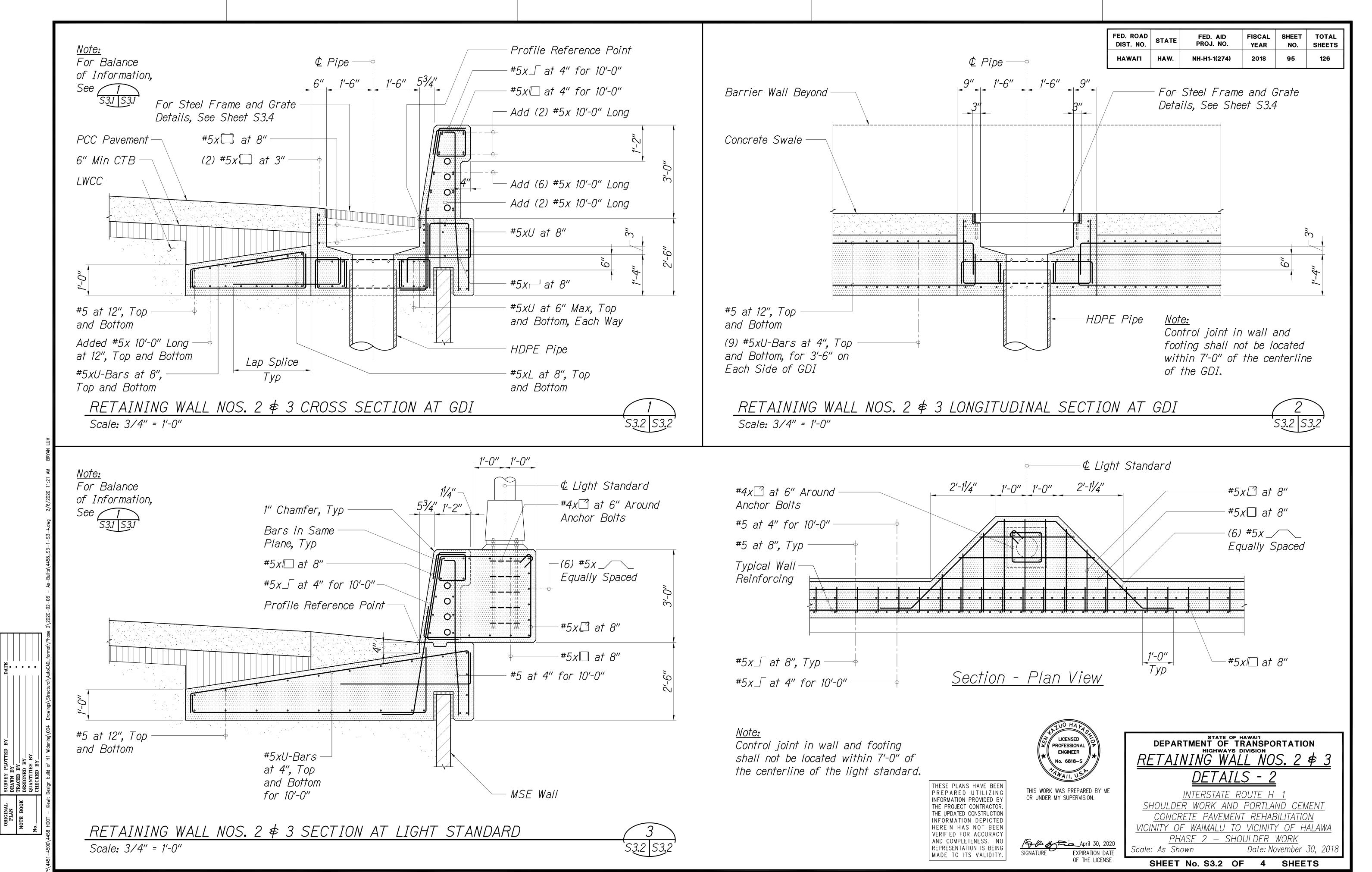




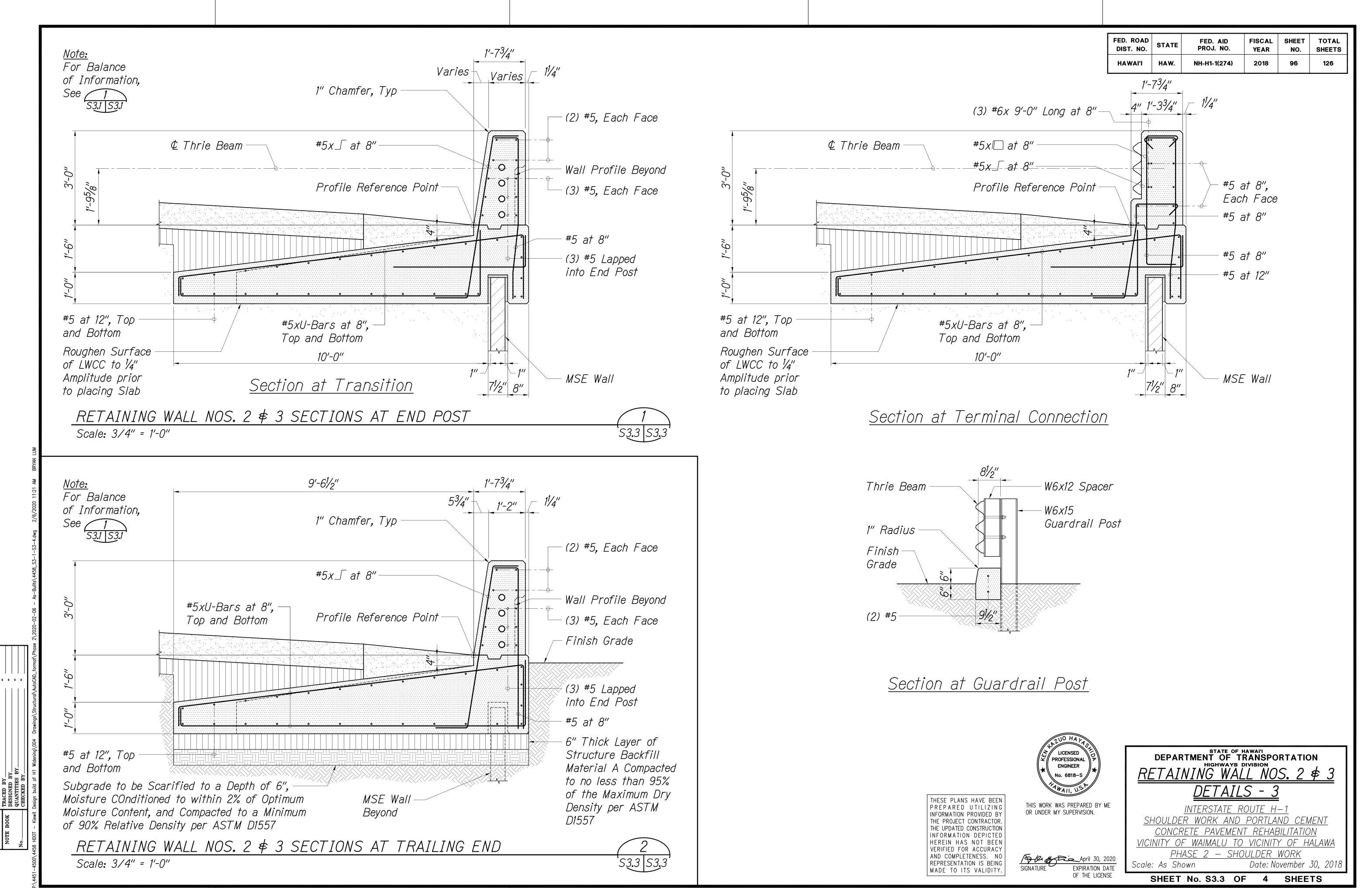




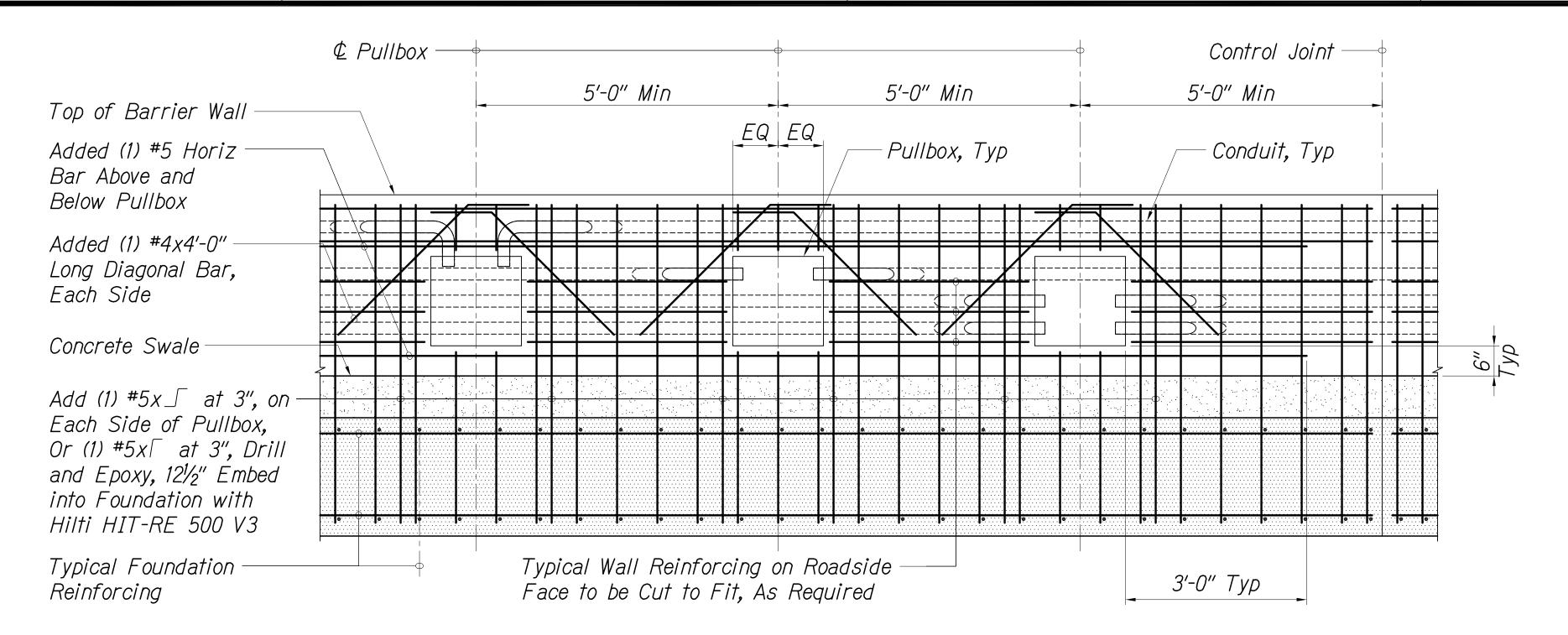
C. Light Standar



95



96



FED. ROAD DIST. NO. FED. AID PROJ. NO. FISCAL SHEET TOTAL YEAR NO. SHEETS 2018 NH-H1-1(274) 97

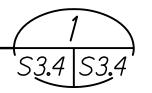
Roadside Elevation

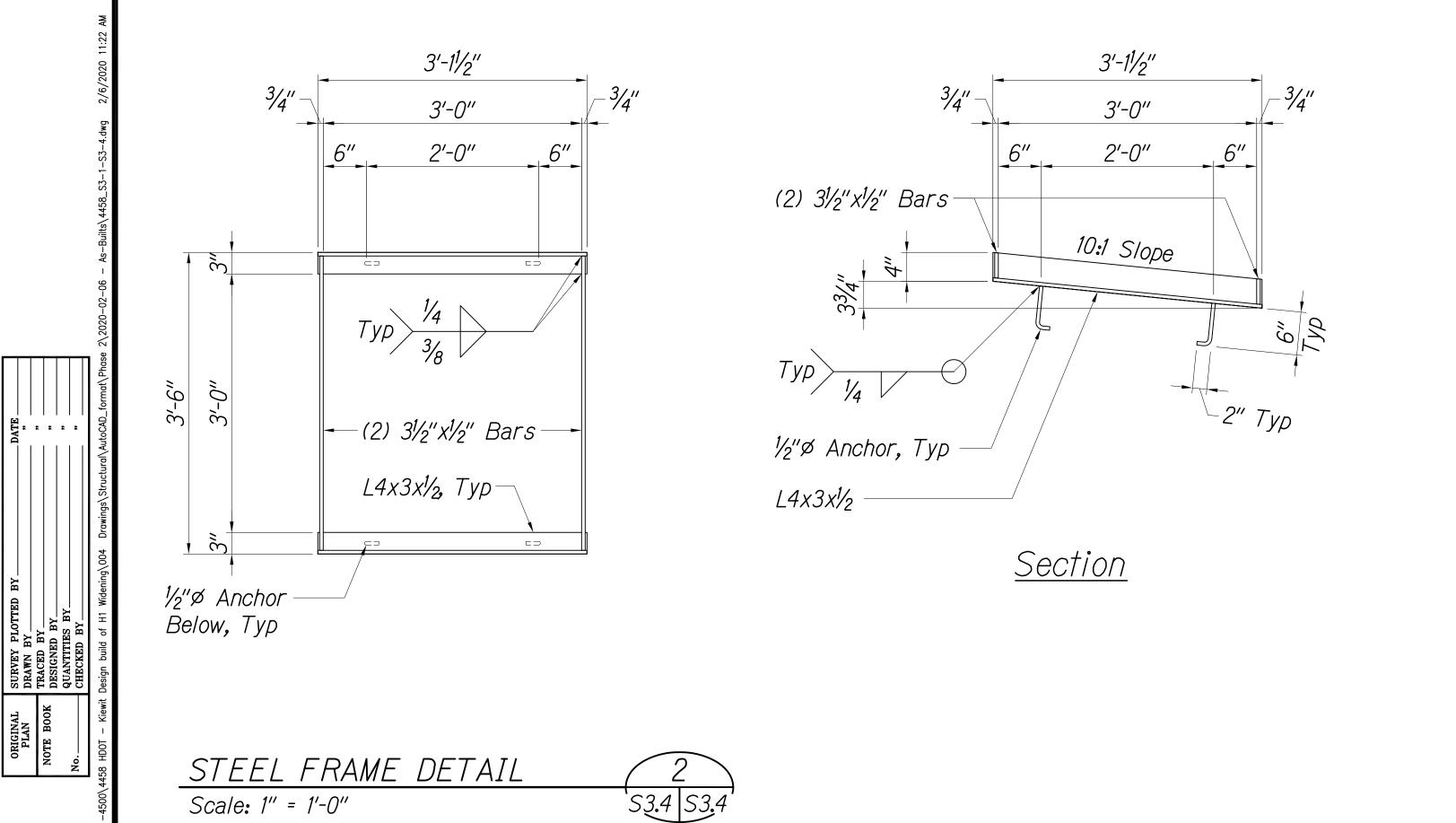
RETAINING WALL NOS. 2 \notin 3 ELEVATION AT PULLBOXES

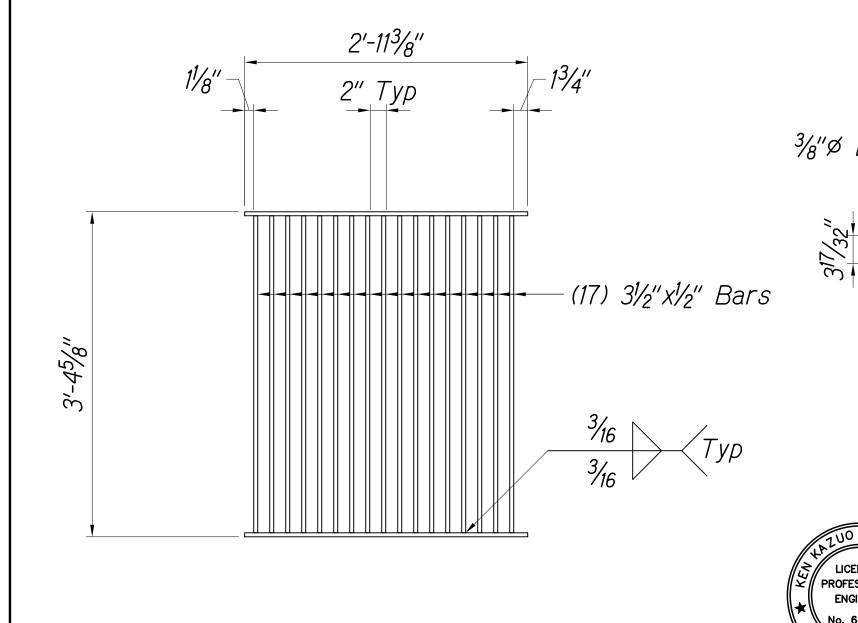
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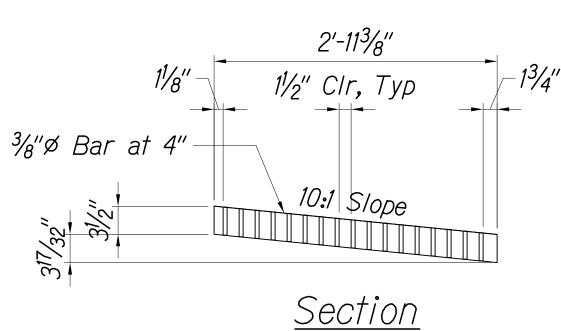
Scale: 1" = 1'-0"

\TE ...

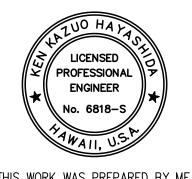






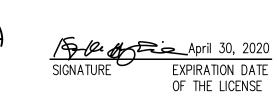


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STEEL GRATE DETAIL S3.4 S3.4 Scale: 1" = 1'-0"



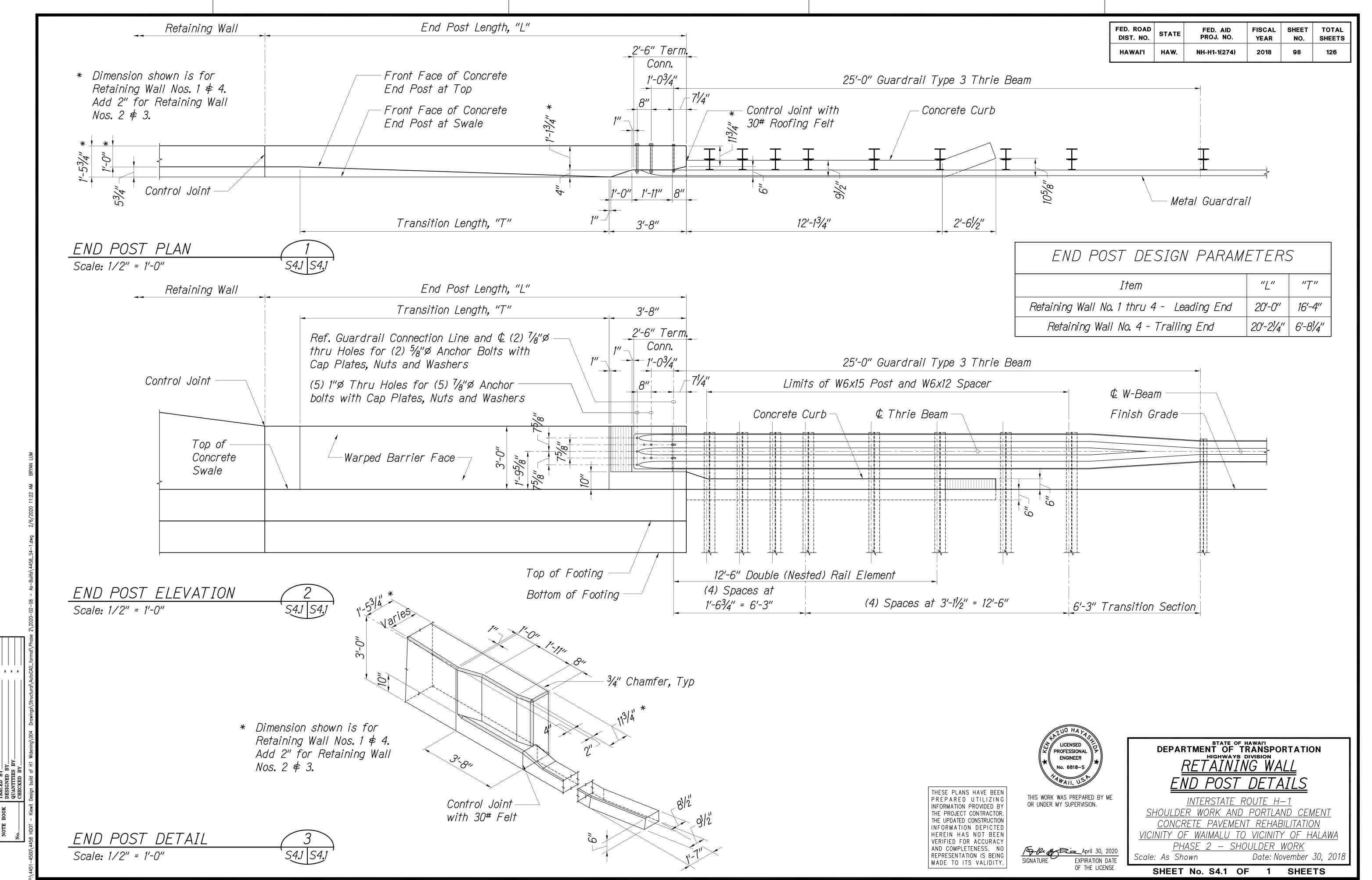
DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION RETAINING WALL NOS. 2 \$ 3 <u>DETAILS - 4</u>

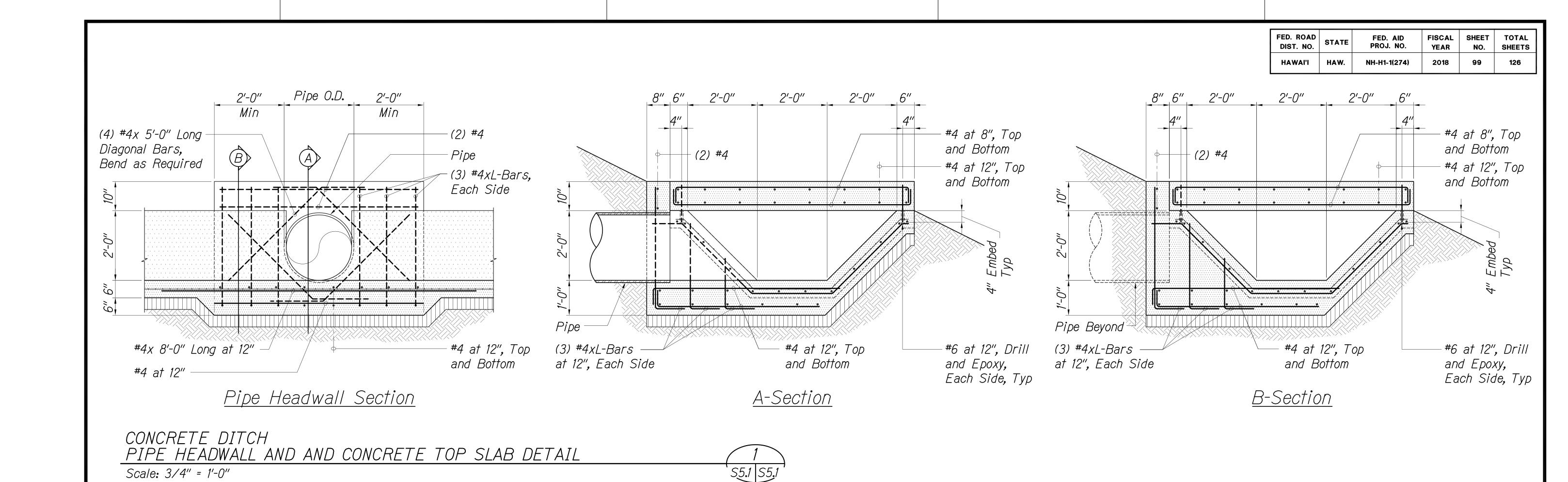
INTERSTATE ROUTE H-1

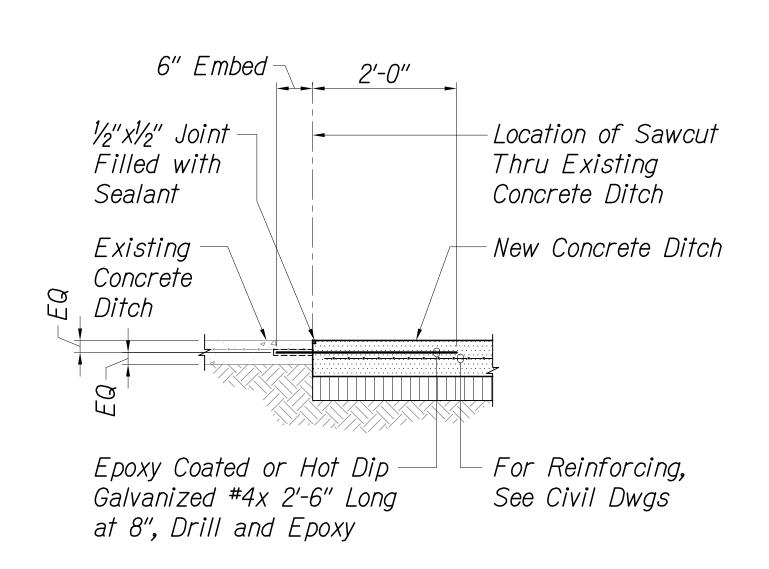
SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 — SHOULDER WORK Date: November 30, 2018 Scale: As Shown

SHEET No. S3.4 OF 4 SHEETS







Existing Ditch to New Ditch Section

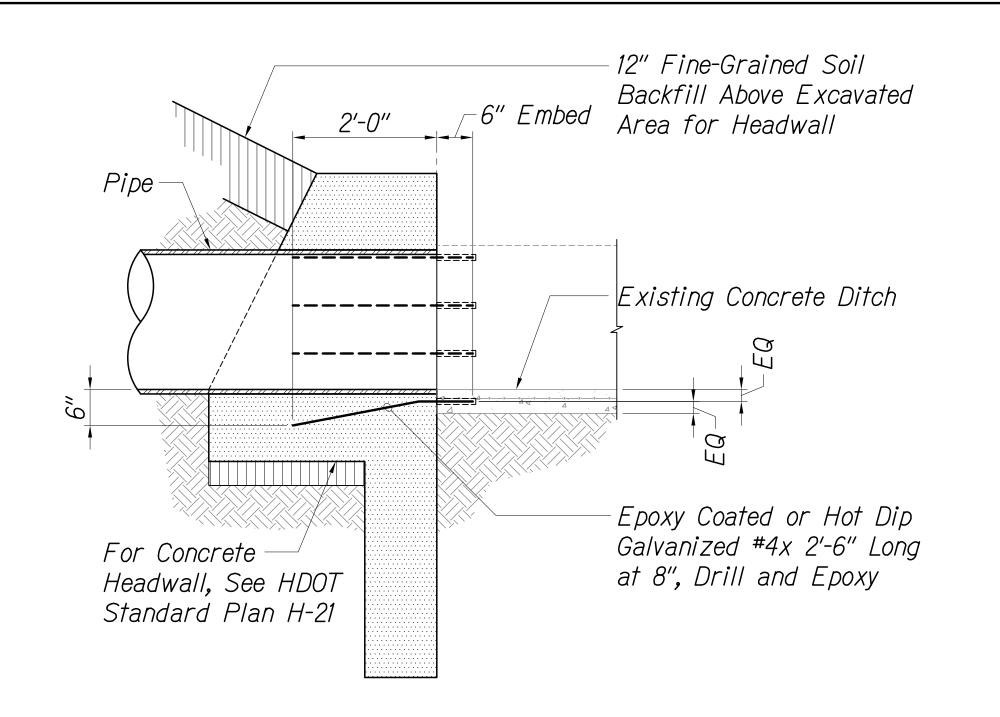
CONCRETE DITCH
CONNECTION DETAILS

Scale: 3/4" = 1'-0"

55.1 S5.1

E

SURVEY PLOTTI
DRAWN BY____
TRACED BY___
DESIGNED BY_
QUANTITIES BY_
CHECKED BY___



Existing Ditch to New Concrete Headwall



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April 30, 2020

SIGNATURE EXPIRATION DATE
OF THE LICENSE

DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION TYPICAL CONCRETE DITCH DETAILS

INTERSTATE ROUTE H-1
SHOULDER WORK AND PORTLAND CEMENT
CONCRETE PAVEMENT REHABILITATION

VICINITY OF WAIMALU TO VICINITY OF HALAWA

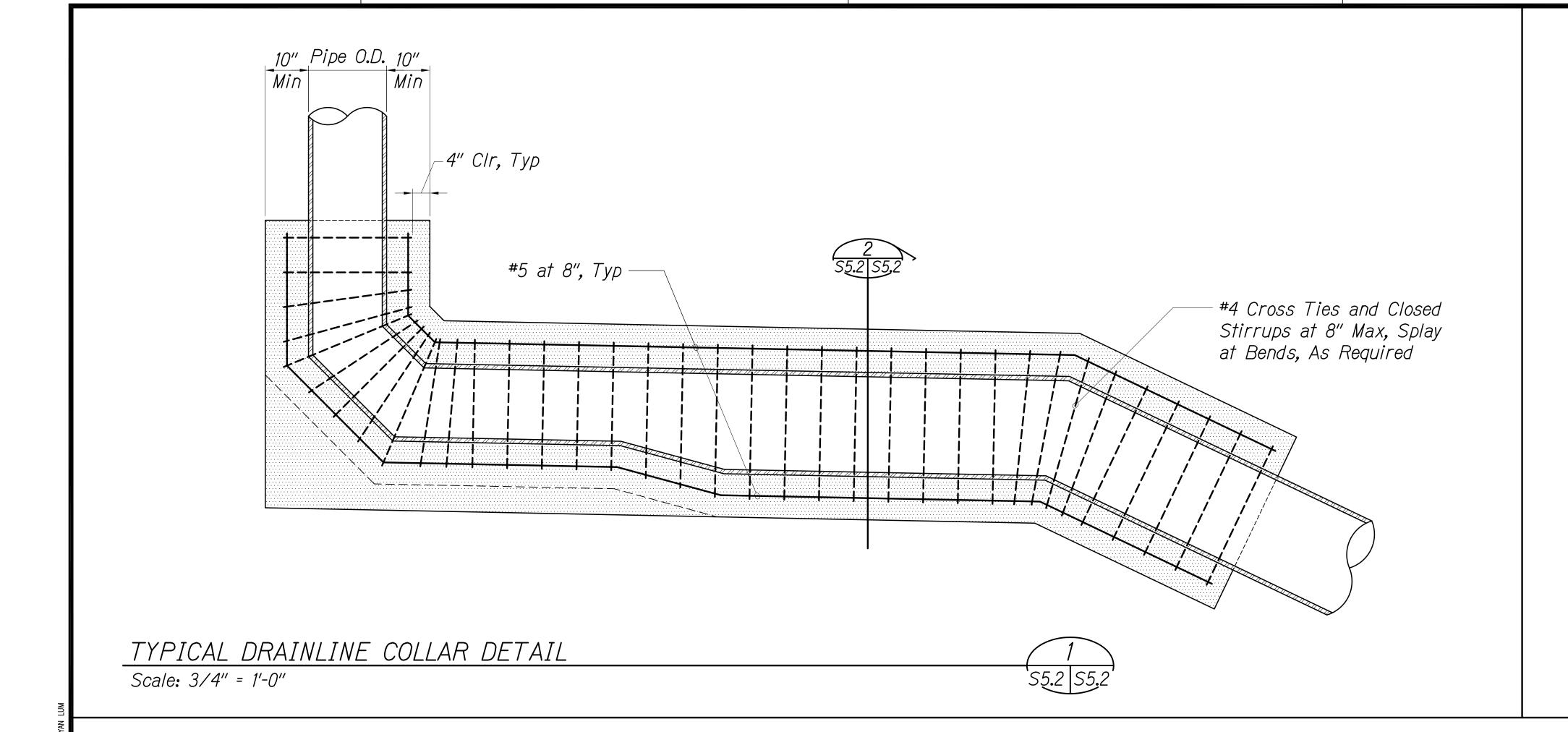
PHASE 2 — SHOULDER WORK

Scale: As Shown Date: November 30, 2018

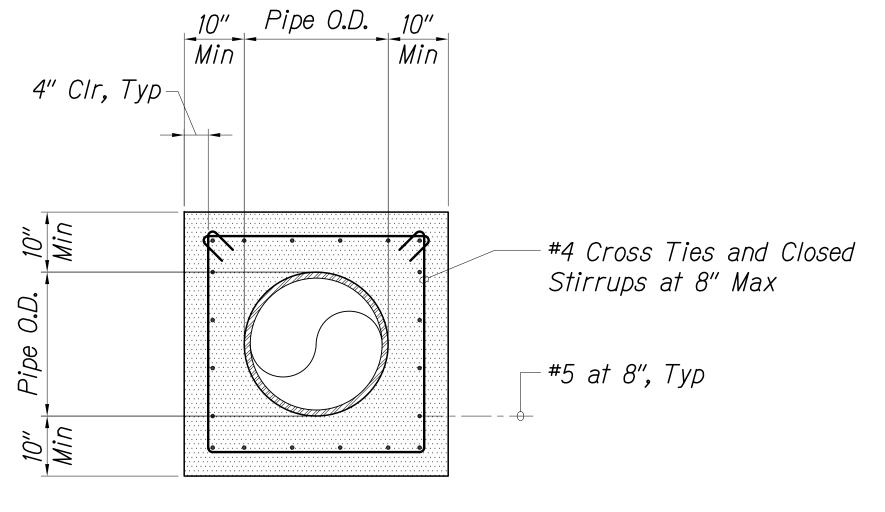
SHEET No. S5.1 OF 2 SHEETS

"AS-BUILT"

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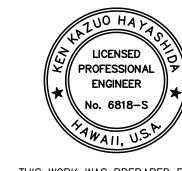
| FED. ROAD DIST. NO. | STATE | FED. AID PROJ. NO. | FISCAL YEAR | SHEET NO. | TOTAL SHEETS |
|------------------------|-------|-----------------------|----------------|--------------|-----------------|
| HAWAI'I | HAW. | NH-H1-1(274) | 2018 | 100 | 126 |



SECTION Scale: 3/4" = 1'-0"

TTE ...

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April 30, 2020
SIGNATURE EXPIRATION DATE
OF THE LICENSE

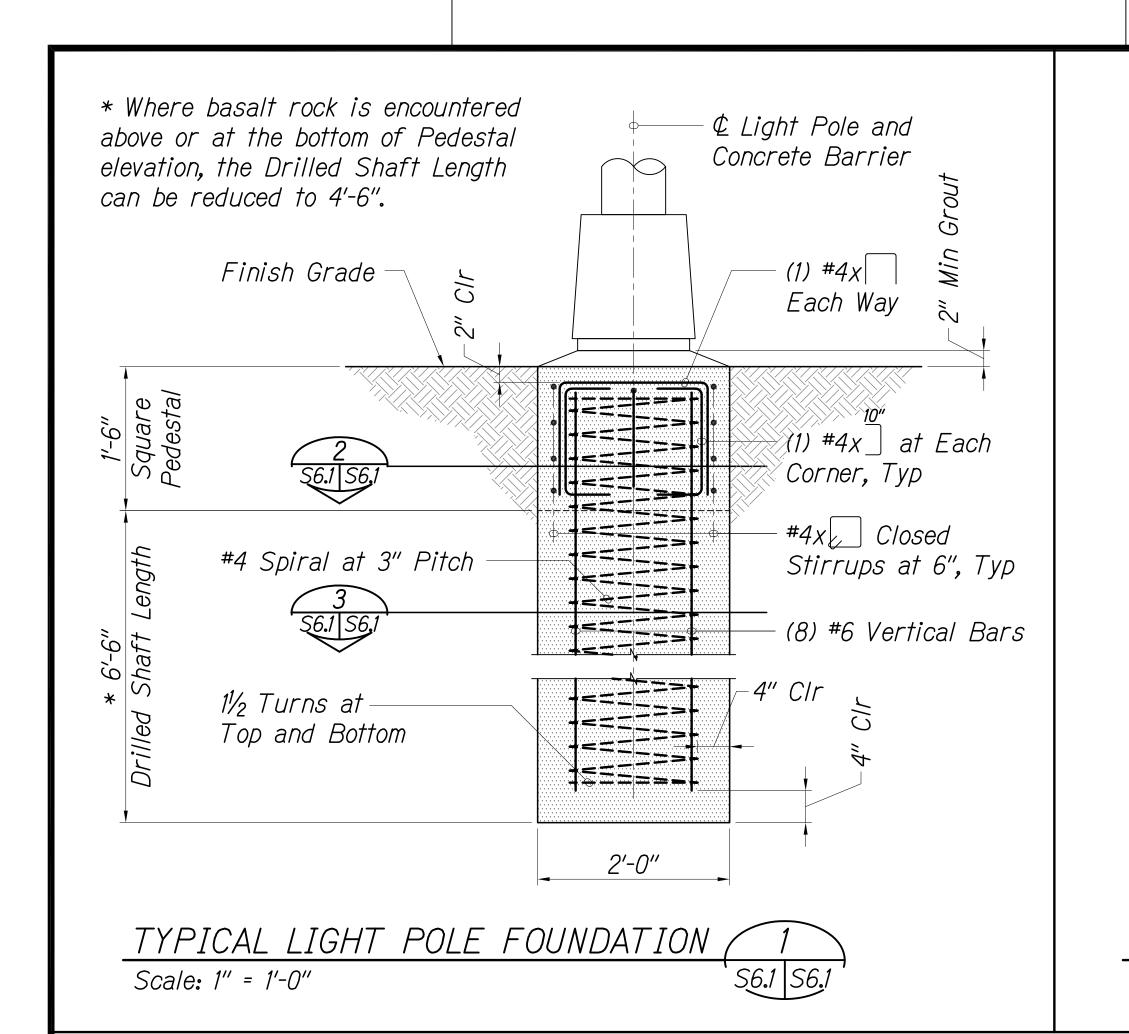
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION
TYPICAL CONCRETE COLLAR DETAILS

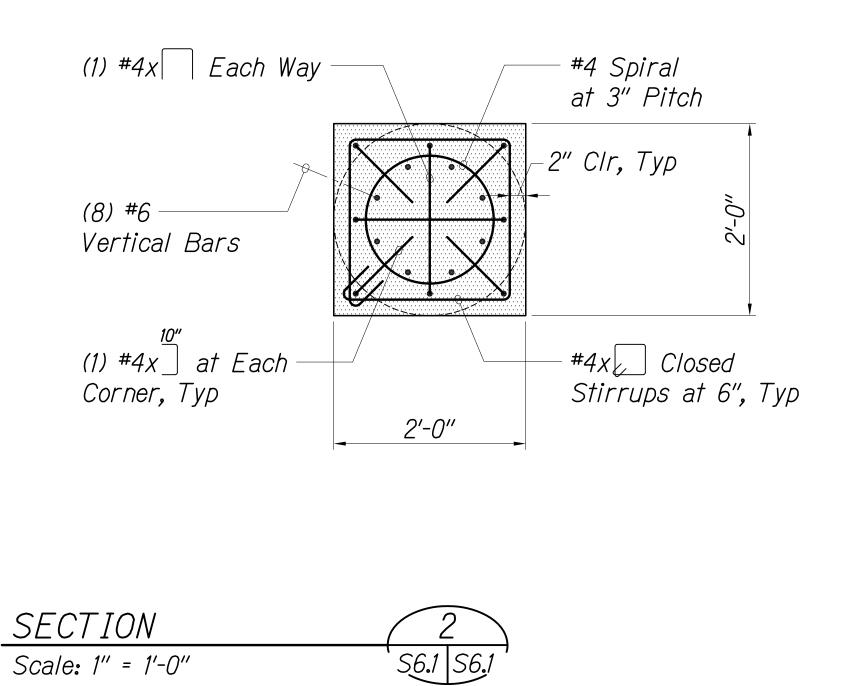
<u>INTERSTATE ROUTE H-1</u>

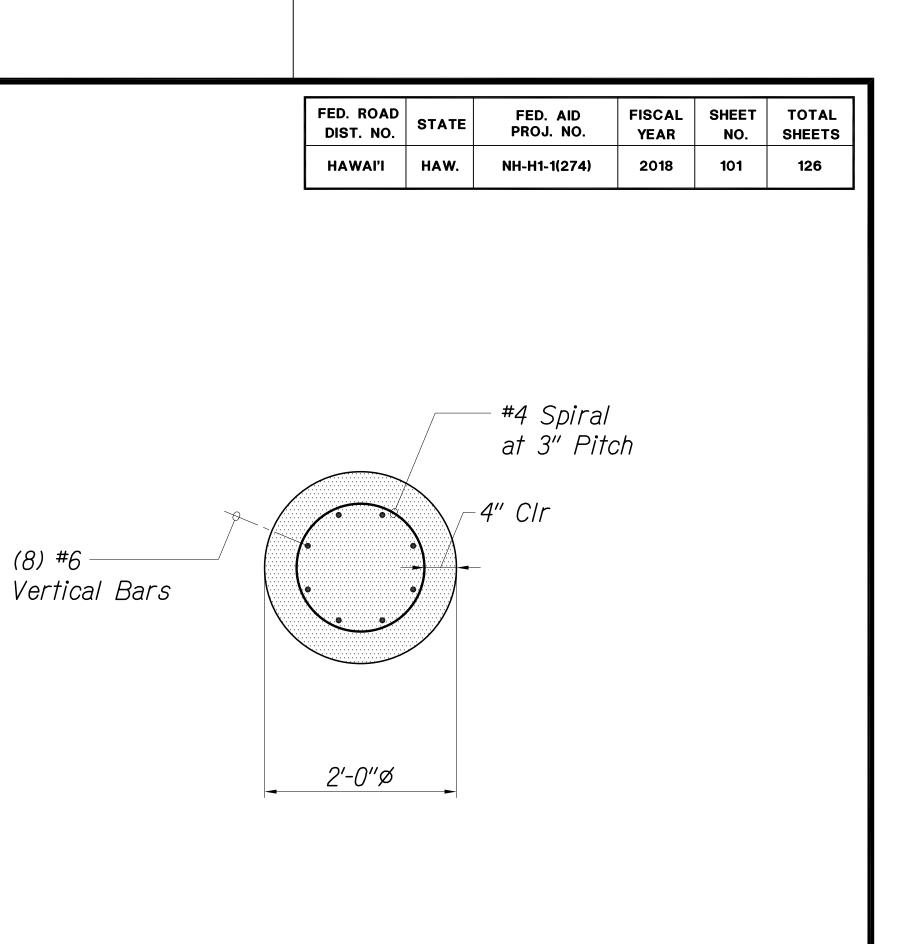
SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 — SHOULDER WORK Date: November 30, 2018 Scale: As Shown

SHEET No. S5.2 OF 2 SHEETS







S6.1 S6.1



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(8) #6

SECTION

Scale: 1" = 1'-0"

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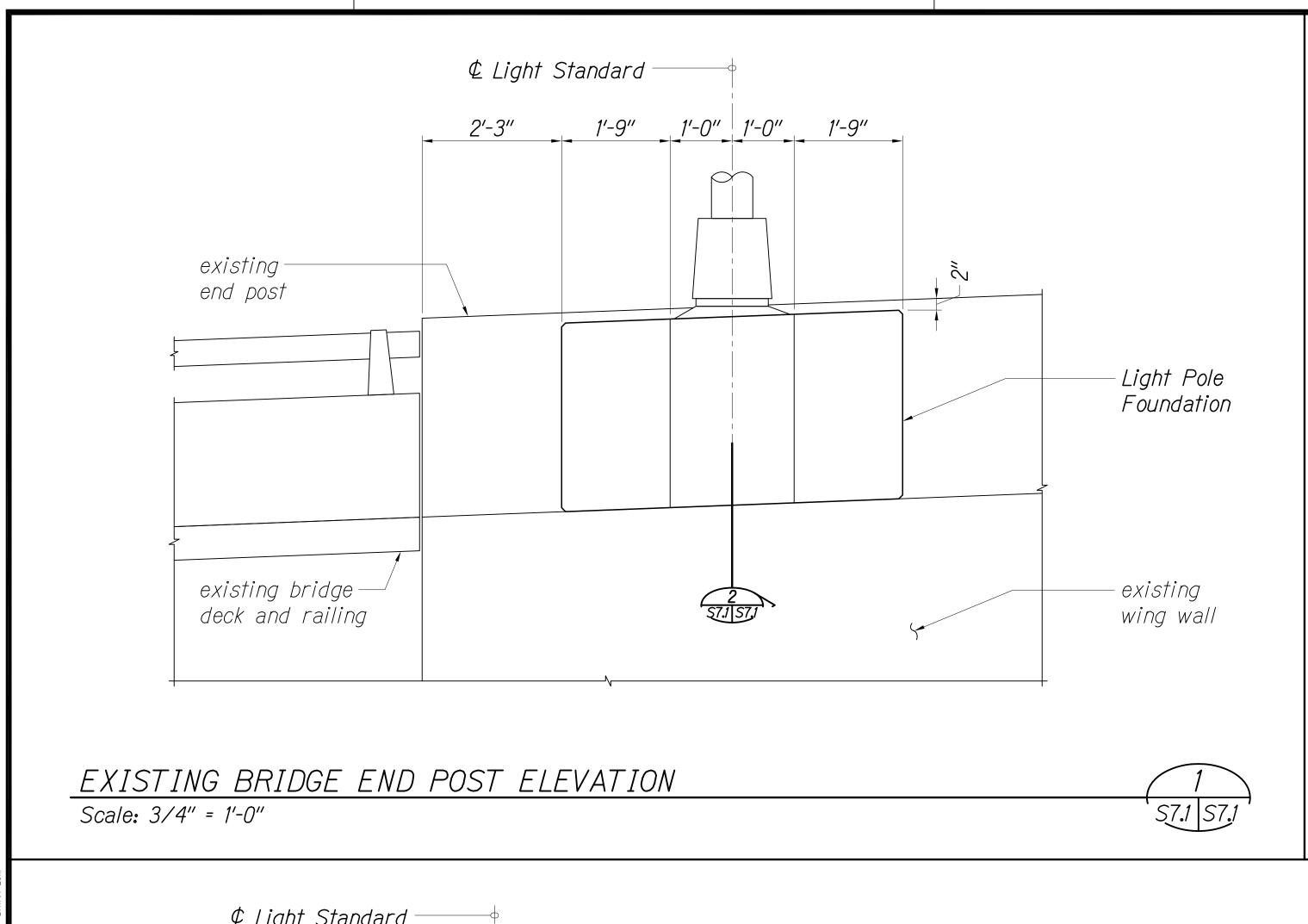
April 30, 2020
SIGNATURE EXPIRATION DATE OF THE LICENSE

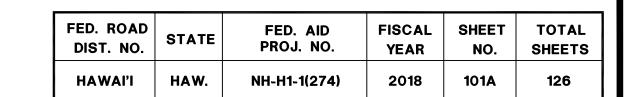
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION
TYPICAL LIGHT POLE <u>FOUNDATION DETAILS</u>

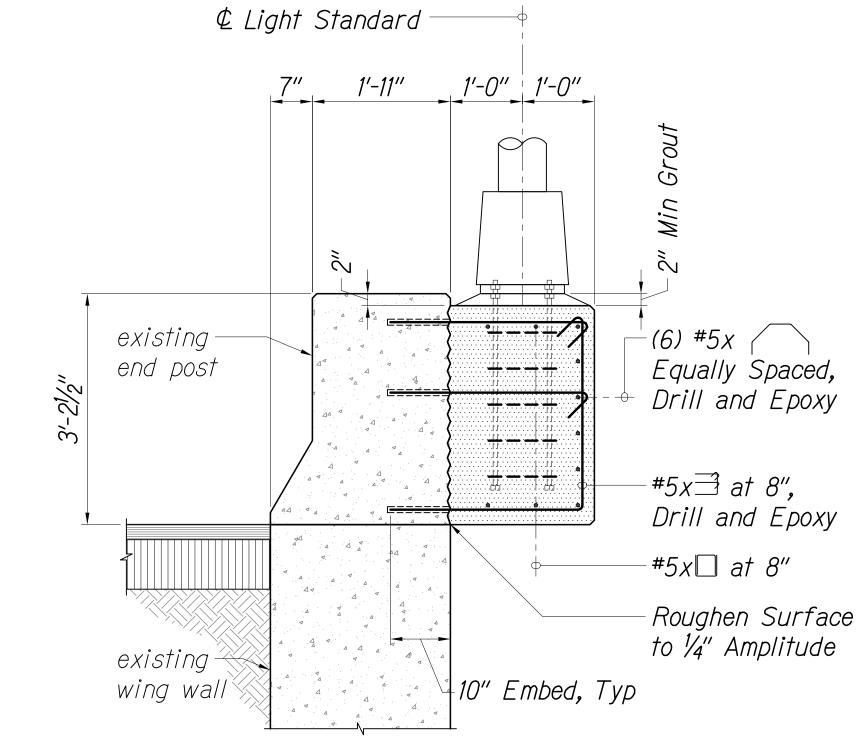
<u>INTERSTATE ROUTE H-1</u> SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 — SHOULDER WORK Date: November 30, 2018 Scale: As Shown

SHEET No. S6.1 OF 1 SHEETS



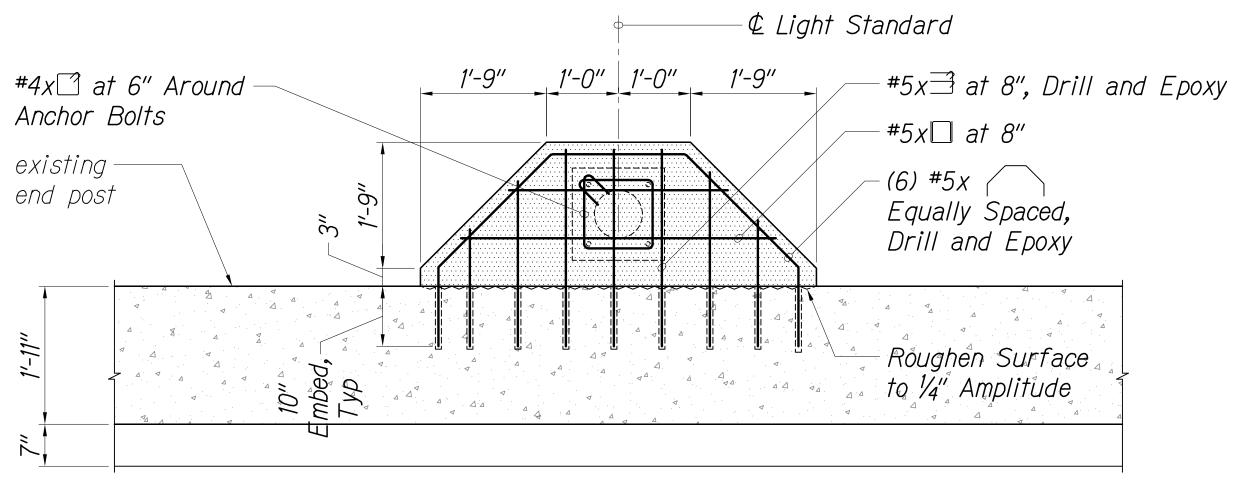




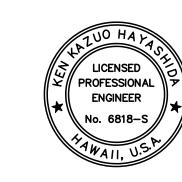
SURVEY PLOTTIEDRAWN BY TRACED BY DESIGNED BY QUANTITIES BY CHECKED BY

SECTION

Scale: 3/4" = 1'-0"



Section - Plan View



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April 30, 2020
SIGNATURE EXPIRATION DATE
OF THE LICENSE

DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION END POST MOUNTED <u>LIGHT POLE FOUNDATION</u>

<u>INTERSTATE ROUTE H-1</u> SHOULDER WORK AND PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION VICINITY OF WAIMALU TO VICINITY OF HALAWA

PHASE 2 — SHOULDER WORK Date: November 30, 2018 Scale: As Shown

SHEET No. S7.1 OF 1 SHEETS

"AS-BUILT"

*S*7.1 *S*7.1