BRIDGE INSPECTION, INVENTORY AND APPRAISAL III, FY 2016

FARRINGTON HIGHWAY BOX CULVERT 1 OVER DITCH BRIDGE NO. 935



Prepared for:

CITY AND COUNTY OF HONOLULU DEPARTMENT OF DESIGN AND CONSTRUCTION CONTRACT NO. SC-DDC-1600113



50 S. Beretania Street, #C-119C Honolulu, HI 96813

NOVEMBER 2016

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This work has been prepared by me or under my supervision.



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SECTION 1.0 - BRIDGE DESCRIPTION

1.1 LOCATION

The culvert is located on Farrington Highway in Kapolei on the island of Oahu.

1.2 DESCRIPTION

No. of Traffic Lanes:

The existing structure is a reinforced concrete box culvert.

Year Built: 1922

Tax Map Key (TMK): 9-1-17

Spans: 1

Bridge Rails: W-beam guardrail

Sidewalks: None

Wearing Surface: A.C. wearing surface
Culvert: Reinforced Concrete

Channel: Natural
Utilities: None
Skew: 0°
Clear Span of Bridge: 6'

Roadway Width Curb to Curb: 22'
Total Bridge Width Out to Out: 27'

Global Positioning System (GPS):

| | DATUM (NAD83) |
|---------------|---------------|
| Latitude (N) | 021° 21' 18" |
| Longitude (W) | 158° 03' 43" |

SECTION 2.0 - PREVIOUS INSPECTIONS

2.1 INSPECTION HISTORY

Previous Bridge Inspection Reports and Bridge Appraisal Sheets on file with the City and County of Honolulu are dated: 8/3/1972, 8/4/1975, 7/10/1980, 6/20/1984, 8/8/1988, 4/16/1990, 5/2/1996, 3/2/2004, 11/19/2013.

2.2 LIST OF PREVIOUS SIGNIFICANT OBSERVATIONS

- Upstream guardrail failed
- Heavy vegetation in channels
- 6"-12" of freeboard height at each culvert end

SECTION 3.0 - OBSERVATIONS

3.1 TRAFFIC FEATURES

- Culvert railings do not appear to be crash tested
- The upstream guardrail is in a failed state and has been since before the 1996 inspection of the culvert (see Photo 9)

3.2 DECK

- A.C. wearing surface is in satisfactory condition

3.3 SUPERSTRUCTURE

N/A

3.4 SUBSTRUCTURE

N/A

3.5 CULVERT

- Corroded upstream guardrail posts embedments in upstream headwall (see Photo 14)
- Spall measuring 8" high x 16" wide x 1" deep occurring at the top East corner of the upstream headwall (see Photo 15)
- 12" long x 72" wide area of delamination occurring in culvert soffit at inlet (see Photo 17)
- Spall measuring 10' long x 30" wide x 2" deep with 8 exposed reinforcing bars occurring in culvert soffit near outlet above the East culvert wall (see Photo 18)
- Spall measuring 10' long x 24" wide x 2" deep with 8 exposed reinforcing bars occurring in culvert soffit near outlet above the West culvert wall (see Photo 19)
- Scaling occurring along the top 12" of the culvert walls (see Photo 22)
- Scattered spalls occurring throughout downstream headwall (see Photo 25)

3.6 STREAM OVERVIEW

- Concrete block built at outlet leaves only the top 16" of the culvert for water to flow through (see Photo 26)

3.7 COMPARISON WITH PREVIOUS INSPECTION

The culvert appears to have been recently cleaned so inspection inside the culvert was possible. Extensive spalling was discovered during this inspection in the culvert soffit with many exposed reinforcing bars,

SECTION 4.0 - LOAD RATINGS

4.1 LOAD RATINGS

This structure is not posted for reduced load carrying capacity. The visual inspection of the structure found no evidence of overload or over stress. Load rating calculations were completed for this structure. The load rating summary can be found on the next page. The calculations are located in Appendix E.

City and County of Honolulu Bridge Load Rating Summary

Existing Bridge Data

| EXISTI | ng Bridge Data | | | | | | | | |
|--|------------------------------------|--------------------|---------------------|--------------|---------------------|--|-----------------|----------------------------------|--|
| Bridge | Number: | | 935 | | Last Loa | nd Rating Date: | | 11/23/2016 | |
| Bridge Name: Structure Number: District: | | Far | rington Hwy Box Cu | lv No. 1 | | pection Date: | | 10/6/2016 | |
| | | | · | | Inspect | ed By: | | KAI Hawaii | |
| | | | | | Fracture | e Critical Members (Y/N): | | N | |
| Span . | Туре: | Reir | nforced Concrete Bo | x Culvert | Item 58 | , Deck Rating: | | N | |
| Bridge | e Plans Available | (Y/N): | Υ | | Item 59 | , Superstructure Rating: | | N | |
| Desig | n Loading: | | | | Item 60 | , Substructure Rating: | | N | |
| Past I | nventory Rating: | | | | Bridge I | oad Posted (Y/N): | | N | |
| Past C | perating Rating: | | | | Posted | Weight Limit: | | N/A | |
| Bridge | e Load Rating Su | mmary | | | | | | | |
| Dead | Load Data | | | | LRFR Ev | aluation Factors: | | | |
| Overla | ау Туре: | | Asphalt | | Surface | Roughness Rating: | | 2 | |
| Overla | ay Depth (IN): | | 3 | | Condition | on Factor: | | 1.00 | |
| Was C | Overlay Depth Me | easured (Y/N): | N | | System | Factor: | | 1.00 | |
| Weigh | nt of Utilities: | | N/A | | ADTT (c | ne way): | | Unknown | |
| Weigh | nt of other Non-S | tructural | | | | | | | |
| Attacl | nments: | | N/A | | | | | | |
| Super | structure/Deck F | Rating Summary | | | | | | | |
| Vehic | le Type | Vehicle GVW (Kips) | Rating Factor | Controlli | ng Member | Controlling Load Effect | IM | Live Load Distribution | |
| _ | (1411.1) | 21/2 | 0.05 | | | EI . | 22.00/ | Factor | |
| Design Load | HL-93 (INV) | N/A | 0.86 | | p slab, inside face | Flexure | 33.0% | 0.138 | |
| | HL-93 (OPR) | N/A | 1.11 | | p slab, inside face | Flexure | 33.0% | 0.138 | |
| | Type 3 | 50.0 | 1.42 | | p slab, inside face | Flexure | 33.0% | 0.138 | |
| | Type 3S2 | 72.0 | 1.56 | | p slab, inside face | Flexure | 33.0% | 0.138 | |
| ре | Type 3-3 | 80.0 | 1.51 | | p slab, inside face | Flexure | 33.0% | 0.138 | |
| Legal Load | NRL | 80.0 | 1.42 | | p slab, inside face | Flexure | 33.0% | 0.138 | |
| Leg | SU4 | 54.0 | 1.42 | Middle of to | p slab, inside face | Flexure | 33.0% | 0.138 | |
| | SU5 | 62.0 | 1.42 | Middle of to | p slab, inside face | Flexure | 33.0% | 0.138 | |
| | SU6 | 69.5 | 1.42 | | p slab, inside face | Flexure | 33.0% | 0.138 | |
| | SU7 | 77.5 | 1.42 | Middle of to | p slab, inside face | Flexure | 33.0% | 0.138 | |
| Load | HP1 | 120.0 | 1 | Middle of to | p slab, inside face | Flexure | 33.0% | 0.138 | |
| Permit Load | HP2 | 157.0 | 0.6 | | p slab, inside face | Flexure | 33.0% | 0.138 | |
| Pe | HP3 | 209.9 | 1.19 | Middle of to | p slab, inside face | Flexure | 33.0% | 0.138 | |
| Subst | ructure Rating Suructure Rated (Y/ | | - Rating Factor | Controlli | ng Member | Controlling Load Effect | IM | Live Load Distribution Factor | |
| | (INV) | N/A | - | | - | - | - | - | |
| | (OPR) | N/A | - | | - | - | - | - | |
| _egal | | N/R | - | | - | - | - | - | |
| 'ermi | t Load | N/R | - | | - | - | - | - | |
| | ng Analysis Sumn | | | | | theck the following boxes that | | | |
| | ning Rating Facto | | 1.42 | | | dge load rating is not governed | | | |
| Governing Load Model: | | | Type 3 | | | dge load rating is not governed | • | rating | |
| Posting Recommended (Y/N): | | | N | | | nnections do not control the bri | | | |
| Recor | nmended Posting | g Load: | - | | | erior girder controls the bridge dge plans do not exist - Rated b | - | ent and current loading | |
| Quali | ty Control/Qualit | ty Assurance | | | 311K | -G2 pand do not exist mateu b | a on Jaagenn | and darrent loading | |
| Load I | Rating Engineer N | Name: | Saeid Pourjalal | i | Remark | s/Recommendations for Bridg | es without Plan | s | |
| | Rating Engineer L | | 11475 | | | | | | |
| | Rating Engineer S | | 800 mile | 6. | | | | | |
| | Ratings Checked I | | Bryan Lum | | | | | | |
| | y Assurance By: | | Mike Hunnemar | n | | | | | |
| Load Rating Date: | | | 11/23/2016 | | | | | | |
| I han I | | | 11/23/2010 | | | | | | |

City and County of Honolulu Bridge Load Rating Summary

Existing Bridge Data

| Bridge Number: | 935 | Last Load Rating Date: | 11/23/2016 |
|-------------------------------|---------------------------------|----------------------------------|------------|
| Bridge Name: | Farrington Hwy Box Culv No. 1 | Last Inspection Date: | 10/6/2016 |
| Structure Number: | | Inspected By: | KAI Hawaii |
| District: | | Fracture Critical Members (Y/N): | N |
| Span Type: | Reinforced Concrete Box Culvert | Item 58, Deck Rating: | N |
| Bridge Plans Available (Y/N): | Υ | Item 59, Superstructure Rating: | N |
| Design Loading: | | Item 60, Substructure Rating: | N |
| Past Inventory Rating: | | Bridge Load Posted (Y/N): | N |
| Past Operating Rating: | | Posted Weight Limit: | N/A |

Bridge Load Rating Summary

| Bridge Load Rating Summary | | | | | |
|------------------------------------|------------------------|--|----------------|--|--|
| Dead Load Data | | LRFR Evaluation Factors: | | | |
| Overlay Type: | Asphalt | Surface Roughness Rating: | 2 | | |
| Overlay Depth (IN): | 3 | Condition Factor: | 1.00 | | |
| Was Overlay Depth Measured (Y/N): | N | System Factor: | 1.00 | | |
| Weight of Utilities: | N/A | ADTT (one way): | Unknown | | |
| Weight of other Non-Structural | | | | | |
| Attachments: | N/A | | | | |
| Superstructure/Deck Rating Summary | | | | | |
| Vehicle Type Vehicle GV | W (Kips) Rating Factor | Controlling Member Controlling Load Effect | IM Live Load D | | |

| Vehic | le Туре | Vehicle GVW (Kips) | Rating Factor | Controlling Member | Controlling Load Effect | IM | Live Load Distribution Factor |
|---------------------------------|---------|--------------------|---------------|---------------------------------|-------------------------|-------|----------------------------------|
| les | REF1 | 51.00 | 1.29 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| Refuse Vehicles | REF2 | 57.18 | 1.21 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| fuse / | REF3 | 45.94 | 1.42 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| - Re- | REF4 | 57.50 | 1.23 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS1 | 30.99 | 1.18 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS2 | 39.60 | 0.96 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS3 | 39.60 | 0.96 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS4 | 64.38 | 0.95 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS5 | 67.24 | 0.84 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| Buses | BUS6 | 67.78 | 0.87 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| _ | BUS7 | 66.79 | 0.86 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS8 | 39.90 | 0.89 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS9 | 39.60 | 0.96 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS10 | 39.60 | 0.96 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | BUS11 | 42.54 | 0.86 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD1 | 38.40 | 1.00 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD2 | 42.74 | 1.00 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD3 | 43.50 | 1.00 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD4 | 49.80 | 0.89 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD5 | 49.80 | 0.89 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD6 | 49.80 | 0.89 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD7 | 52.20 | 0.76 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD8 | 62.74 | 1.09 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| sk | HFD9 | 73.50 | 0.92 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| Honolulu Fire Department Trucks | HFD10 | 59.24 | 1.00 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| ment | HFD11 | 60.00 | 1.20 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| part | HFD12 | 51.18 | 1.27 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| re De | HFD13 | 58.00 | 1.20 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| = | HFD14 | 44.00 | 0.79 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| onolu | HFD15 | 44.00 | 0.80 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| Ĩ | HFD16 | 44.00 | 0.99 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD17 | 42.74 | 1.00 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD18 | 76.60 | 0.77 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD19A | 77.56 | 0.90 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD19B | 77.56 | 0.92 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD20A | 87.56 | 0.90 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD20B | 87.56 | 0.92 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD21 | 42.00 | 1.00 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |
| | HFD22 | 37.00 | 1.05 | Middle Of Top Slab, Inside face | Flexure | 33.0% | 0.138 |

Substructure Rating Summary

Substructure Rated (Y/N):

Recommended Refuse Vehicle

Recommended Refuse LR Factor: Recommended Refuse Load Model: Recommended Max Payload:

| | 1.21 |
|--------|--------------|
| | ALL |
| | FULL |
| `~ ~ ~ | ing Conscitu |

*Payload is Allowable Vehicle Load Caryying Capacity

Quality Control/Quality Assurance

Load Rating Engineer Name: Load Rating Engineer License No.: Load Rating Engineer Signature: Load Ratings Checked By: Quality Assurance By: Load Rating Date:

| Saeid Pourjalali |
|------------------|
| 11475 |
| Sai pull. |
| Bryan Lum |
| Mike Hunnemann |
| 2/28/2017 |
| |

Please check the following boxes that apply:

Bridge load rating is not governed by deck rating

✓ Bridge load rating is not governed by substructure rating

✓ Connections do not control the bridge load rating

Exterior girder controls the bridge load rating

Bridge plans do not exist - Rated based on judgement and current loading

Remarks/Recommendations for Bridges without Plans

SECTION 5.0 - GENERAL CONDITION ASSESSMENT

5.1 CONDITION ASSESSMENT

In general, the culvert is in poor condition.

5.2 MAINTENANCE RECOMMENDATIONS

| MAINTENANCE RECOMMENDATIONS |
|--|
| No maintenance recommendations at this time. |

5.3 REPAIR RECOMMENDATIONS

In consideration of the limited resources available for bridge rehabilitation, the City should prioritize the remediation of the deficiencies as it deems to be appropriate.

| REPAIR RECOMMENDATIONS | ESTIMATED COST | | |
|---|----------------|--|--|
| Upgrade bridge railings | \$10000.00 | | |
| Repair spalls in culvert headwalls and soffit | \$30000.00 | | |