

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	2-11-1(111)-214	1974	155	234

NOTES

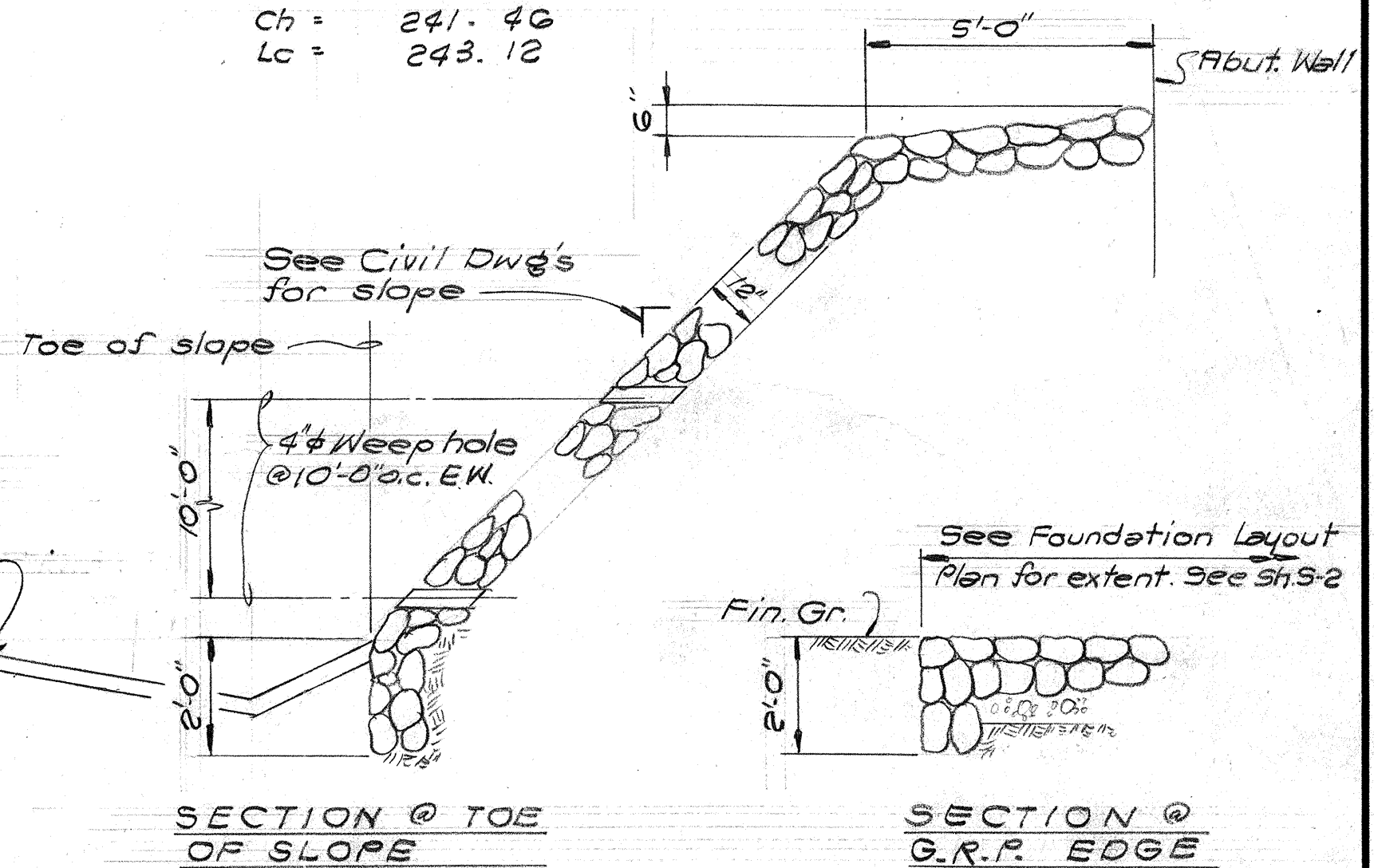
1. Date shown shall be the year structure is completed.
2. For letter size & detail see "Standard Bridge Detail" Sh. S-25

PEARL HARBOR INTERCHANGE
STRUCTURE #1
19 -

NAME & DATE OF STRUCTURE

PLANTATION DRIVE #2 CURVE DATA #2

$\Delta = 23^\circ 13' 00''$
 $\Delta/2 = 11^\circ 36' 30''$
 $R = 600.00$
 $T = 123.25$
 $Ch = 241.46$
 $Lc = 243.12$



TYPICAL G.R.P. DETAIL

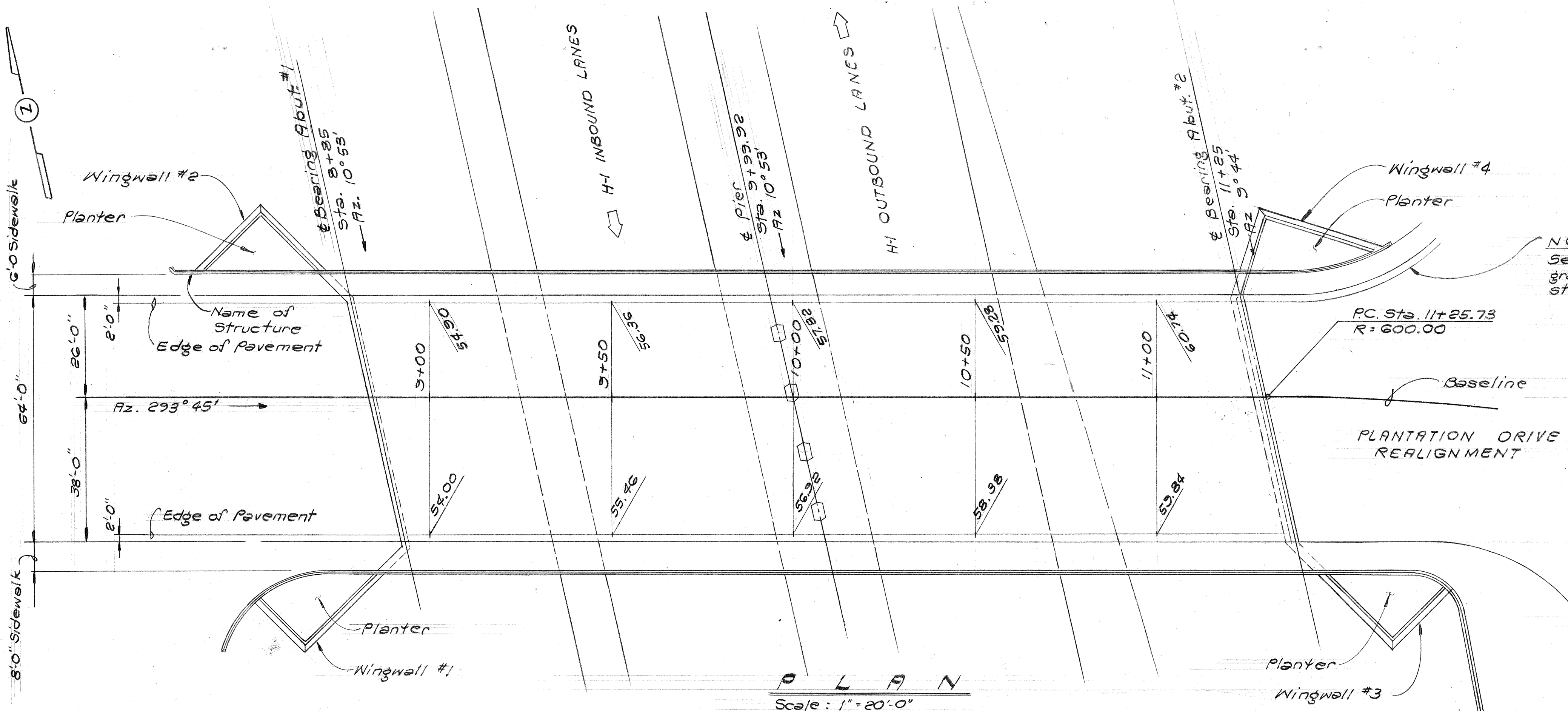
3/8" = 1'-0"

SPECIAL NOTE

All footings shall bear 1'-0" min. into competent tuff - Subject to soils Engineers approval.

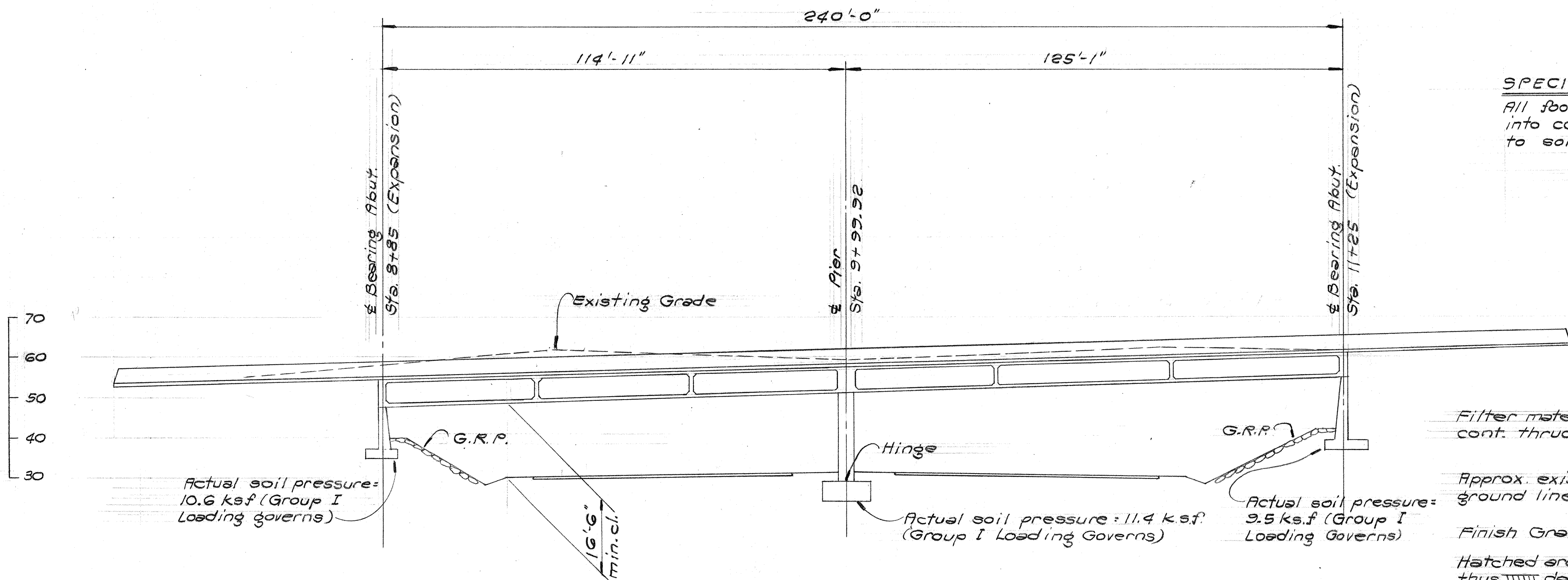
61618 Gutter

Retaining Wall
See Sh. R-1
for details



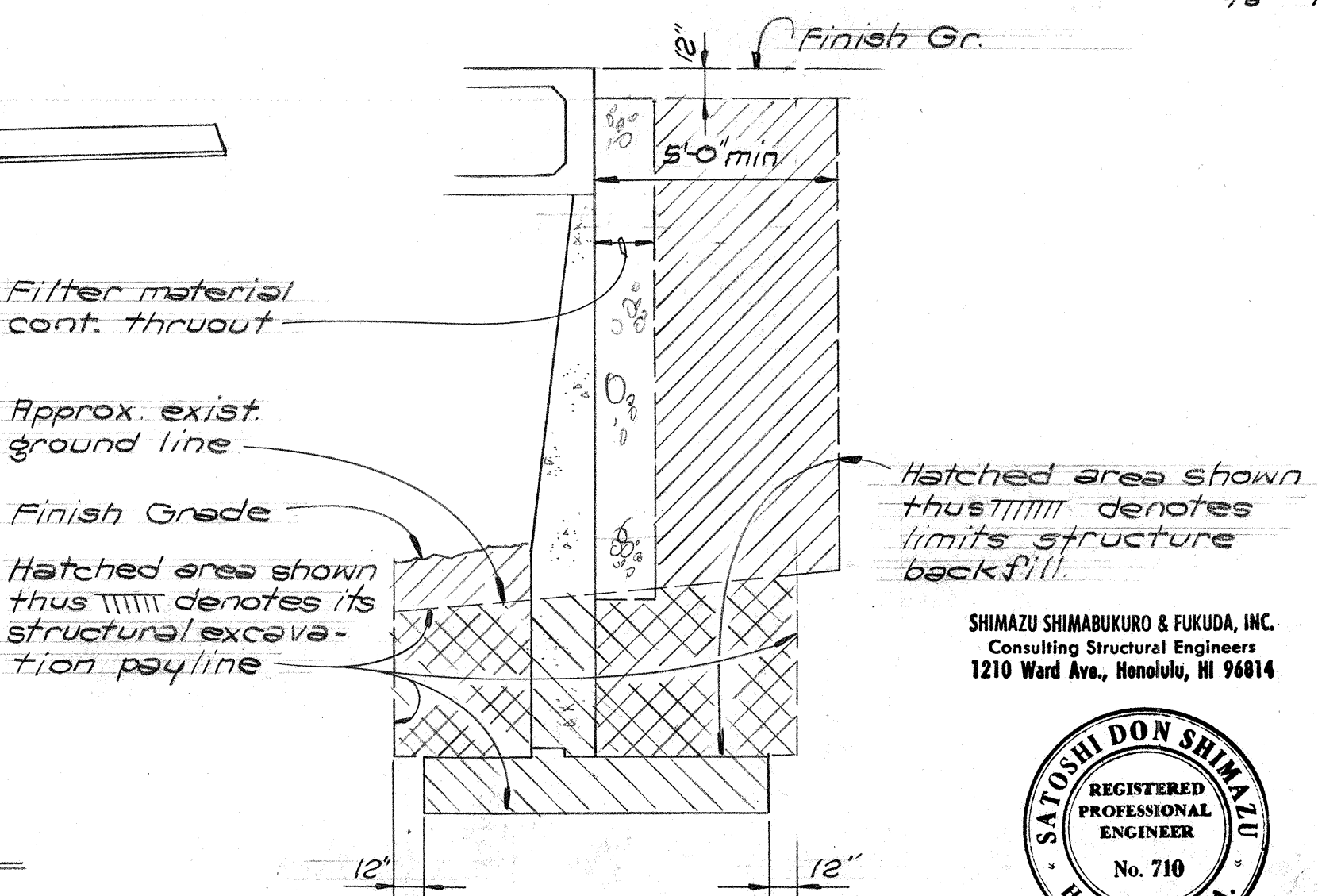
PLAN

Scale: 1" = 20'-0"



LONGITUDINAL SECTION ALONG BASELINE OF STRUCTURE

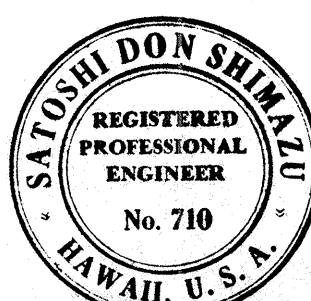
Scale: 1" = 20'-0"



STRUCTURAL EXCAVATION & BACKFILL DETAIL

N.T.S.

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Consulting Structural Engineers
1210 Ward Ave., Honolulu, HI 96814



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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE No. 1

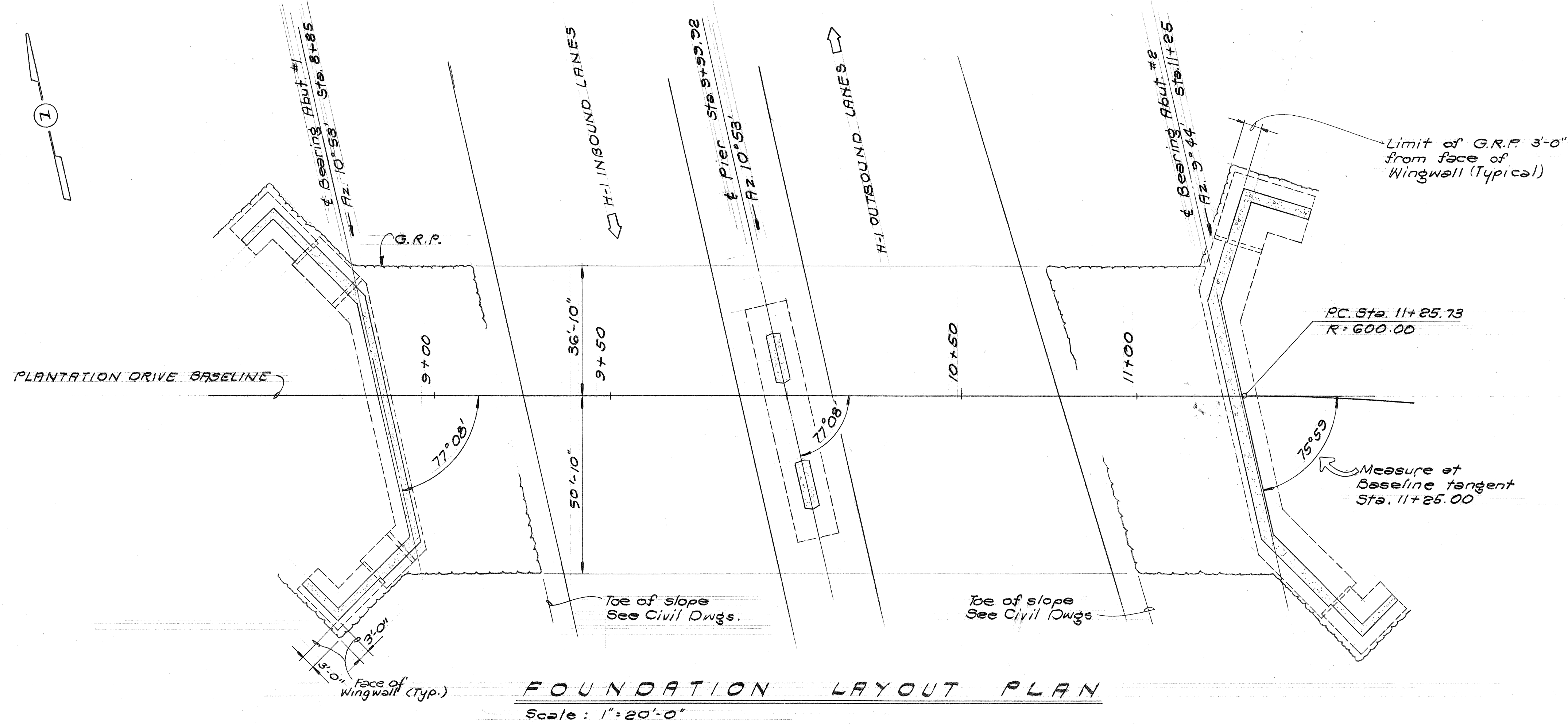
LAYOUT PLAN &
LONGITUDINAL SECTION

PEARL HARBOR INTERCHANGE
F.A.I.-Proj. No. I-H-1(111)-14

Scale: AS SHOWN Date: DEC. 14, 1973

SHEET No. 5-1 OF 34 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(11):14	1974	156	234



FOUNDATION LAYOUT PLAN
Scale: 1"=20'-0"

BRIDGE QUANTITY ESTIMATES

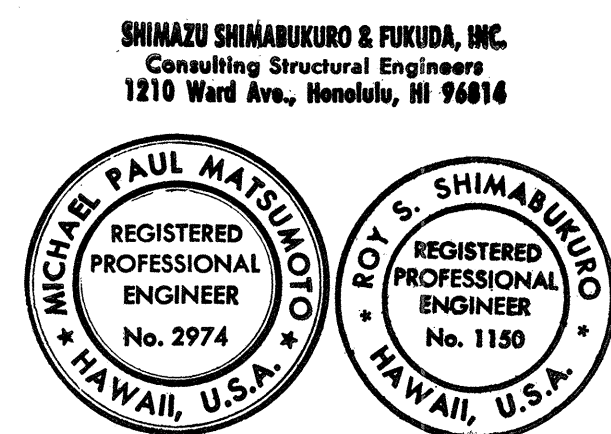
ITEM No.	ITEM	UNIT	QUANTITY
206.60	Structural Excavation	C.Y.	4,784
206.65	Structural Backfill	C.Y.	1,782
503.071	Concrete in Footings	L.S.	513 C.Y.
504.51	Post-Tensioning Operation for Structure	L.S.	84,630#
503.072	Concrete for Cast-in-Place Structure	L.S.	2238 C.Y.
602.80	Reinforcing Steel	L.S.	648,342#
206.80	Filter Material	C.Y.	261
507.70	Concrete Bridge parapet, including End Post	L.F.	679
612.01	Grouted Rubble Paving	C.Y.	219
	Sidewalk	S.Y.	373

DESIGN CRITERIA

- Loads = HS 20-44
- Specifications = Standard Specifications for Highway Bridges, AASHTO, 11th Edition & Standard Specifications for Road & Bridge Construction, Hawaii, 1973
- Allowable Foundation Pressure = 12,000 p.s.f. (Footings bearing 1'-0" min. into tuff. - subject to Soils Engineer's approval)
- 28-Day Concrete Strengths
Superstructure, Post-Tensioned = $f'_c = 5,000$ p.s.i.
Abutments, walls & footings = $f'_c = 3,000$ p.s.i. (Class A Concrete)
Columns = $f'_c = 5,000$ p.s.i.
- Reinforcing steel: ASTM A615 Grade 60
- Prestressing Strands: 270 k.s.i. Ultimate strength, seven wire stress-relieved $\frac{1}{8}$ " ϕ strands

REFERENCE DRAWINGS

- Abutment #1 Foundation ---- Sh. S-3
- Abutment #2 Foundation ---- Sh. S-6



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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE No. 4
FOUNDATION
LAYOUT PLAN

PEARL HARBOR INTERCHANGES
F.A.I. Proj. No. I-HI-1(11):14

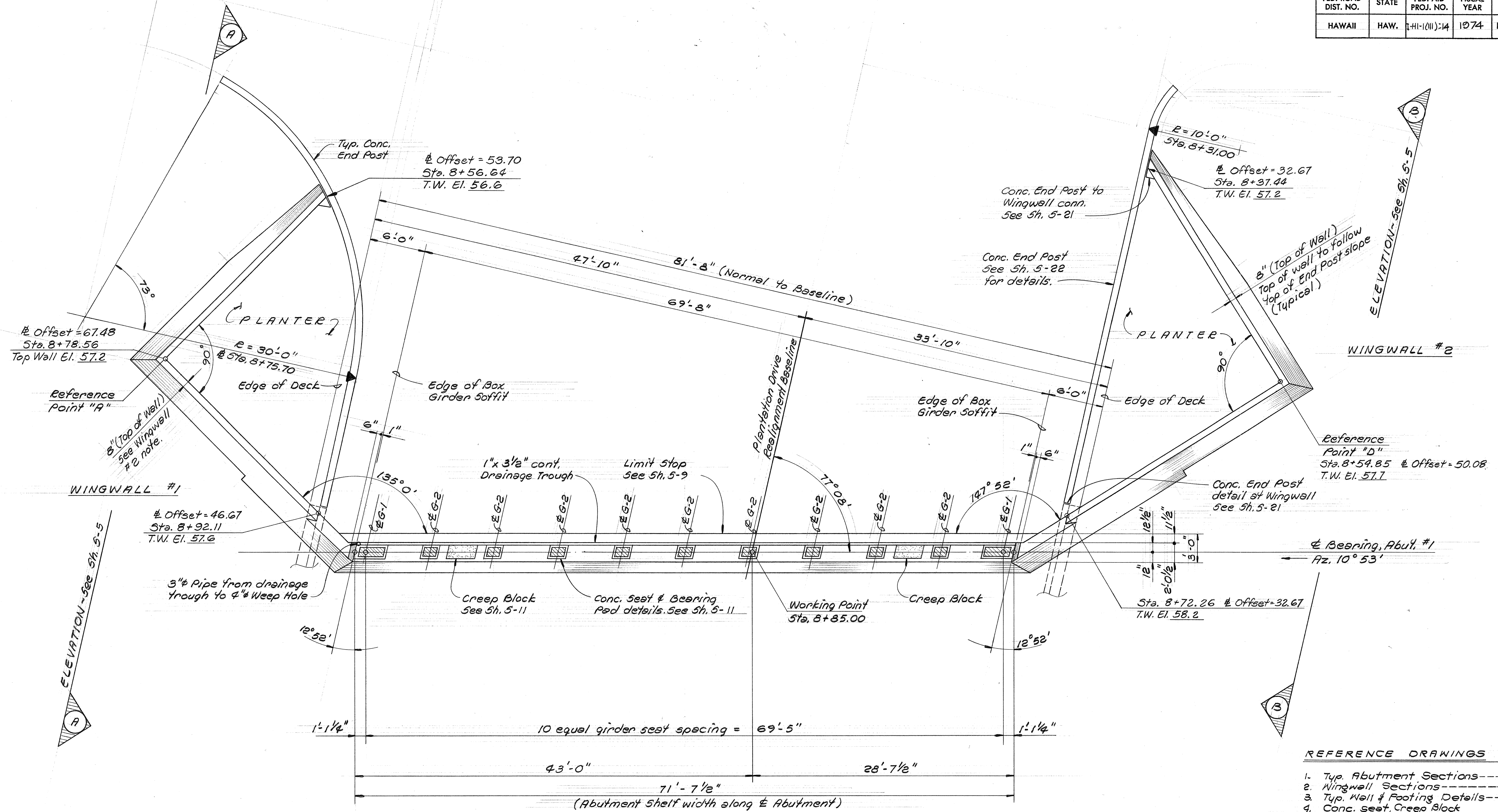
Scale: As Shown Date: DEC. 14, 1973

SHEET No. 9-2 OF 34 SHEETS

SURVEY PLANNED BY	DATE
DRAWN BY	DATE
CHECKED BY	DATE
NOTED BY	DATE
QUANTITIES BY	DATE
CHECKED BY	DATE

ORIGINAL PLAN	NO.
NOTE BOOK	NO.

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-HI-1(11)-14	1974	158	234



- REFERENCE DRAWINGS
1. Typ. Abutment Sections-----Sh. 5-9
 2. Wingwall Sections-----Sh. 5-10
 3. Typ. Wall & Footing Details-----Sh. 5-23
 4. Conc. Seat, Creep Block Bearing Details-----Sh. 5-11

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No. 1180
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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE No. 1
ABUTMENT #1 - PLAN
AT ABUTMENT SHELF

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-HI-1(11)-14

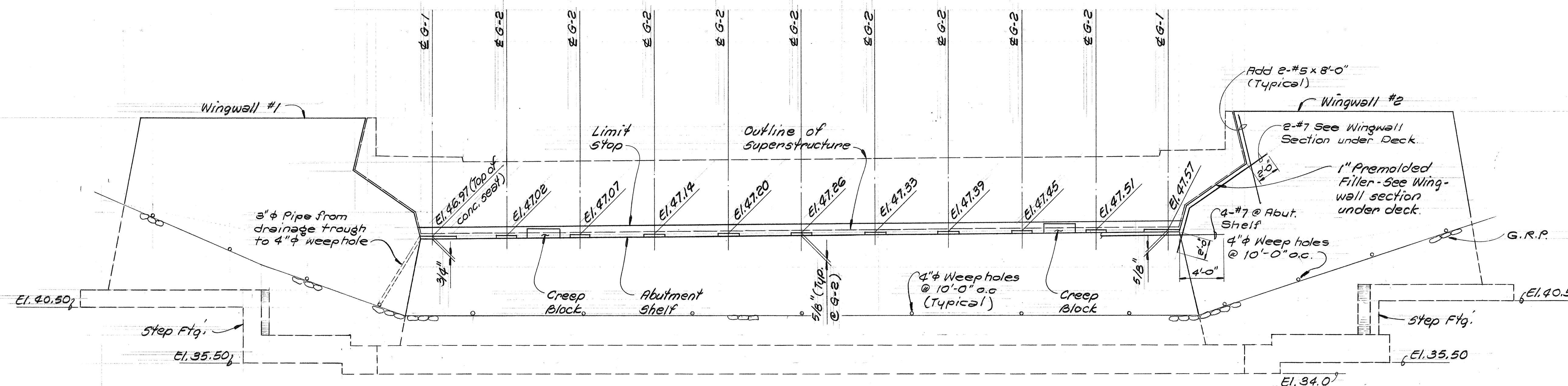
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SHEET No. 5-4 OF 34 SHEETS

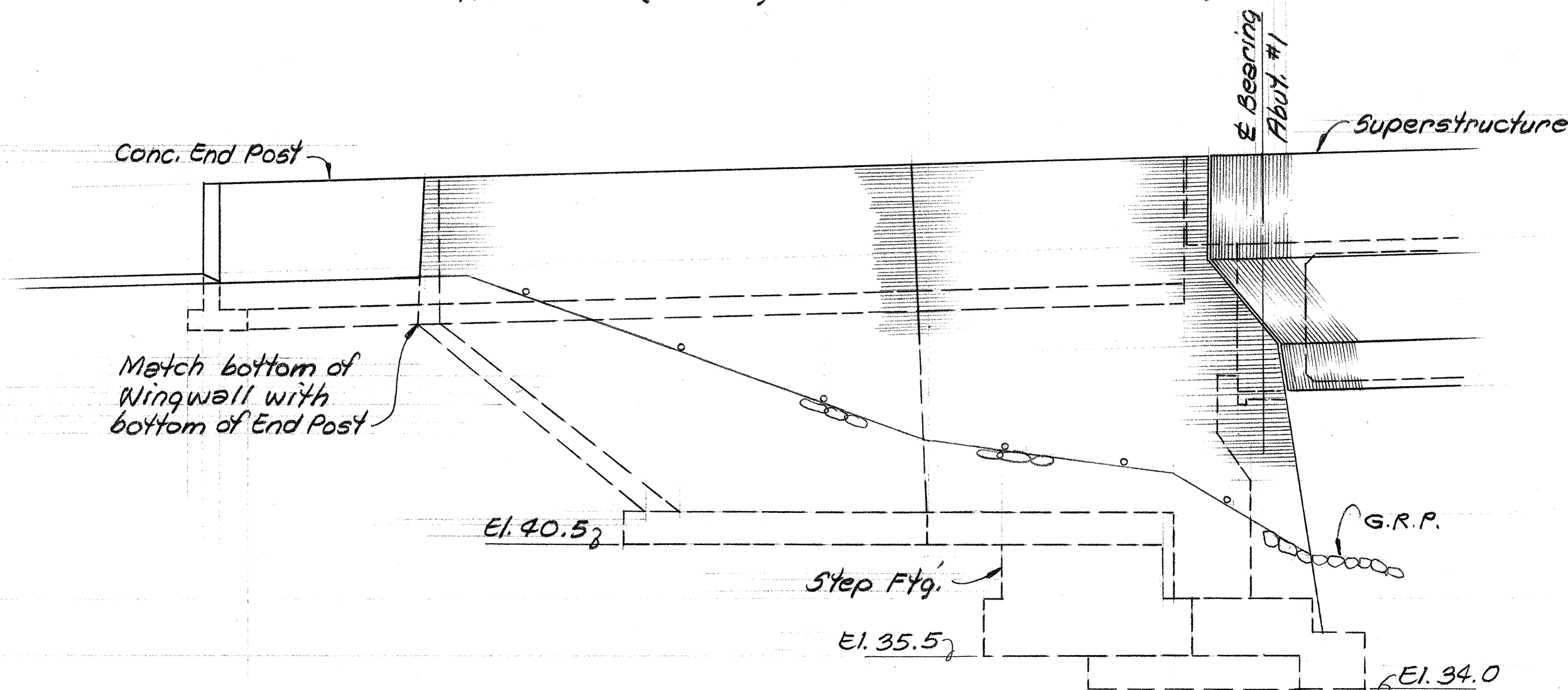
This work was prepared by
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[Signature]

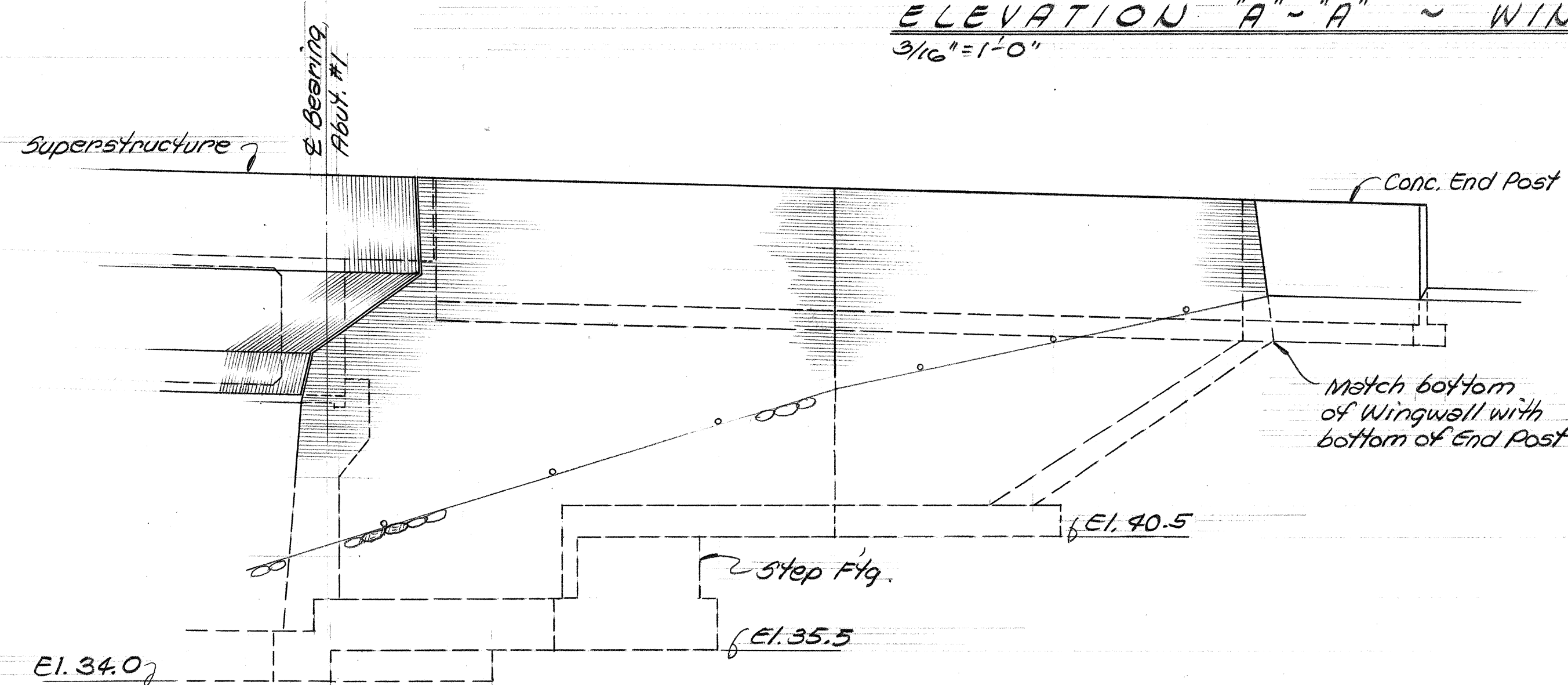
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(III)-14	1974	150	234



ELEVATION ~ ABUTMENT #1
3/16" = 1'-0" (Looking normal to Plantation Drive)

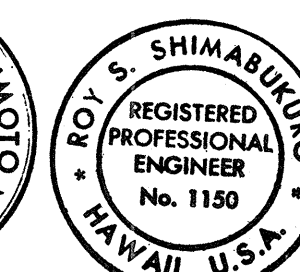


ELEVATION "A"-A ~ WINGWALL #1
3/16" = 1'-0"



ELEVATION "B"-B ~ WINGWALL #2
3/16" = 1'-0"

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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE No. 1

ABUTMENT #1 & WINGWALLS
#1 & #2 ELEVATIONS

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-HI-1(III)-14

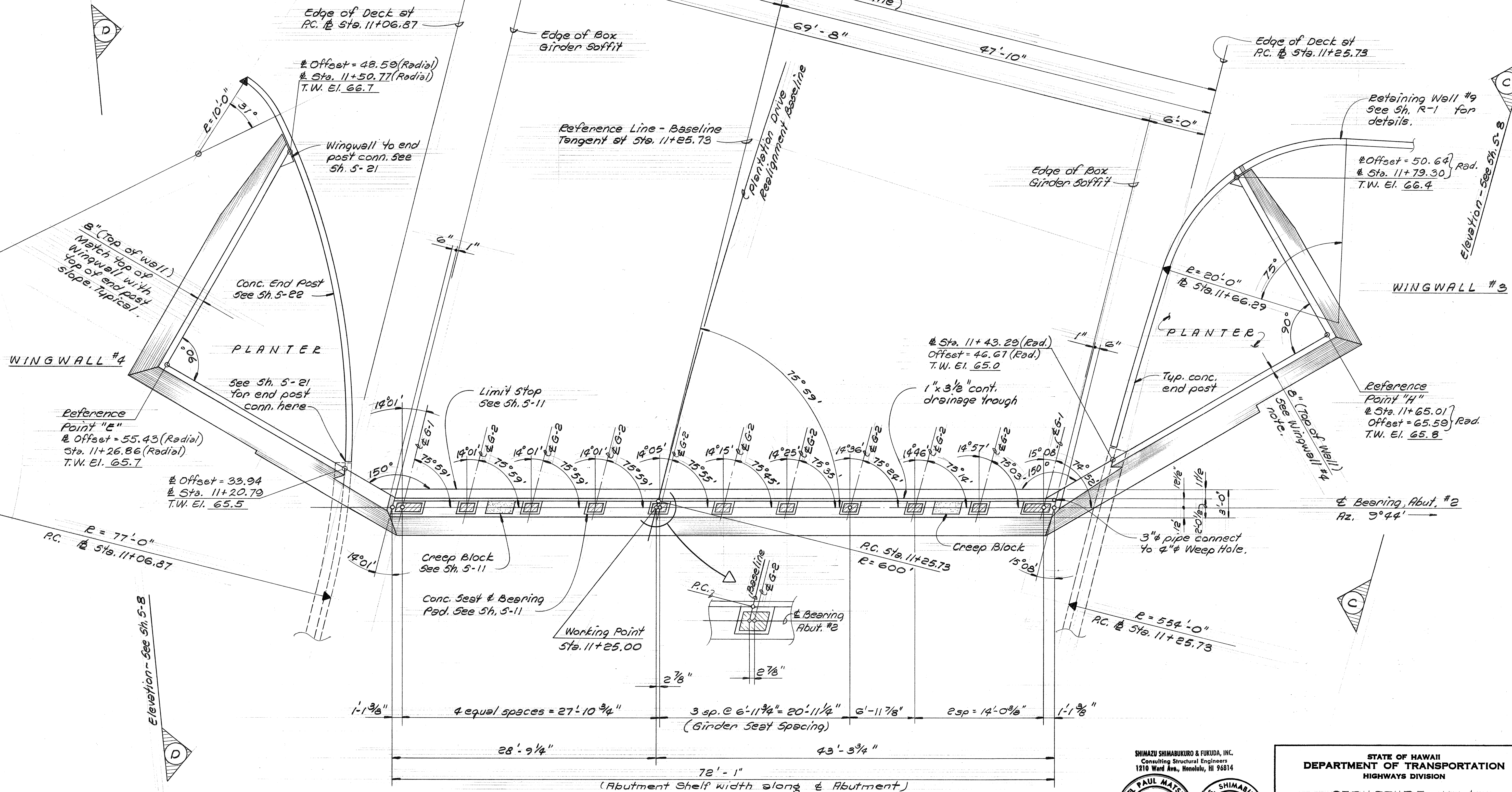
Scale: As Shown Date: DEC. 14, 1973

SHEET No. 9-5 OF 34 SHEETS

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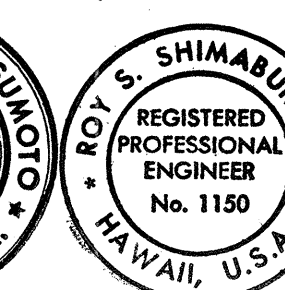
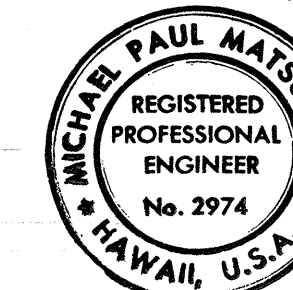
[Signature]

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	3H-1(111)-14	1974	161	234



ABUTMENT #2 ~ PLAN AT ABUTMENT SHELF
3/16" = 1'0"

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**STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION**

STRUCTURE No. 1

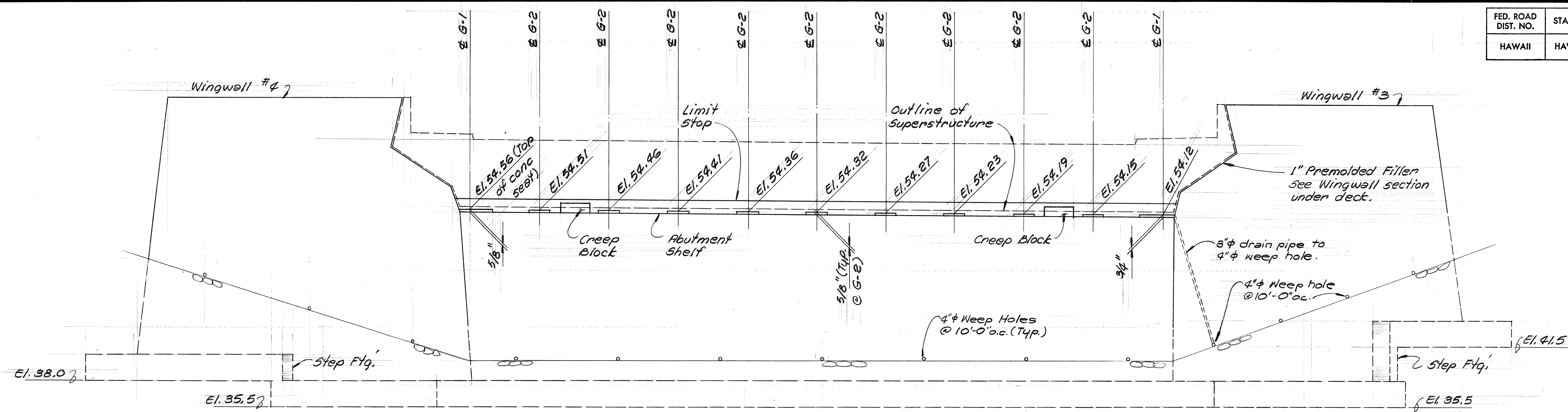
ABUTMENT #2 - PLAN
AT ABUTMENT SHELF

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. 1-H-1 (11): 14

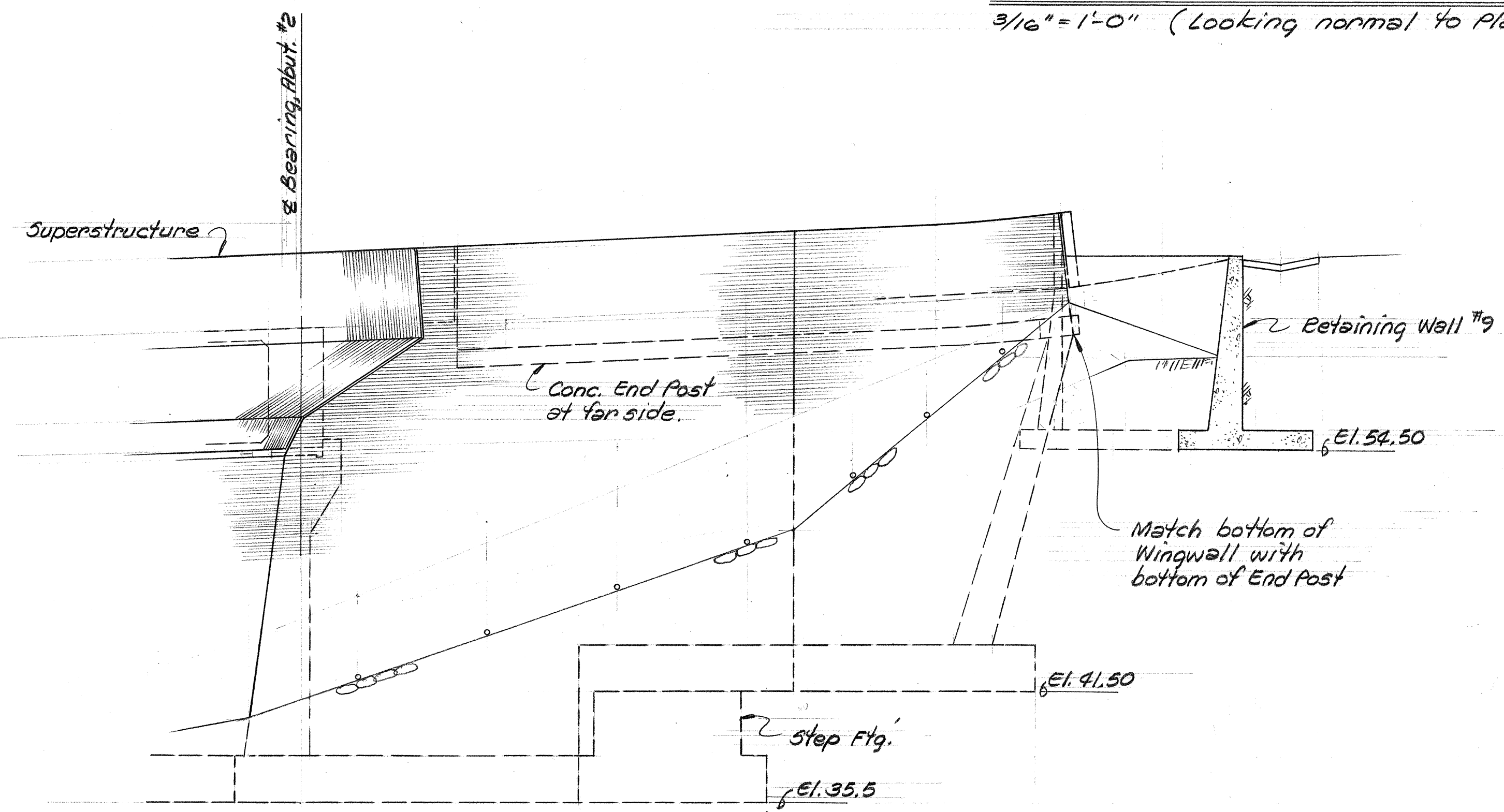
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SHEET No. 5-7 OF 34 SHEETS

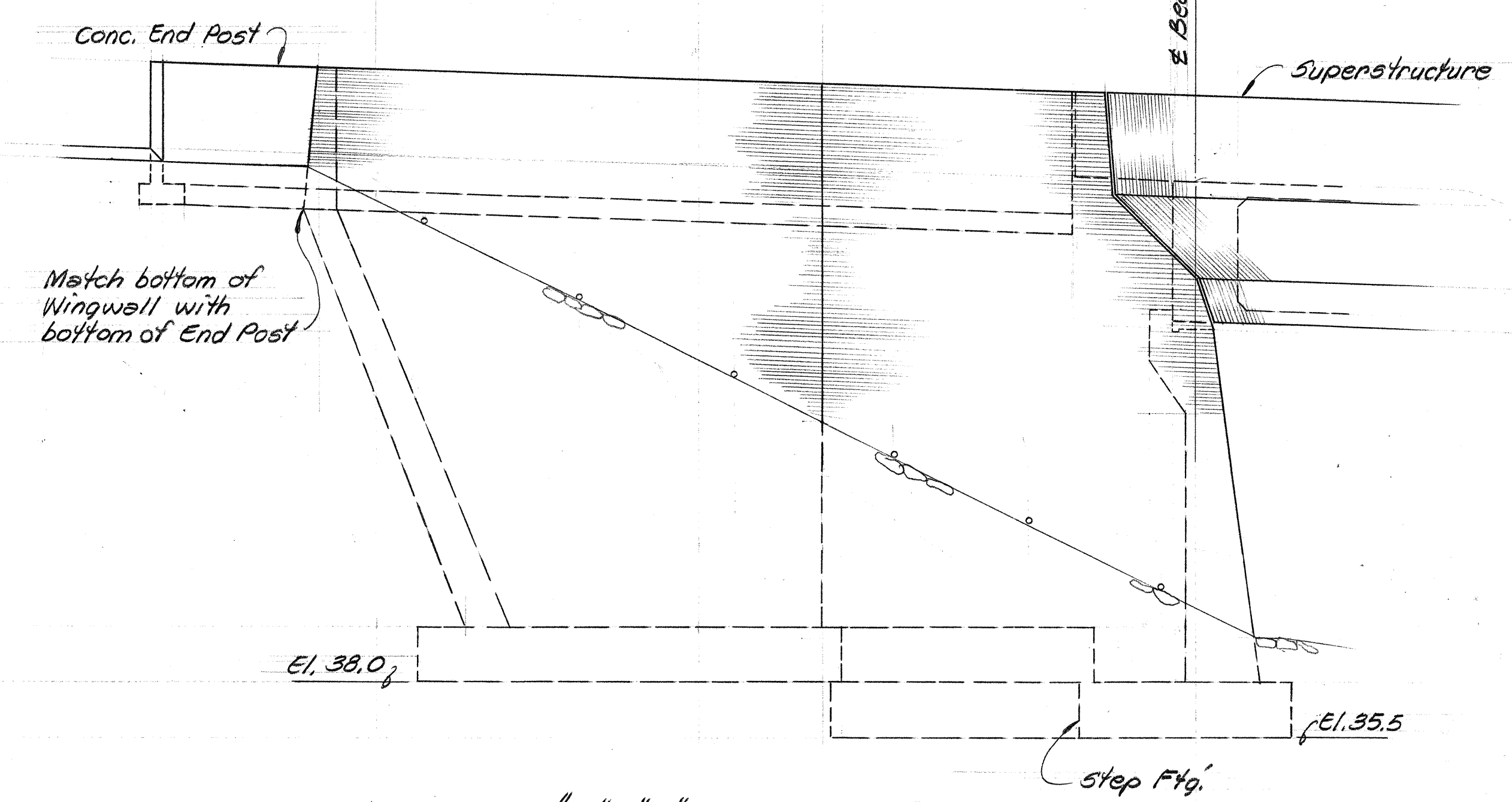
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(III)-14	1974	162	234



ELEVATION ~ ABUTMENT #2
 3/16" = 1'-0" (Looking normal to Plantation Drive)



ELEVATION "C"~"C" - WINGWALL #3
 3/16" = 1'-0"



ELEVATION "D"~"D" ~ WINGWALL #4
 3/16" = 1'-0"

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REGISTERED PROFESSIONAL ENGINEER
 No. 1150
 HAWAII, U.S.A.

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STATE OF HAWAII
 DEPARTMENT OF TRANSPORTATION
 HIGHWAYS DIVISION

STRUCTURE No. 1

ABUTMENT #2 & WINGWALLS #3 & #4 ELEVATION

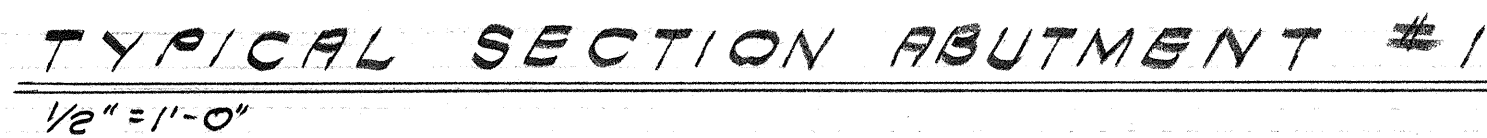
PEARL HARBOR INTERCHANGE
 F.A.I. Proj. No. I-HI-1(III)-14

Scale: As Shown Date: DEC. 14, 1973

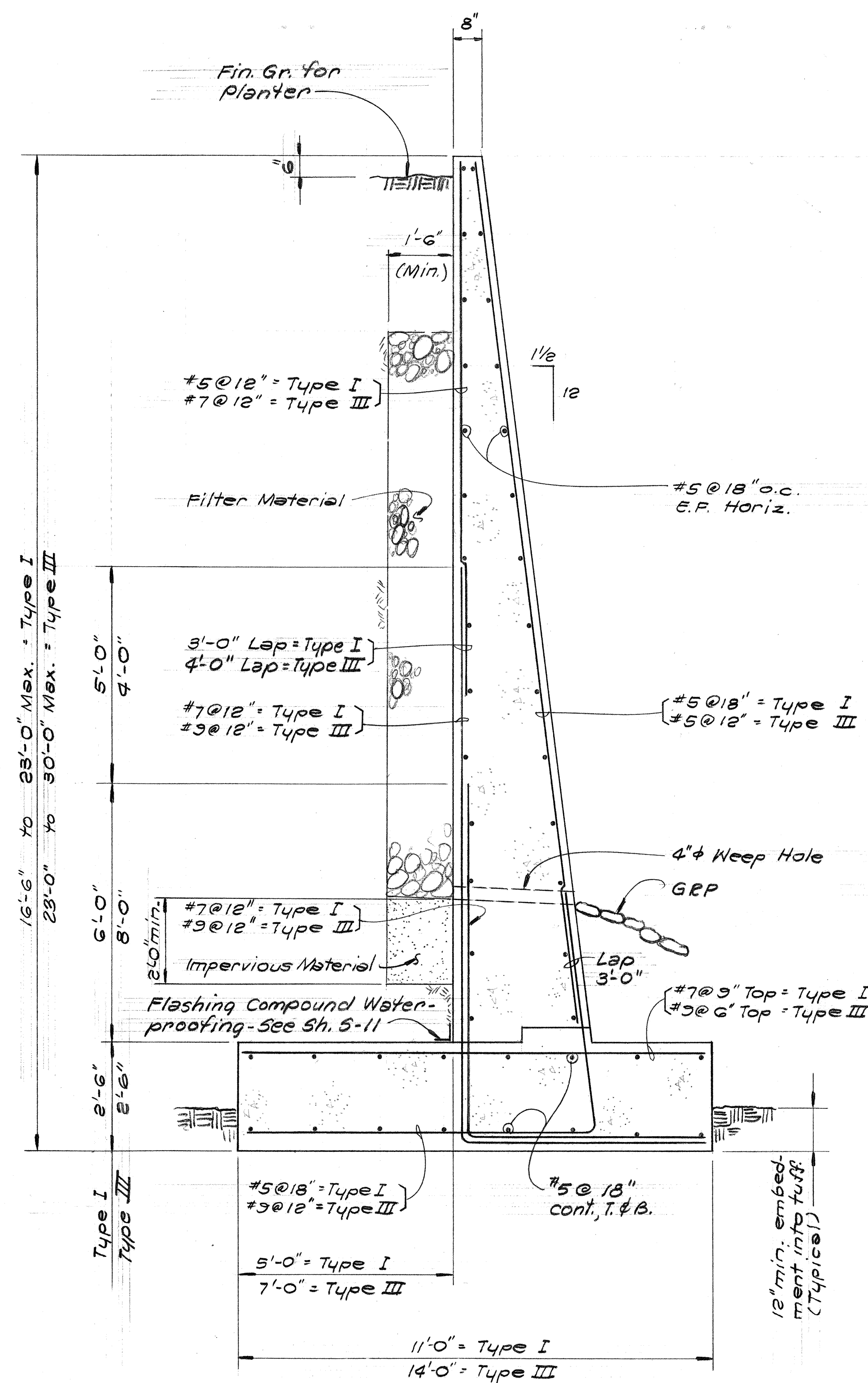
SHEET No. 5-8 OF 34 SHEETS

SURVEY PLOTTED BY	DATE
DRAWN BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
No.	

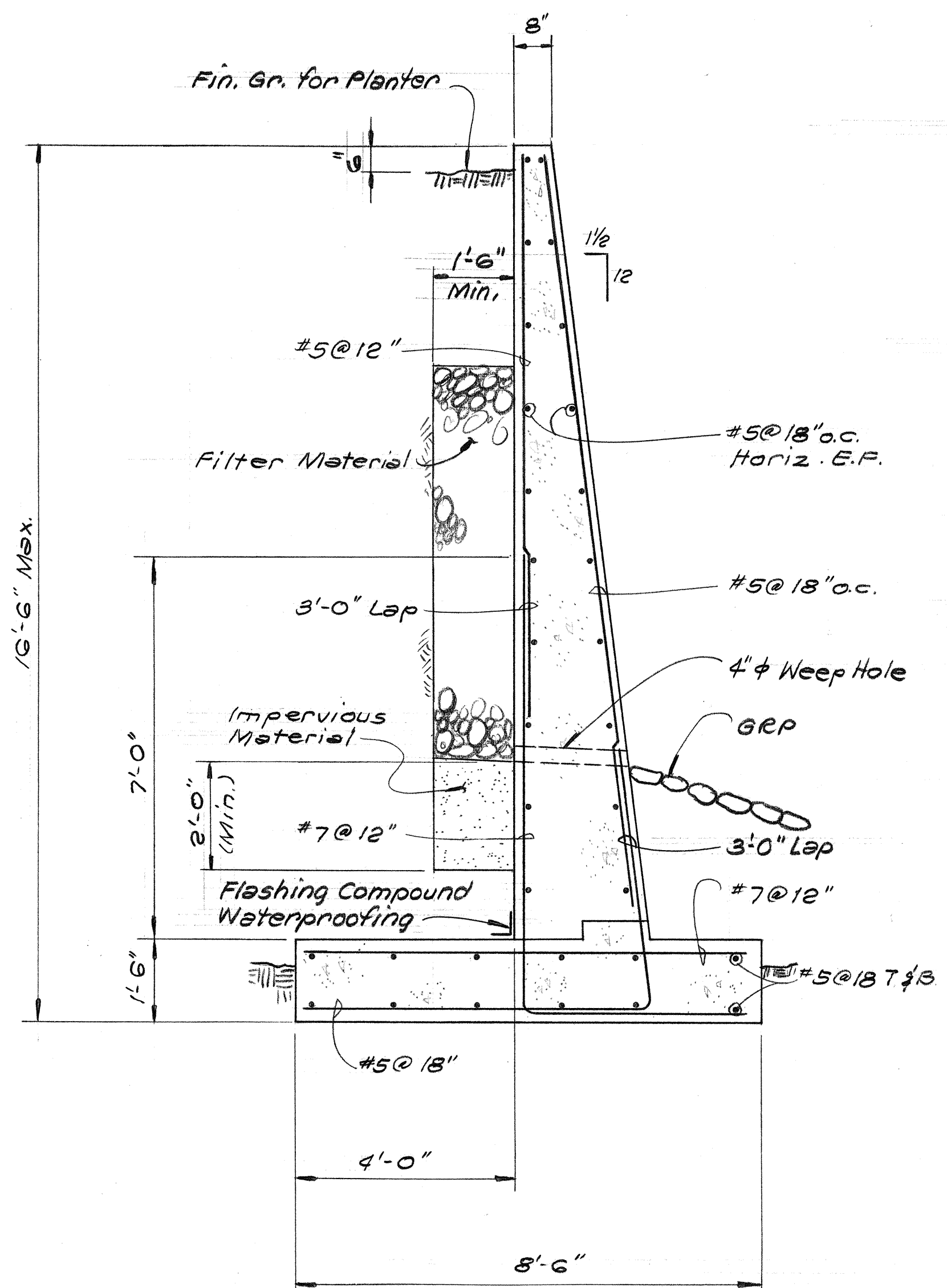
ORIGINAL PLAN	SURVEY PLOTTED BY	DATE
NOTE BOOK	DRAWN BY	"
	TRACED BY	"
	DESIGNED BY	"
	QUANTITIES BY	"
No.	CHECKED BY	"

[illegible]

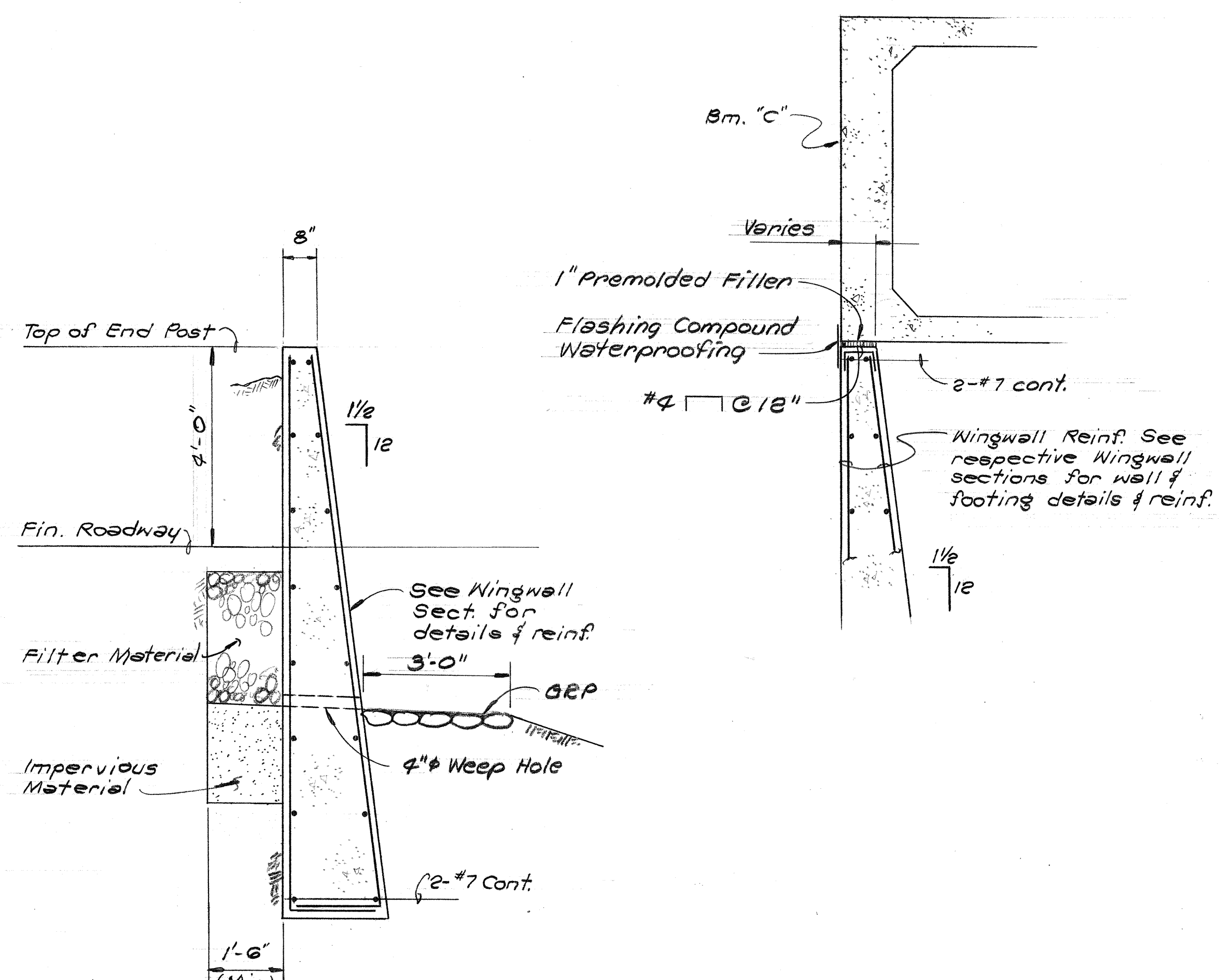
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-H-1(111) 14	1974	164	234



TYPE I & II WINGWALL SECTION
1/2" = 1'-0"

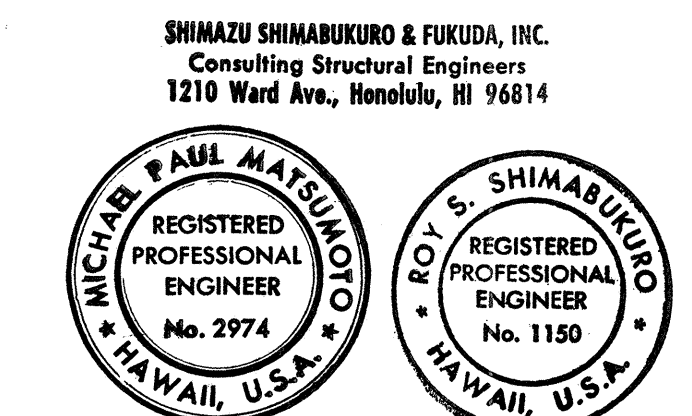


TYPE II WINGWALL SECTION
1/2" = 1'-0"



TYPICAL CANTILEVERED WINGWALL SECTION
1/2" = 1'-0"

WINGWALL SECTION UNDER DECK
1/2" = 1'-0"



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HIGHWAYS DIVISION

STRUCTURE No. 1

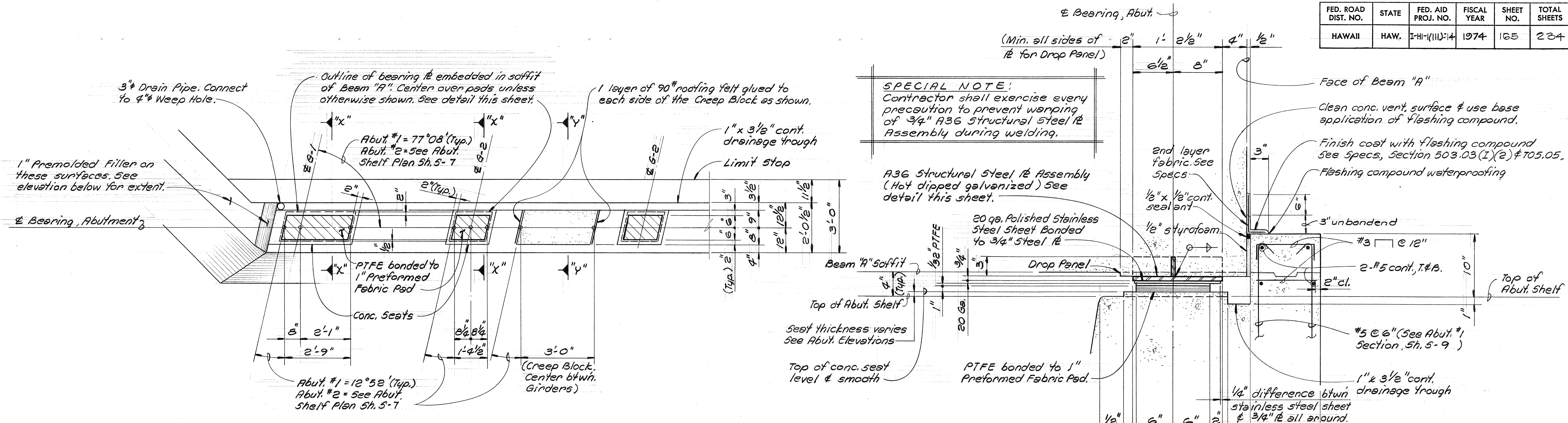
TYPICAL WINGWALL SECTIONS

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-H-1(111) 14

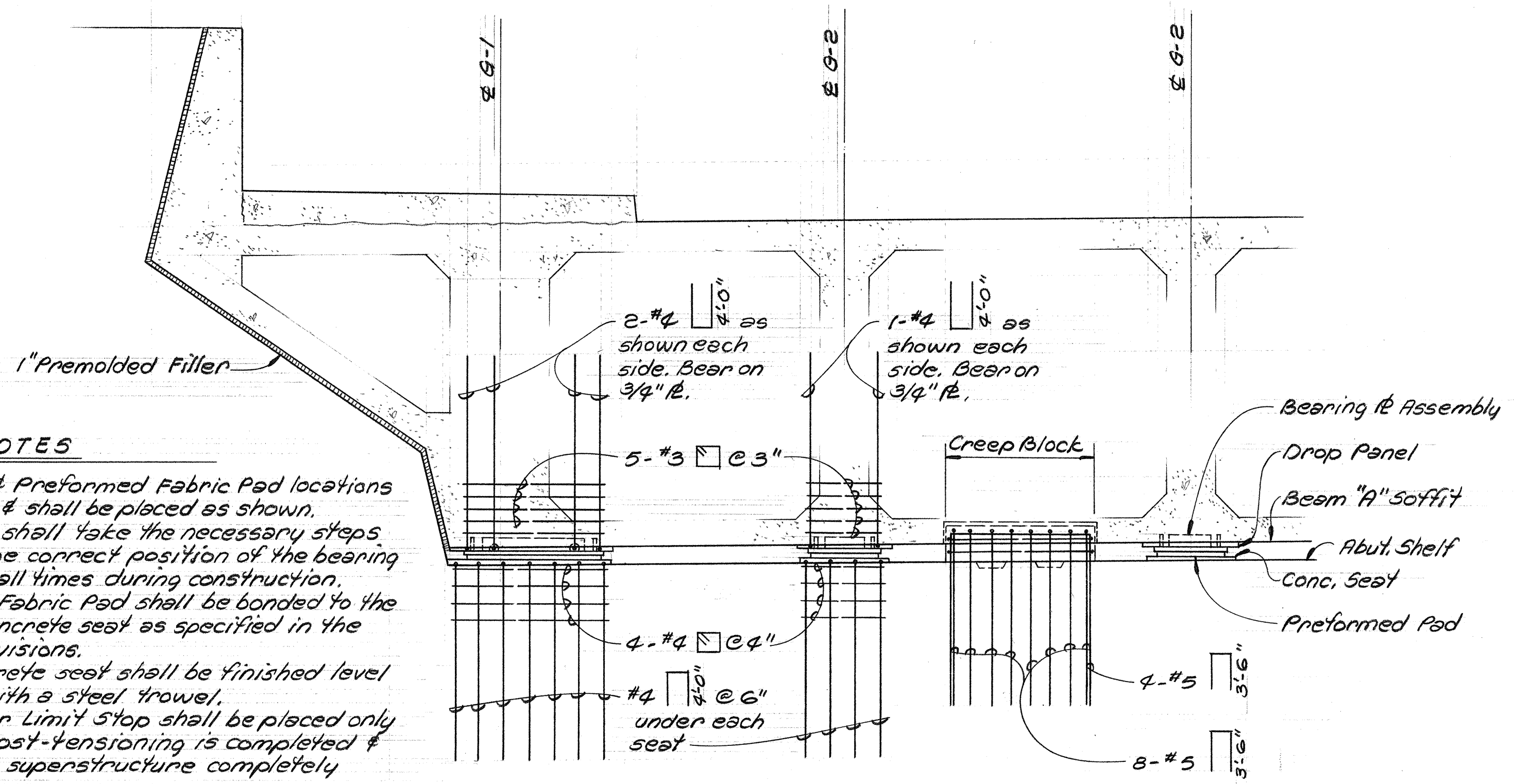
Scale: As Shown Date: DEC. 14, 1973

SHEET No. 5-10 OF 24 SHEETS

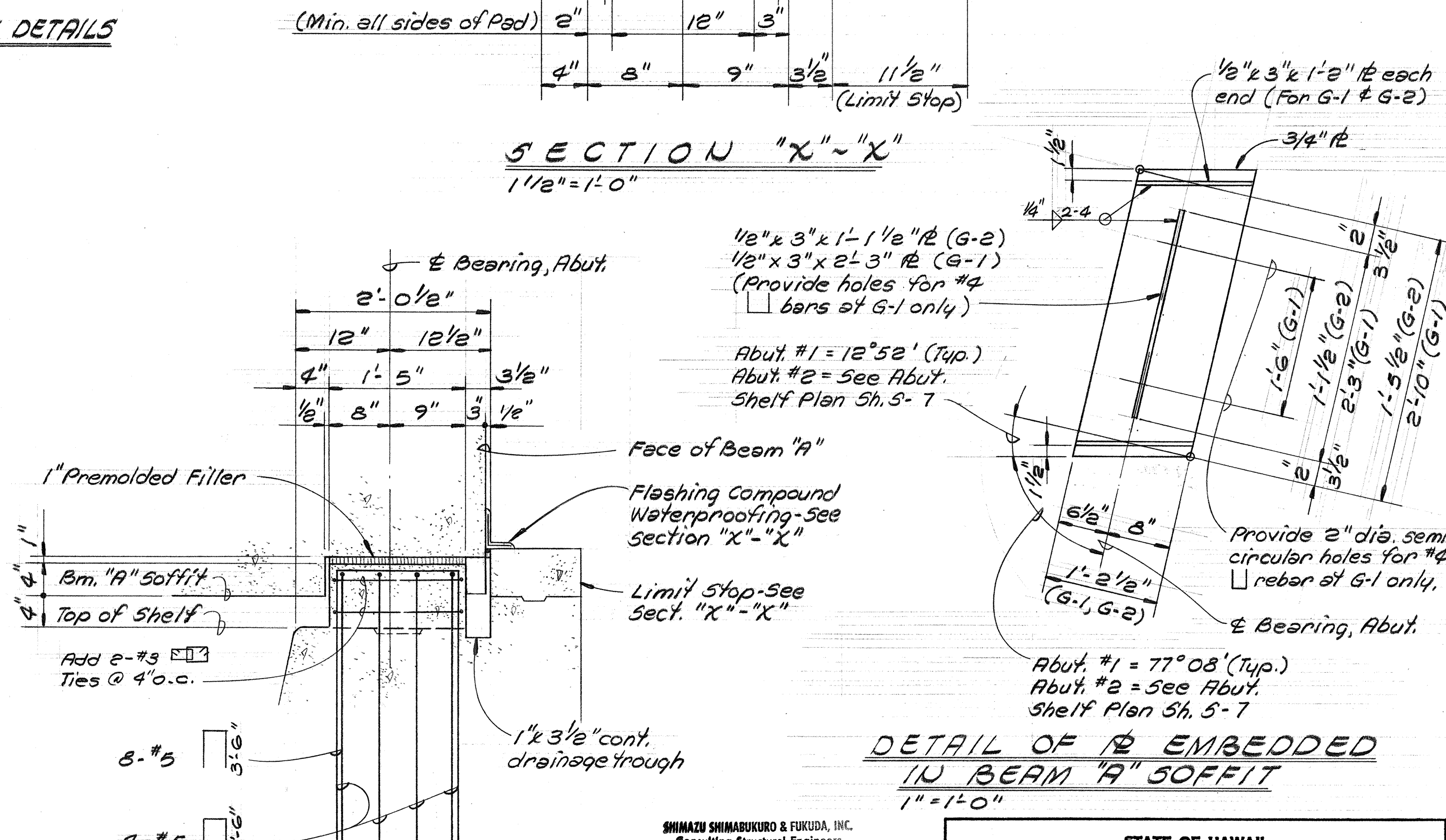
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(111)-14	1974	165	234



PLAN - ABUTMENT SHELF SHOWING CONCRETE SEATS, BEARING PADS & CREEP BLOCK DETAILS
1/2" = 1'-0"



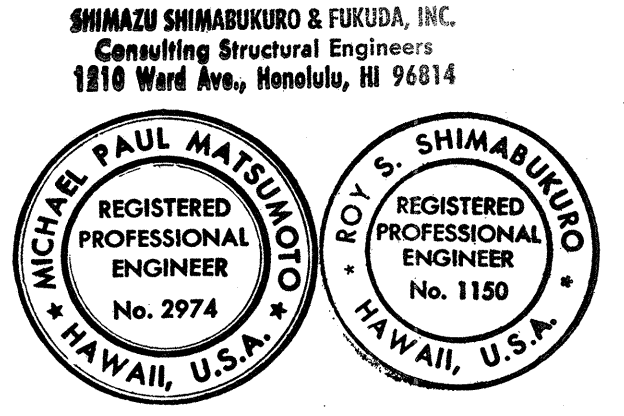
ELEVATION - ABUTMENT SHELF SHOWING SPECIAL REINF. FOR CONC. SEATS, DROP PANELS & CREEP BLOCK
1/2" = 1'-0"



SECTION "Y"-Y"
1" = 1'-0"

SPECIAL NOTES

1. Bearing $\bar{r}$ & Preformed Fabric Pad locations are critical & shall be placed as shown. Contractor shall take the necessary steps to insure the correct position of the bearing assembly at all times during construction.
2. Preformed Fabric Pad shall be bonded to the abutment concrete seat as specified in the Special Provisions.
3. Top of concrete seat shall be finished level & smooth with a steel trowel.
4. Concrete for Limit Stop shall be placed only after all post-tensioning is completed & shoring for superstructure completely removed.
5. Concrete Box Girder shall be poured next to Creep Block faces (2 sides) with Roofing Felt.
6. Flashing Compound Waterproofing shall be continuous full width of superstructure between Beam "A" & Abutment Shelf & Beam "C" & Wingwall.



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE No. 1
ABUTMENT SHELF PLAN & DETAILS

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-HI-1(111)-14

Scale: As Shown Date: DEC. 14, 1973

SHEET No. 5-11 OF 34 SHEETS

DATE	
SURVEY PLATTED BY	
DRAWN BY	
DESIGNED BY	
NOTED BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
No.	

26'-9 ¹⁷/₃₂"

12'-0"

6'-0"

6'-0"

4'-0"

2'-6"

2'-0"

2'-6"

2'-0"

2'-6"

1'-3"

1'-3"

3'-2" x 8'-6" sq. keys in footing top.

Plantation Drive Realignment Baseline

77°08'

Working Point Sta. 9+99.92

7'-3"

7'-3"

15'-6"

36'-11 ¹/₂"

15'-6"

67'-11 ¹/₂"

7'-3"

7'-3"

to Pier Column & Footing

Non shaded bars within spiral are #18 bars (20 total)

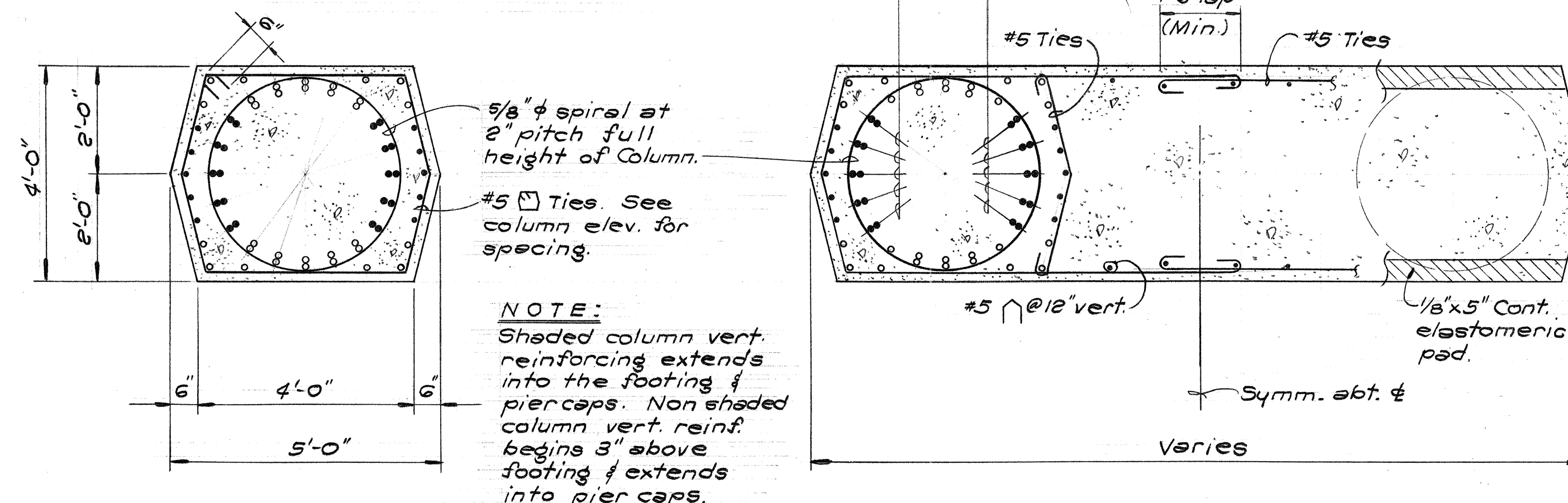
Shaded bars within spiral are #18 bars (20 total)

Non shaded bars outside of spiral are #9 bars (12 total)

Shaded bars outside of spiral are #9 bars (10 total)

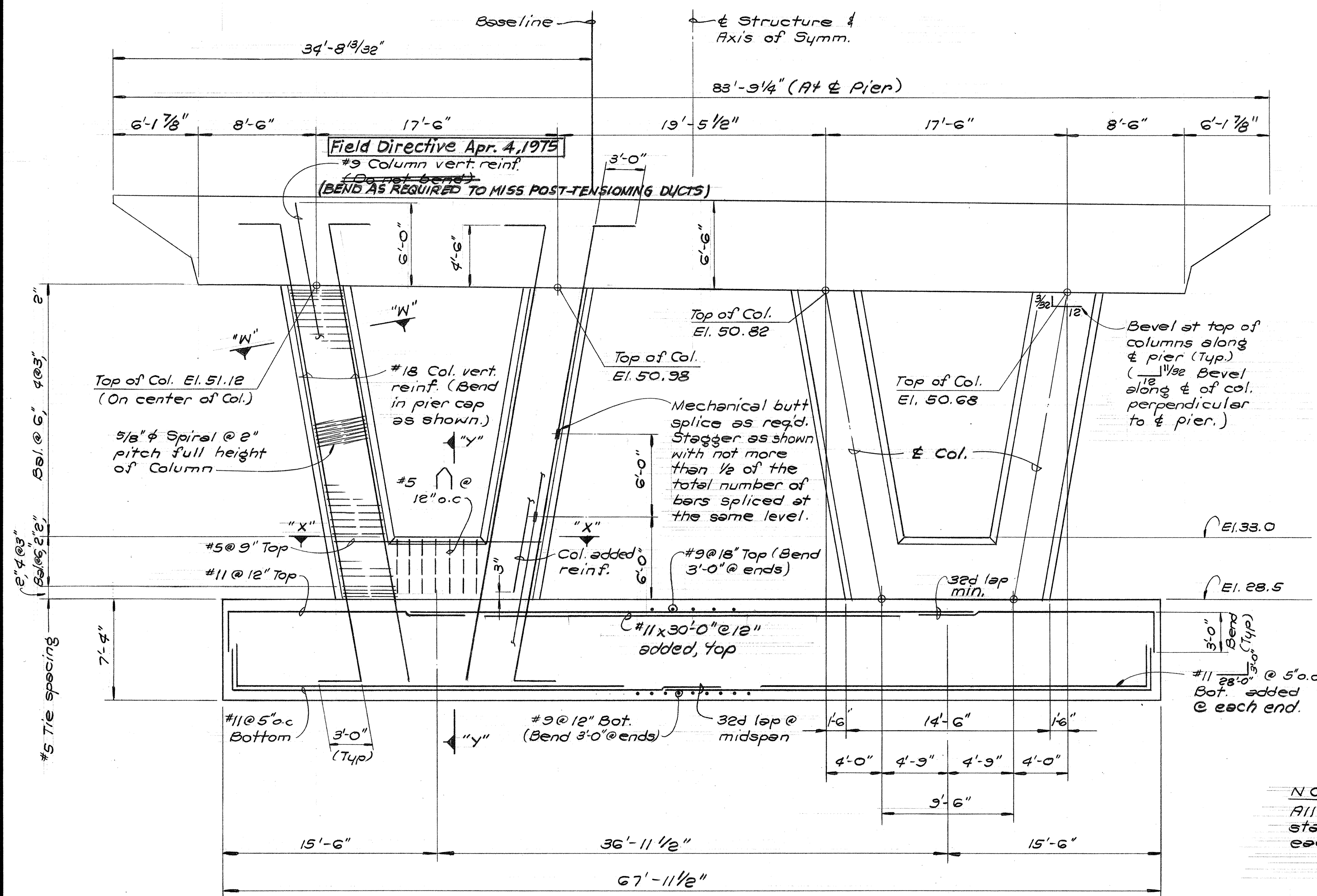
Extend these 10-#18
bars in footing &
bend 3'-0". See Elev.

Extend these 10-#18
bars 6'-6" in footing
(Do not bend)

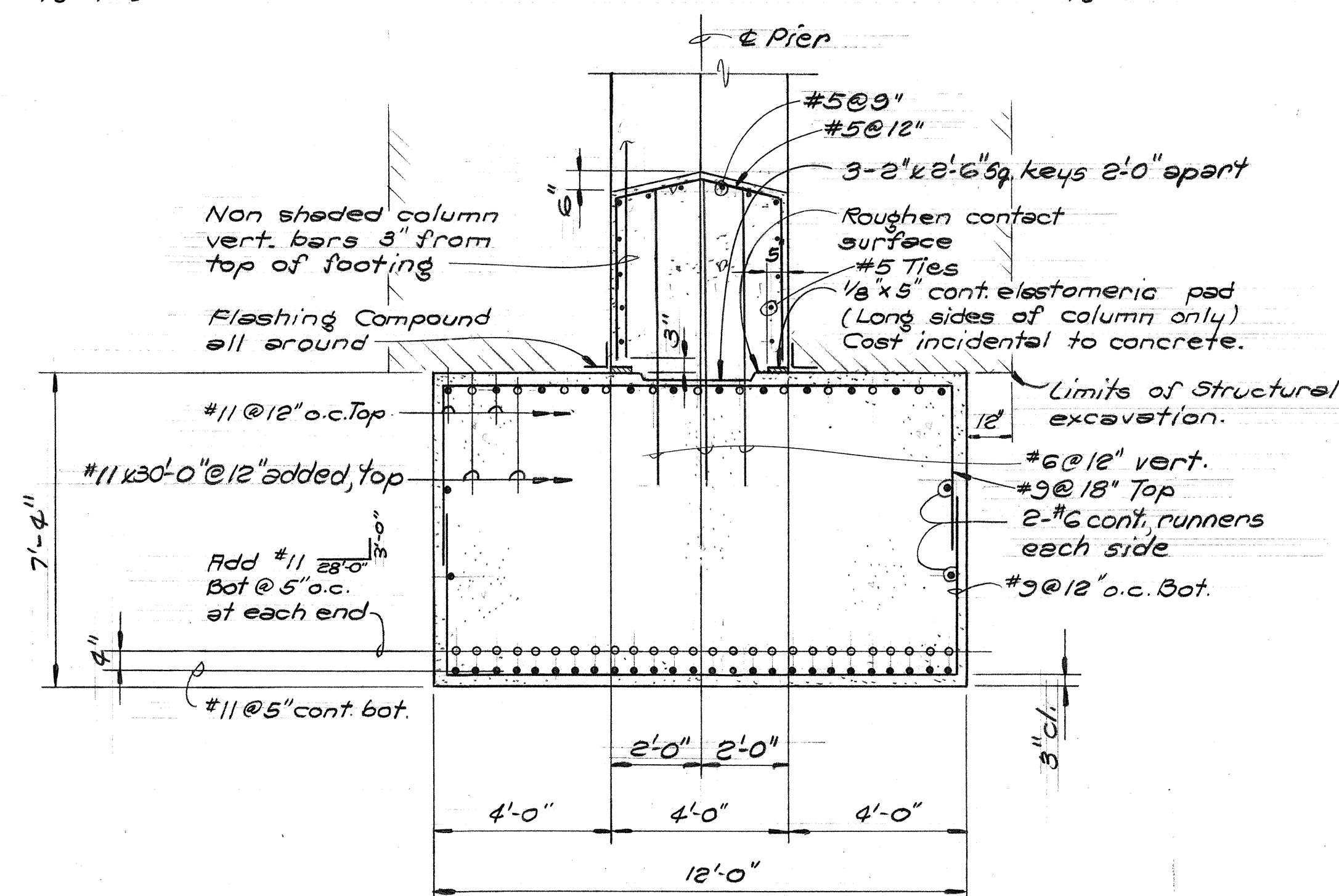


SECTION "W" - "W"
1/2" = 1'-0"

SECTION "X"-"X"
1/2"=1'-0"

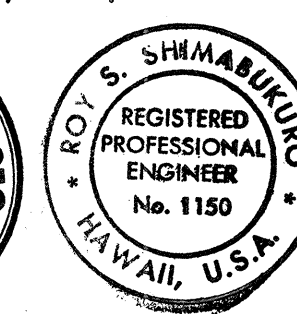


NOTE:
All bar bends shall be standard bends for each size, respectively.



SECTION "Y" - "Y"
3/8" = 1'-0"

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**STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION**

STRUCTURE No. 1

PIER PLAN, ELEVATION
& SECTIONS

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-H-1 (III) : 4

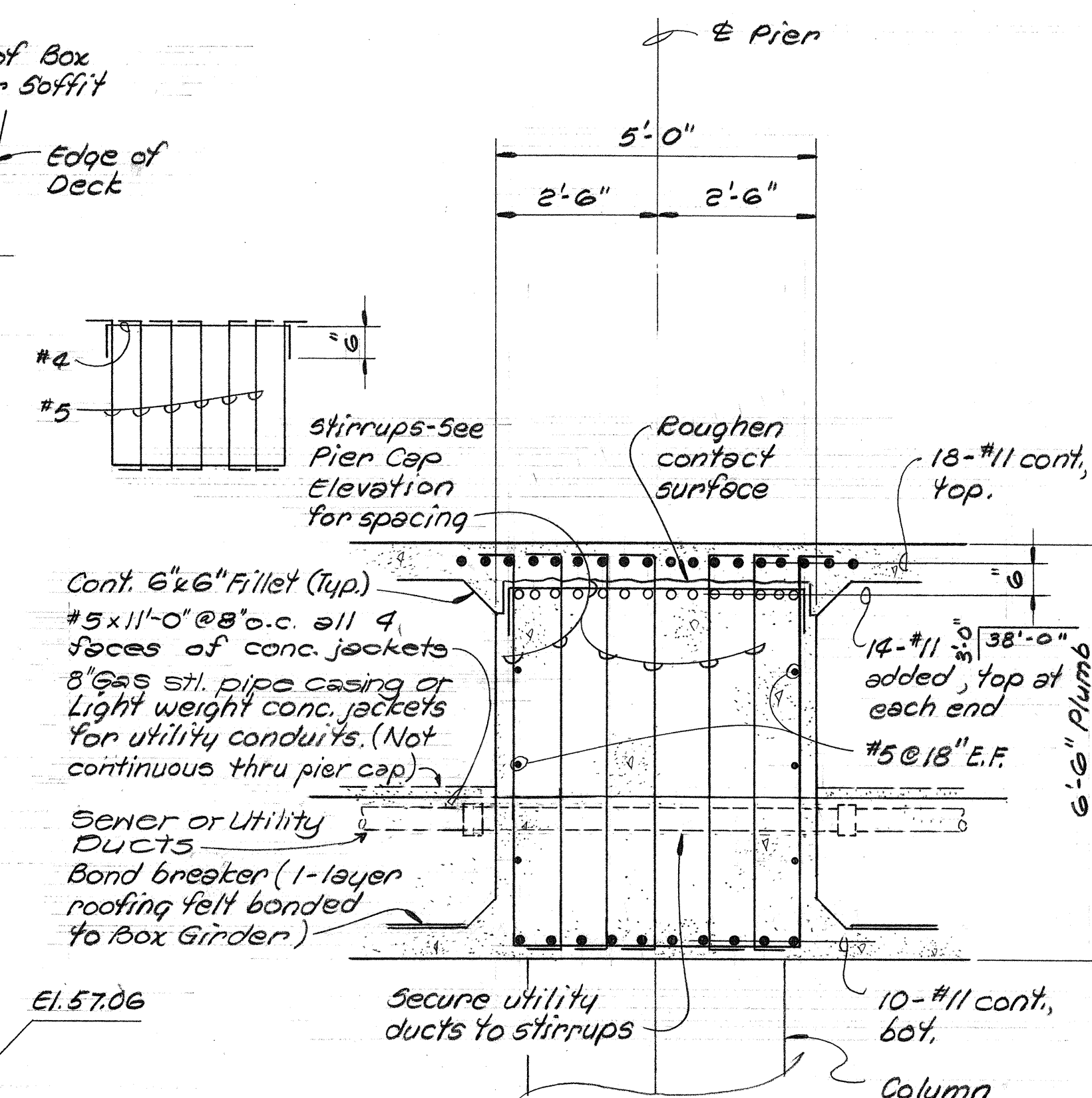
Scale : As Shown Date : DEC. 14, 1973
SHEET No. 5-12 OF 34 SHEETS

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MICHAEL PAUL MATSUMOTO
REGISTERED
PROFESSIONAL
ENGINEER
No. 2974
HAWAII, U.S.A.

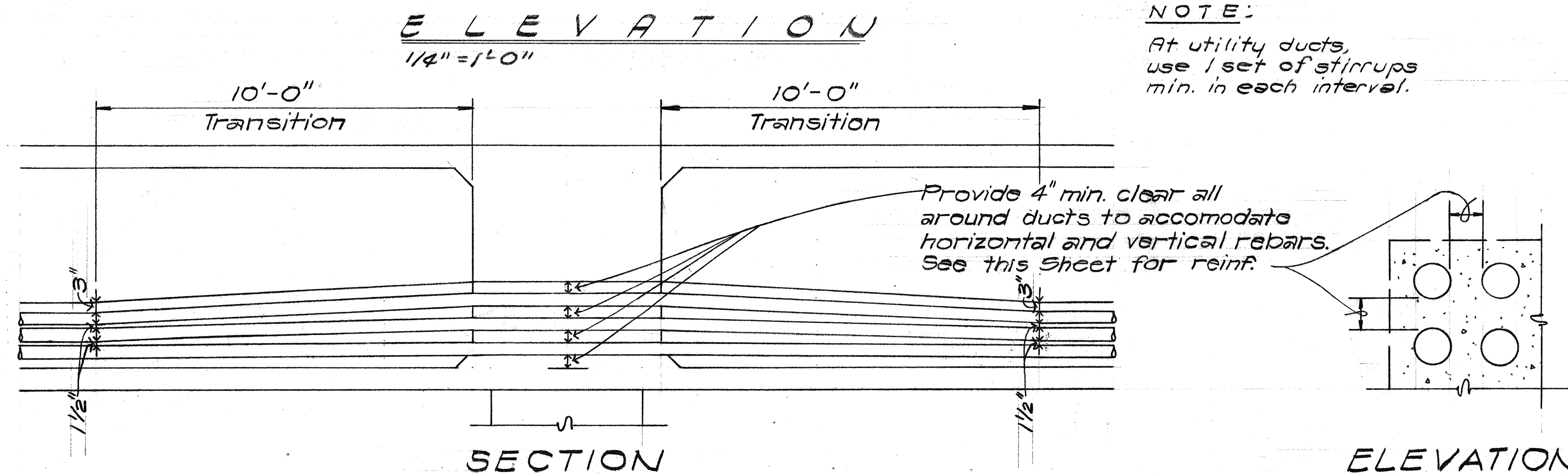
ROY S. SHIMABUKURO
REGISTERED
PROFESSIONAL
ENGINEER
No. 1150
HAWAII, U.S.A.

Scale: As Shown Date: DEC. 14, 1973
SHEET No. 5-13 OF 34 SHEETS



TYPICAL SECTION
1/2" = 1'-0"

At utility ducts,
use 1 set of stirrups
min. in each interval.



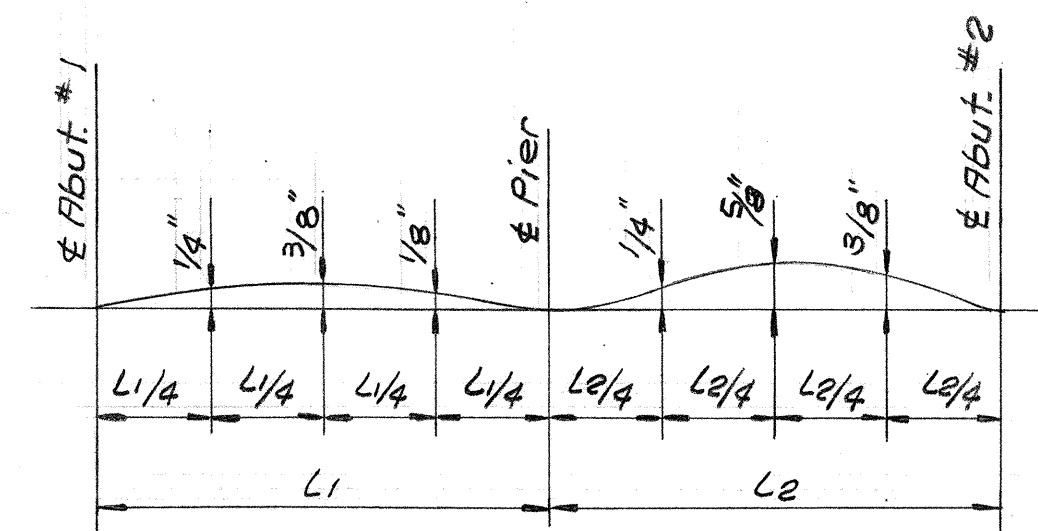
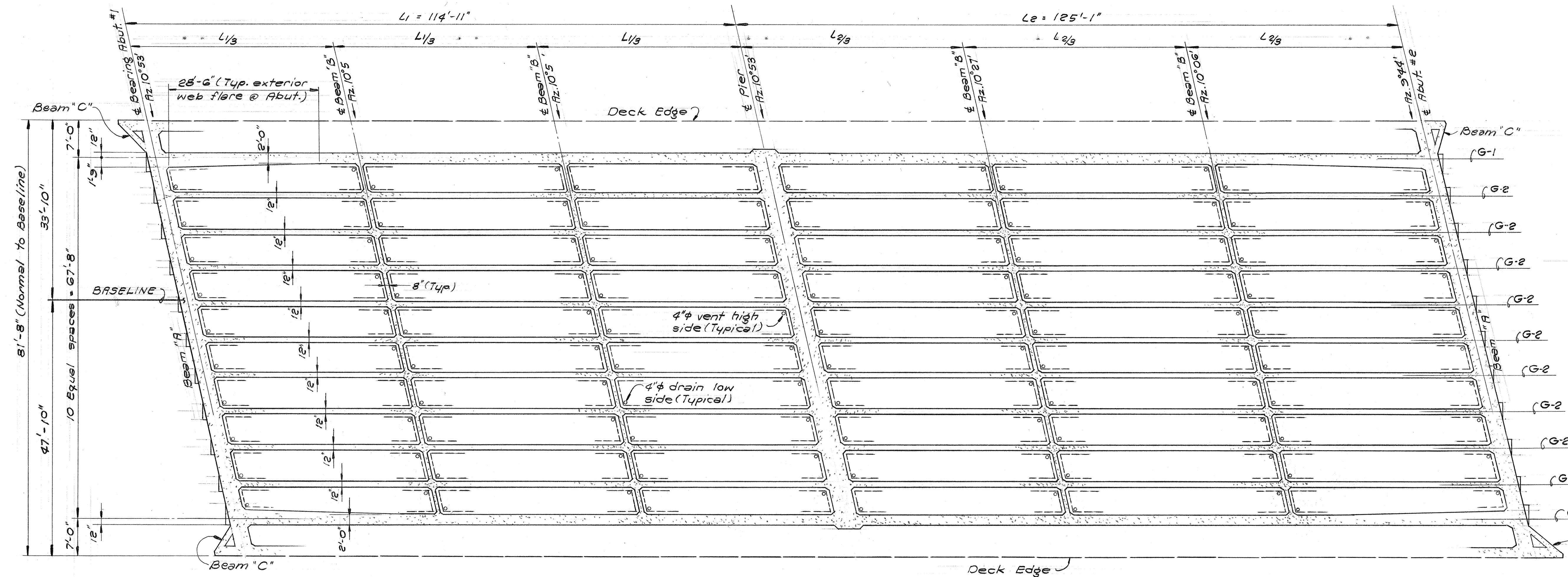
SECTION ELEVATION
PARTIAL CONCRETE JACKET DETAIL AT PIER CAP
 (TYPICAL FOR ALL ELECTRICAL & COMMUNICATION DUCTS)
 NO SCALE

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Consulting Structural Engineers
1210 Ward Ave., Honolulu, HI 96813



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FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-H-1(III)-14	1974	168	234

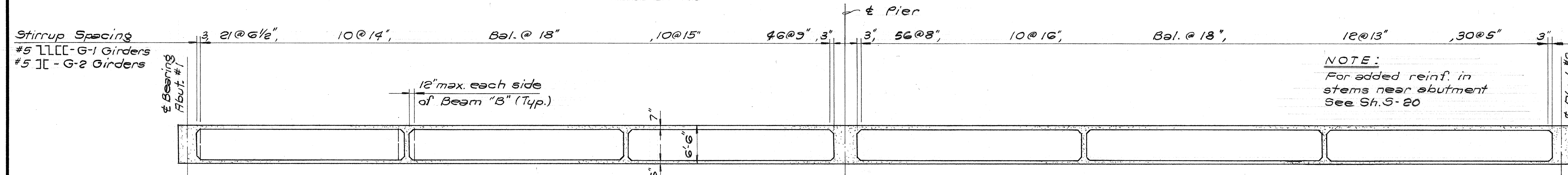


CAMBER DIAGRAM
N.T.S.

SPECIAL NOTES

1. All post tension ducts in each stem shall be placed to satisfy the locations shown for the Center of Gravity of Post Tension Force.
2. All curves in ducts shall be parabolic & smooth.

DECK FRAMING PLAN
Scale: 3/32" = 1'-0"



NOTE:
For added reinf. in stems near abutment
See Sh. S-20

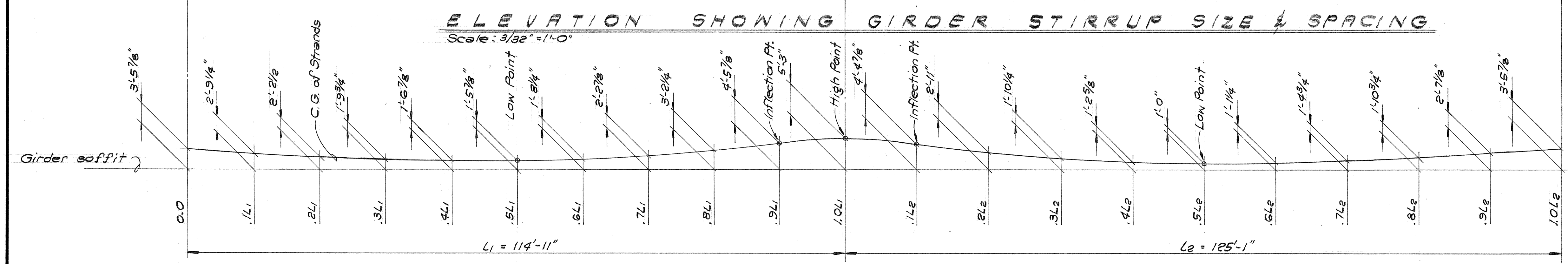
POST TENSION NOTES

1. See "TYPICAL POST TENSION NOTES" Sh. S-18
2. Effective post tension force after all losses (in kips) shall be:
Girder #1 (Jacking end), #4L, #10L (Pier), #6L, #10L (4L)
G-1-----2096k-----2276k-----2218k-----2282k-----2096k
G-2-----1048k-----1138k-----1103k-----1141k-----1048k
3. All strands shall be post tensioned simultaneously from Abutment #1 & Abutment #2.
4. Maximum number of ducts per stem shall be three (3) for G-2 Girders & six (6) for G-1 Girders.
5. Prior to final tensioning of the prestressing steel, friction tests shall be conducted by the contractor to verify the friction losses used in calculating the working force. At least two friction tests shall be conducted for each tendon or duct length and profile. The cost of such tests shall be incidental to the prestressing operation and shall not be paid for separately.

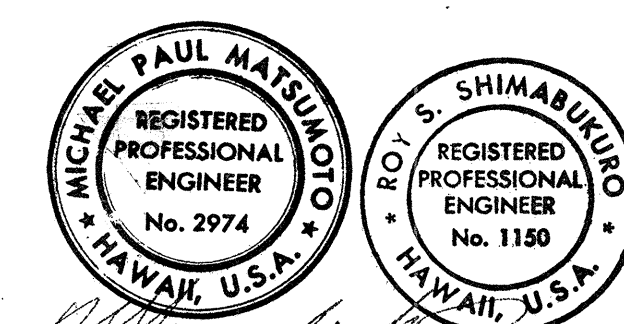
REFERENCE DRAWINGS

1. Post Tension Notes-----Sh. S-18
2. Typical Deck Cross Section-----Sh. S-17
3. Beam "A"-----Sh. S-19
4. Beam "B"-----Sh. S-26

CENTER OF GRAVITY OF POST TENSION FORCES
Scale: 3/32" = 1'-0"



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Consulting Structural Engineers
1210 Ward Ave., Honolulu, HI 96814



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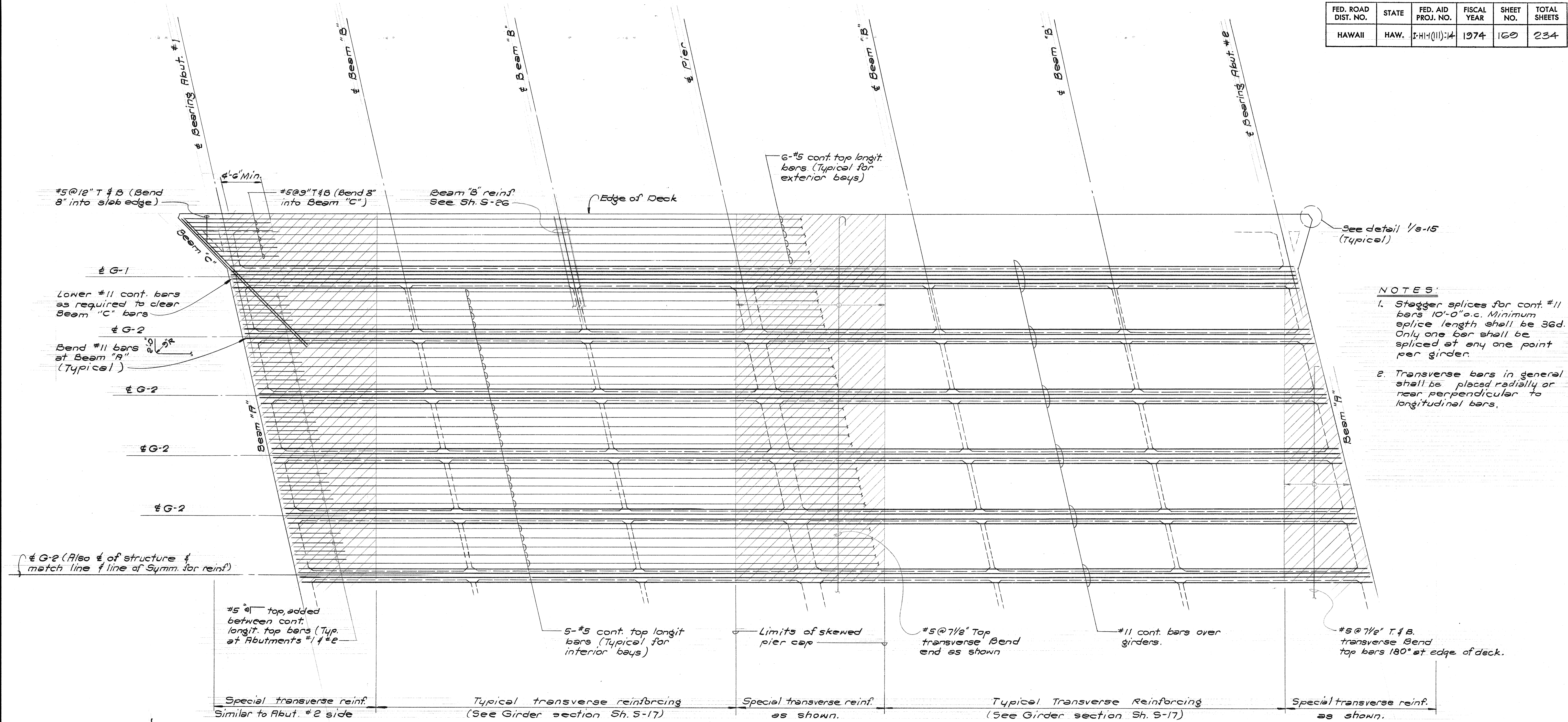
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE No. 1

DECK FRAMING PLAN, ELEVATION
SHOWING GIRDER STIRRUP SIZE &
SPACING & CENTER OF GRAVITY OF
POST TENSION FORCES
PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-H-1(III)-14

Scale: As Shown Date: DEC. 14, 1973
SHEET No. 5-14 OF 34 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-H-1(11):14	1974	169	234

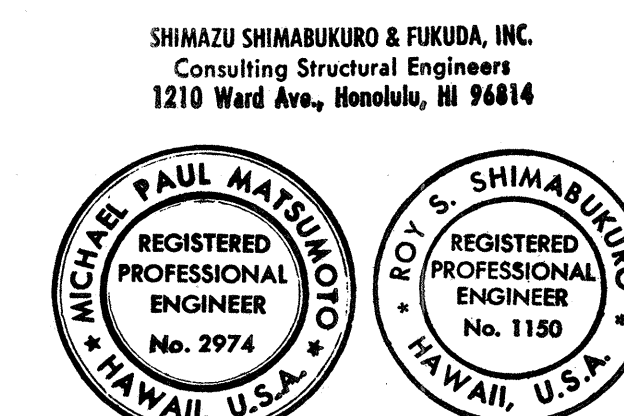
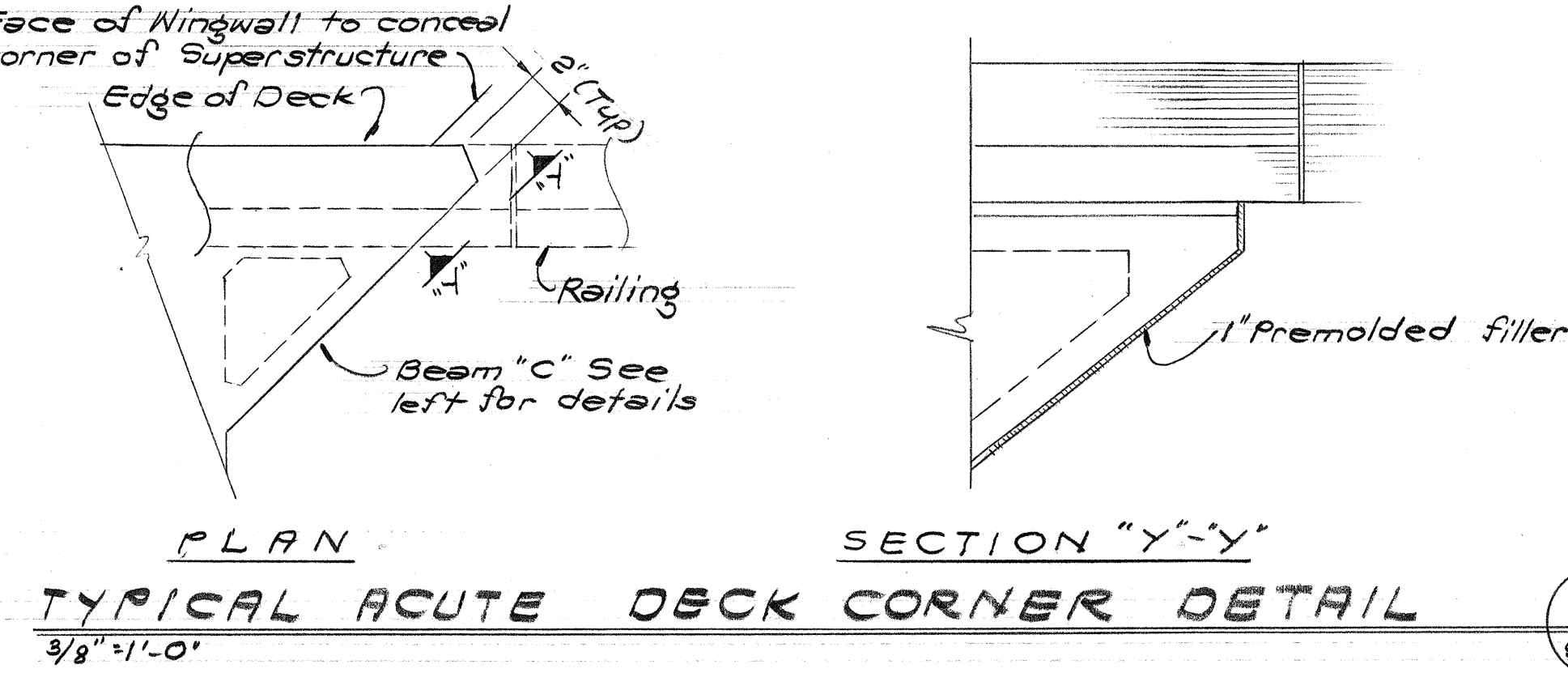
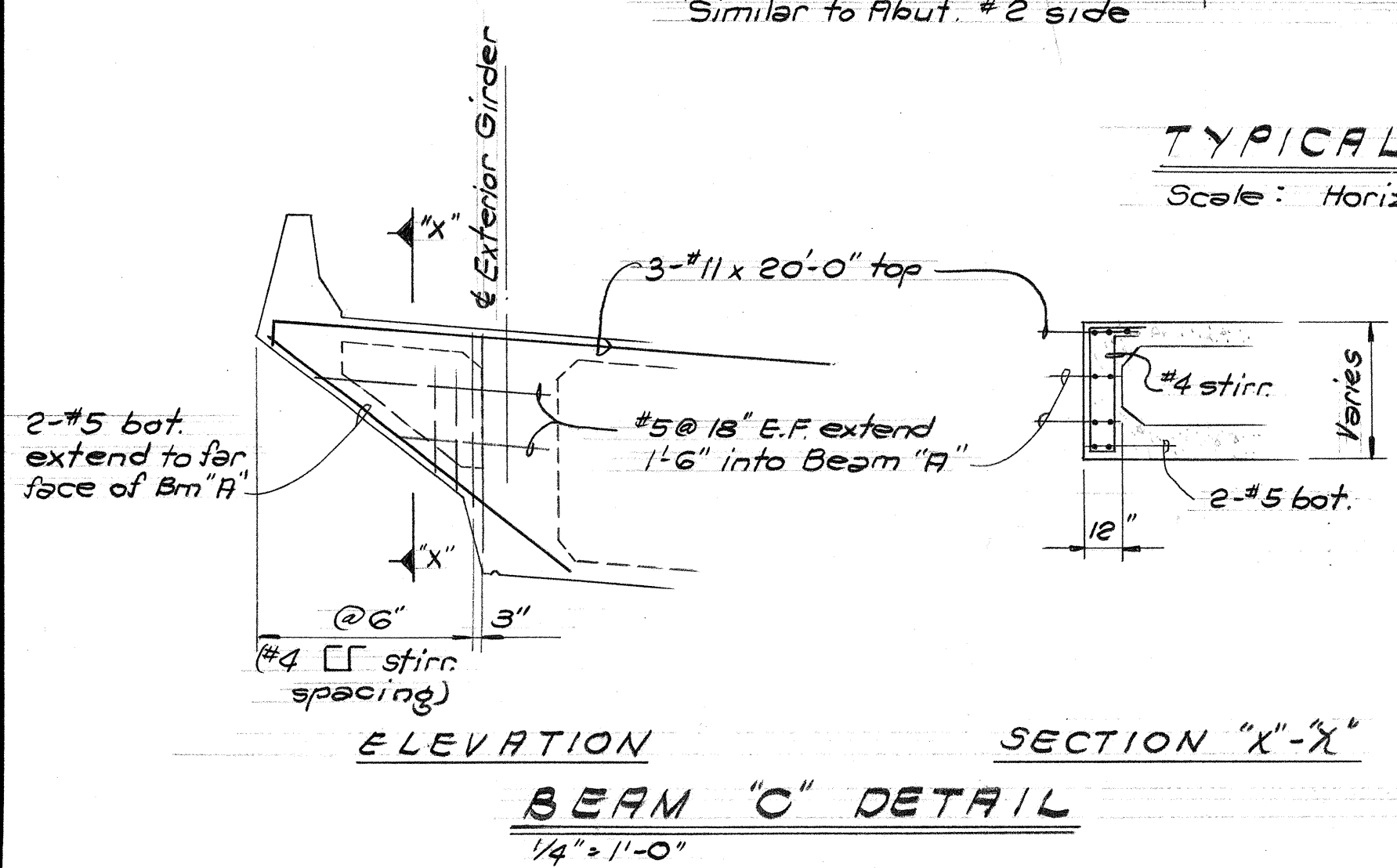


TYPICAL ARRANGEMENT OF TOP SLAB TOP LAYER LONGITUDINAL & TRANSVERSE DECK REINFORCEMENT

Scale: Horiz. 3/32" = 1'-0" Vert. 3/16" = 1'-0"

REFERENCE DRAWINGS

1. Beam "A" ----- Sh. S-19
2. Beam "B" ----- Sh. S-26



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE No. 1

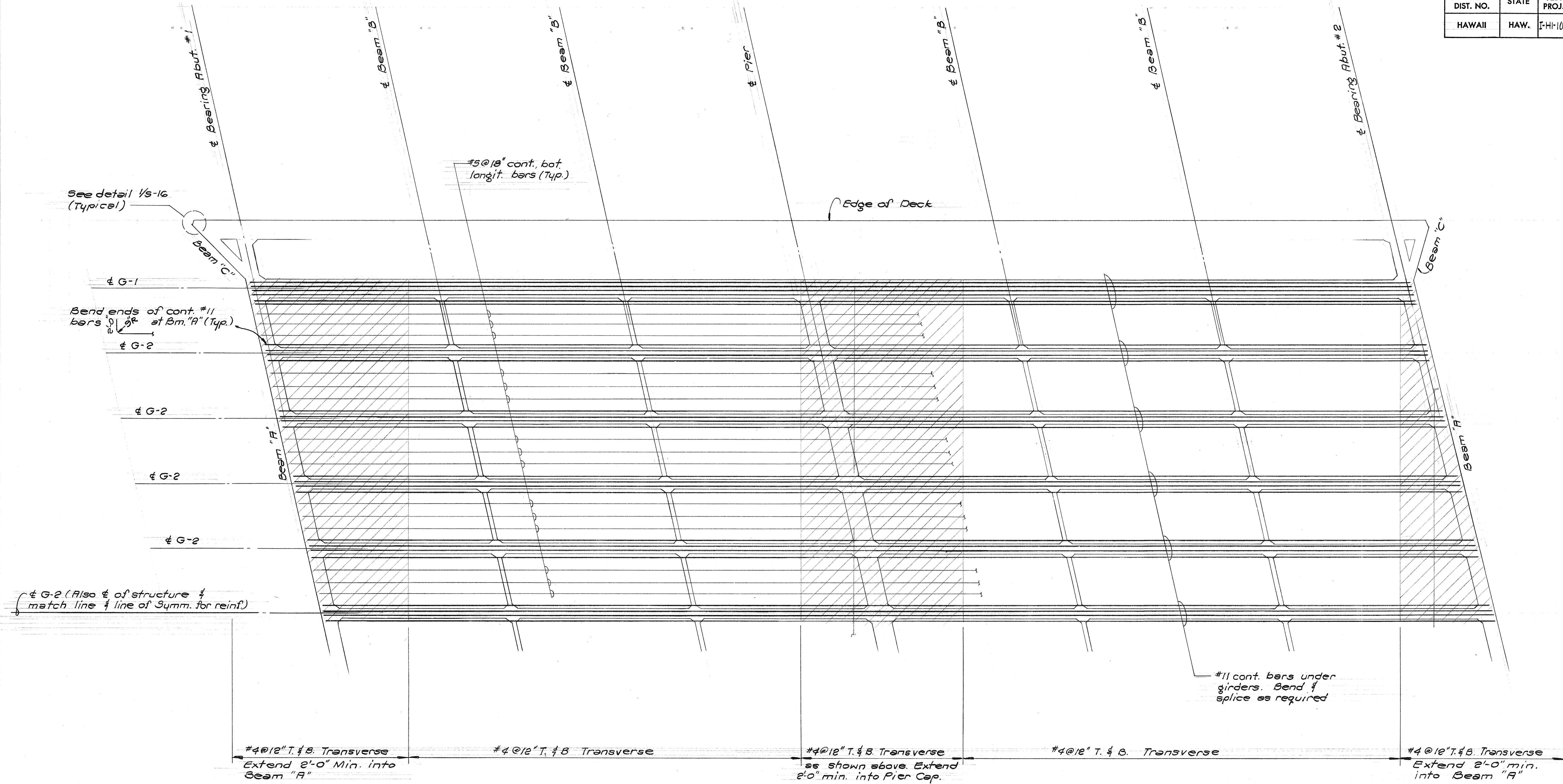
TOP SLAB TOP LAYER LONGITUDINAL & TRANSVERSE DECK REINF. & BEAM "C" DETAIL & ACUTE CORNER DET.

PEARL HARBOR INTERCHANGE
P.A.I. Proj. No. I-H-1(11):14

Scale: As Shown Date: DEC. 14, 1973

SHEET No. S-15 OF 34 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-H-1(11):14	1974	170	234



TYPICAL ARRANGEMENT OF BOTTOM SLAB, BOTTOM LAYER LONGITUDINAL & TRANSVERSE DECK REINFORCEMENT
 Scale: Horiz 3/32" = 1'-0" Vert. 3/16" = 1'-0"

- NOTES**
1. Stagger splice for cont. #11 bars 10'-0" o.c. Minimum splice length shall be 36". Only one bar shall be spliced at any one point per girder.
 2. Transverse bars in general, shall be placed radially or near perpendicular to longitudinal bars.

REFERENCE DRAWINGS

1. Beam "A"	Sh. S-13
2. Beam "B"	Sh. S-20
3. Beam "C"	Sh. S-15

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REGISTERED PROFESSIONAL ENGINEER
 No. 2974
 HAWAII, U.S.A.

REGISTERED PROFESSIONAL ENGINEER
 No. 1150
 HAWAII, U.S.A.

STATE OF HAWAII
 DEPARTMENT OF TRANSPORTATION
 HIGHWAYS DIVISION

STRUCTURE No. 1

BOTTOM SLAB, BOTTOM LAYER
 LONGITUDINAL & TRANSVERSE
 DECK REINFORCEMENT.

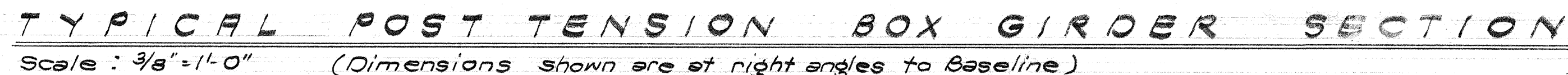
PEARL HARBOR INTERCHANGE
 F.A.I. Proj. No. I-H-1(11):14

Scale: As Shown Date: DEC. 14, 1973

SHEET No. 5-16 OF 34 SHEETS

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 me at under my supervision.

ORIGINAL PLAN	SURVEY PLOTTED BY _____	DATE _____
NOTE BOOK	DRAWN BY _____	" _____
	TRACED BY _____	" _____
	DESIGNED BY _____	" _____
	QUANTITIES BY _____	" _____
N ^o _____	CHECKED BY _____	" _____



1. Bend all bottom longitudinal #11 bars 90° at Abutment ends.

Splice minimum of 20'-0" from bend.
2. Bend all longitudinal #5 top bars in top slab & all bars in bot. slab 90° at Abutment end.

Splice minimum of 10'-0" from bend.
3. DO NOT splice #11 conf. bars in top slab at the Pier.
4. All slab transverse reinforcing shall be placed normal to the $\bar{B}$.

1. Longitudinal deck reinf. along edge of deck to follow curvature of deck edge.
2. All vertical dimensions are measured plumb.



1. Post Tension Notes-----Sh.S-14
2. Stirrup size & spacing-----Sh.S-14
3. Concrete railing-----Sh.S-24
4. Top slab added reinf. Plan-----Sh.S-15

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DEPARTMENT OF TRANSPORTATION
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STRUCTURE No. 1

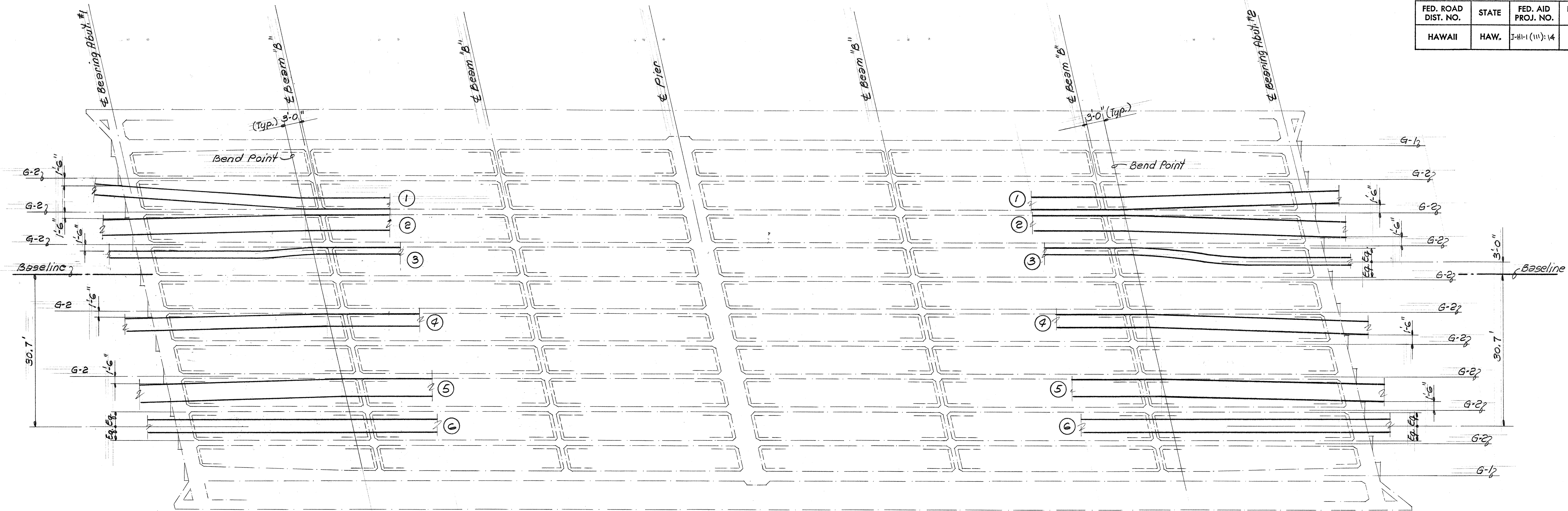
TYPICAL BOX GIRDER SECTION

PEARL HARBOR INTERCHANGE
E.A.J. Proj. No. 1-H-1-1 (III) : 14

Scale: As Shown Date: DEC. 14, 1973

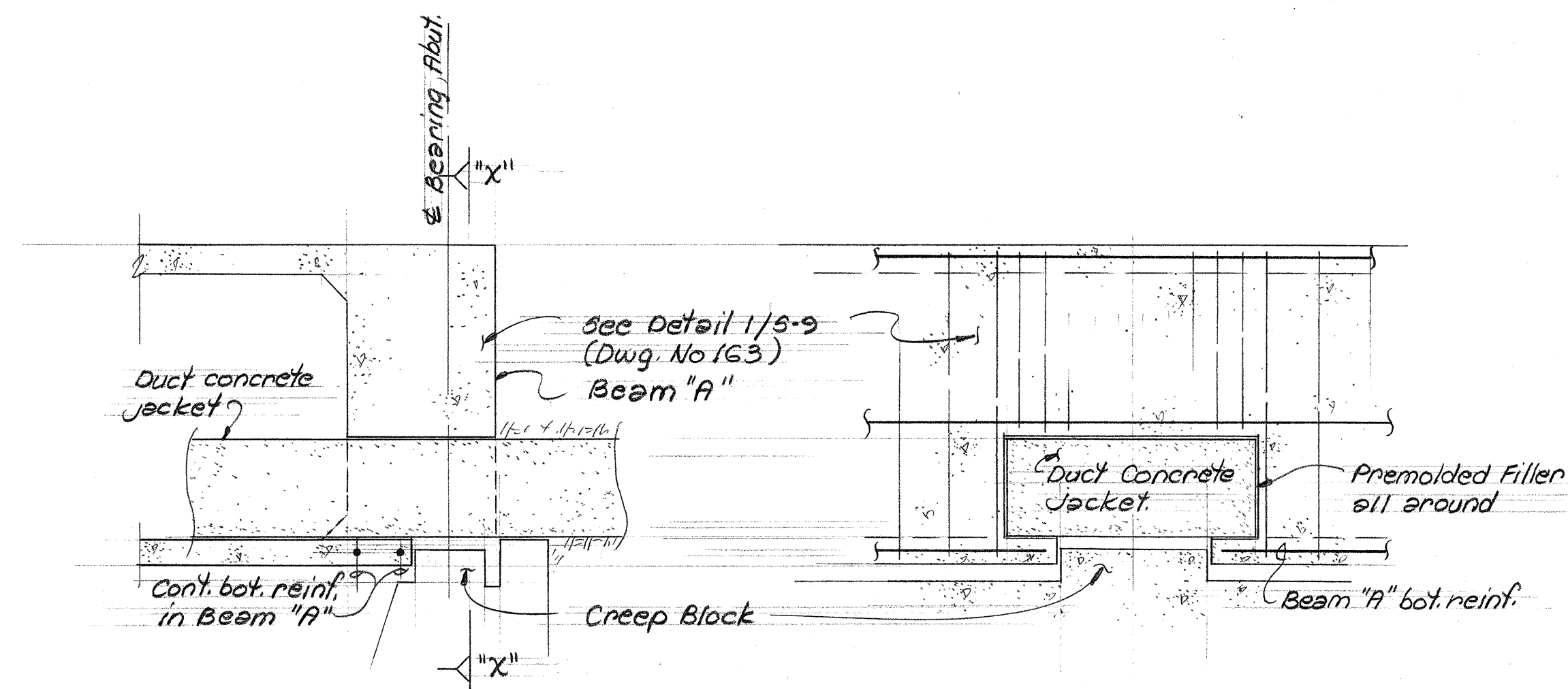
SHEET No. 5-17 OF 34 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1 (III): 14	1974	CO1715-1	234



DECK FRAMING PLAN SHOWING DUCT LAYOUT

Scale: 3/8" = 1'-0"



SECTION THRU BM. "A" @ CREEP BLOCK

3/8" = 1'-0"

SECTION "X"-"X"

3/8" = 1'-0"

GENERAL NOTES

Contract Dwg 174 (Plan of skewed abutment) shows #6 $\frac{3}{8}$ " added top & bottom of each duct. Wherever these encroach into utility jackets, bend bars on place in skewed direction to clear jackets.

Item No	Description	Change From Original Plan	Remarks
① ② ④ ⑤	Utility Cable Ducts	3'-0" from Bm. "B" nearest end Abutment, bend duct jacket to exit structure in manner shown.	Duct bends shall be smooth transitions in accordance with utility company requirements
③	Gas Line & sleeve	Ditto above	Specified bend points outside the structure shall be relocated to suit in field.
⑥	Sewer Line	Move alignment so this line falls midway between stem walls for entire length of structure.	Sewer M.H. #C-1 and C-2 shall be shifted so offset from Baseline is 30.7' (e'-0" shift)

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STRUCTURE No. 1

LAYOUT OF UTILITIES
THRU STRUCTURE

PEARL HARBOR INTERCHANGE
P.A.I. Proj. No. I-HI-1 (III): 14

Scale: As Shown Date AUG. 1975

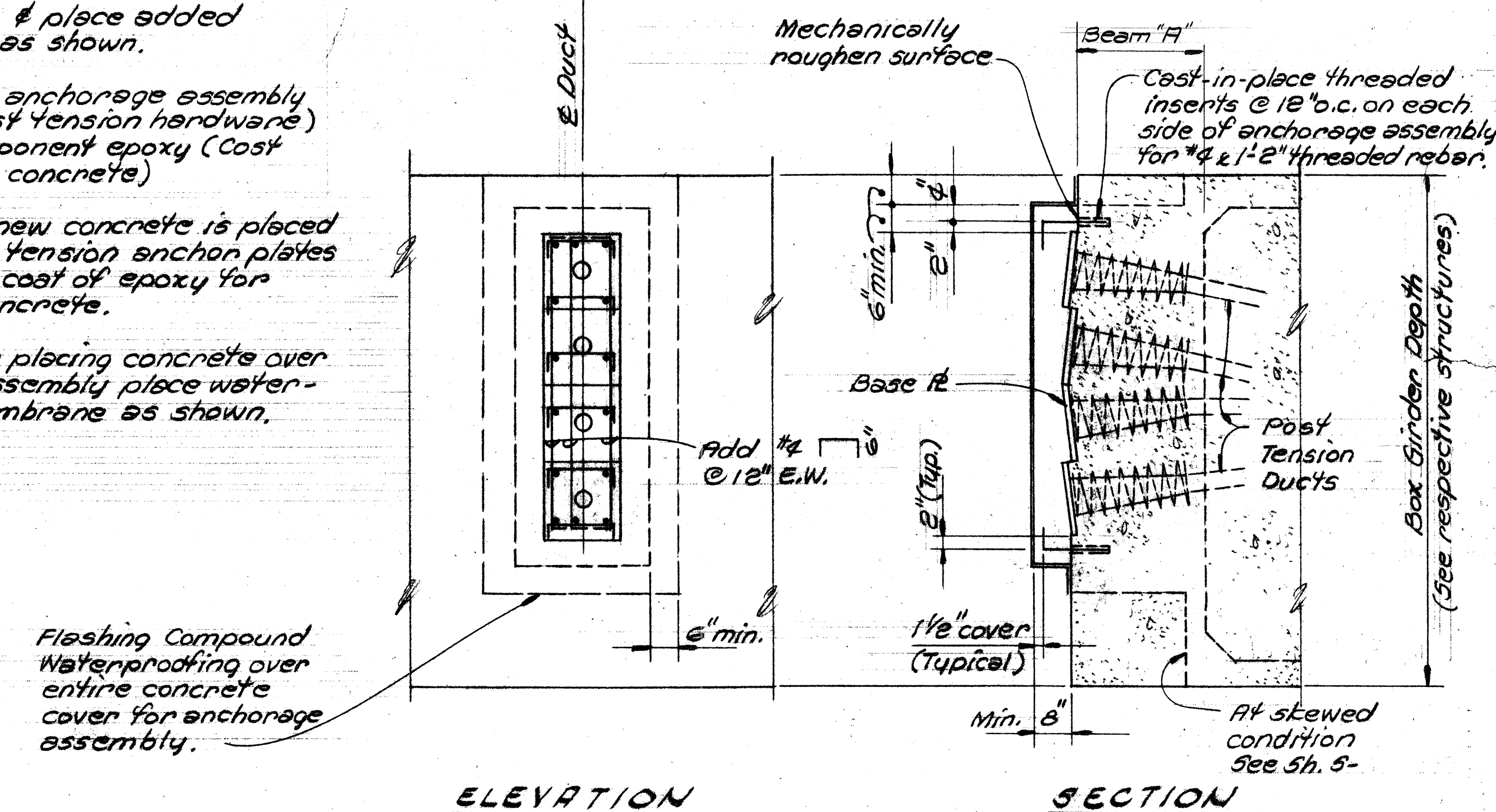
SHEET No. 51A OF 34 SHEETS

CO1715-1

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAWAII	1-111(III)-14	1974	172	234

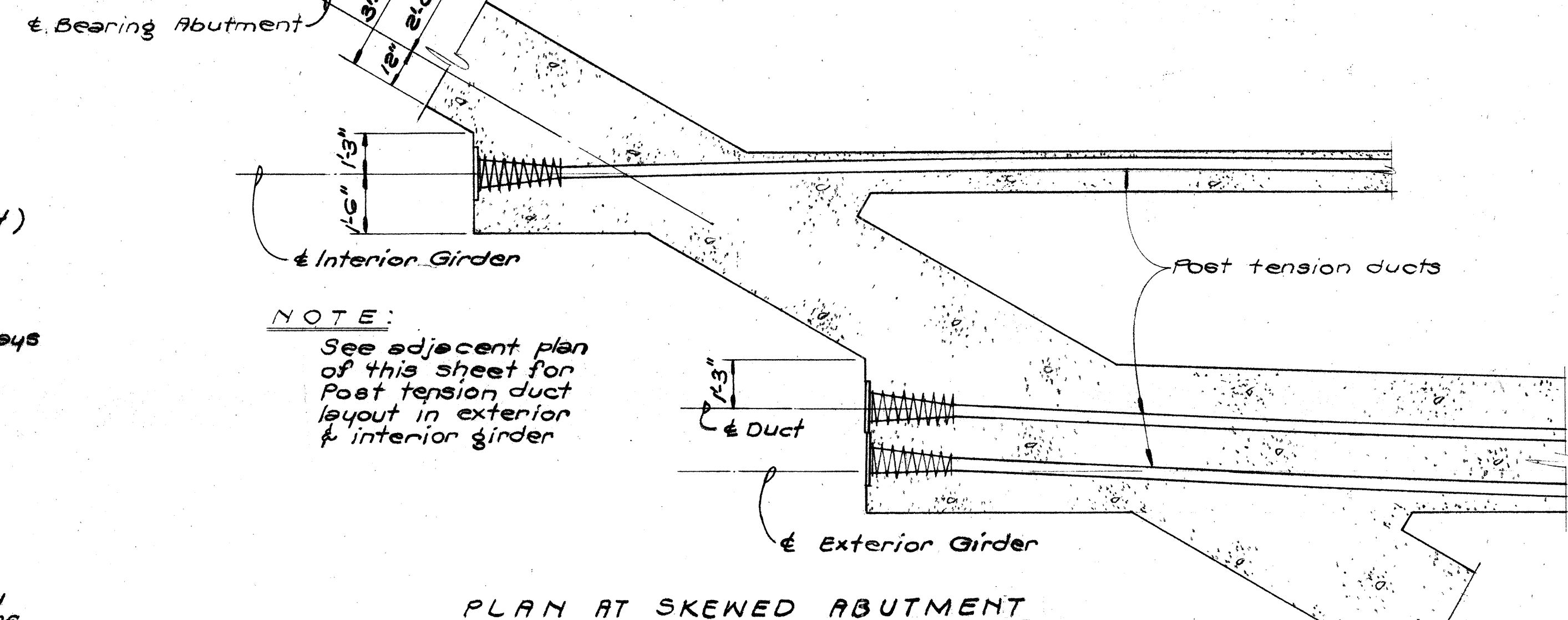
ANCHORAGE ASSEMBLY PROTECTION NOTES

1. After all post tension work is completed, clean hardware as required in order to provide good bonding surface.
2. Bend dowels & place added reinforcing as shown.
3. Paint entire anchorage assembly (including post tension hardware) with two component epoxy (Cost incidental to concrete)
4. Just before new concrete is placed against post tension anchor plates apply a new coat of epoxy for bond with concrete.
5. 7 days after placing concrete over anchorage assembly place water-proofing membrane as shown.



POST TENSION DUCT ANCHORAGE GROUTING DETAILS

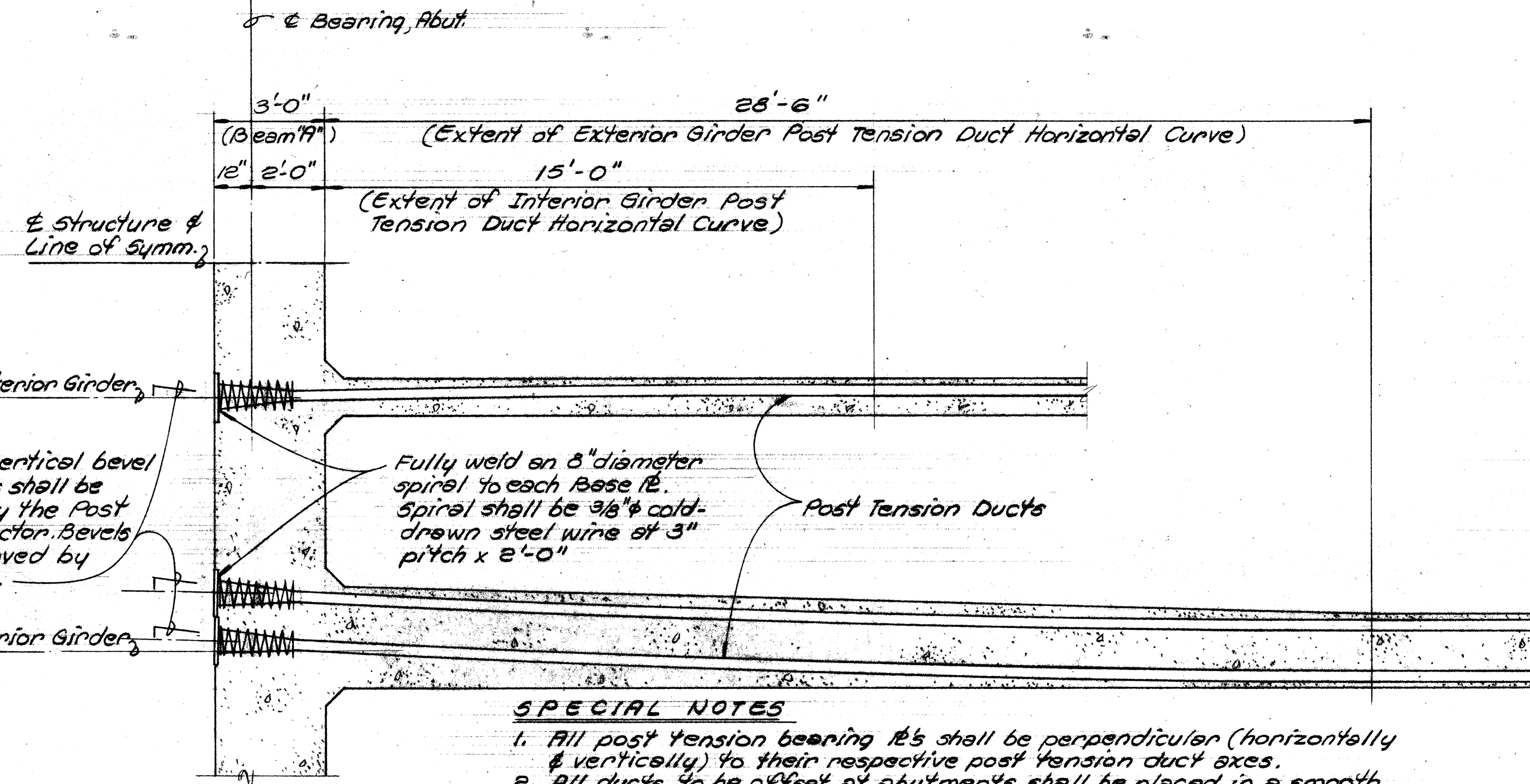
3/8" = 1'-0"



PLAN AT SKEWED ABUTMENT

POST TENSION DUCT LAYOUT IN EXTERIOR & INTERIOR GIRDER

3/8" = 1'-0"



SPECIAL NOTES

1. All post tension bearing IRs shall be perpendicular (horizontally & vertically) to their respective post tension duct axes.
2. All ducts to be offset at abutments shall be placed in a smooth horizontal curve as shown.
3. Details & notes shown on this sheet shall govern conditions at skewed abutments.

PLAN - POST TENSION DUCT LAYOUT IN EXTERIOR & INTERIOR GIRDER

3/8" = 1'-0"

TYPICAL POST TENSION NOTES

1. See Specifications for other post tension requirements.
2. All ducts shall be securely anchored at all times during construction to prevent any damage. Contractor shall exercise the maximum amount of care during placement of ducts, reinforcing & concrete in order to protect the ducts from damage.
3. 1/2" & 270K strands shall be used for post tensioning.
4. All strands are to be placed after the concrete is poured & the following criterion is met:
The last concrete placed must have attained a compressive strength equal to 4,000 p.s.i. & a minimum of 14 days must have elapsed since its placement.
5. All prestressing steel shall be tensioned by means of hydraulic jacks so that the force in the prestressing steel shall not be less than the value shown on the plans. The force in the prestressing steel shall be considered as the smaller of the two values as determined by the measured elongation and the gage pressure.
6. All ducts shall be cleaned out before placing strands.
7. Strands shall be end anchored at an equivalent "anchor set" of 1/2".
8. Post tensioning sequence shall be as follows:
a) All lower-most ducts in each girder shall be completely tensioned in the same day.
b) A minimum of 30% of the remaining ducts above the lower most ducts in each girder shall be completely tensioned in the same day.
c) The remaining ducts in each girder shall be completely tensioned.
9. After all post tension operations & testing, if required, are completed, all ducts shall be grouted in accordance with the Specifications.
10. After grouting is completed, the anchorage assembly shall be reinforced & protected as shown above.

POST TENSION DESIGN NOTES

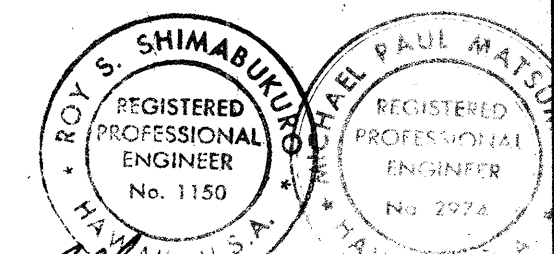
1. Design is based on $K=0.0002$ (friction wobble coefficient) & $M=0.25$ (curvature friction coefficient)
2. Jacking stress for all strands shall be 70% of ultimate.
3. Minimum concrete compressive strength at 28 days for the concrete box girder shall be 5,000 p.s.i.

SPECIAL NOTES:

1. Contractor shall take steps necessary to allow Superstructure to contract during tensioning operations. During tensioning operations the box girder soffit forms will be subject to the same compressive force as the concrete box girder unless compressible joints &/or bond breakers are provided to relieve this force.
2. No tensioning or grouting shall be performed in the absence of the Engineer.

DATE	
SURVEY PLOTTED BY	
ORIGINAL PLAN	
DESIGNED BY	
NOTE BOOK	
CHECKED BY	
No.	

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HIGHWAYS DIVISION

TYPICAL DETAIL

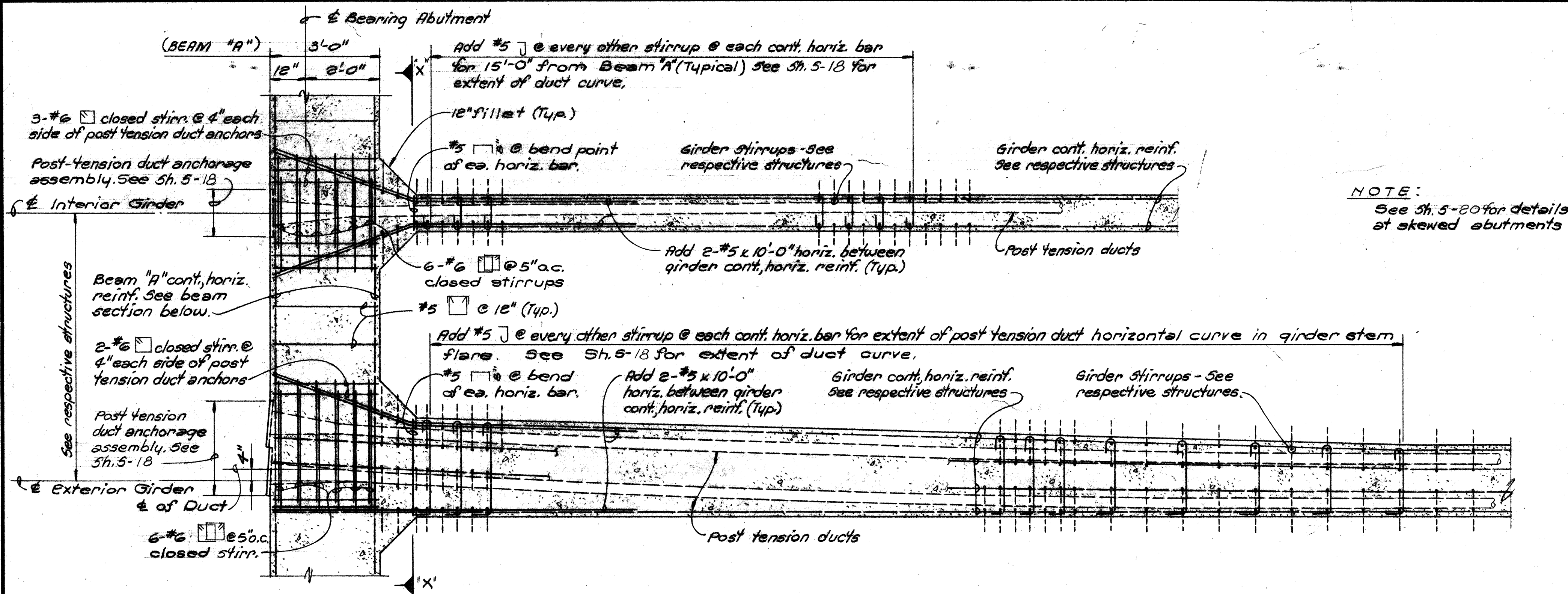
POST TENSION DUCT LAYOUT,
DETAILS & NOTES AT BEAM "A"

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-HI-1(III)-14

Scale: As Shown Date: DEC. 14, 1973

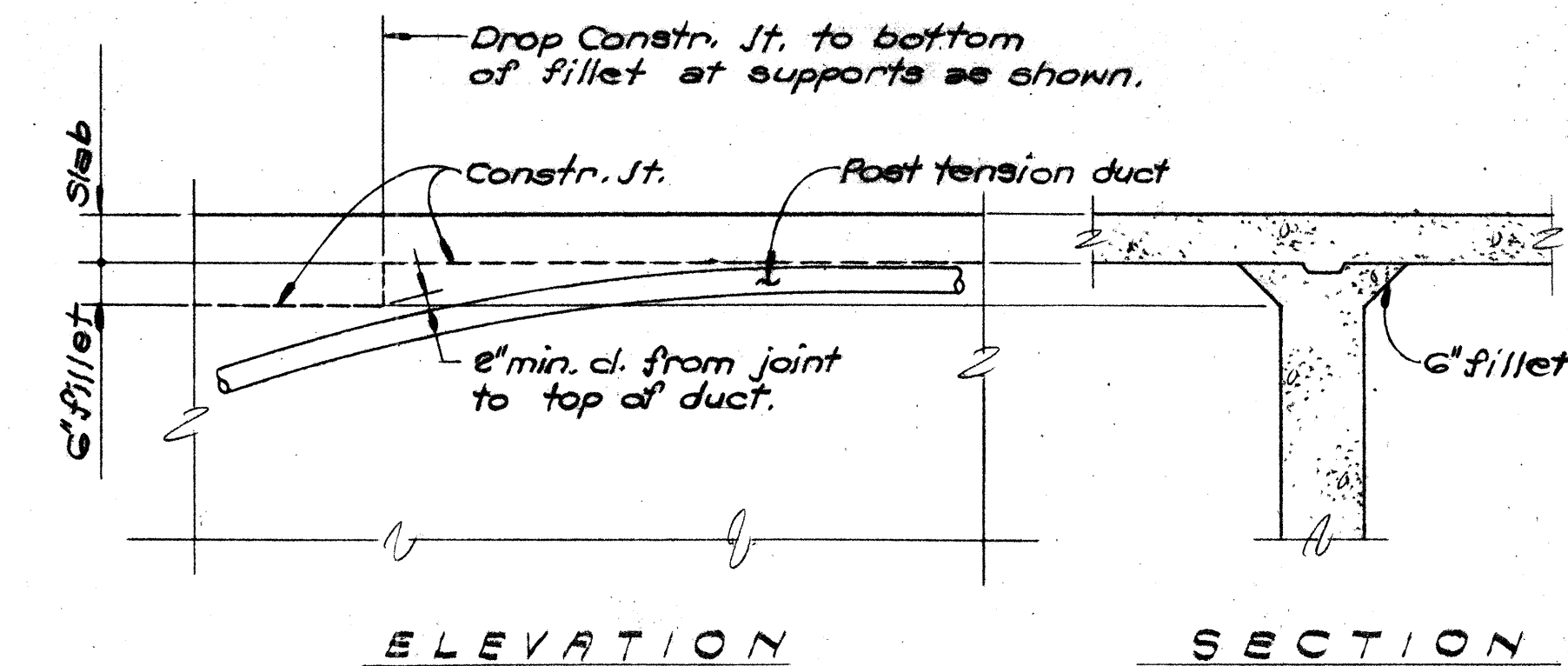
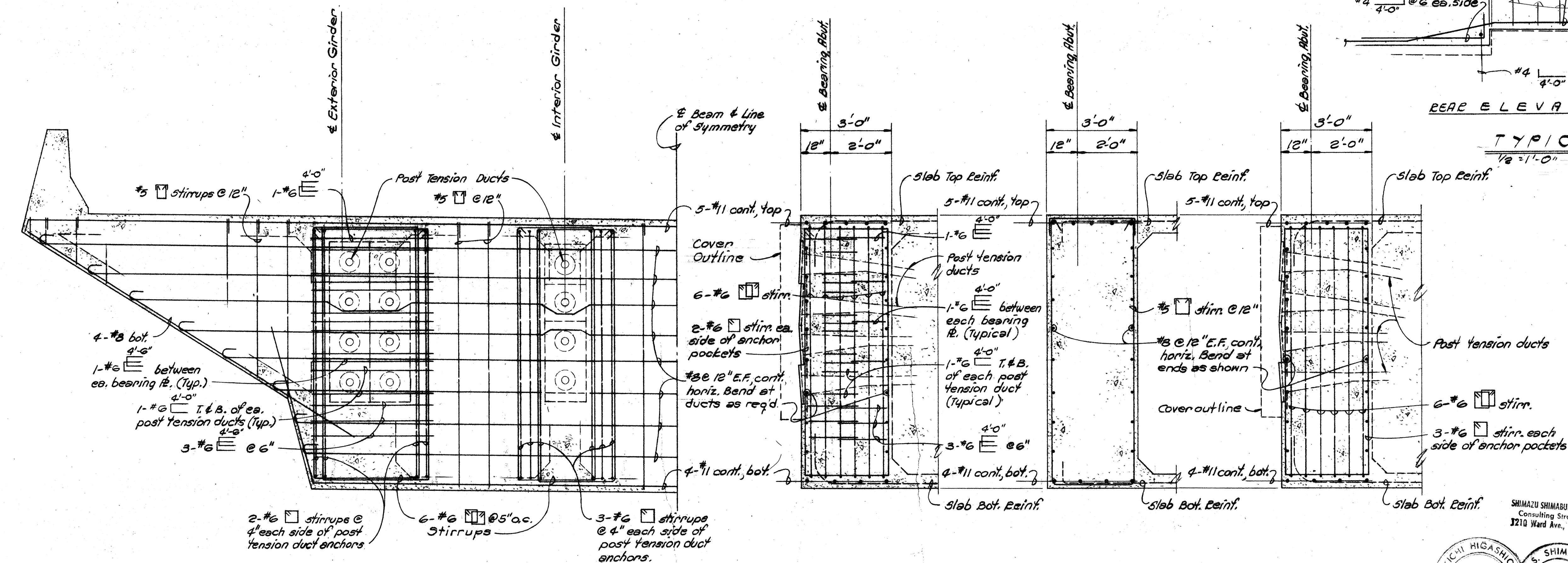
SHEET NO. 5-18 OF 34 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-H-11(11)-14	1974	173	234



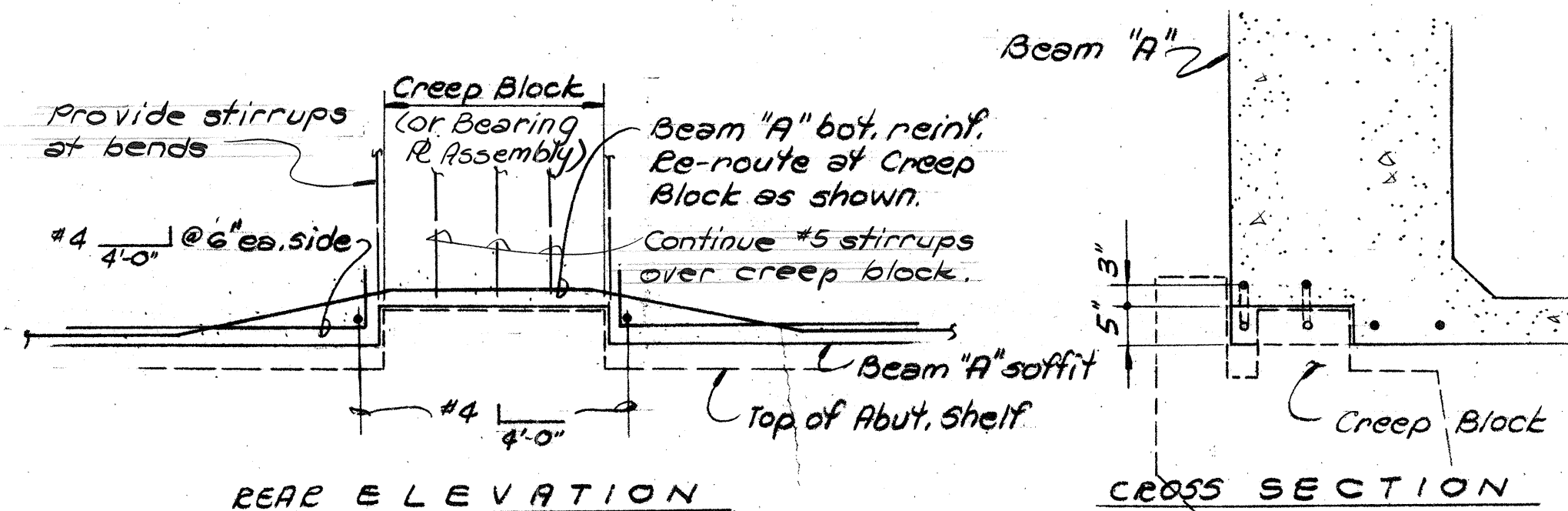
PLAN SHOWING REINFORCING LAYOUT FOR BEAM "A" & GIRDERS "G-1" & "G-2" AT BEAM "A"

1/2" = 1'-0"



STEM CONSTRUCTION JOINT DETAIL

1/2" = 1'-0"



TYPICAL CONDITION @ CREEP BLOCK & BEARING PLATES

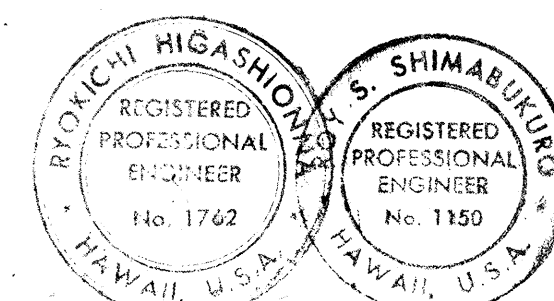
1/2" = 1'-0"

BEAM "A" ~ ELEVATION "X" - "X"

1/2" = 1'-0"

BEAM "A" ~ SECTIONS

1/2" = 1'-0"



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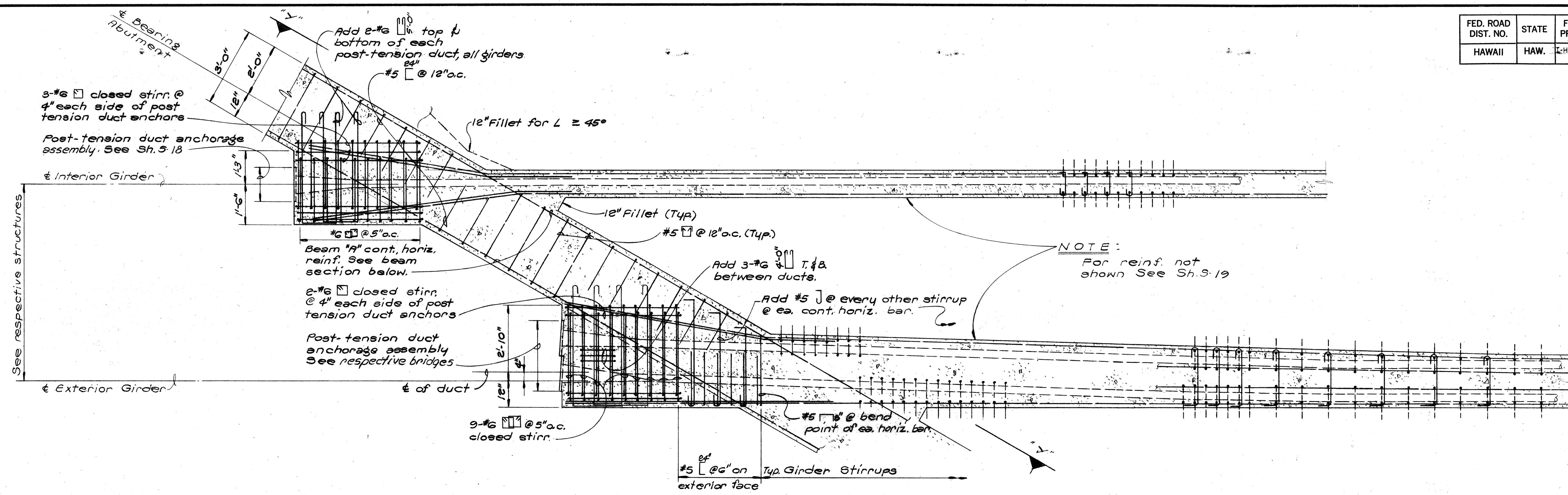
TYPICAL DETAIL
REINFORCING DETAILS FOR
BEAM "A" & GIRDERS

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-H-11(11)-14

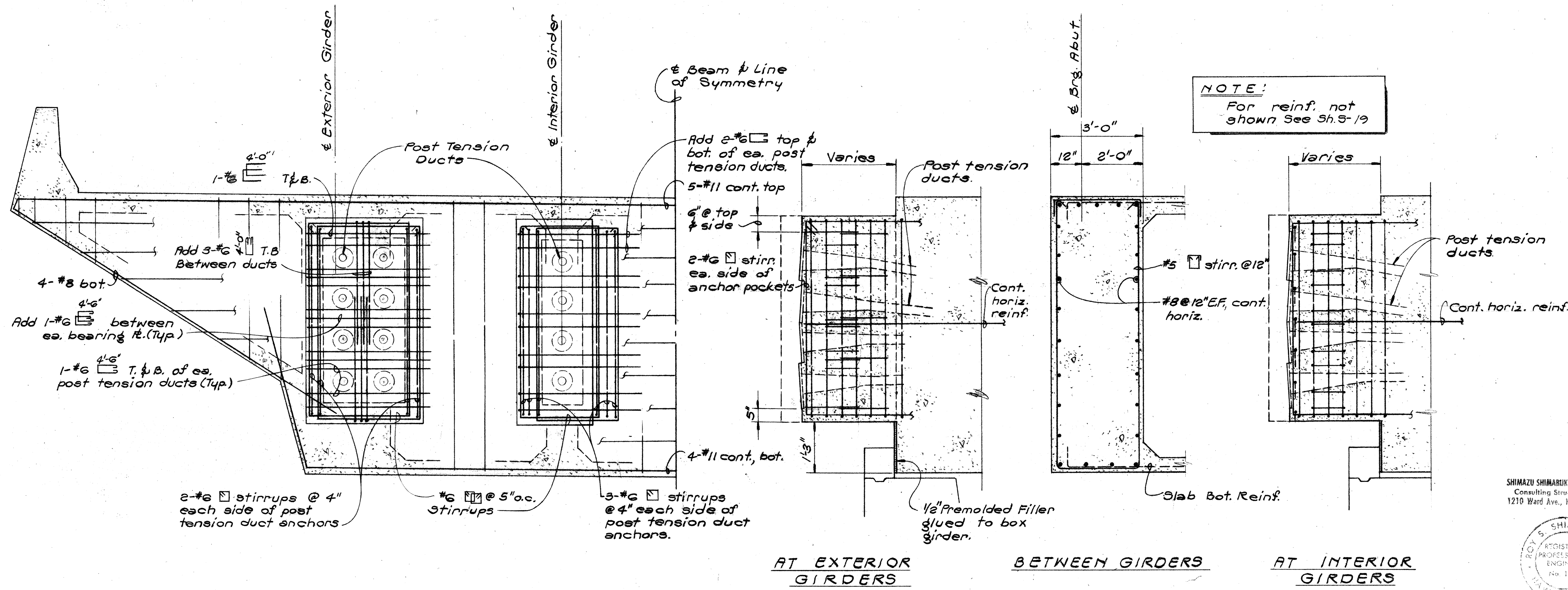
Scale: As shown Date: DEC. 14, 1973

SHEET NO. 9-19 OF 34 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HI-1(11)-14	1974	174	234



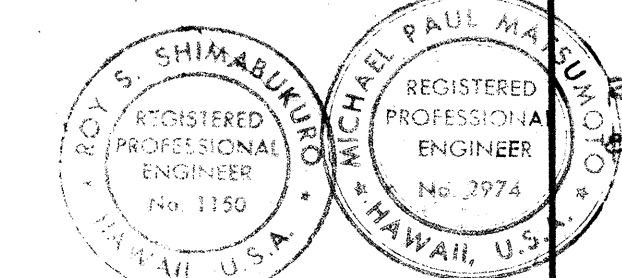
PLAN AT SKEWED ABUTMENTS
Scale = 1/2" = 1'-0"



BEAM "A" - ELEVATION "Y"- "Y"
1/2" = 1'-0"

BEAM "A" SECTIONS
1/2" = 1'-0"

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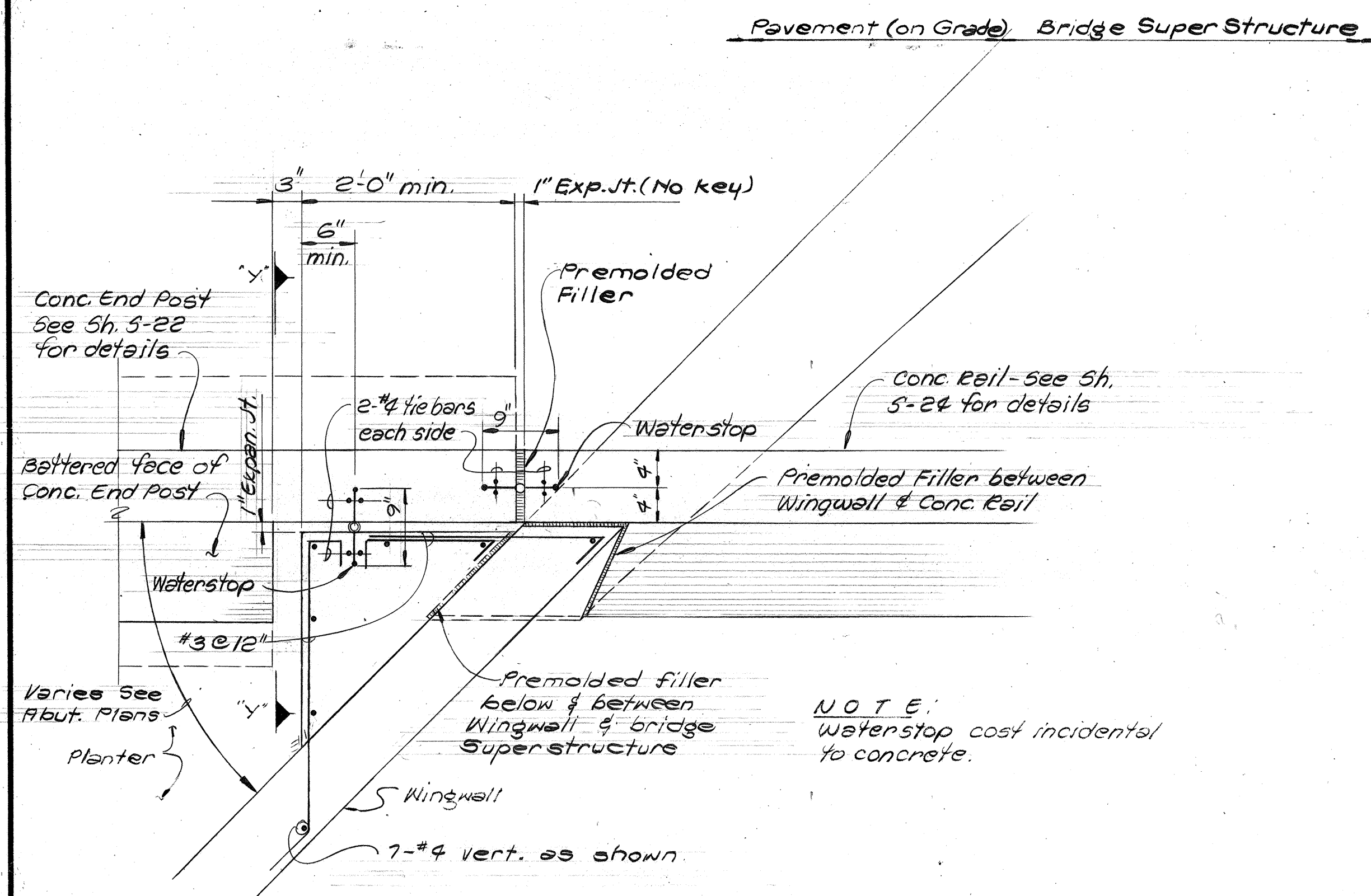
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

TYPICAL DETAIL
REINFORCING DETAIL FOR BEAM "A" &
GIRDERS AT SKEWED ABUTMENTS

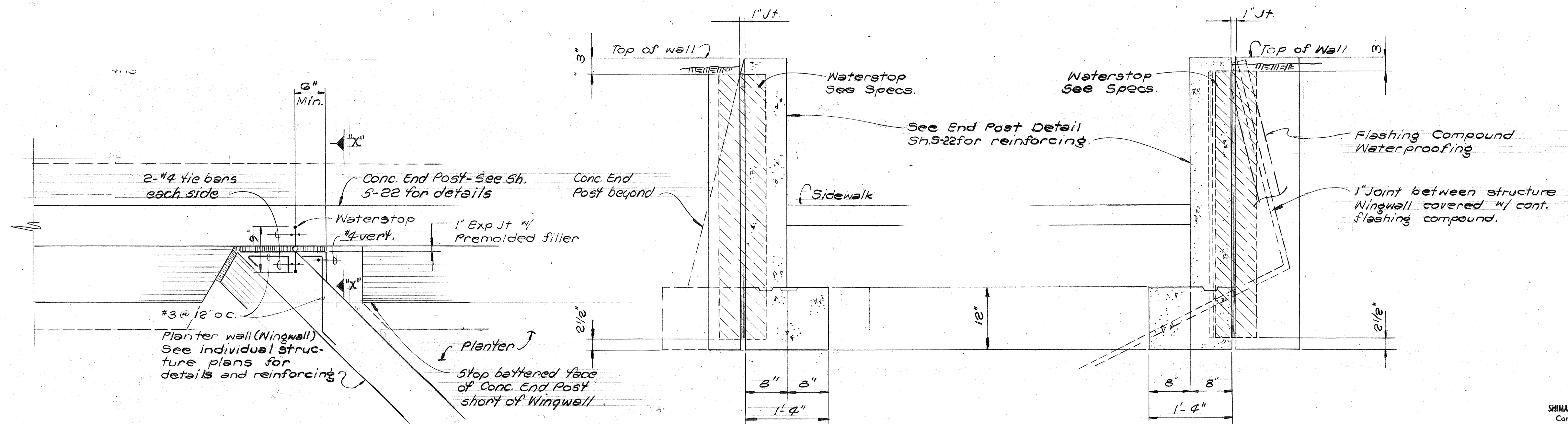
PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-HI-1(11)-14

Scale: As Shown Date: DEC. 14, 1973
SHEET NO. 3-20 OF 34 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-111-14	1974	175	234



PLAN AT BEGINNING OF WINGWALL



PLAN AT END OF WINGWALL

SECTION "X"-X"

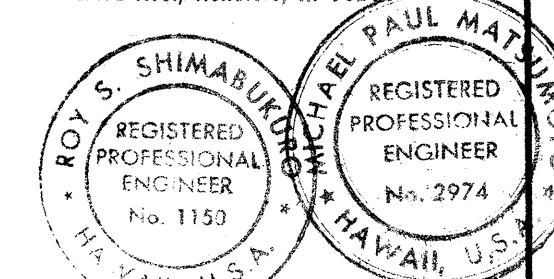
SECTION "Y"-Y"

WINGWALL & END POST CONNECTION DETAILS Scale: 1"=1'-0"

See Sht. 176R
Field Directive
Feb. 3, 1975
Typical Detail Revisions
for End Posts & Wingwall
Connections.

DATE	REVISION
4/16/74	BATTERED BACK FACE OF CONC. END POST

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HIGHWAYS DIVISION

TYPICAL DETAIL
WINGWALL TO END POST
CONNECTION DETAIL

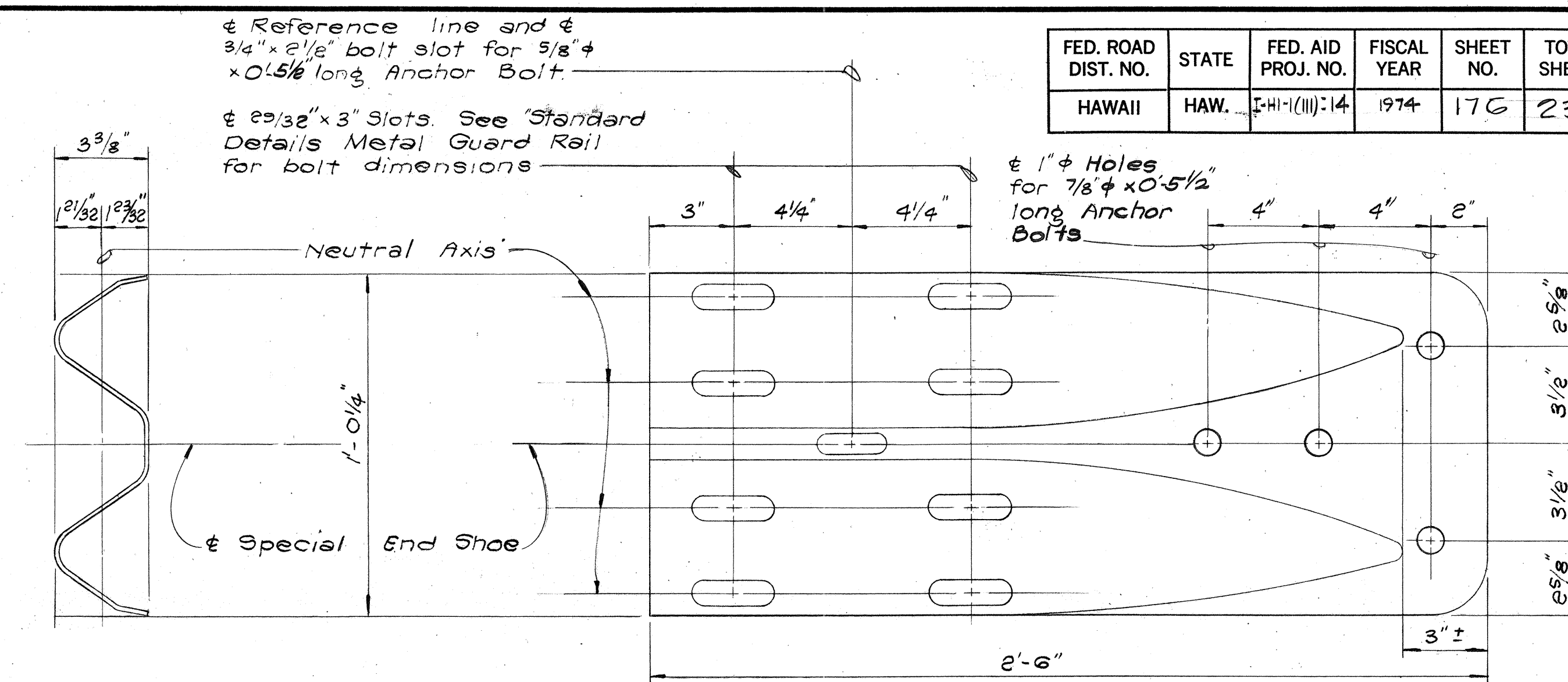
PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-H-1(III)-14

Scale: As Shown Date: DEC 14, 1973
SHEET No. 5-21 OF 24 SHEETS

175R

ORIGINAL	DATE
PLAN	
NOTE BOOK	
QUANTITIES BY	
CHECKED BY	

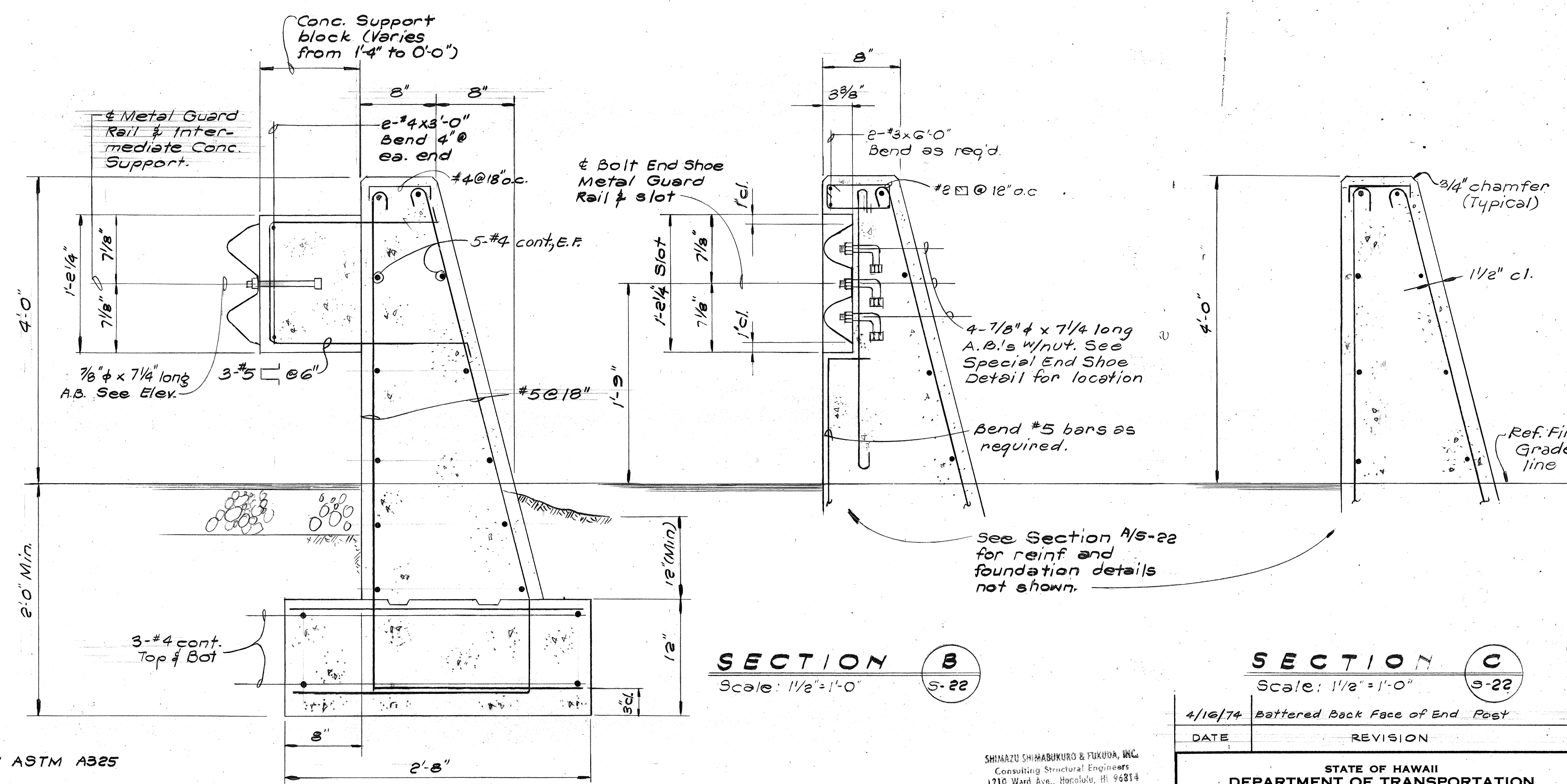
4/16/74	Battered Back Face of End Post
DATE	REVISION



SECTION ELEVATION

SPECIAL END SHOE DETAILS

Scale: 3"=1'-0"



SECTION B
Scale: 1 1/2" = 1'-0"
S-22

SECTION C
Scale: 1 1/2" = 1'-0"

- NOTES:
1. All Anchor Bolts shall be High Strength bolts conforming to the requirements of ASTM A325
 2. All Shoe assembly, including anchor bolts, nuts and washers shall be galvanized
 3. Special End Shoe shall be fabricated from 10 gauge steel conforming to the requirements of AASHTO 180
 4. First 25'-0" of guard rail adjoining "Special End Shoe" shall be galvanized steel.
 5. Anchor Bolt length given from under bolt head.

SPECIAL NOTE:
For concrete end post
section at planter
See Sh. 3-24

See Sht. 175 R

Field Directive
Feb. 3, 1975

Typical Detail Revisions for
End Posts and Wingwall
Connections.

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HIGHWAYS DIVISION

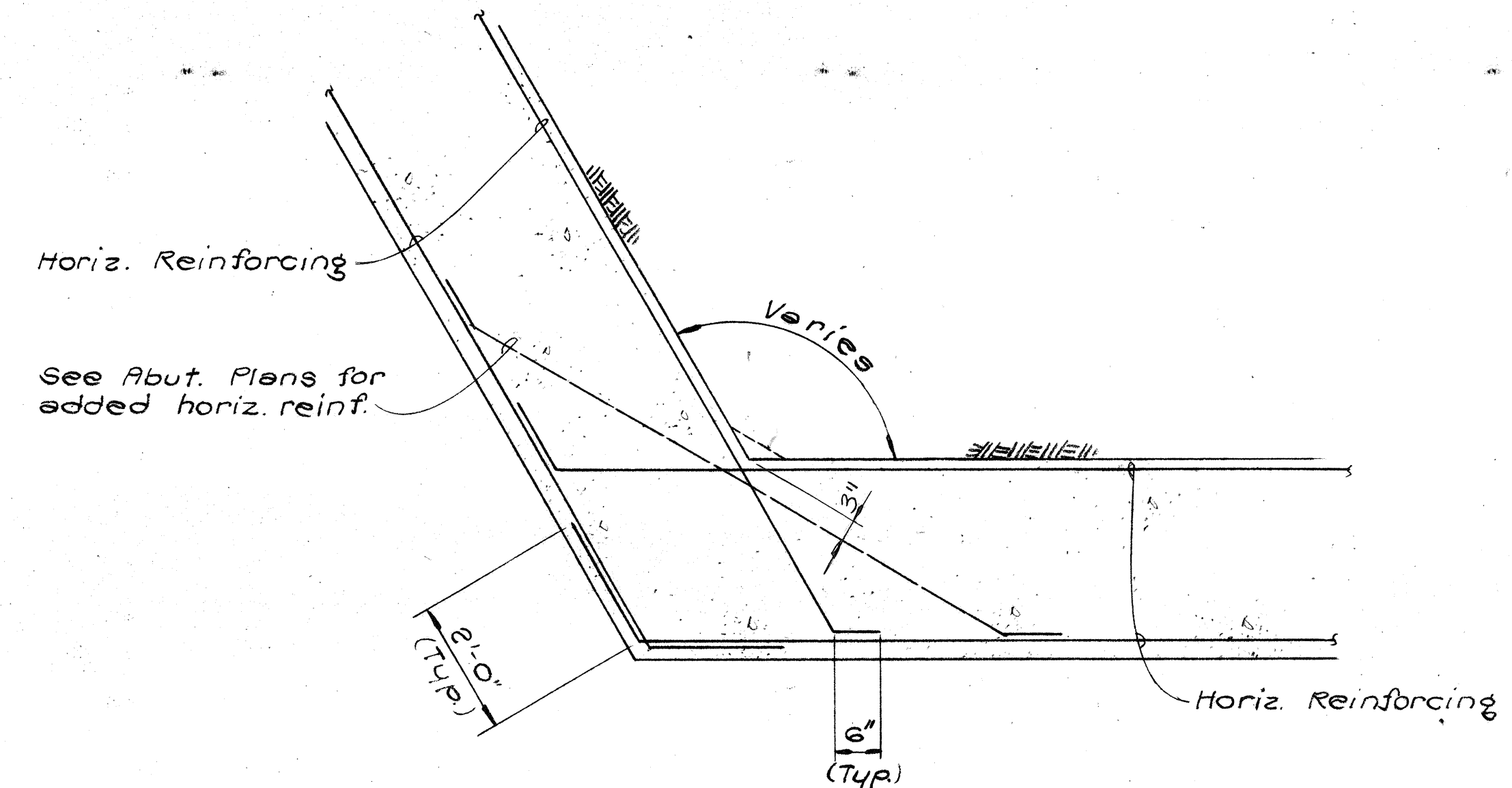
TYPICAL DETAIL
END POST DETAILS

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-HI-1(111)-14

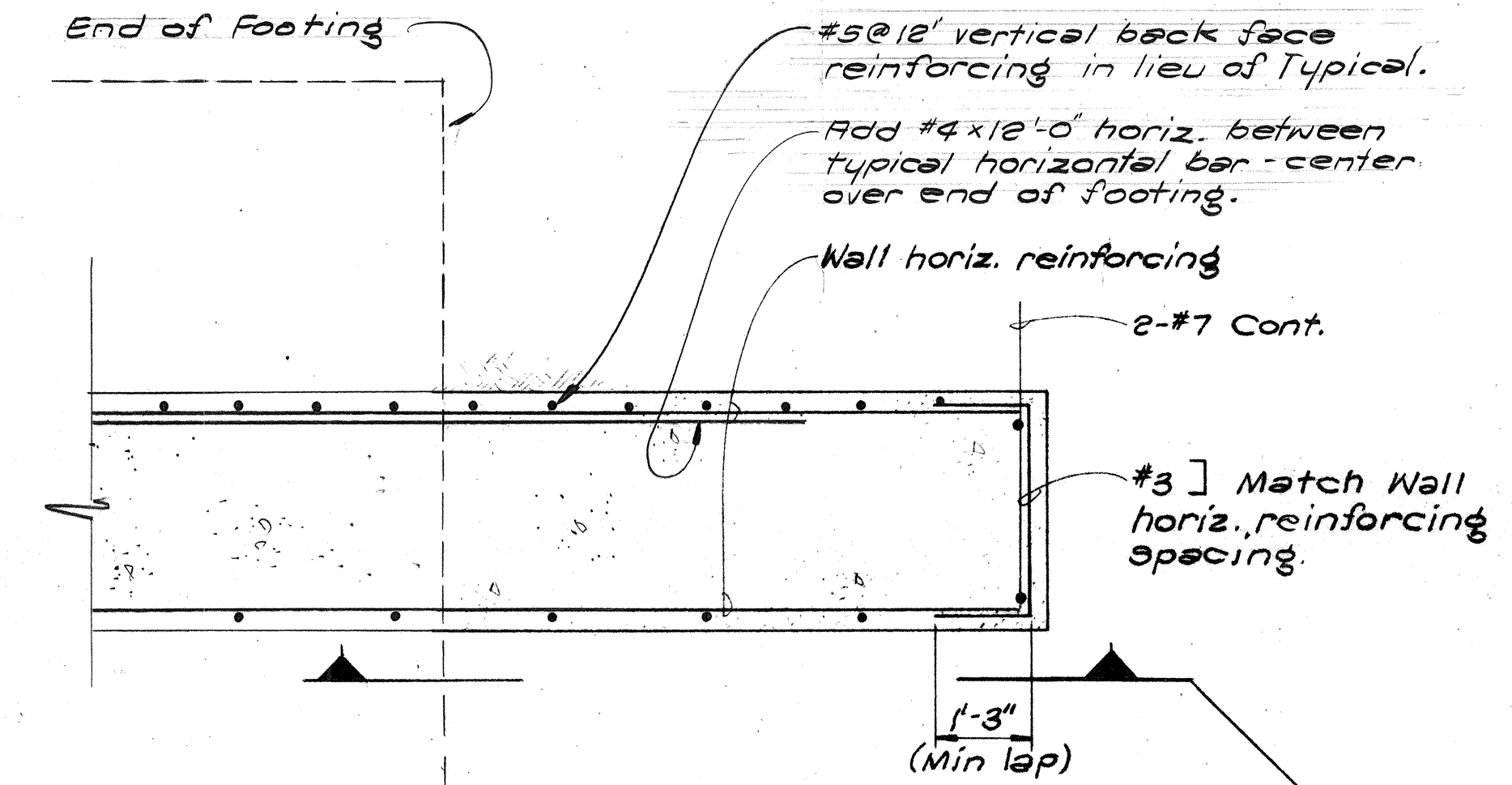
Scale: As Shown Date: DEC. 14, 1973
SHEET NO. 5-22 OF 34 SHEETS

176F

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(11)-14	1974	177	234



PLAN AT CORNERS



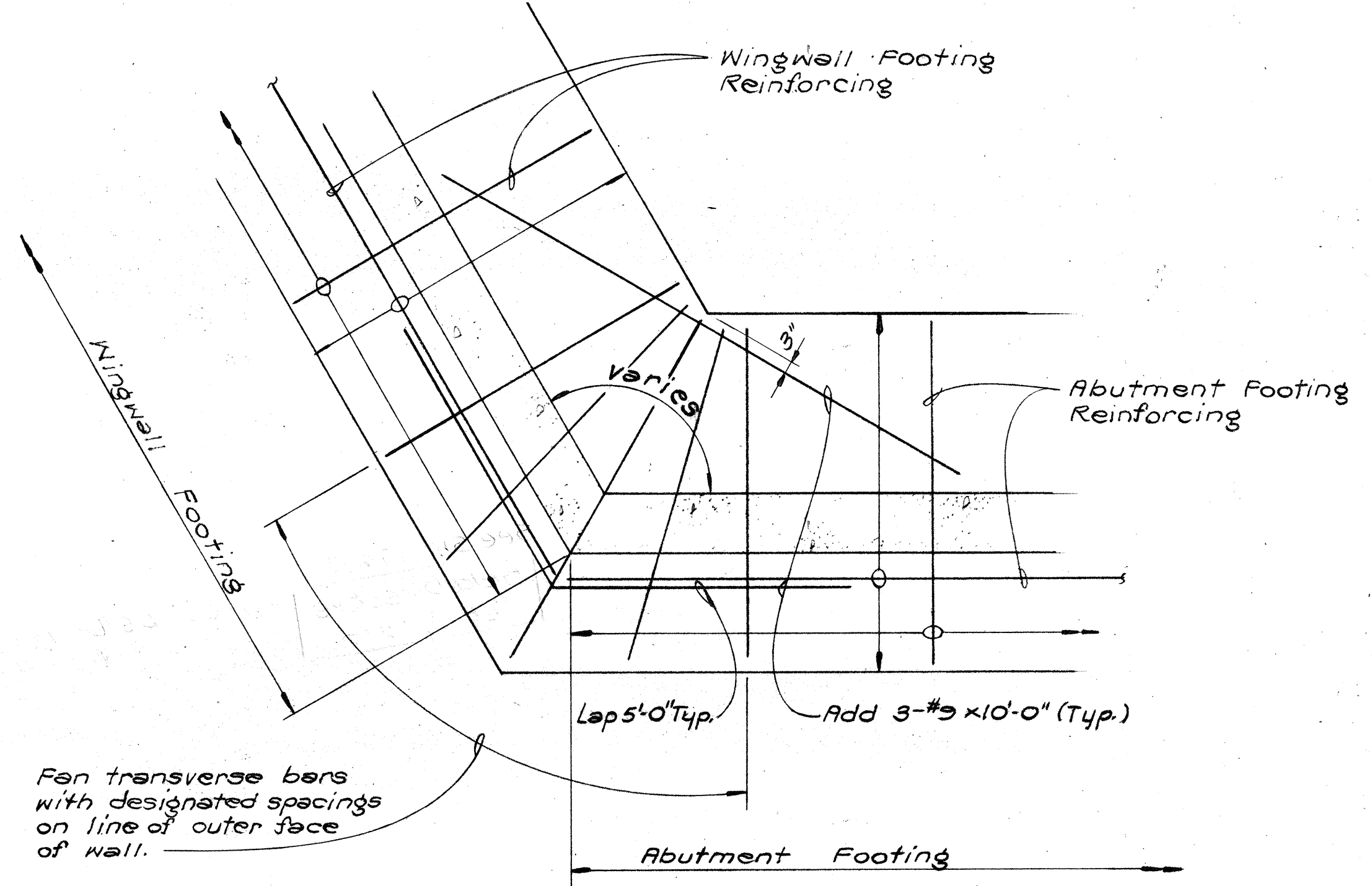
PLAN AT CANTILEVER ENDS

TYPICAL WALL REINFORCING DETAILS

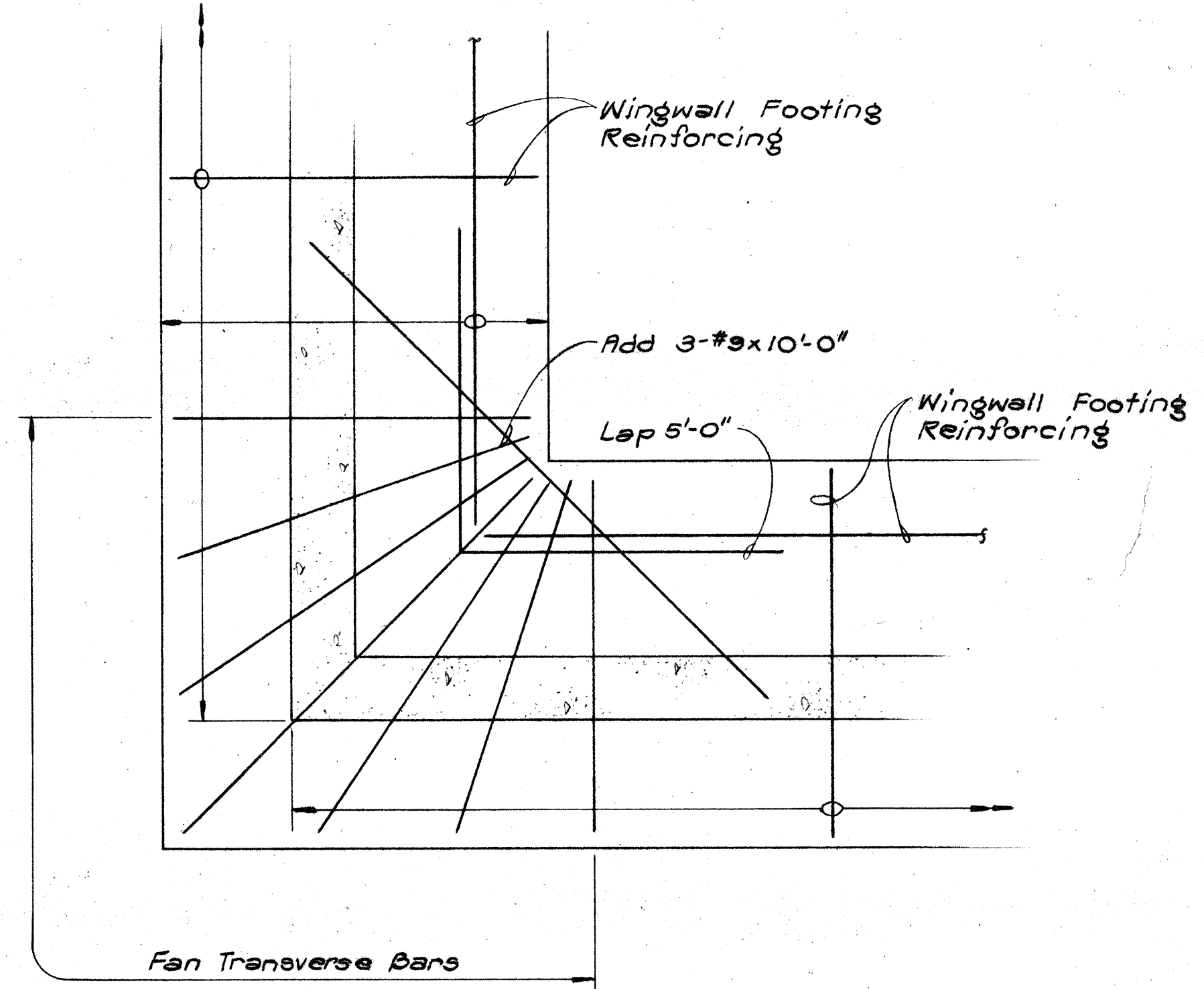
Scale: 1/2" = 1'-0"

WALL & FOOTING NOTES

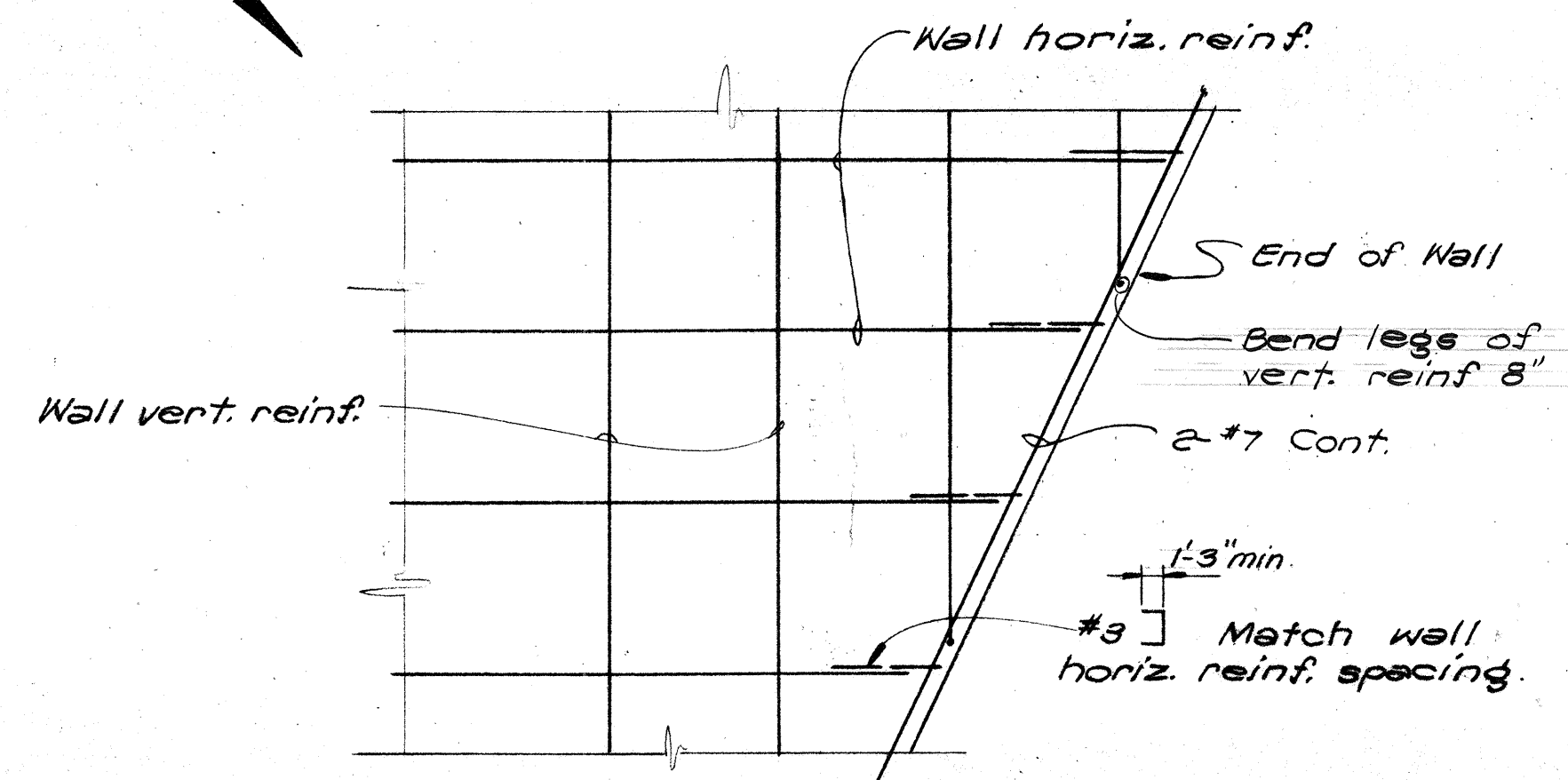
1. Concrete for all footings shall be poured against neat excavations.
2. All footings shall be embedded 1'-0" (minimum) into competent tuff.
3. Bottom of footings shall be to elevations shown in these plans unless minimum embedment into competent tuff is not attainable - in which case the Engineer shall be notified of this situation.
4. Concrete walls may be poured full height, provided pour windows 6'-0" o.c., E.W. are utilized for placing concrete.
5. Minimum 28 day concrete strengths for walls & footings shall be - 3000 p.s.i.
6. Reinforcing steel shall conform to A.S.T.M. A615, Grade 60.



PLAN - ABUTMENT & WINGWALL CORNER FOOTING REINF.



PLAN - WINGWALL CORNER FOOTING REINF.

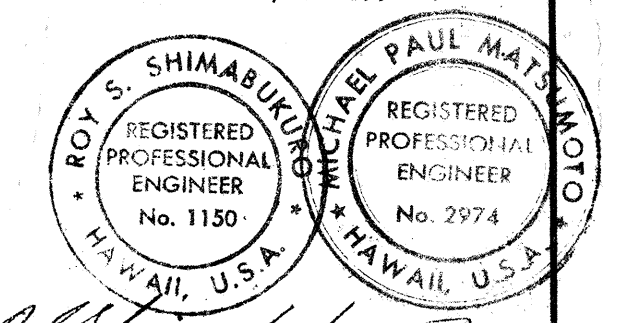


ELEVATION

ABUTMENT WALL & FOOTING DETAILS

Scale: 1/2" = 1'-0"

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DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

TYPICAL DETAIL
TYPICAL WALL AND FOOTING
REINFORCEMENT DETAILS

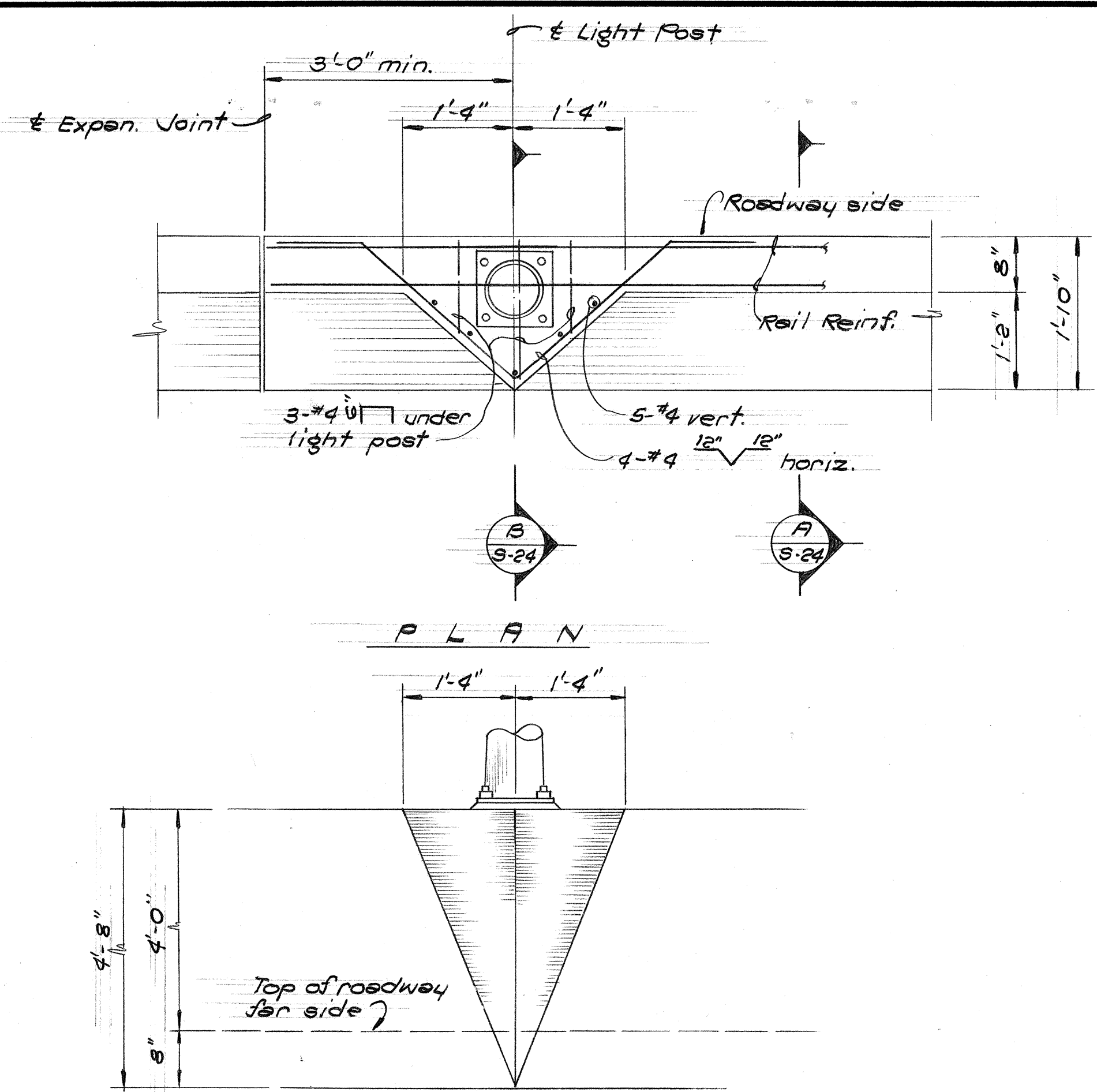
PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-HI-1(11)-14

Scale: As Shown Date: DEC. 14, 1973

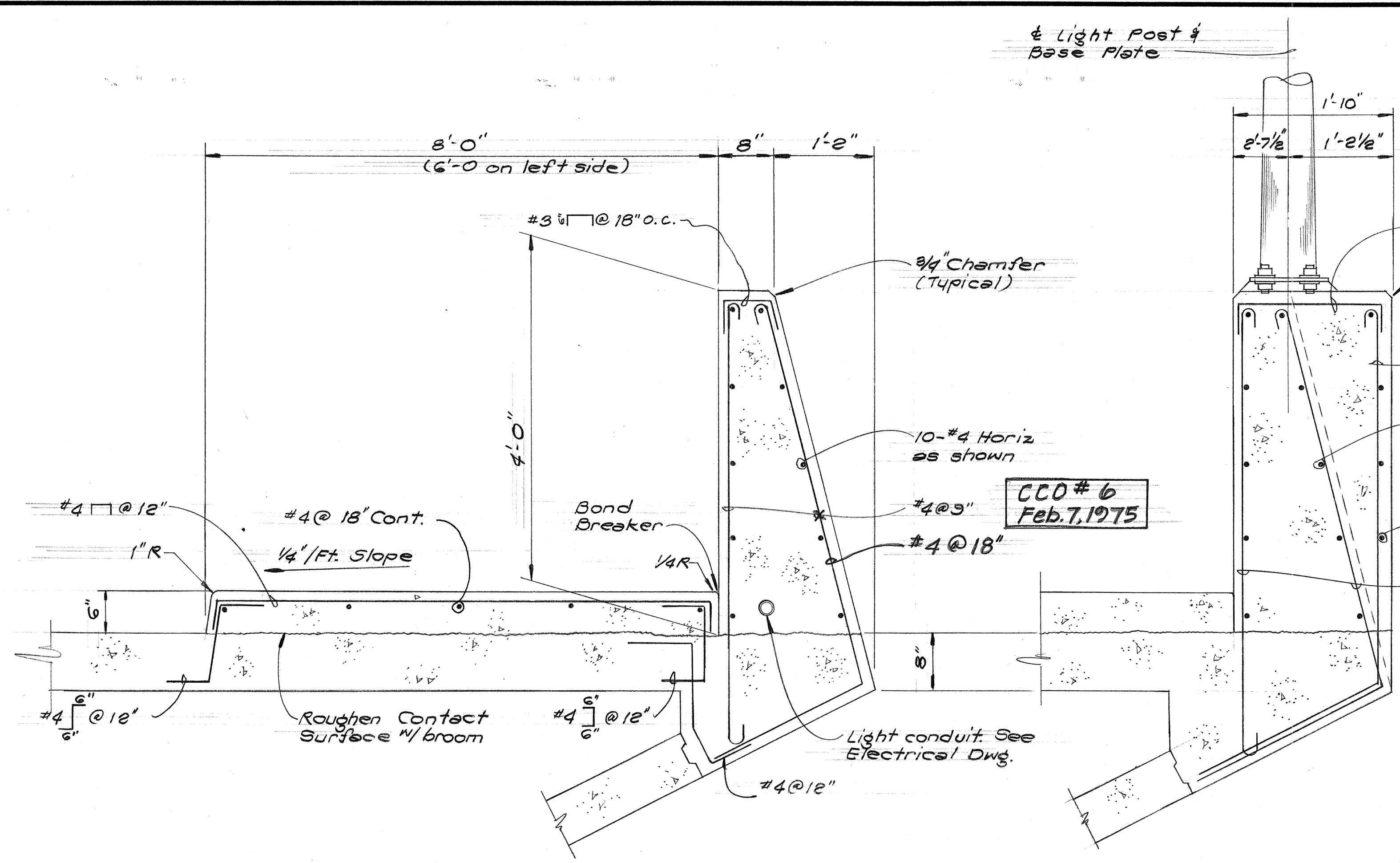
SHEET NO. 23 OF 34 SHEETS

DATE	_____
SURVEY PLOTTED BY	_____
DESIGNED BY	_____
TRACED BY	_____
NOTE BOOK	_____
QUANTITIES BY	_____
CHECKED BY	_____
No.	_____

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-H-1(11):14	1974	178	234



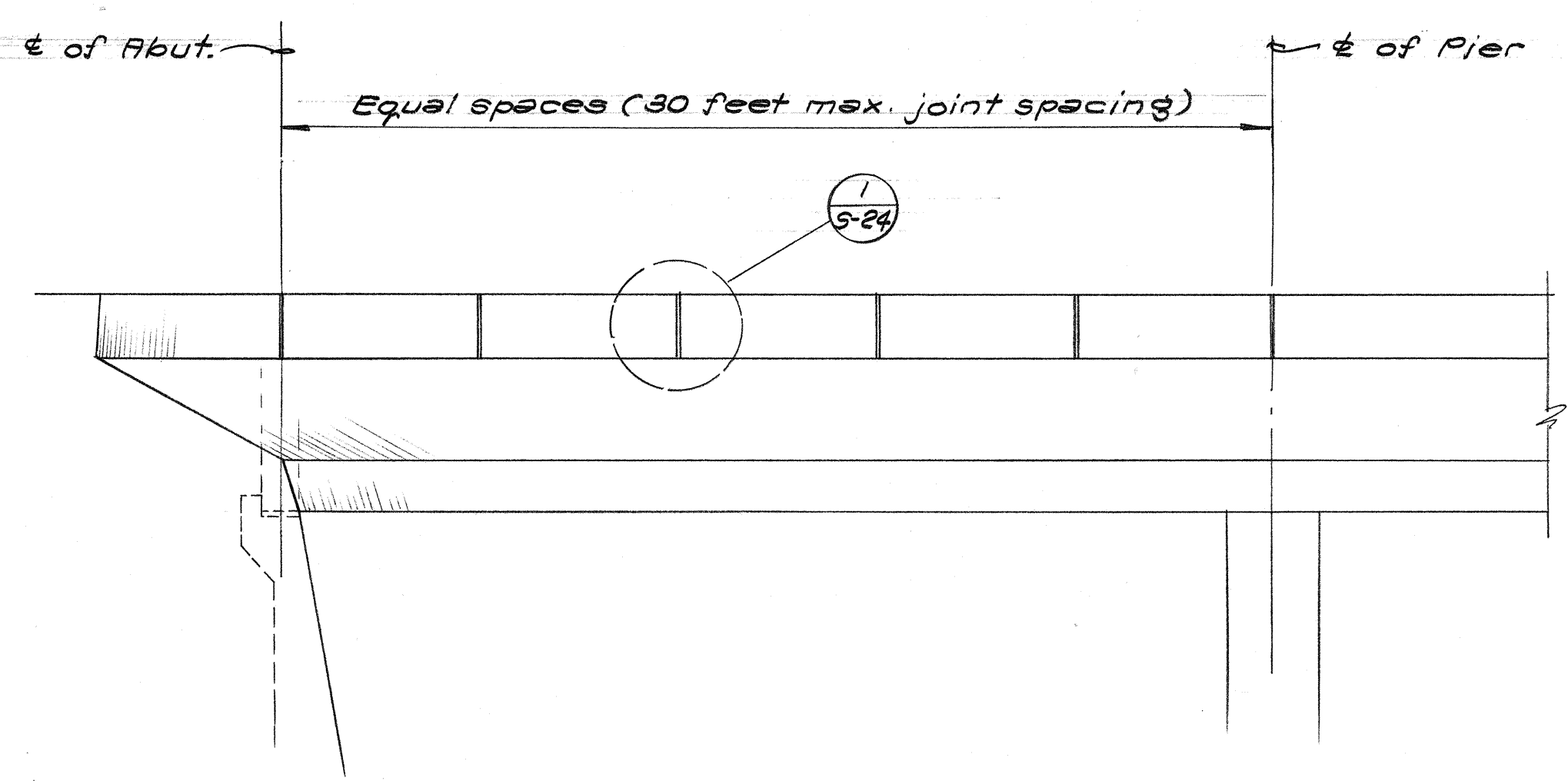
CONC. RAIL AT LIGHT STANDARD
3/4" = 1'-0"



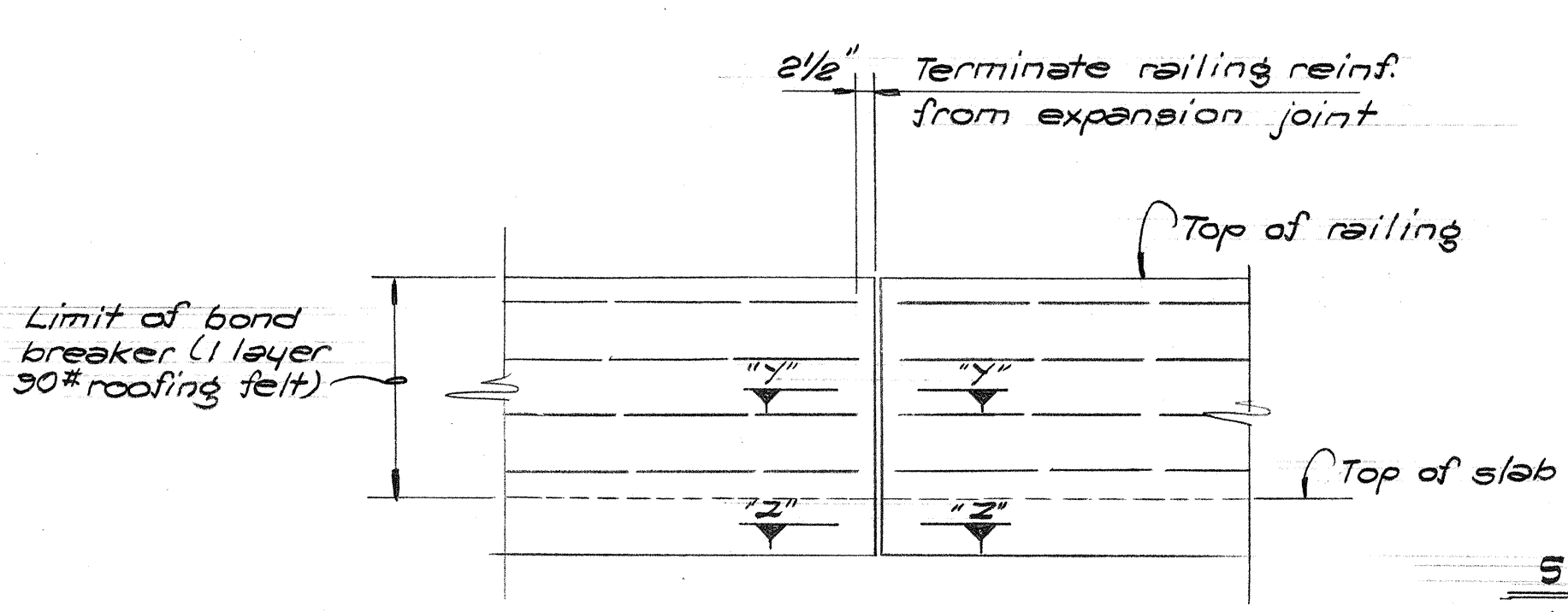
SECTION A
1" = 1'-0"

SECTION B
1" = 1'-0"

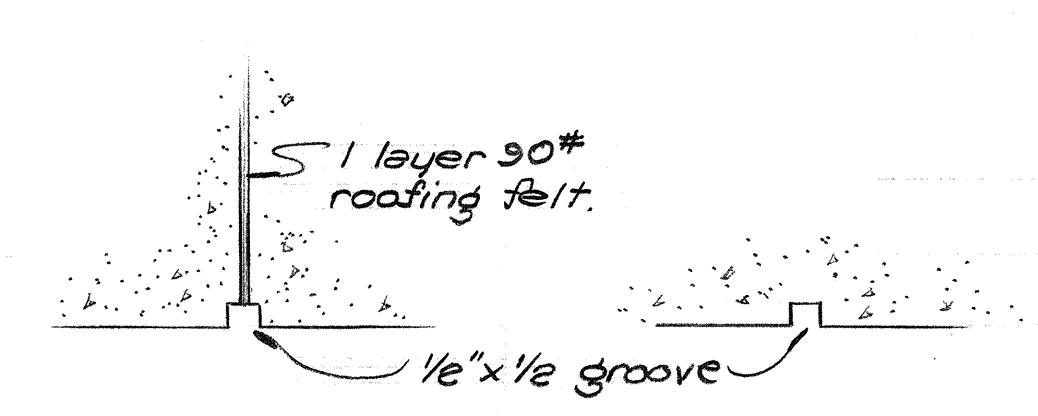
NOTE:
For Highway Lighting Standard Mounting detail See Electrical Drawings



RAIL ELEVATION SHOWING EXPANSION JOINT SPACING
N.T.S.

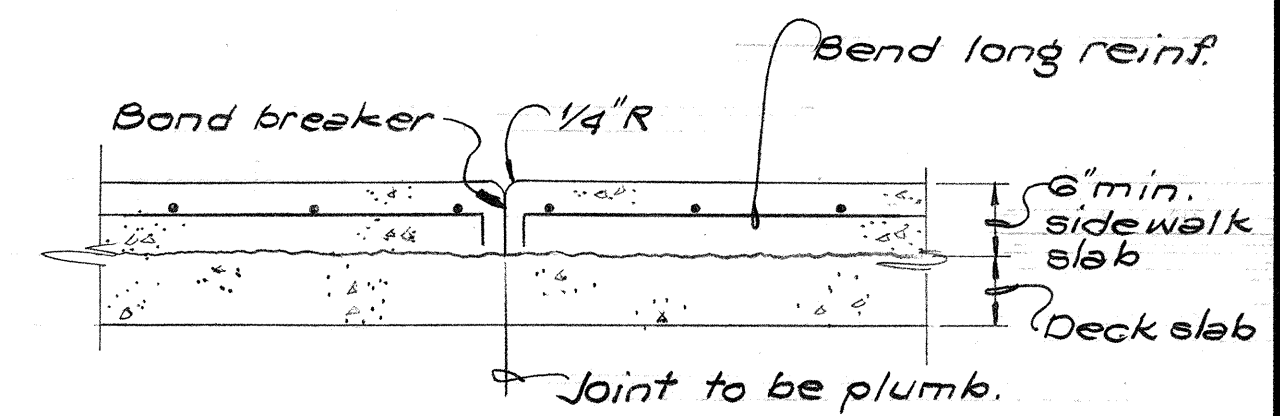


DETAIL
1/2" = 1'-0"



SECTION "Y-Y"
N.T.S.

SECTION "Z-Z"
N.T.S.



SIDEWALK CONST. JOINT DETAIL
(Maximum joint spacing 30'-0" match rail expansion joint spacing)

TYPICAL CONCRETE RAILING DETAILS

SHIMAZU SHIMABUKURO & FUKUDA, INC.
Consulting Structural Engineers
1210 Ward Ave., Honolulu, HI 96814

REGISTERED PROFESSIONAL ENGINEER
No. 2974
HAWAII, U.S.A.

REGISTERED PROFESSIONAL ENGINEER
No. 1150
HAWAII, U.S.A.

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

TYPICAL DETAIL
CONCRETE RAILING DETAIL
CONCRETE SIDEWALK DETAIL

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-H-1(11):14

Scale: As Shown Date: DEC. 14, 1973
SHEET No. 5-24 OF 34 SHEETS

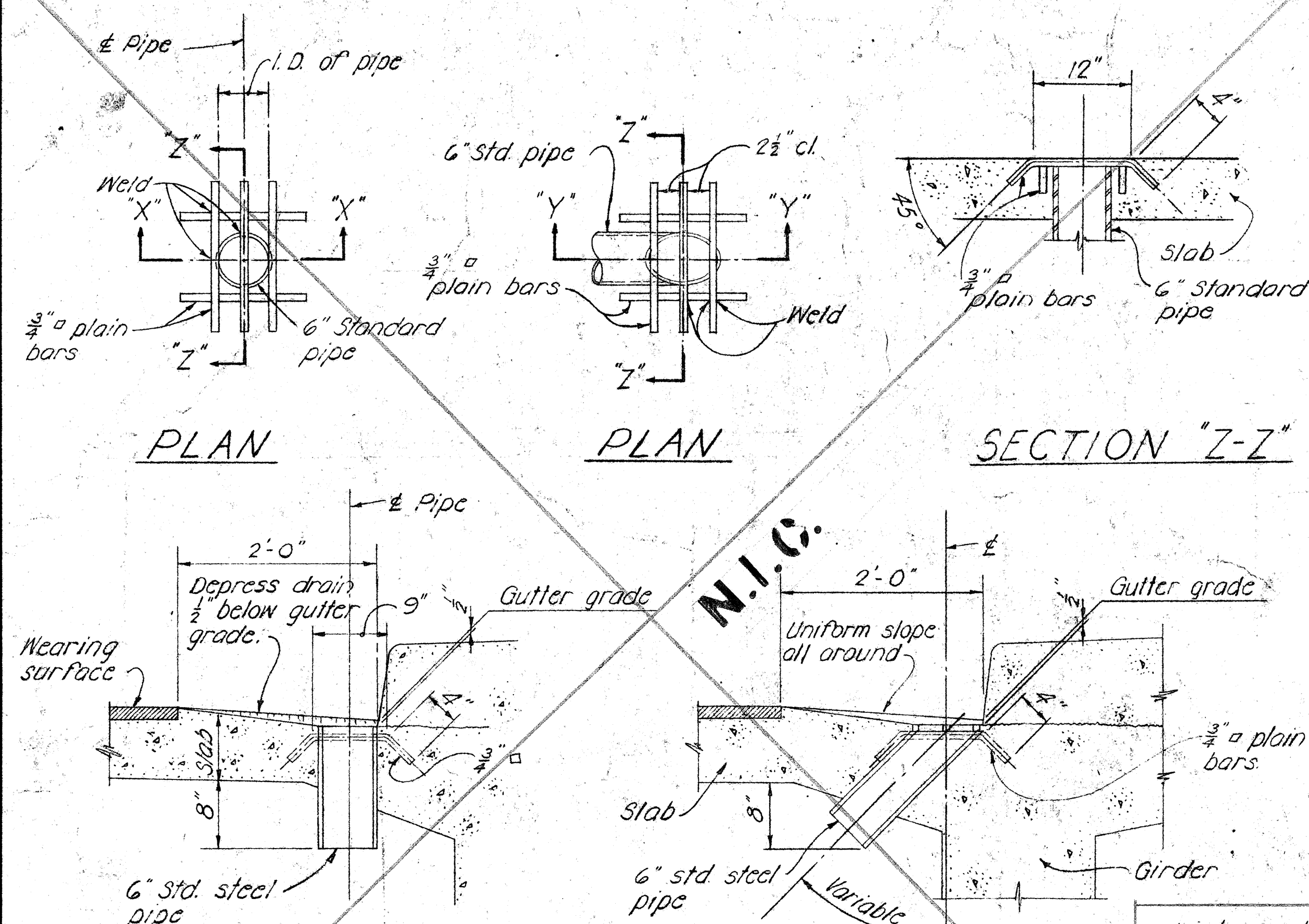
This work was prepared by me or under my supervision

GENERAL NOTES

- All concrete shall be Class A unless otherwise noted.
- Reinforcing shall be detailed in accordance with A.C.I. Manual of Standard Practice for Detailing Reinforced Concrete Highway Structures unless otherwise noted.
- Clearance for reinforcing bars shall be as follows, unless otherwise noted:
 - 1/2" For stirrups of beams & pier caps.
 - 2" For ties of columns.
 - 2" For formed walls.
 - 3" For footings.
- Minimum spacing between parallel bars shall be 2 1/2 times the diameter of bars, but in no case shall the clear distance between the bars be less than 1 1/2 times the maximum size of the coarse aggregate.
- All dimensions relating to reinforcing are to centers of bars unless otherwise noted.
- Reinforcing bars shall be securely tied at all intersections & lap splices & shall be held in place during pouring to maintain their proper locations as shown on plans.
- Vertical column bars shall be arranged in such a manner as to miss pier cap bars above as directed by the Engineer.
- Except as otherwise noted on drawings, all exterior corners & re-entrant angles in concrete work shall be chamfered 3/4" x 3/4".
- Concrete seats receiving steel plates, lead plates or neoprene pads shall be poured monolithically with supporting structures & top of concrete seats shall be finished with a steel trowel to a smooth level, surface at the elevation shown on the plans.
- Elastomeric Pads: Bottom of bridge bearing pads shall be secured against displacement with adhesives approved by the Engineer. Pads shall be incidental to concrete and will not be paid for separately.
- Forms for all exposed surfaces of separation structures and surfaces of retaining walls visible from a traveled way or from a populated area shall be plywood except that metal forms may be permitted provided a surface finish satisfactory to the Engineer can be obtained and the use of such forms is approved in writing by the Engineer. Forms for round columns shall be made of materials that will give a smooth finish and true dimensions as given in the plans. Forms for prestressed concrete members shall be either metal or plywood.
- Gothic letters and figures approximating dimensions shown will be acceptable if approved by the Engineer.
- The contractor shall verify the location of all existing utility lines and notify the respective owners before commencing work of excavation or driving of piles.
- Standard Detail Drawings refer to all structures in general, except for modifications as may be required for special conditions. For such modifications refer to the corresponding detailed drawings.

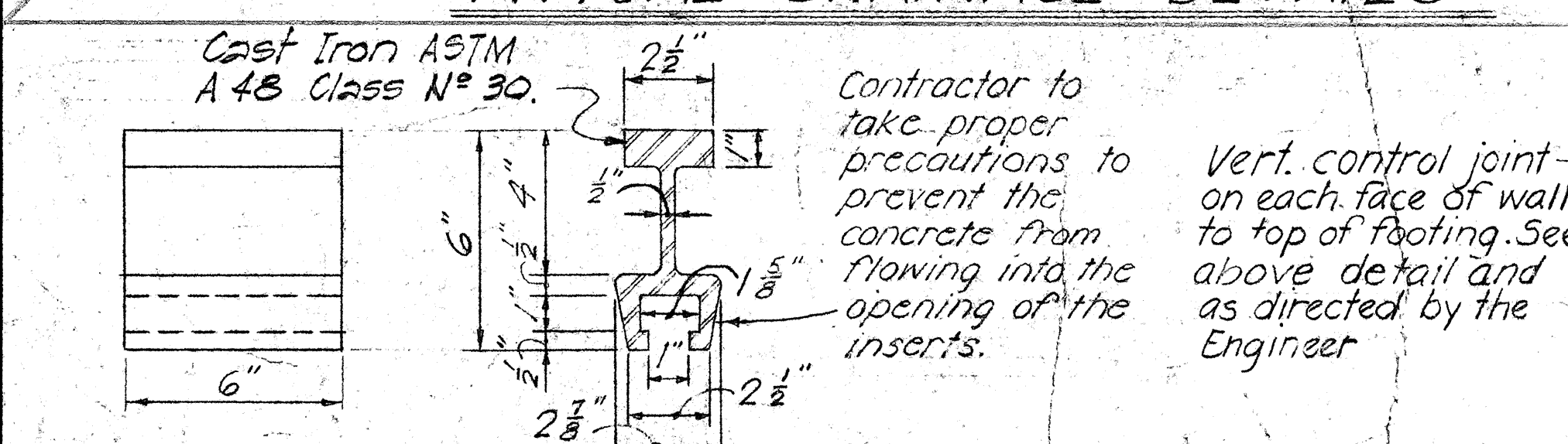
TYPICAL CONCRETE GROOVE DETAIL

DRIP DETAIL

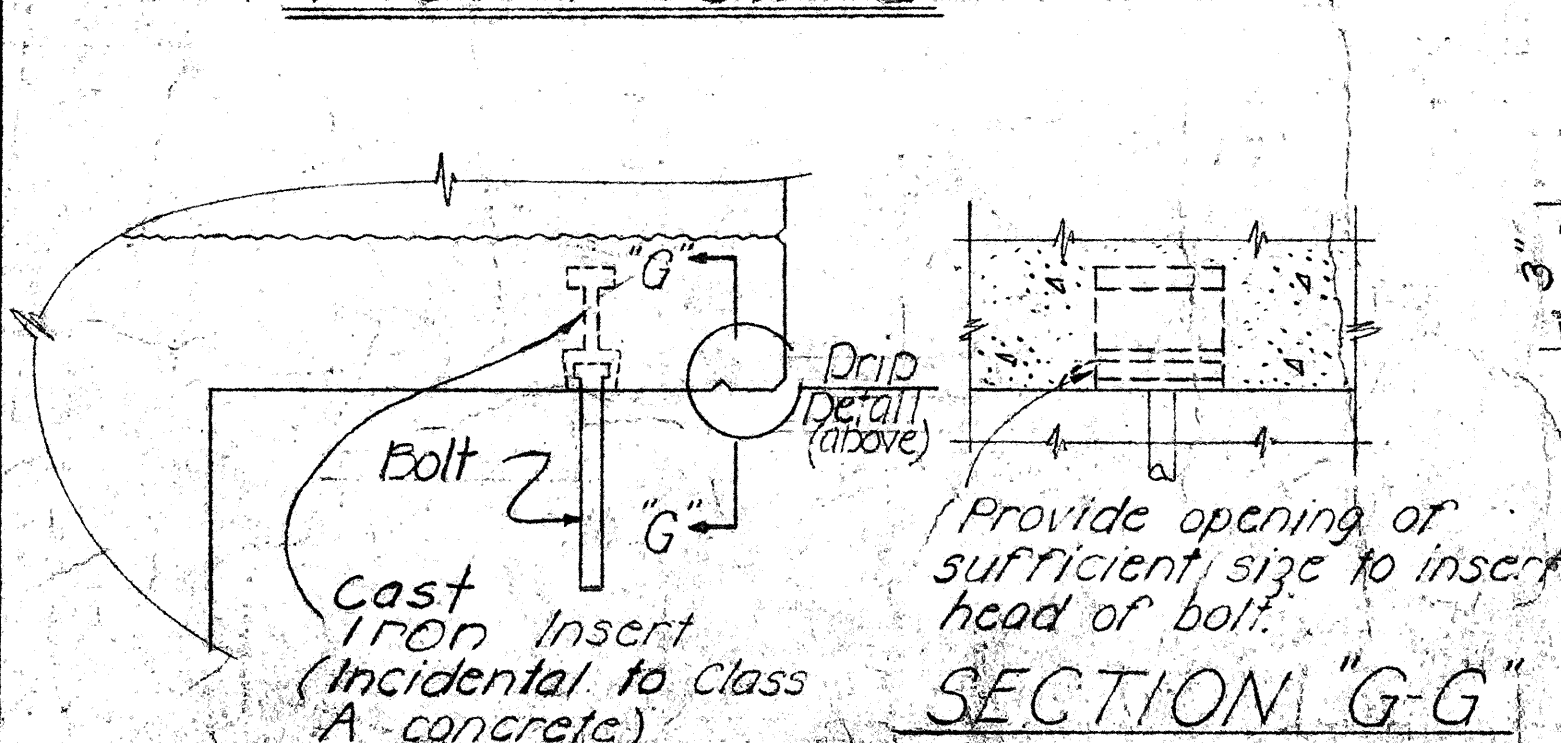


NOTE: Metal drains shall be considered incidental to Class A Concrete in both concrete and steel bridges and will not be paid for separately. Assembly of grillage and pipe shall be hot-dip galvanized after all welding is completed.

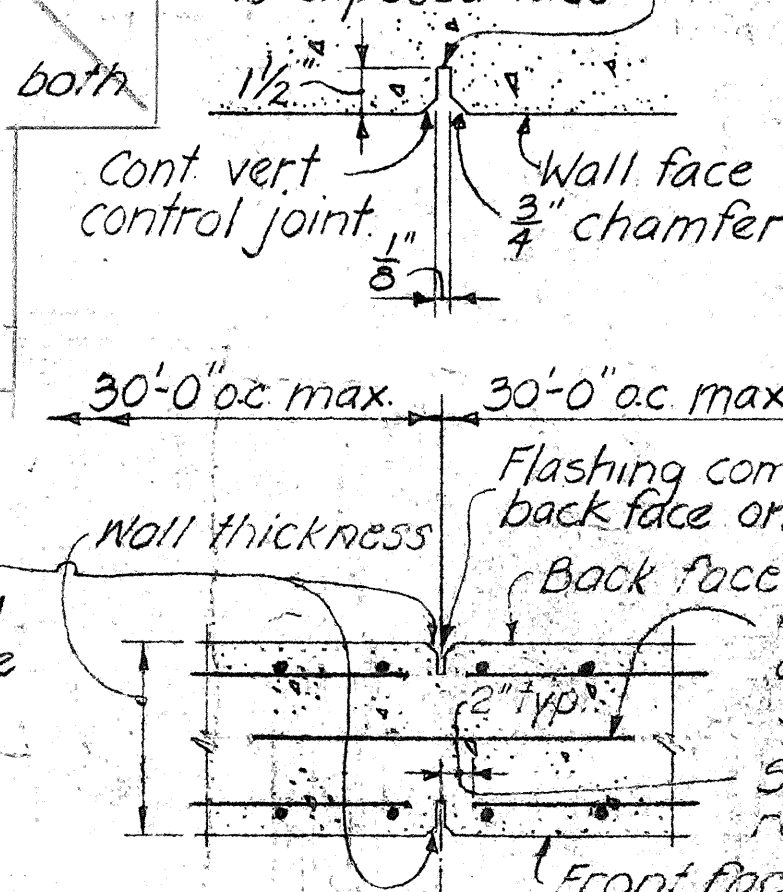
TYPICAL DRAINAGE DETAILS



INSERT DETAIL

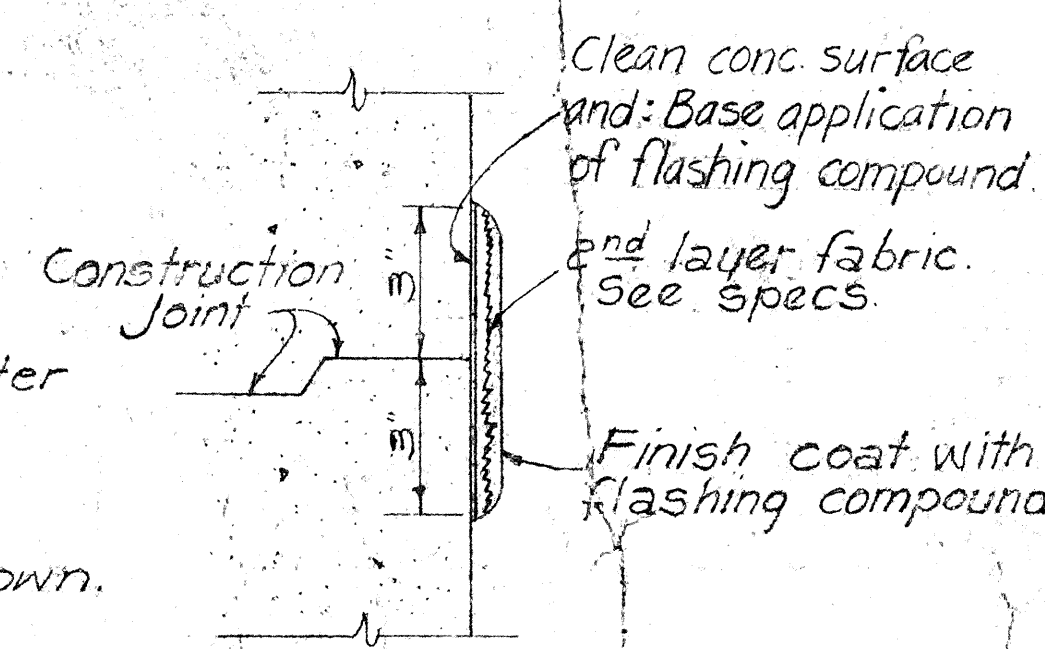


Joint may be formed with a masonite & cut back to the root of the chamfer on the exposed face.



CONTROL JOINT DETAILS

TYP FLASHING COMPOUND WATERPROOFING DETAIL



NAME DATE

Use correct name of bridge

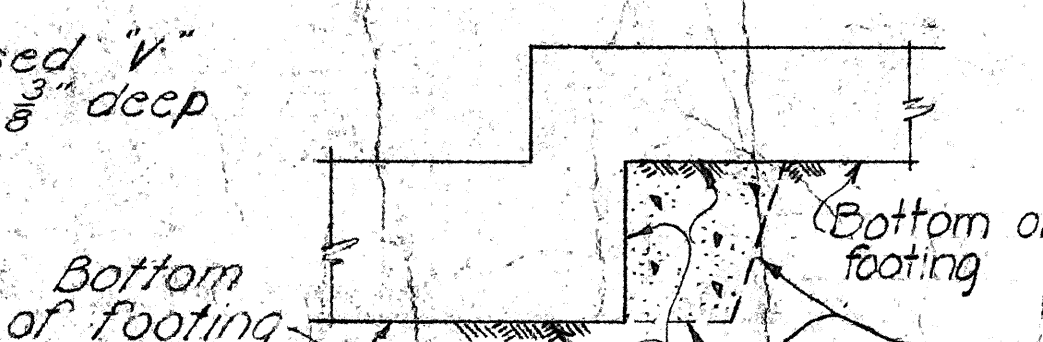
Date of Year built

NOTE: Refer to corresponding detail drawings for placement of such names & dates at end post. Exact details & spacing of letters & figures shall be as directed by the Engineer. See Note #12.

TYPICAL DETAIL OF LETTERS & FIGURES AT CONCRETE END POST

DETAIL OF PIPE HANGERS & INSERTS

Cast Iron Insert (Incidental to Class A concrete)



Note! In case of over excavation, the contractor shall backfill this area with minimum class D concrete at his own expense.

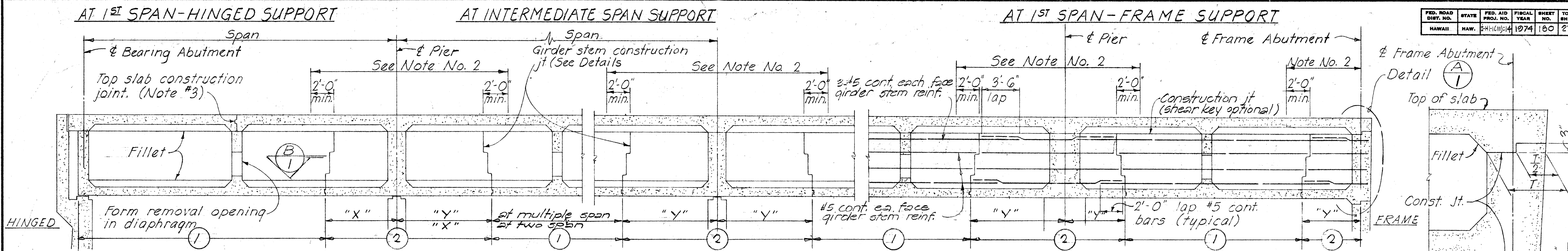
TYPICAL EXCAVATION FOR CONTINUOUS FOOTING STEP-UP

APPROVAL RECOMMENDED:
Paul T. Yamashita
BRIDGE DESIGN ENGINEER
DATE 1-23-76

APPROVED
Paul T. Yamashita
ASSISTANT CHIEF, ENGINEERING
DATE 1-28-76

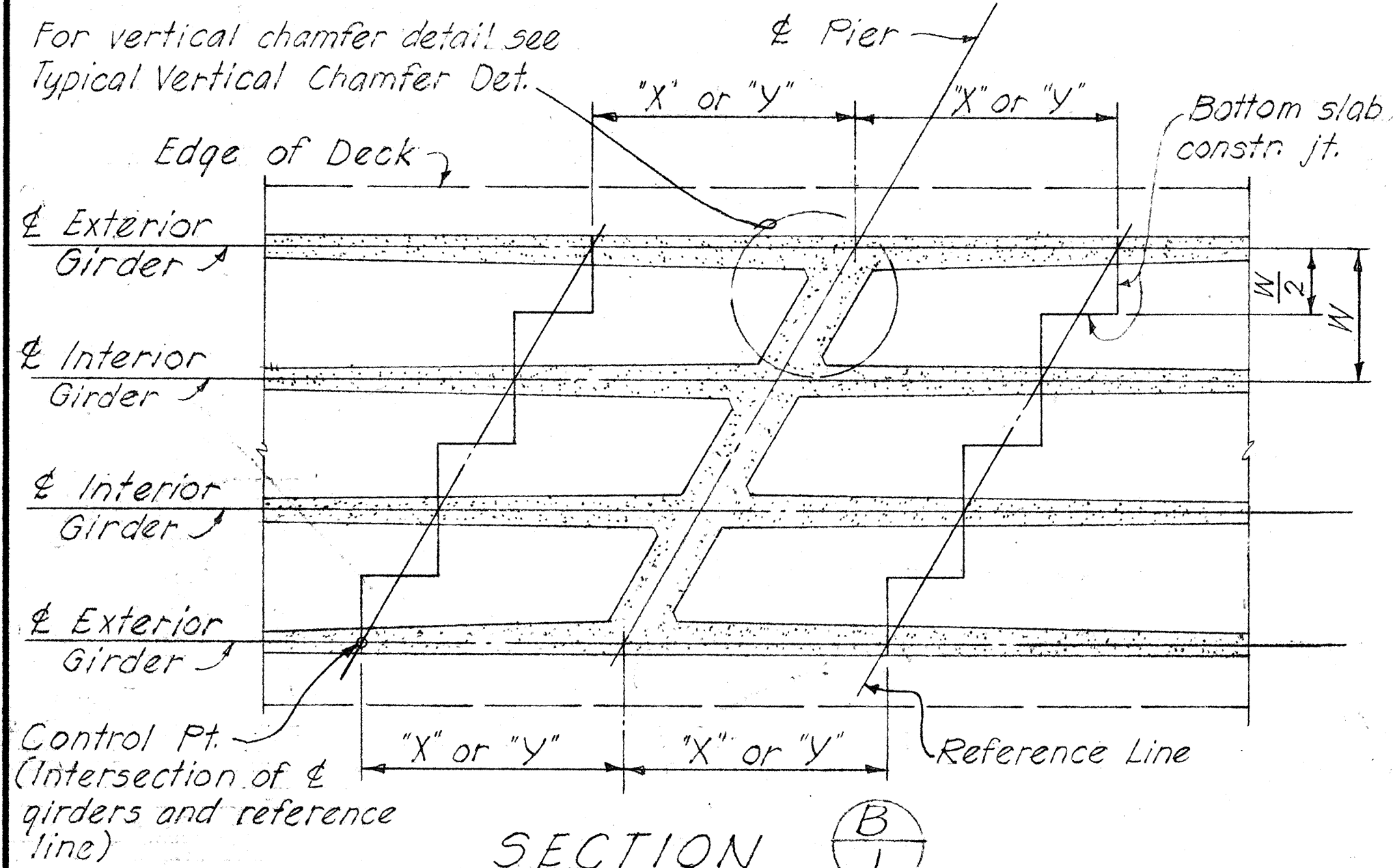
NO.	REVISION	APPROVED BY	DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION
STANDARD DETAILS
NOTES AND MISCELLANEOUS
DETAILS
Date: October 1969
SHEET No. 525 OF 34 SHEETS DB-100



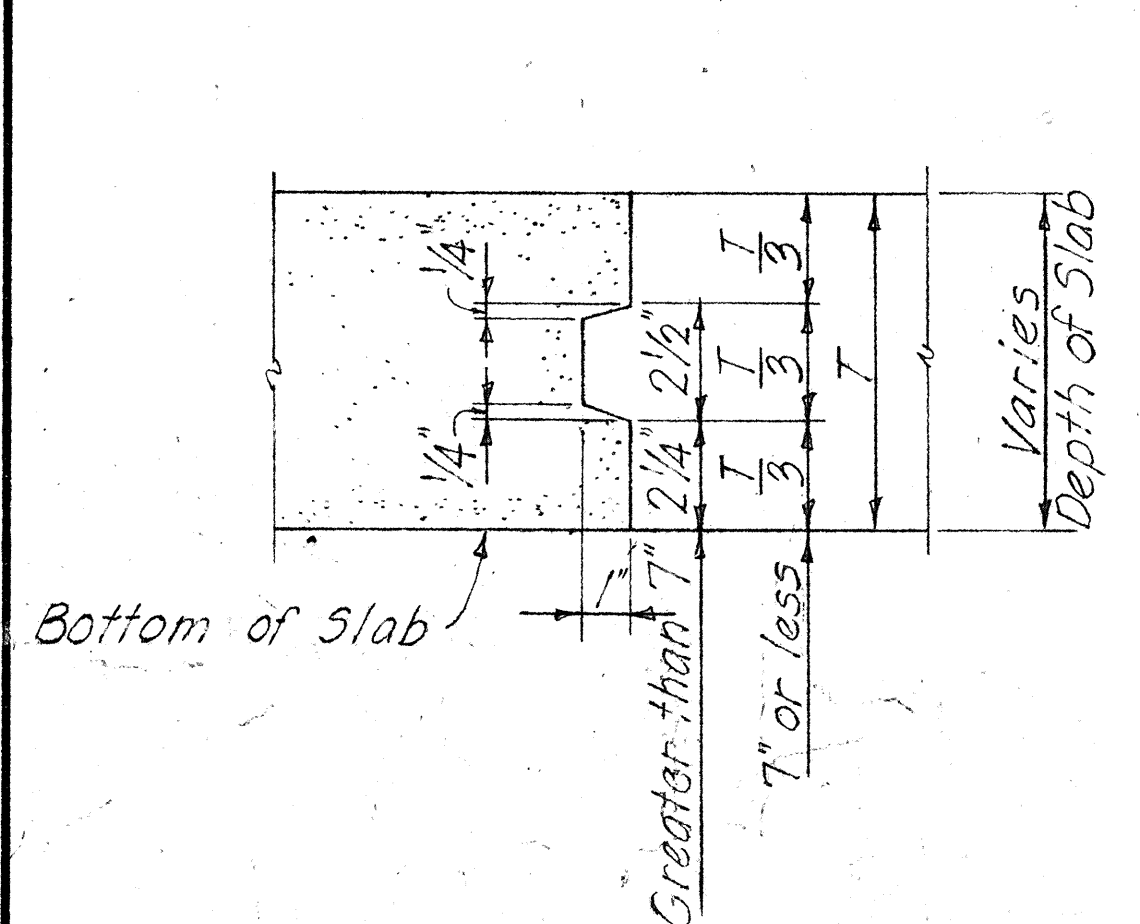
PLACING DIAGRAM FOR SUPERSTRUCTURE CONCRETE

- NOTE:**
- Numbers ① & ② indicate sequence of placing bottom slab & girder stem concrete. ② may be placed with ① when approved by the Engineer provided that the ① sections adjoining the ② sections are placed in this pour. For any deviation from the above the Contractor shall submit a diagram to the Engineer for approval.
 - No top slab construction joint or form removal opening shall be located in the region over the piers as noted, nor in the supporting section of a hinged span.
 - When top slab is not poured continuously, and top slab forms are not removed, construction joint in top slab shall be located over an intermediate diaphragm.
 - Top slab concrete shall be placed separately from the stem. Top slab concrete may be placed continuously or in parts as approved by the Engineer, providing pour is not in conflict with the above notes.
 - Dimension "X" = $\frac{1}{4}$ span. "Y" = $\frac{1}{8}$ span.
 - No lapping of main bars shall be permitted at construction joints.



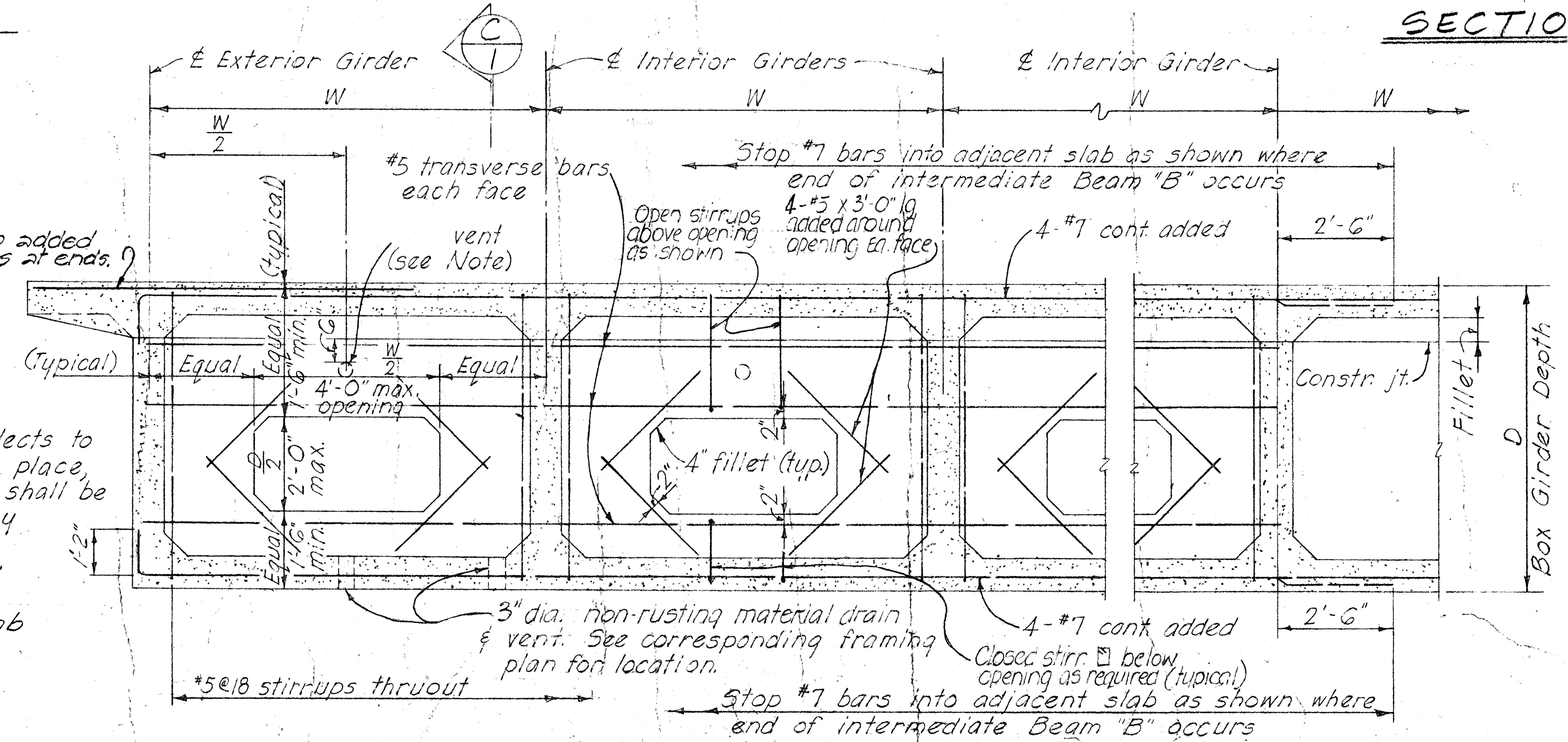
SECTION B

TREATMENT OF CONSTRUCTION JOINT BRIDGES ON SKEW

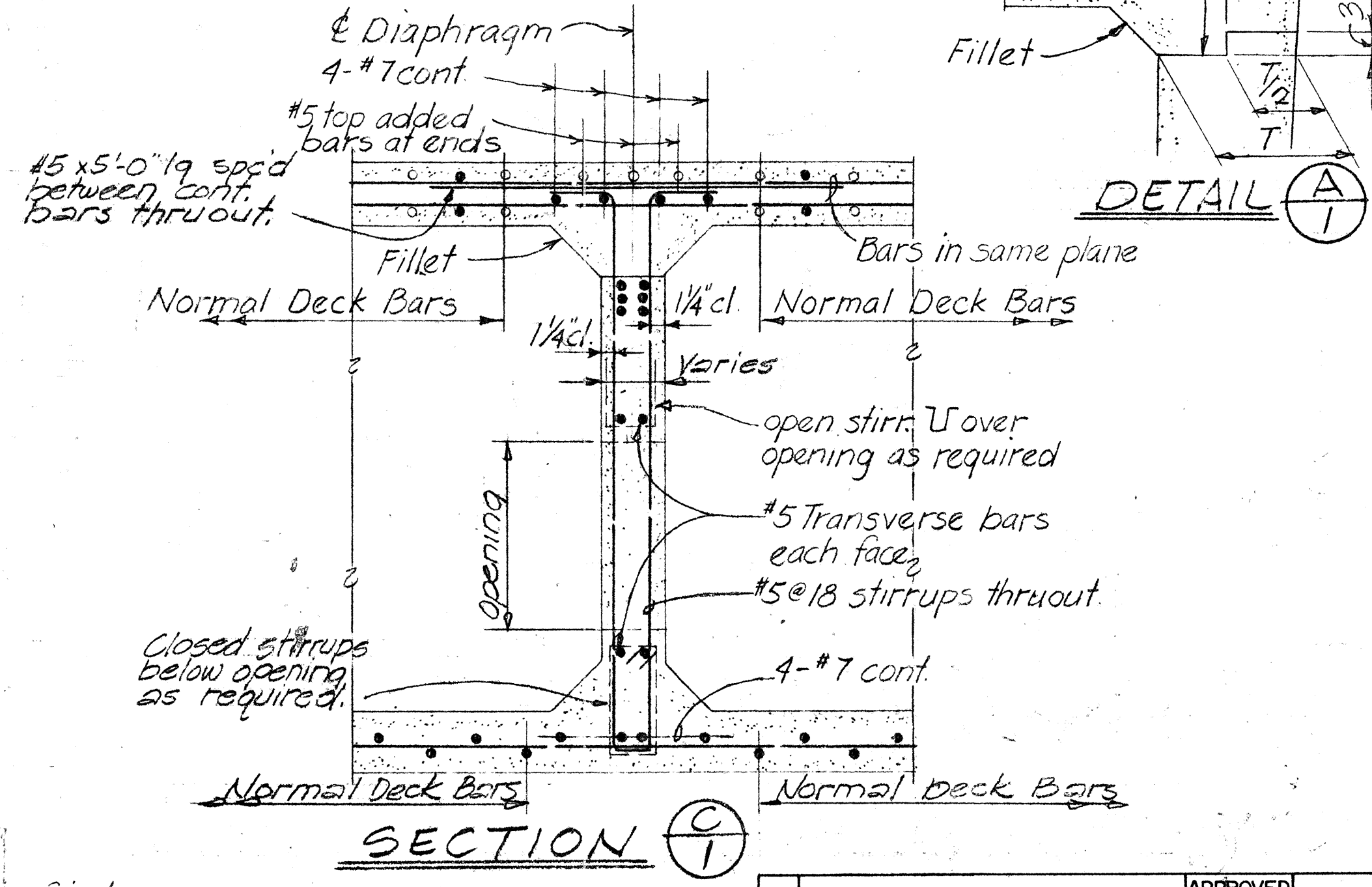


TYPICAL SLAB CONSTRUCTION JOINT DETAIL

- NOTE:**
- If the Contractor elects to leave top slab forms in place, form removal opening shall be omitted and replaced by 3" vent.
- No vertical shoring shall be placed in the center half of top slab span W.



AT NON-CONTINUOUS ENDS TYPICAL INTERIOR SPAN



SECTION C

NO.	REVISION	APPROVED BY	DATE

APPROVAL RECOMMENDED:
Paul T. Uemashita
 BRIDGE DESIGN ENGINEER 1-23-70
 DATE

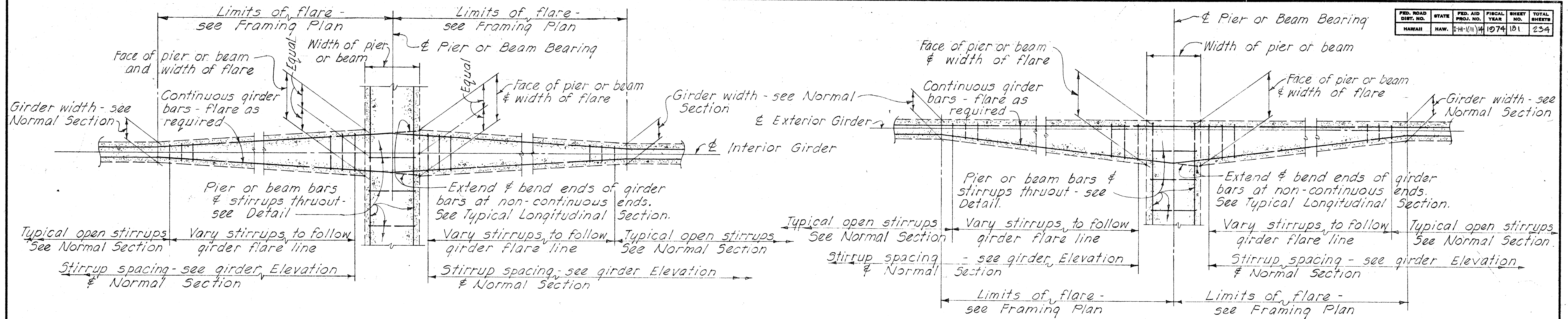
APPROVED:
Paul T. Uemashita
 ASSISTANT CHIEF, ENGINEERING 1-28-70
 DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STANDARD DETAILS

CONCRETE BOX GIRDER

Date: _____

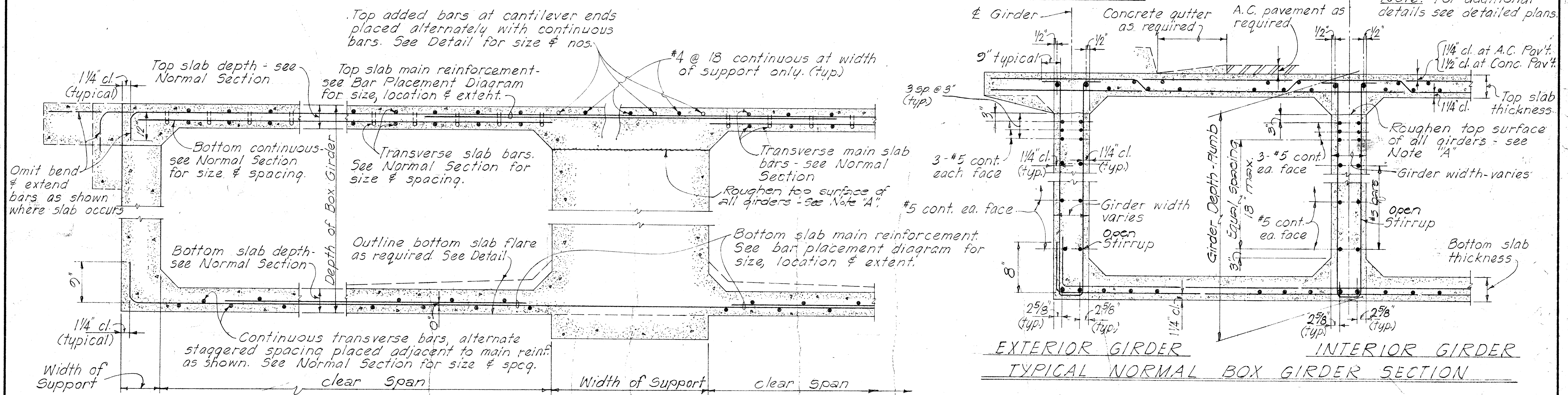


INTERIOR GIRDER

EXTERIOR GIRDER

TYPICAL DETAILS OF GIRDER FLARE AT NORMAL OR RADIAL SUPPORTS - CONTINUOUS ENDS AS SHOWN

(NON-CONTINUOUS ENDS SIMILAR EXCEPT AS OTHERWISE NOTED)



AT NON-CONTINUOUS END

AT CONTINUOUS SUPPORT

TYPICAL LONGITUDINAL DECK SECTION

ALONG GIRDER AT NORMAL OR RADIAL SUPPORTS

Note "A" The entire surface of the joint shall be thoroughly cleaned of surface laitance, curing compound, and other material foreign to the concrete and clean aggregate be exposed by abrasive blast cleaning after the curing period or immediately before placing concrete at the joint.

NO.	REVISION	APPROVED BY	DATE

APPROVAL, RECOMMENDED
Paul T. Yamashita 1-23-70
 BRIDGE DESIGN ENGINEER DATE

APPROVED
Paul T. Yamashita 1-28-70
 For ASSISTANT CHIEF, ENGINEERING DATE

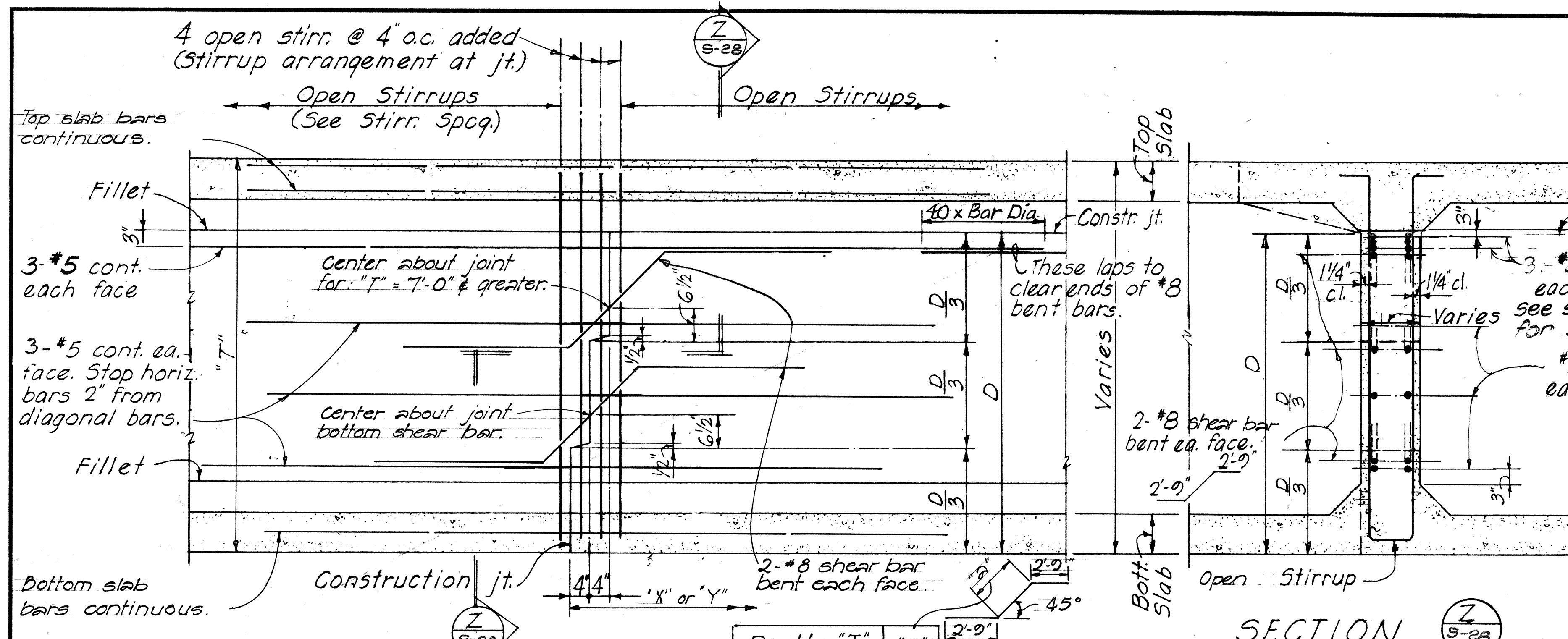
STATE OF HAWAII
 DEPARTMENT OF TRANSPORTATION
 HIGHWAYS DIVISION

STANDARD DETAILS

CONCRETE BOX GIRDER

Date

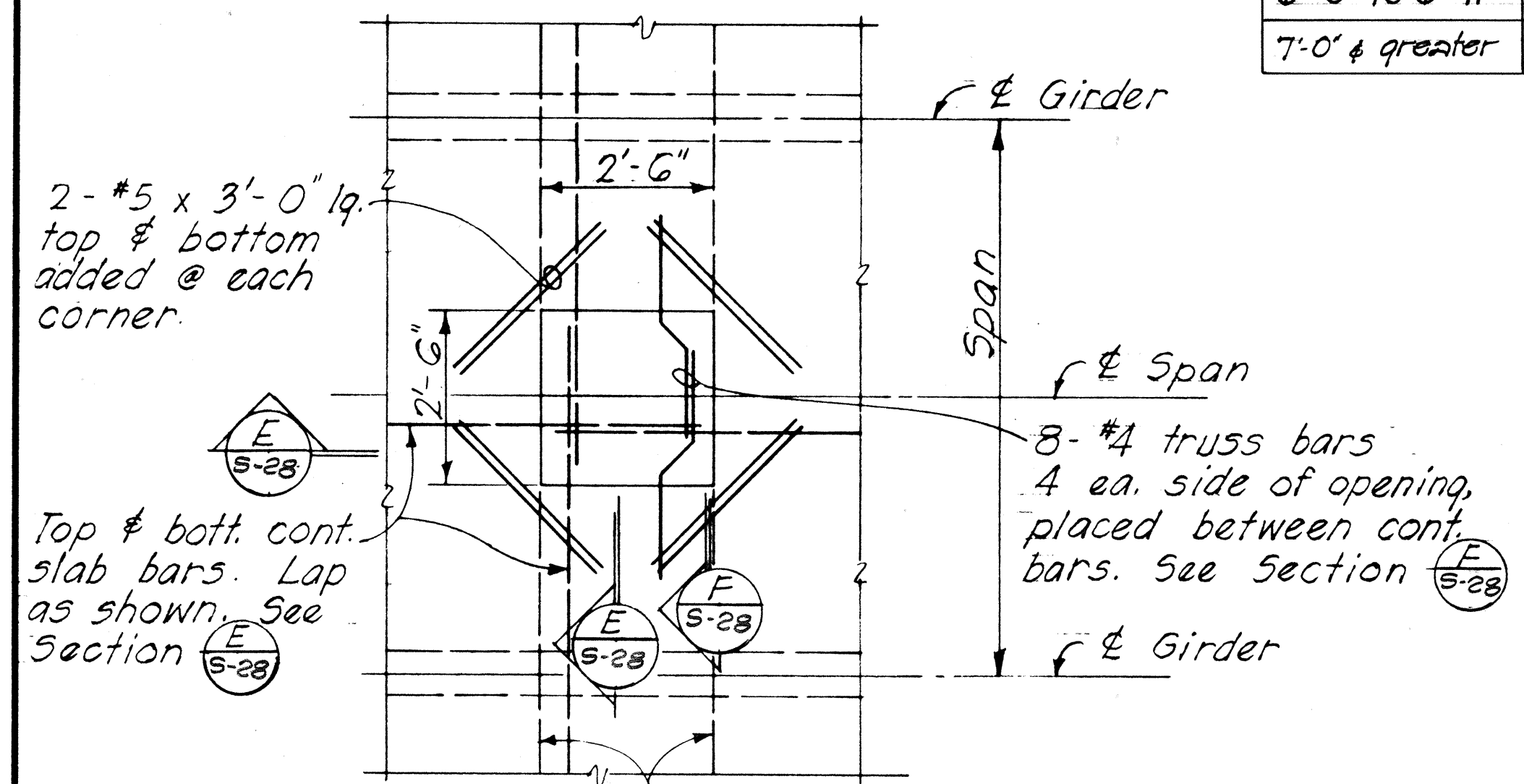
SHEET No. 27 OF 34 SHEETS DB-101-2



GIRDER STEM CONSTRUCTION JOINT

Depth "T"	"a"
4'-11" & smaller	1'-0"
5'-0" to 5'-11"	2'-0"
6'-0" to 6'-11"	2'-3"
7'-0" & greater	3'-0"

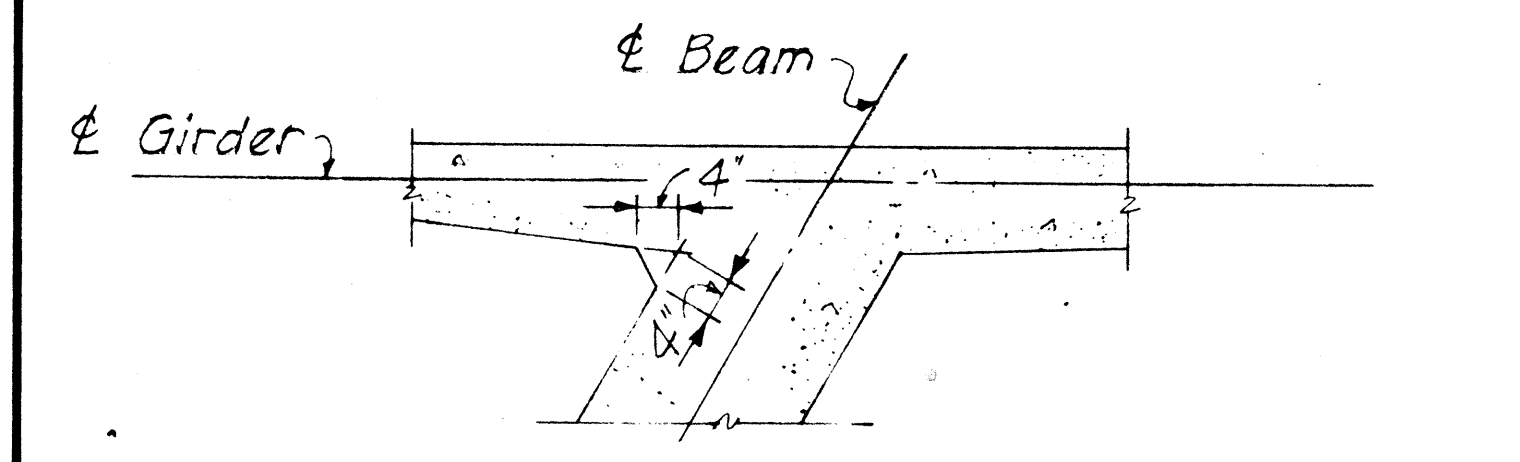
NOTE: D = Depth of girder stem to bott. of slab.



Note: Final location of opening shall be determined by the Engineer.

Note: Form removal opening may extend from edge of deck with approval of the Engineer. Lap small longitudinal deck bars as shown on Section E-28.

PART PLAN - DECK SHOWING FORM REMOVAL OPENING



TYPICAL DETAIL OF VERTICAL CHAMFER AT ACUTE INTERSECTION OF SKEWED CONCRETE MEMBER

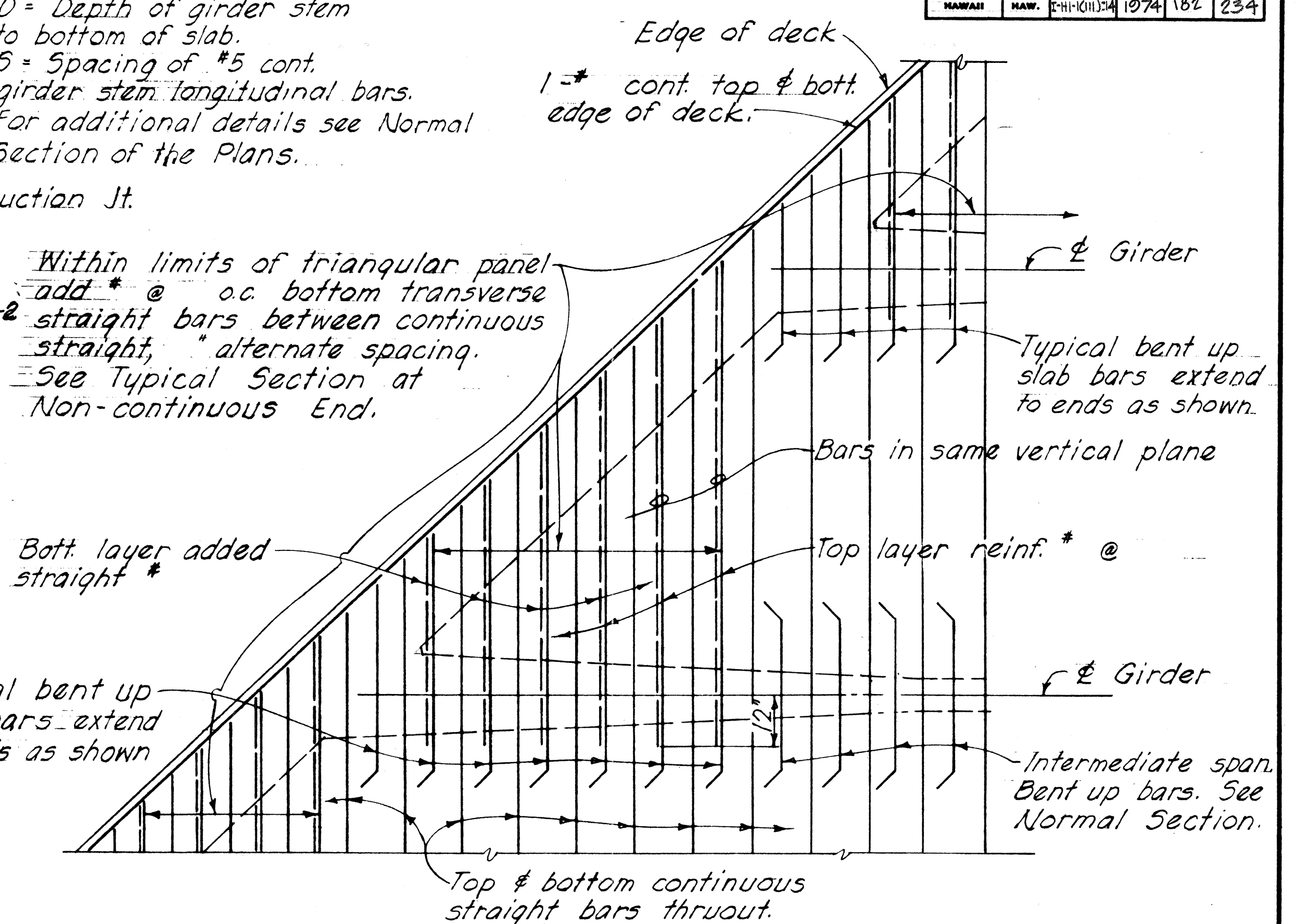
NOTE:
D = Depth of girder stem to bottom of slab.
S = Spacing of #5 cont. girder stem longitudinal bars.
For additional details see Normal Section of the Plans.

Within limits of triangular panel add * @ o.c. bottom transverse straight bars between continuous straight, "alternate spacing." See Typical Section at Non-continuous End.

Bott. layer added straight *

Typical bent up slab bars extend to ends as shown

PART PLAN - TYPICAL ARRANGEMENT OF TOP SLAB REINFORCEMENT AT SKEWED ENDS



APPROVAL RECOMMENDED: Paul T. Yamechita 1-23-70
BRIDGE DESIGN ENGINEER DATE

APPROVED: Paul T. Yamechita 1-28-70
For ASSISTANT CHIEF, ENGINEERING DATE

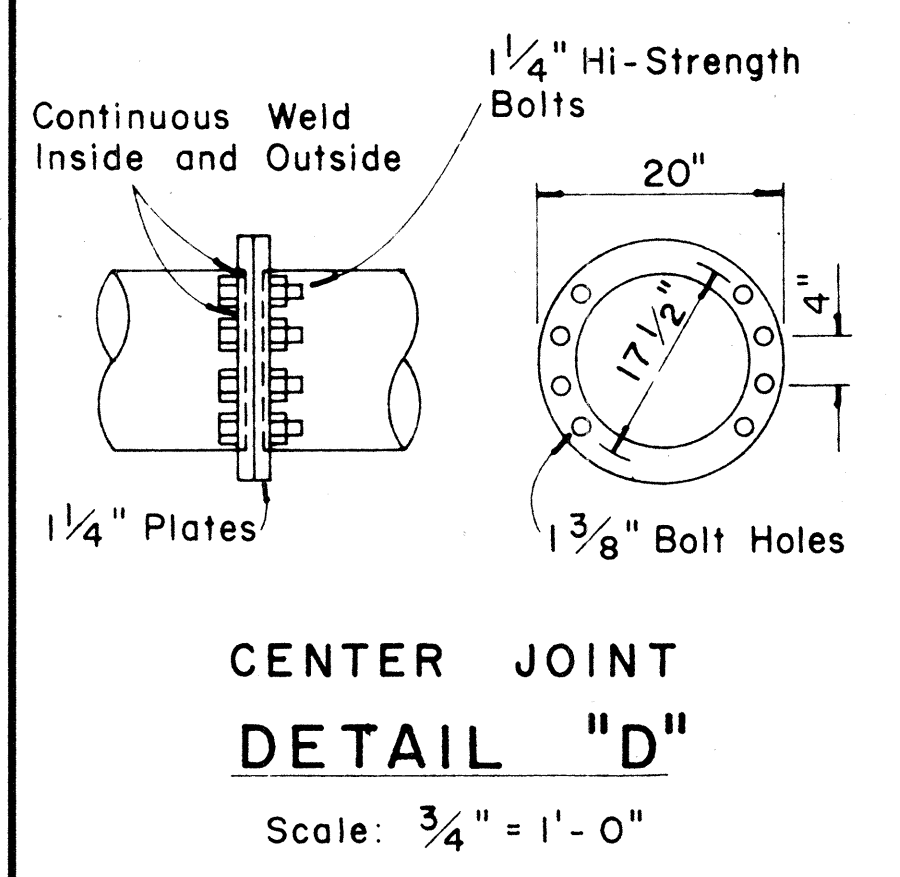
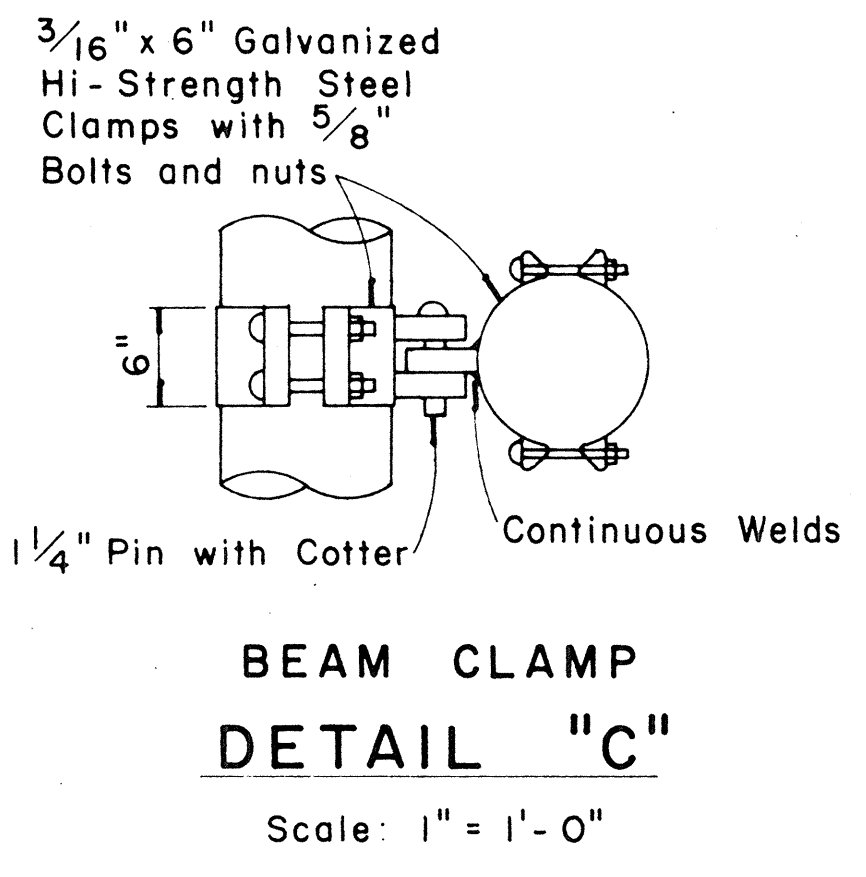
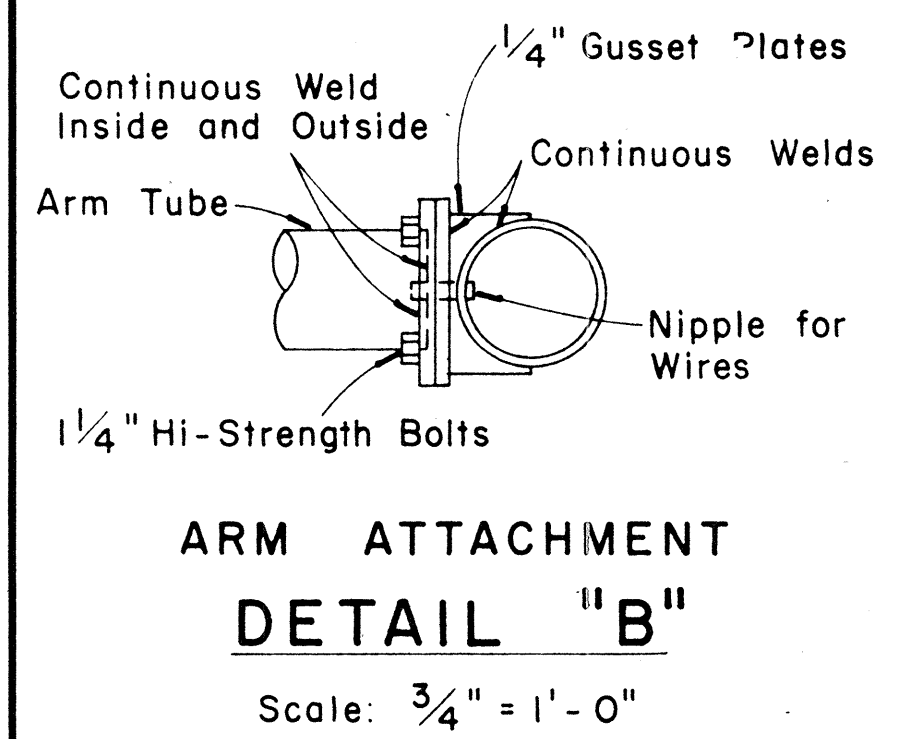
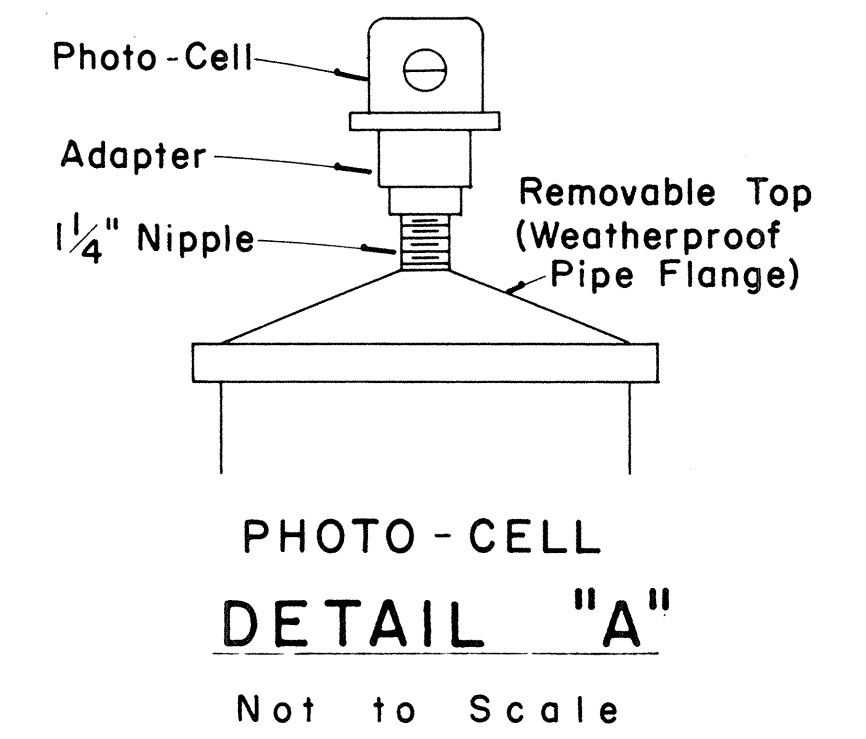
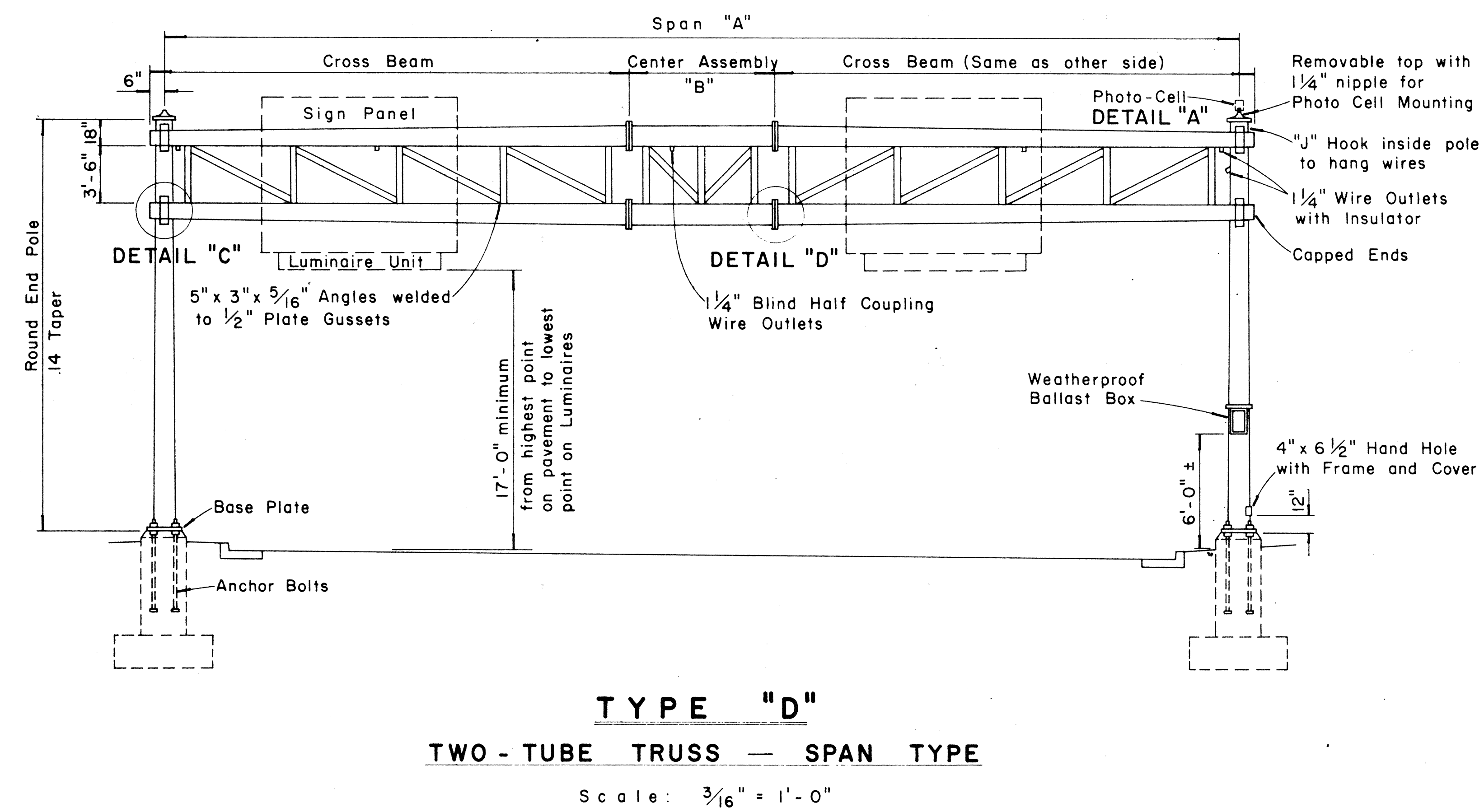
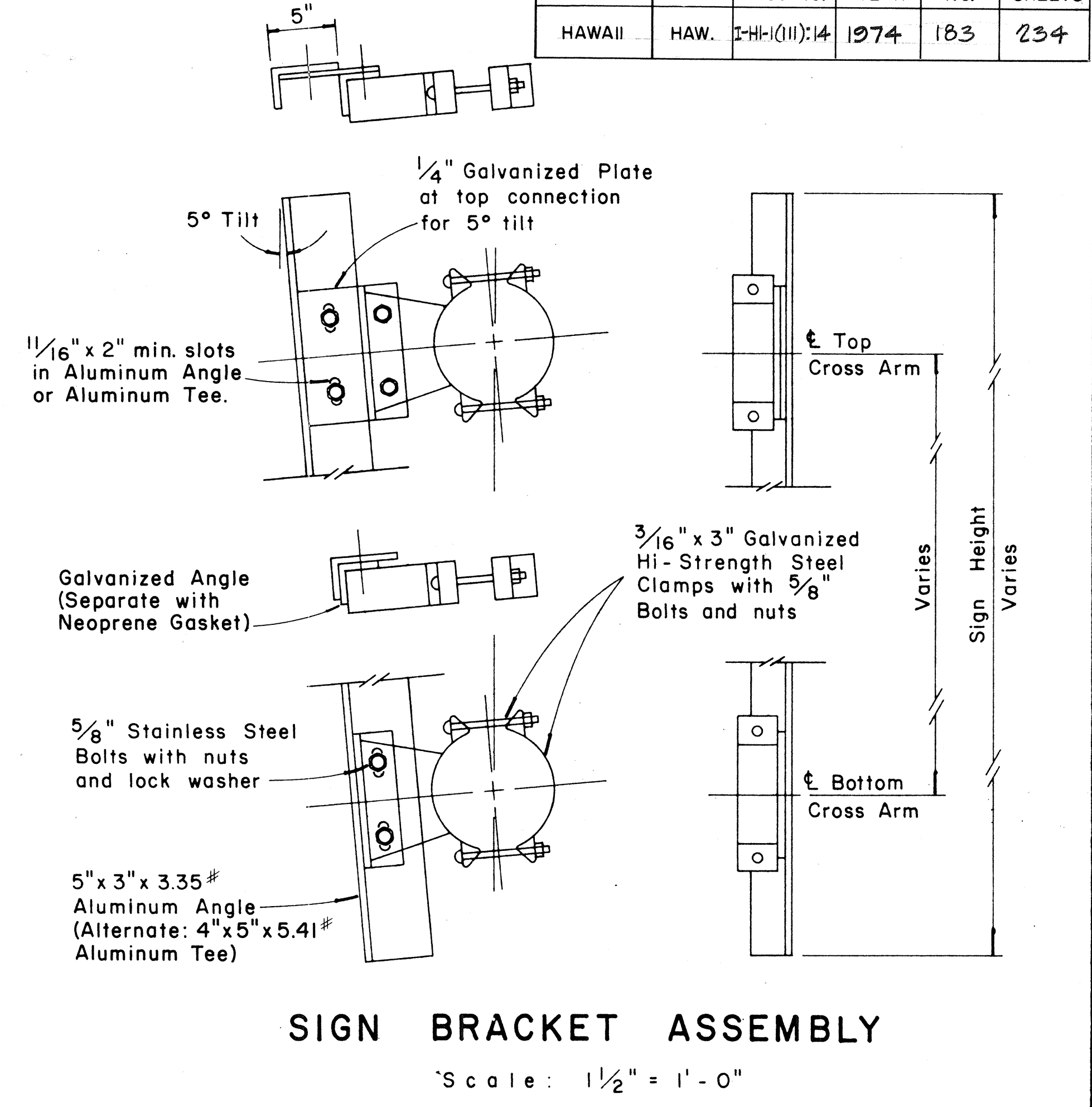
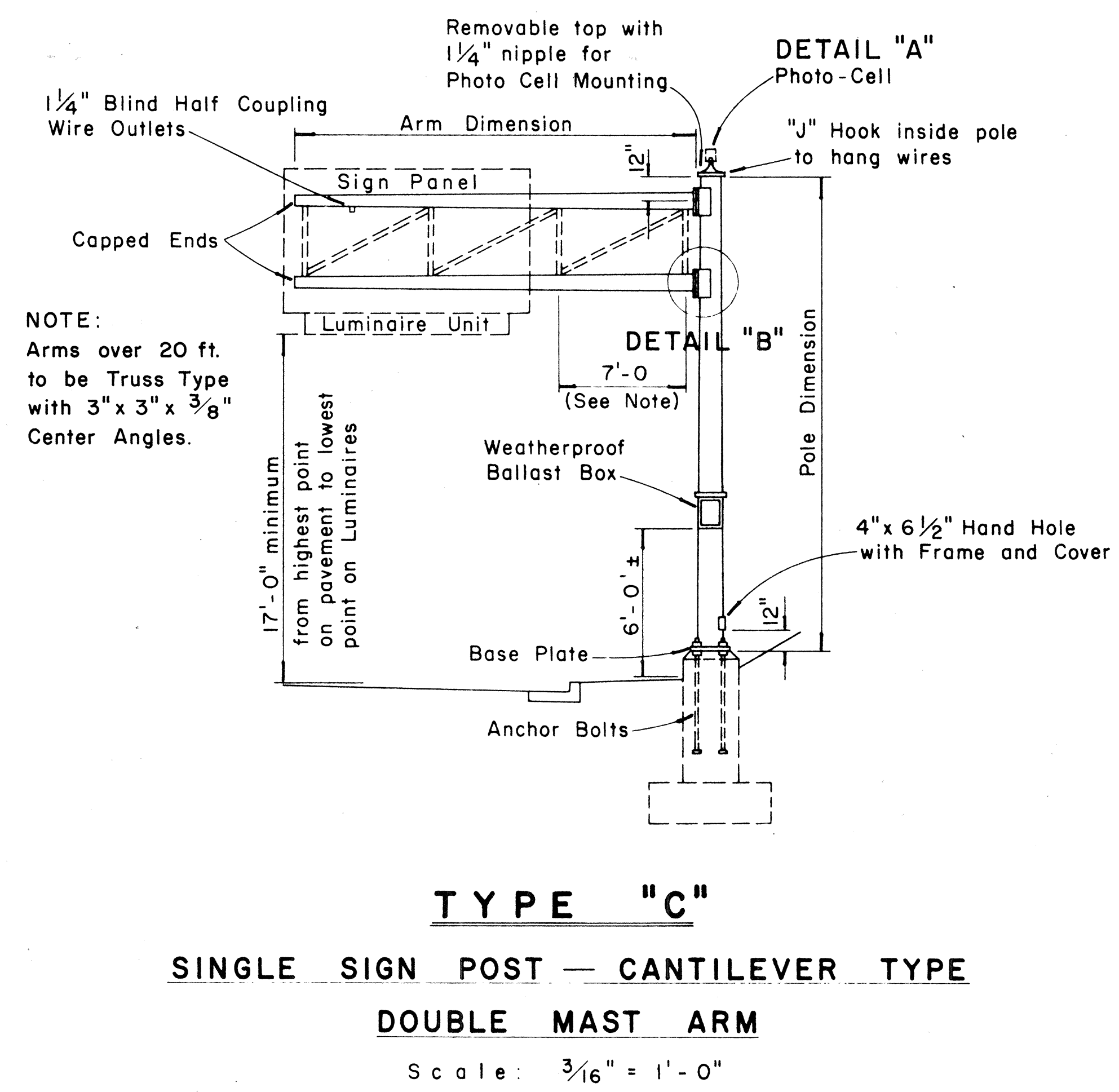
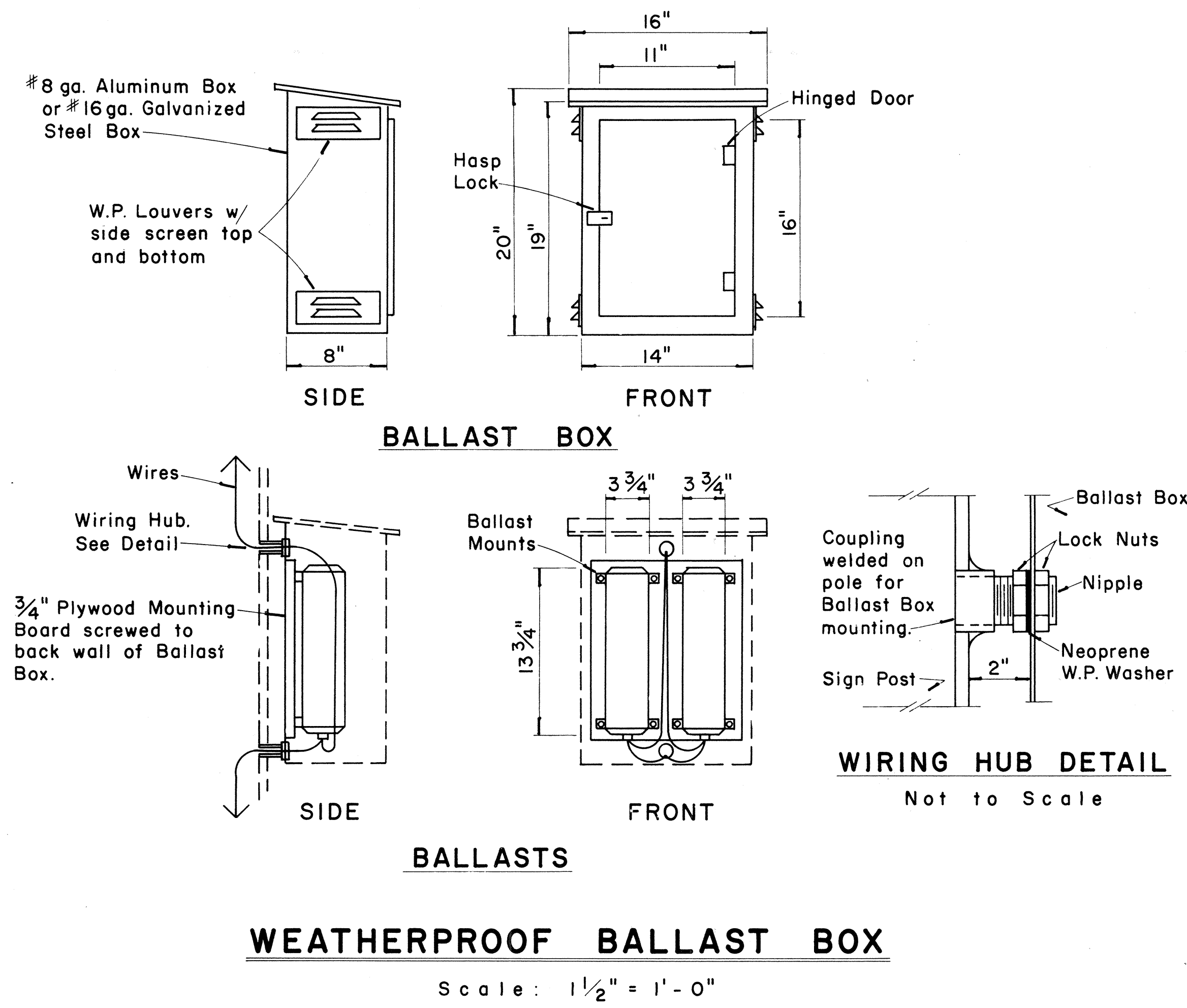
NO.	REVISION	APPROVED BY	DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STANDARD DETAILS
CONCRETE BOX GIRDER

SCALE AS NOTED DATE: _____

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-H-1(11):14	1974	183	234



APPROVAL	RECOMMENDED:	DATE
	<i>Eishi Tanaka</i>	12/29/69
	TRAFFIC ENGINEER	
APPROVED:		DATE
	<i>Alvin S. S. S.</i>	12-30-69
	ASSISTANT CHIEF, ENGINEERING	

NO.	REVISION	APPROVED BY	DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

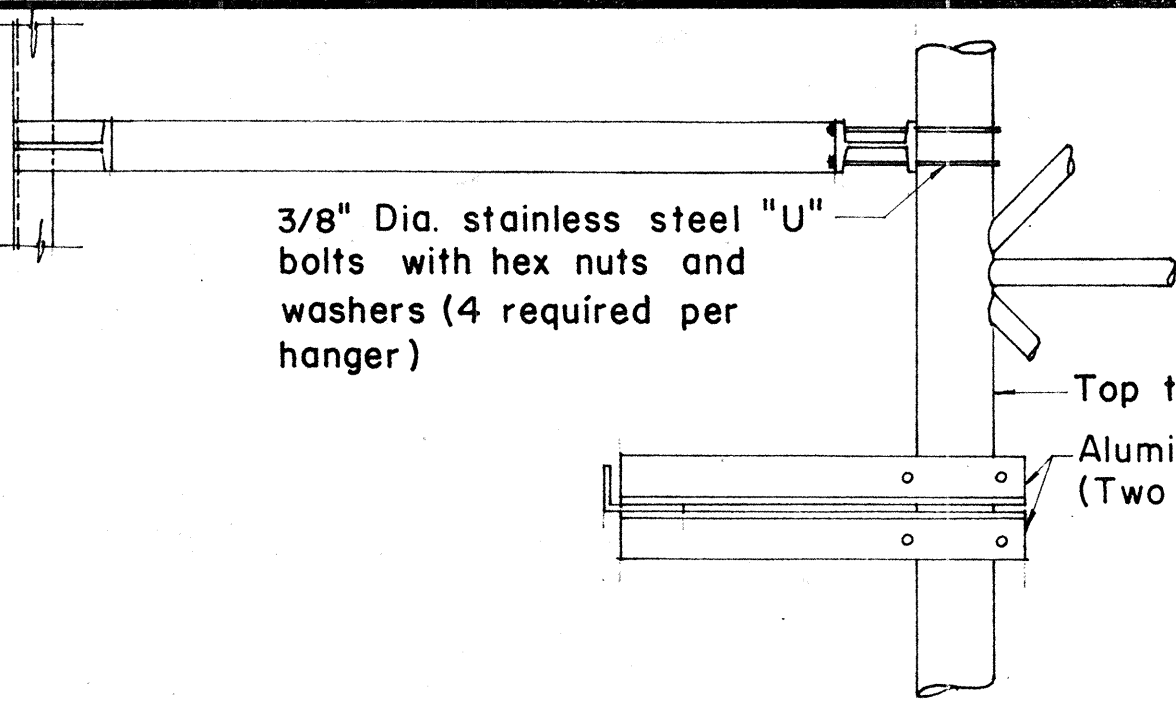
STANDARD DETAILS
OVERHEAD SIGN SUPPORTS

Scale: As Shown

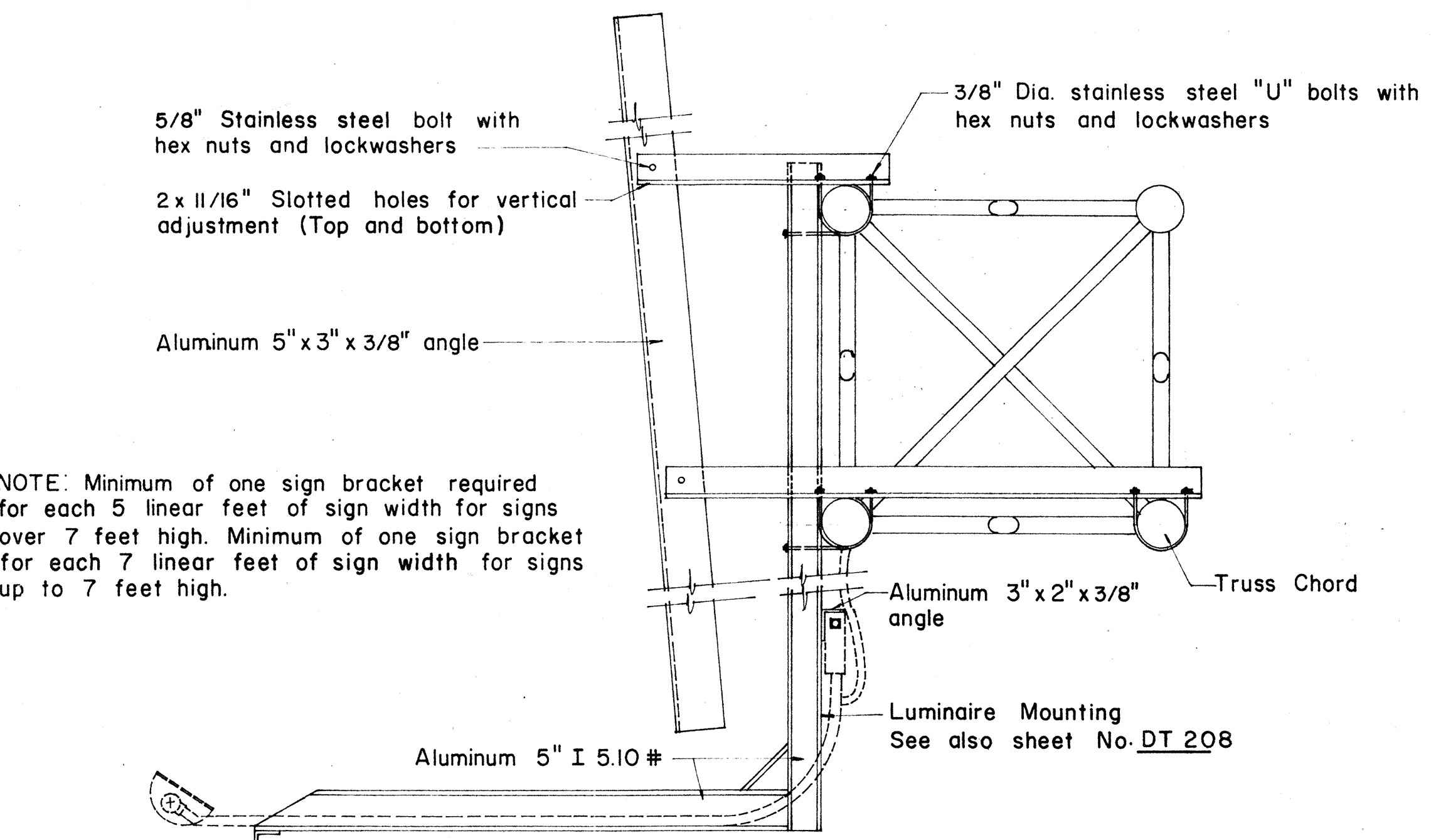
SHEET No. 529 OF 34 SHEETS DT 200

SURVEY PLOTTED BY	DATE
DRAWN BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-HH(III)-14	1974	184	234



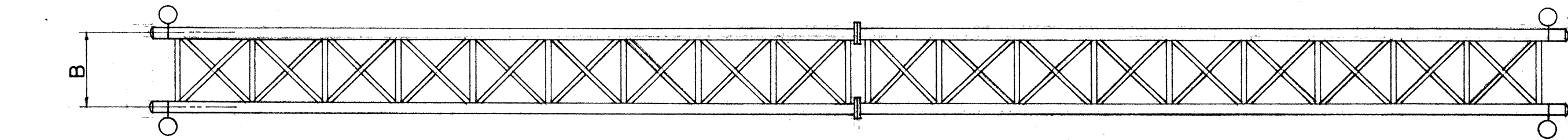
PLAN



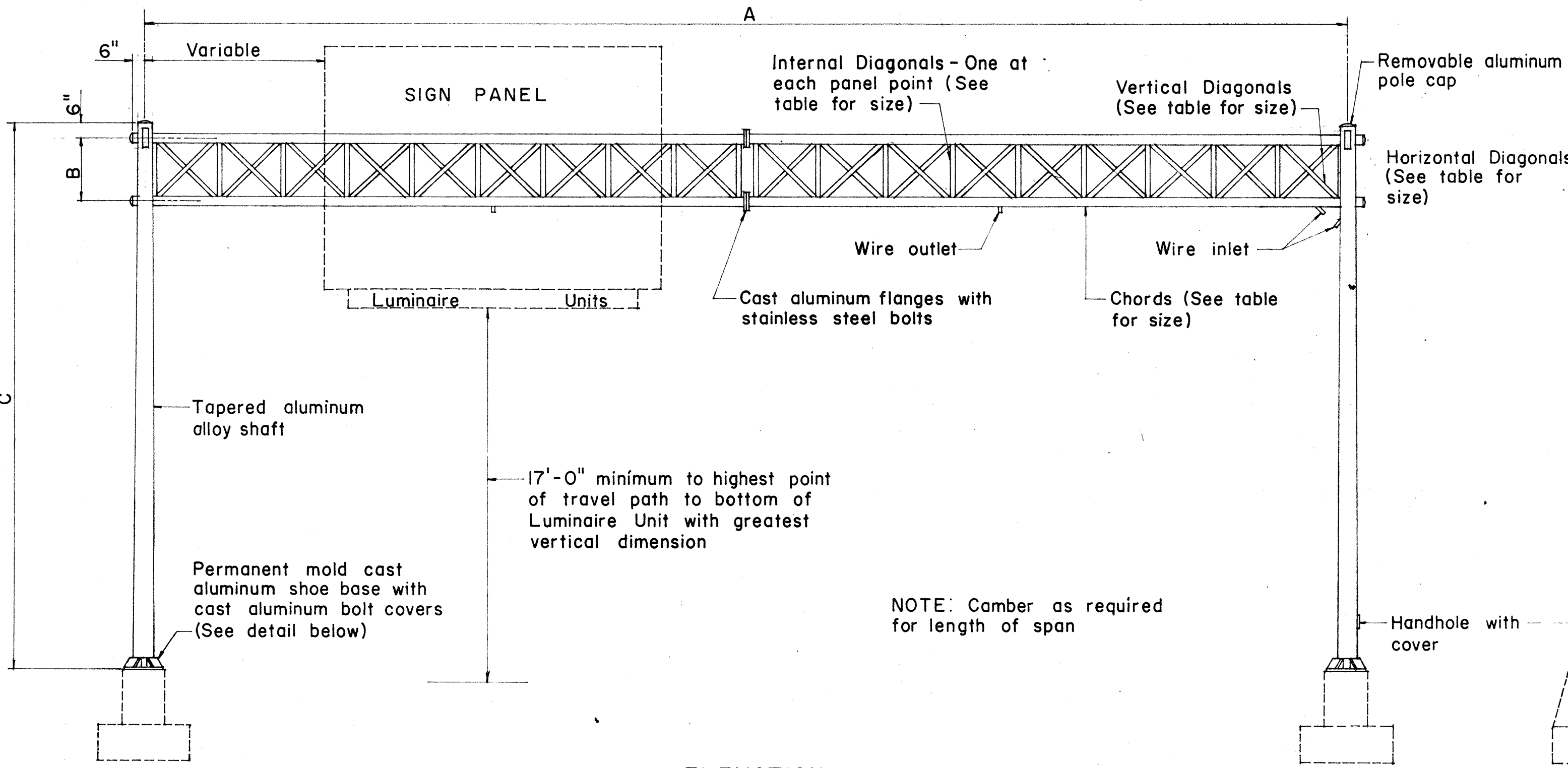
NOTE: Minimum of one sign bracket required for each 5 linear feet of sign width for signs over 7 feet high. Minimum of one sign bracket for each 7 linear feet of sign width for signs up to 7 feet high.

ELEVATION

SIGN BRACKET ASSEMBLY

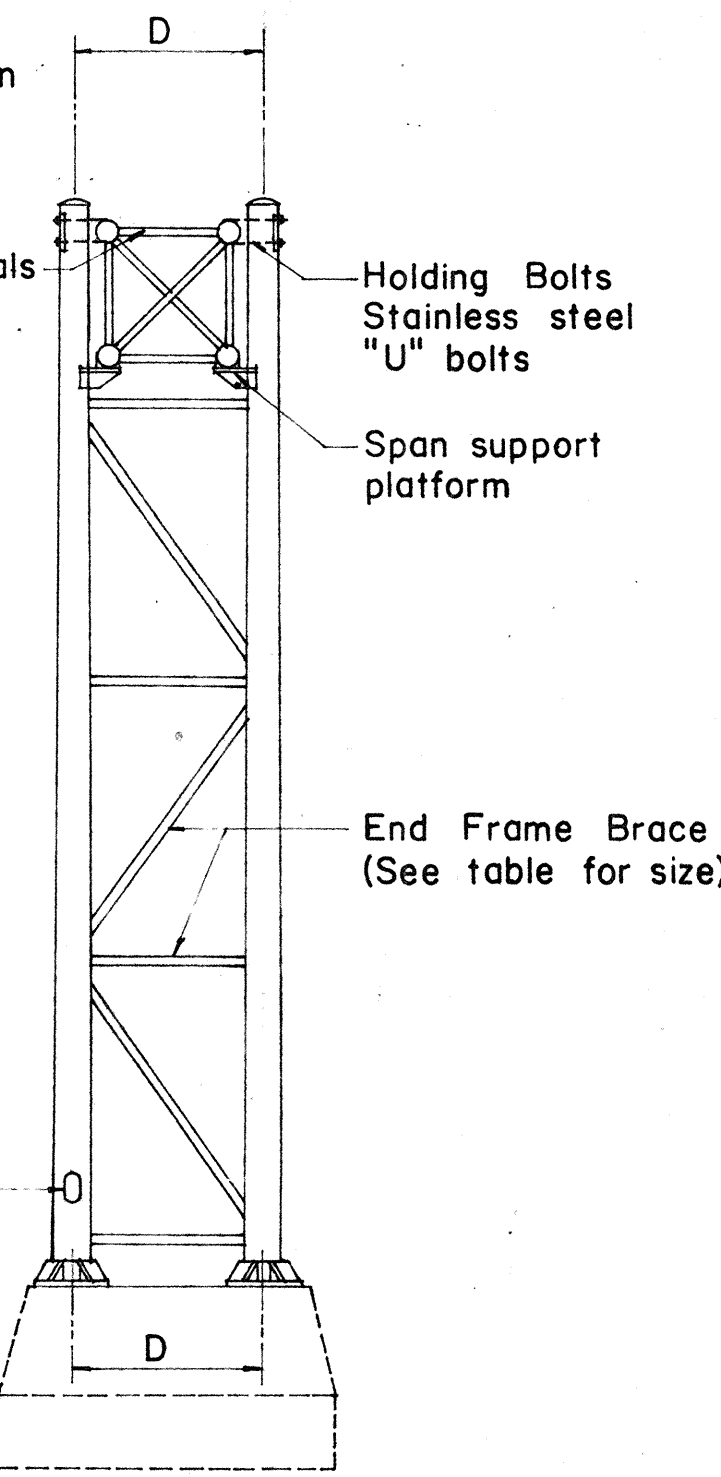


PLAN



ELEVATION

TYPE "F"



END VIEW

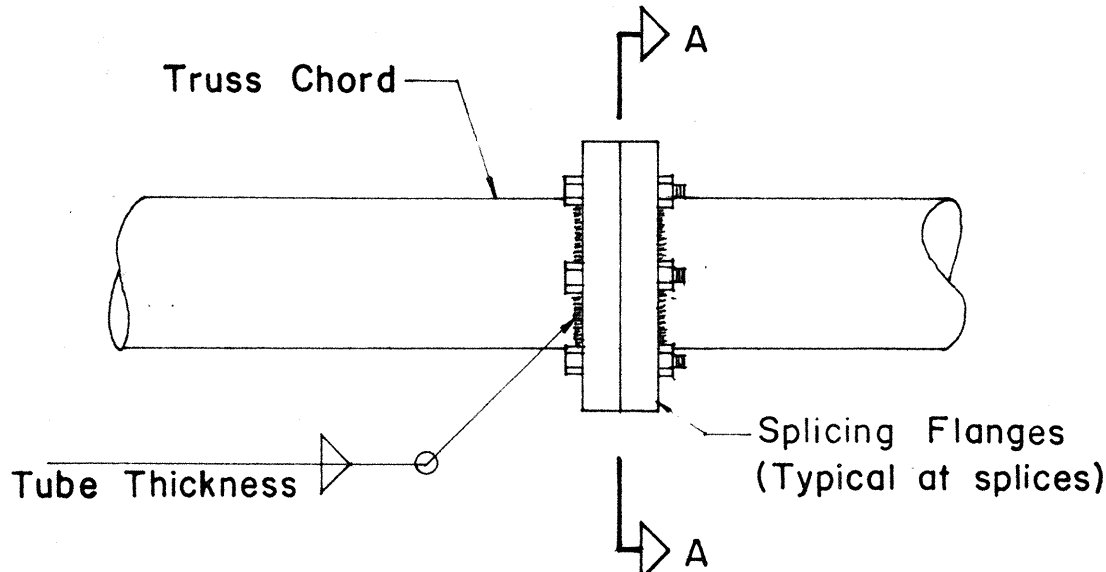
SIGN NO.	A	B	C	D	E	F	SPAN SECTIONS REQ'D.	CHORD SIZE	END FRAME SHAFT SIZE AND TAPER	VERTICAL DIAGONAL SIZE	HORIZONTAL DIAGONAL SIZE	INTERNAL DIAGONAL SIZE	END FRAME BRACE SIZE	BOLT CIRCLE DIA.	ANCHORAGE

APPROVAL RECOMMENDED:

Eichi Tanaka 12/29/69
TRAFFIC ENGINEER DATE

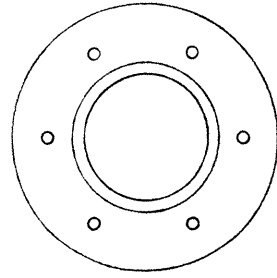
APPROVED:

John J. Suter 12-30-69
ASSISTANT CHIEF, ENGINEERING DATE

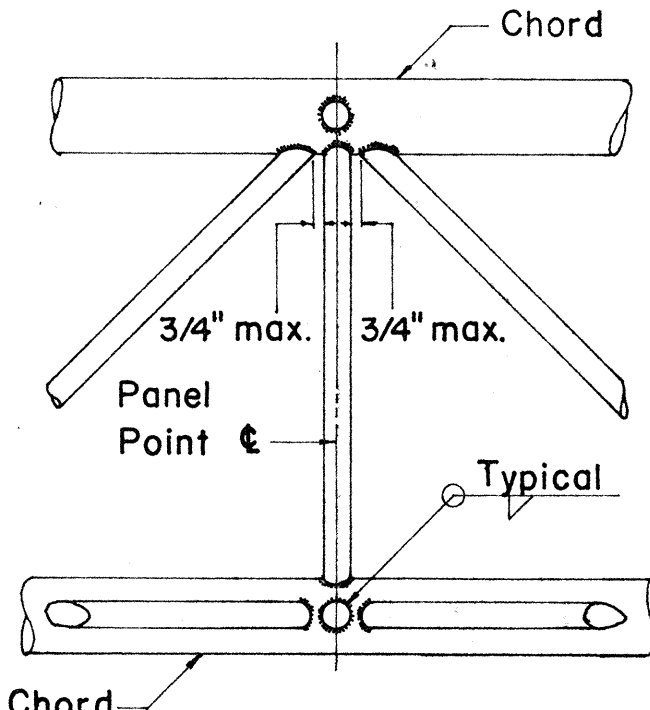


PLAN

DETAIL OF CHORD SPLICES

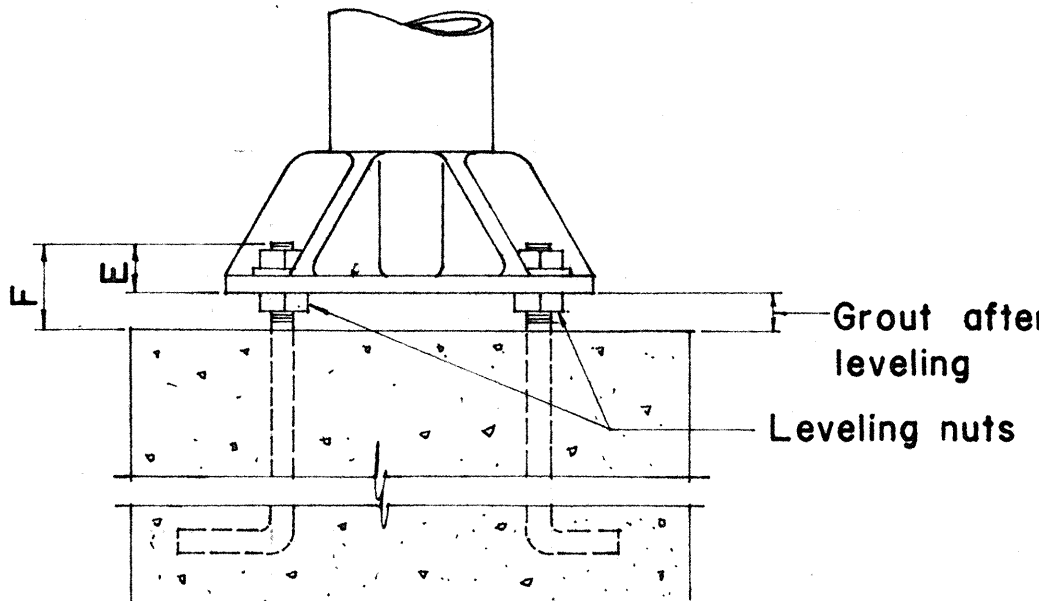


SECTION "A"- "A"



NOTE: Fillet weld size to be tube thickness plus 1/16".

TYPICAL JOINT DET.



DETAIL OF BASE

NO.	REVISION	APPROVED BY	DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

TYPE "F"
OVERHEAD SIGN SUPPORT
BOX TRUSS TYPE
ALUMINUM

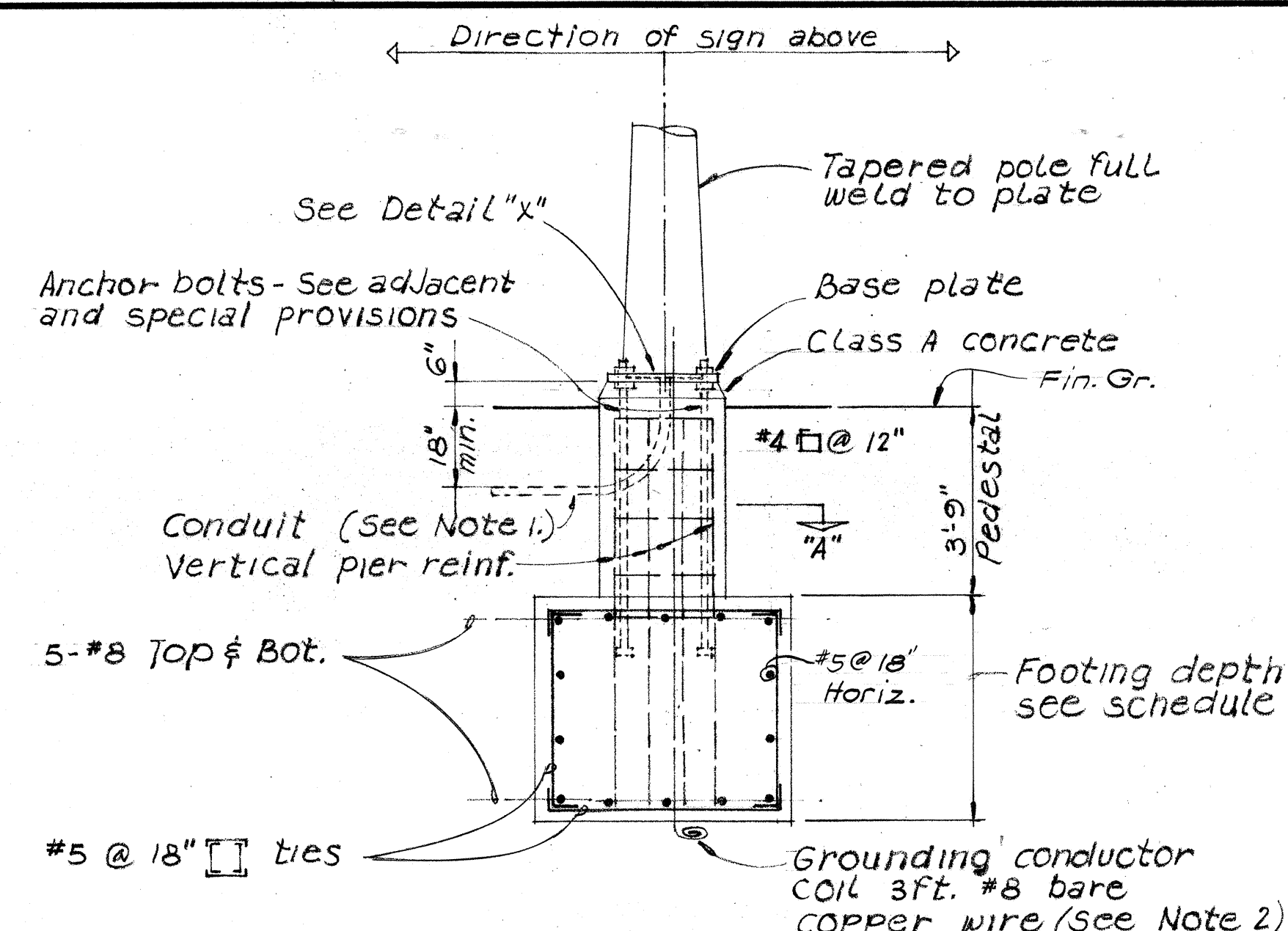
SHEET No. 30 OF 34 SHEETS DT 201

DATE	
DESIGNED BY	
CHECKED BY	
NO.	

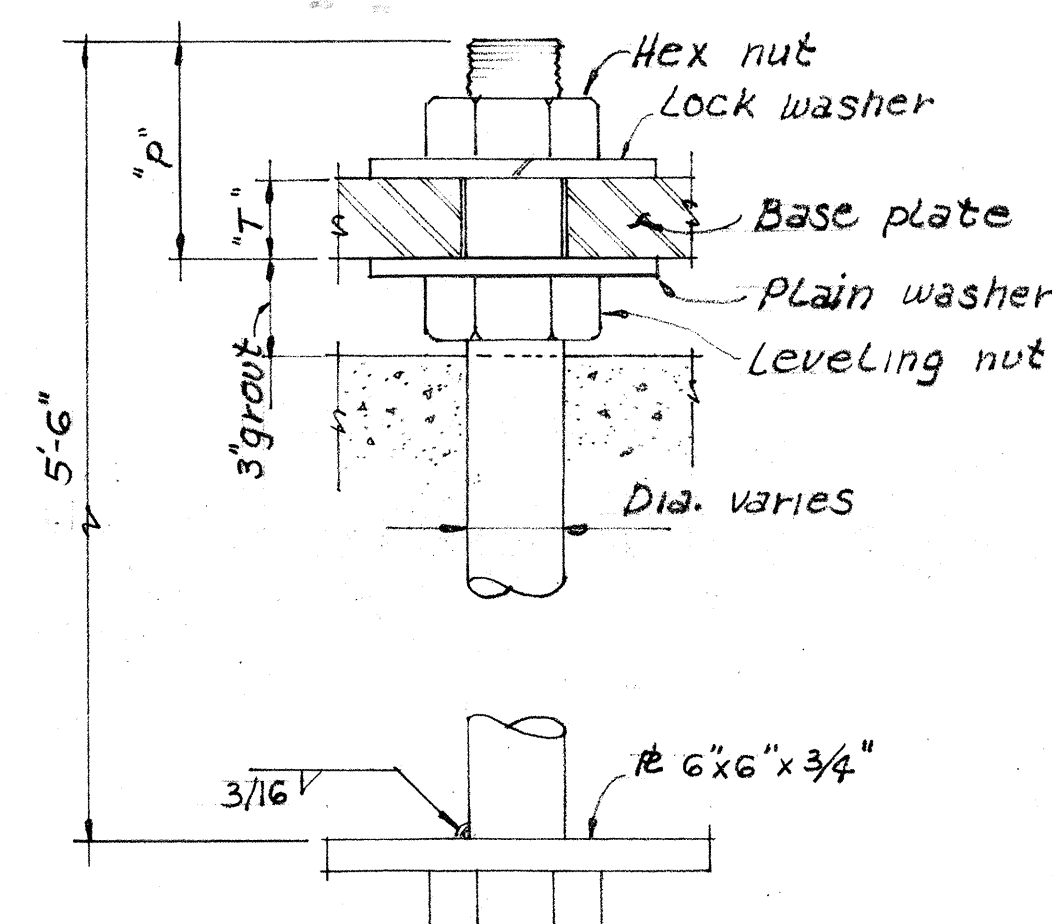
FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-HI-1(III):14	1974	185	234

NOTES

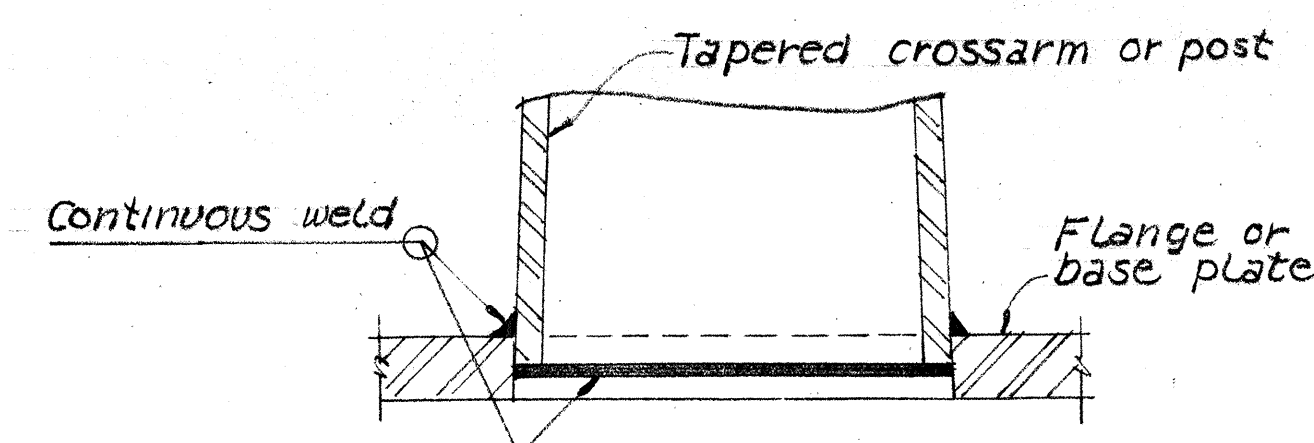
1. Total number of conduits into pole foundation as indicated on Lighting plans. Conduits shall slope away from pole foundation. Conduit shall stub out 5'-0" min. for direct burial entry.
2. Grounding conductor shall be bonded to sign support.
3. Except specified otherwise, contractor shall determine the detail requirements of the pedestal, base plate, and anchor bolts in accord with type of sign support selected.
4. For exact location of signs and clearance requirements, see frame layout sheets.



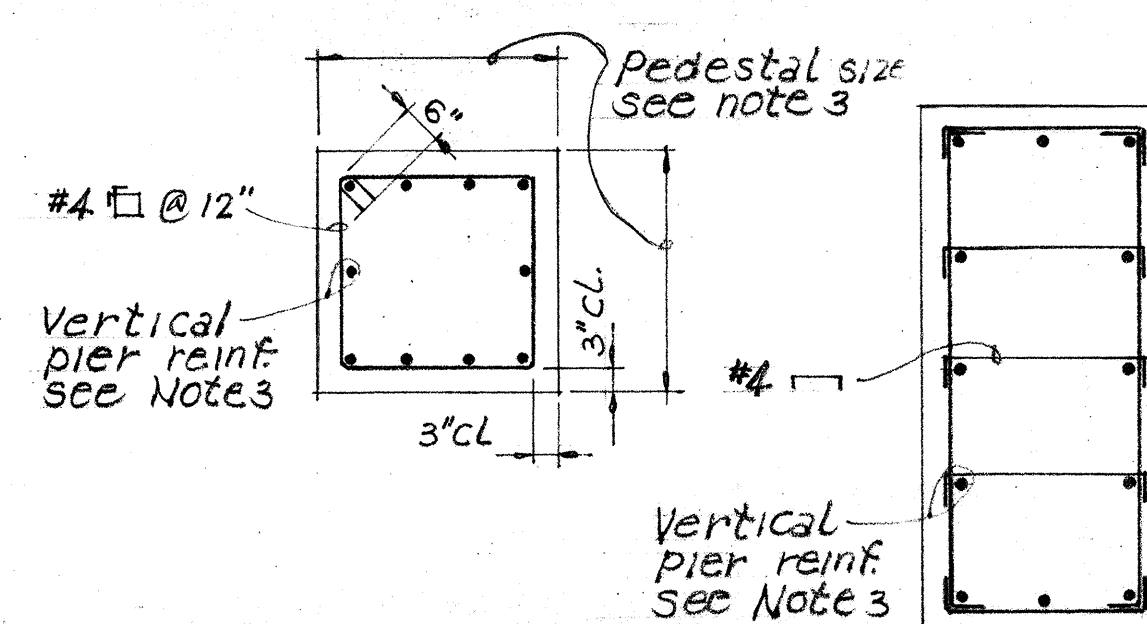
TYPICAL POLE FOUNDATION DETAIL



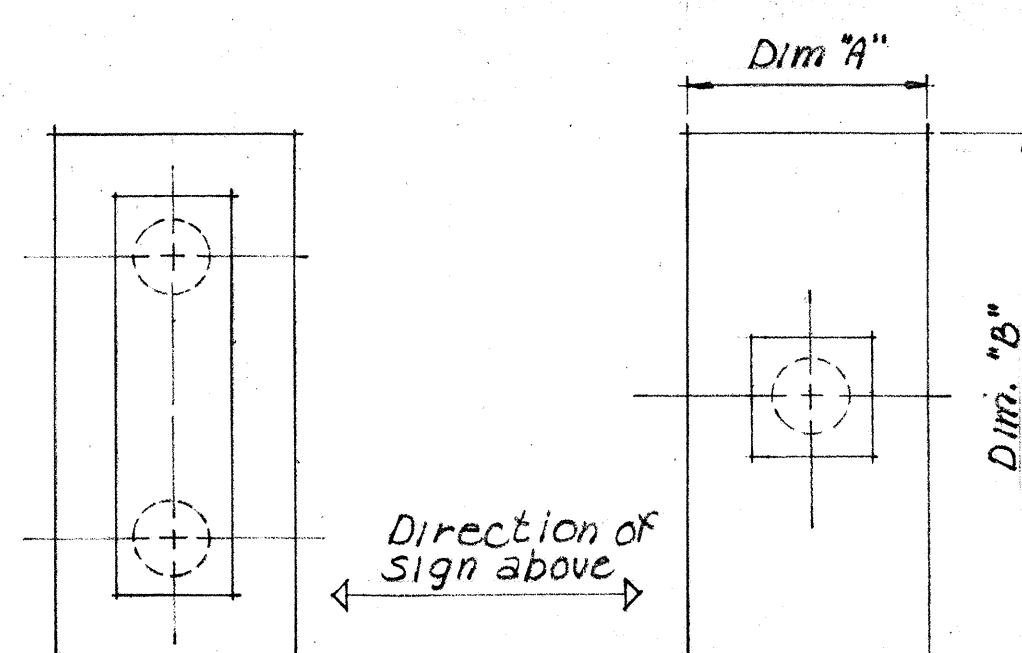
TYPICAL ANCHOR BOLT DETAIL



DETAIL "X"



SECTION "A"



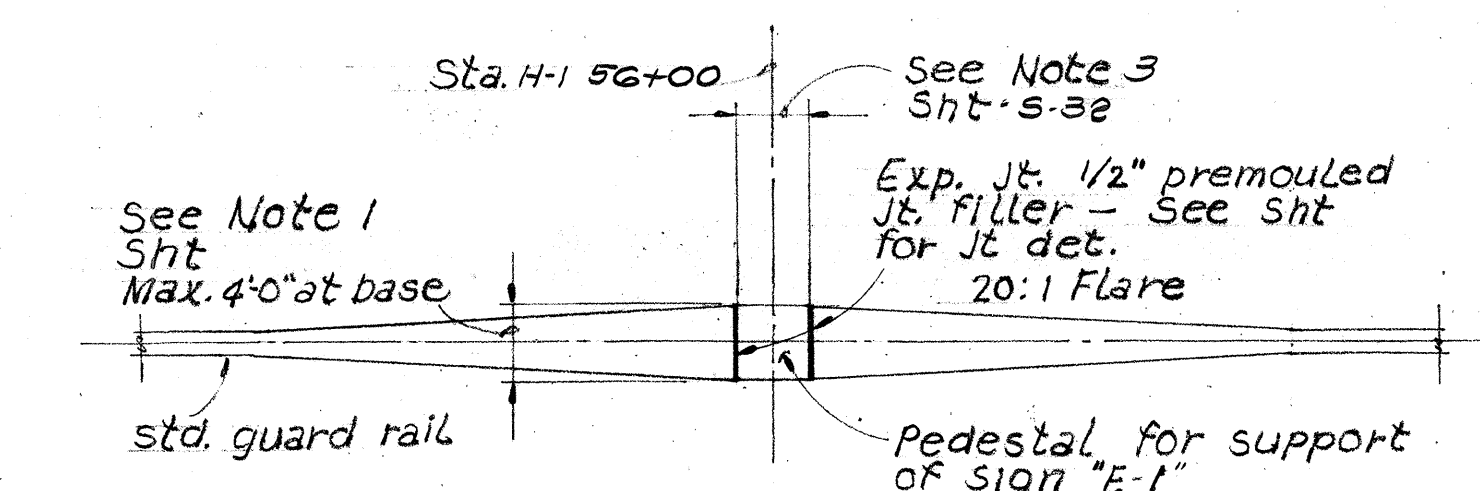
PEDESTAL PLAN

TYPICAL FOOTING DETAIL

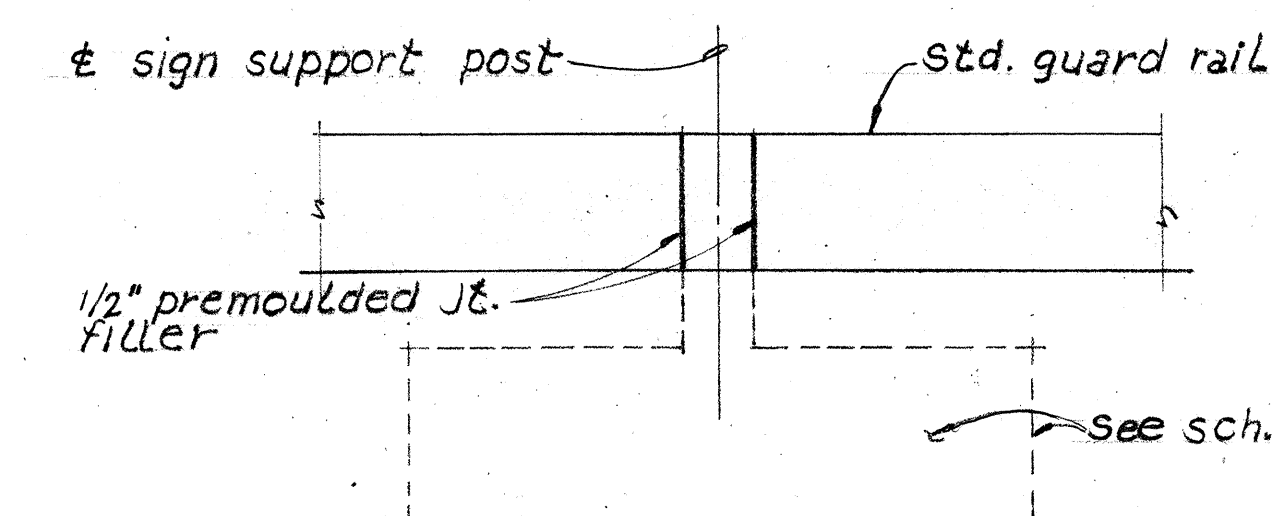
FOUNDATION SCHEDULE					
SIGN	SUPPORT TYPE	FOUNDATION SCH.			REMARKS
		A	B	DEPTH	
E-1	Type "D" or "F"	5'	14'	5'-6"	See note "a."
E-5	Type "D" or "F"	6'	15'	5'-6"	
E-6	Type "C"	8'	14'	6'-9"	
E-13	Standard Detail				

NOTE:

- a. Left side on Median Barrier. See "Pedestal at Conc. Guard Rail detail"



PLAN

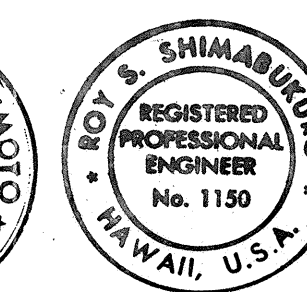
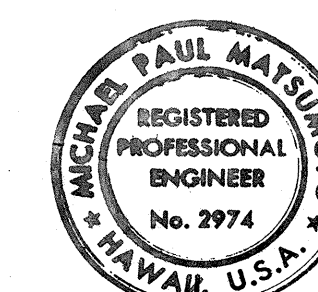


ELEV.

PEDESTAL AT CONC. GUARD RAIL

DATE	_____
SURVEY PLOTTED BY	_____
PLAN	_____
DETAILED BY	_____
NOTE BOOK	_____
QUANTITIES BY	_____
CHECKED BY	_____
No.	_____

SHIMAZU SHIMAMUKURO & FUKUDA, INC.
Consulting Structural Engineers
1210 Ward Ave., Honolulu, HI 96814



STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

STANDARD SIGN
SUPPORT FOUNDATION
DETAILS & SCHEDULES

PEARL HARBOR INTERCHANGE
F.A.I. Proj. No. I-HI-1(III):14

Date: DEC. 14, 1973

THIS WORK WAS PREPARED BY ME
OR UNDER MY SUPERVISION

SHEET NO. 5-31 OF 34 SHEETS

(OVERHEAD SIGNS)

SUPPORT SPACING TABLE

APPROVAL RECOMMENDED:
Eishi Tanaka 12/29/69
TRAFFIC ENGINEER DATE

NO.	REVISION	APPROVED BY	DATE

SHEET No. 9-32 OF 34 SHEETS DT 205



Aluminum 4" x 5" Tee 5.41 #

Sign Bracket

Aluminum Post Clip, Bolt, Nut and Washers

Aluminum Mounting Extrusion

Porcelain Enameled Laminated Panel

Support Tube

3/8"

Stainless Steel Flathead Bolt w/ Nut and Washer.
4 Bolts per Panel unless otherwise specified.

A schematic diagram of the experimental setup. It shows a long, narrow rectangular platform tilted at an angle. A label '5° Tilt' with an arrow indicates the angle of the platform. Along the length of the platform, there are several small rectangular blocks, each with a circle on top. At the top of the platform, there is a larger rectangular block with a circle on top, and a line with an arrow pointing to it. At the bottom of the platform, there is another larger rectangular block with a circle on top, and a line with an arrow pointing to it. A scale bar at the bottom indicates a length of 100 cm.

Aluminum
5" x 3" Angle 3.35#

$\frac{5}{8}$ " Bolt w/ Nut
and Lock Washer

Sign Bracket

$\frac{3}{8}$ " Stainless Steel
Flathead Bolt w/ Nut
and Lock Washer. Min.
4 Bolts per Panel unless
otherwise specified.

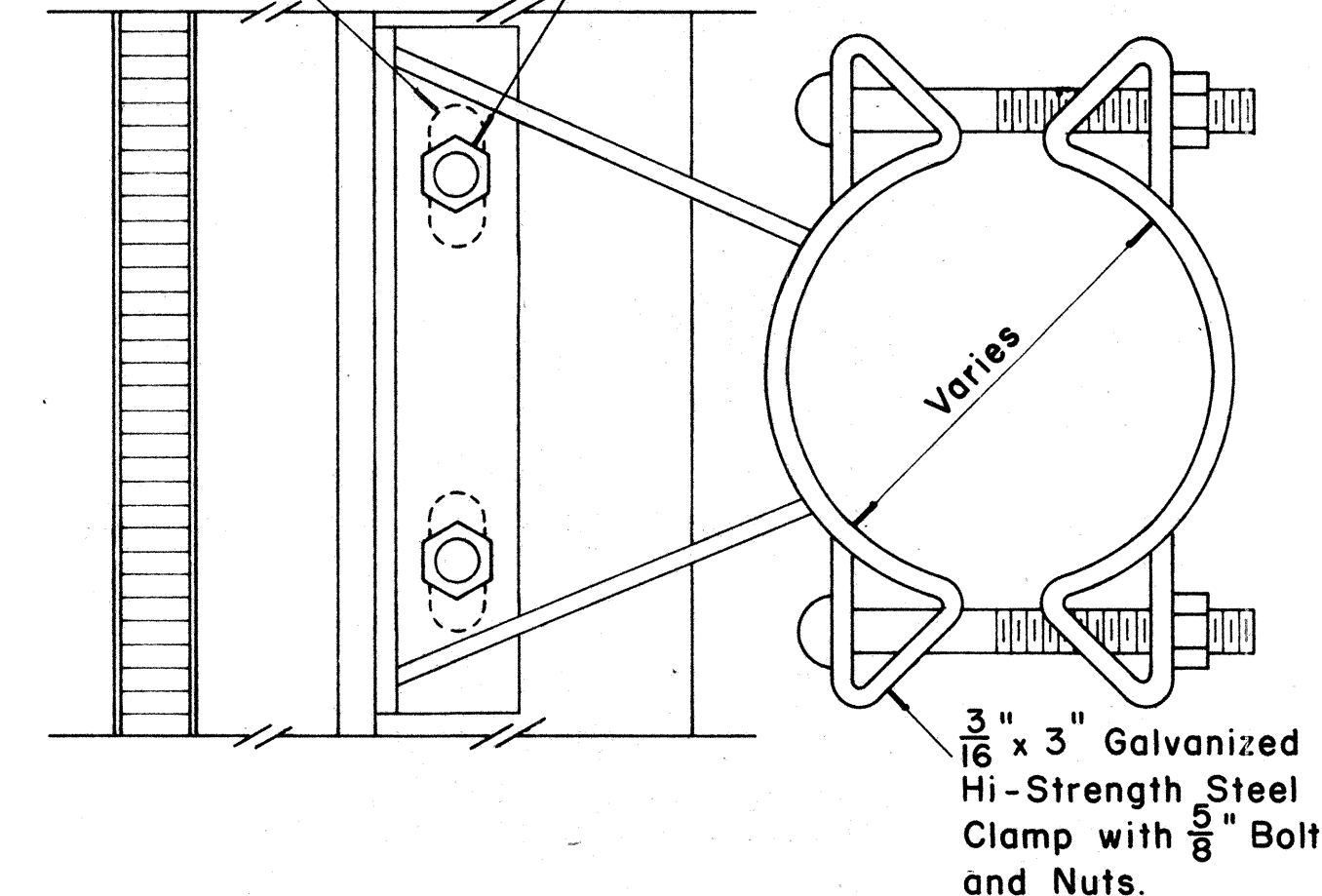
Support Tube

Porcelain Enameled
Laminated Panel

The diagram shows a cross-section of a cable assembly. A central cable with a braided shield is depicted. To the left, a dimension line indicates a length of 5 inches. Two rectangular components, likely connectors or relays, are connected to the cable via wires and connectors. The components are shown in a perspective view, with one component positioned above the other. The entire assembly is enclosed in a rectangular frame.

$\frac{11}{16}$ " x 2" min. slots
in Aluminum Angle
or Aluminum Tee—

$\frac{5}{8}$ " Stainless Steel
Bolts with Nuts
and Lock Washer



Scale: $\frac{3}{8}'' = 1''$

1"

.063" Aluminum Porcelain Enameled Face

.040" Aluminum Back

.062" Aluminum Alignment Extrusion

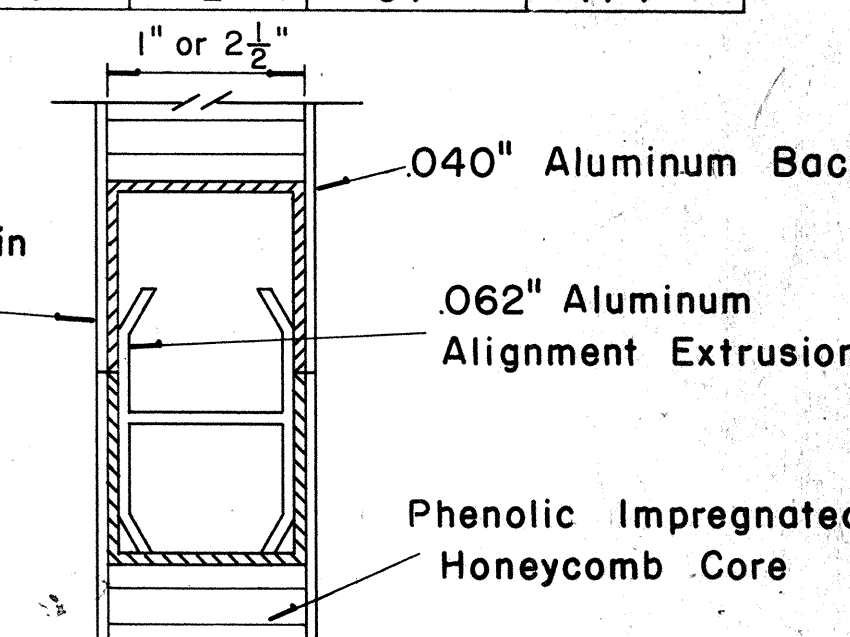
Phenolic Impregnated Honeycomb Core

FULL SCALE

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	1-HH(III)-14	1974	187	234

(GROUND MOUNTED SIGNS)
SUPPORT SPACING TABLE

PANEL THICK.	LENGTH OF SIGN	NO. OF SUPPORTS	OVERHANG	SUPPORT SPACING
	A		B	C
1"	6'-0"	2	12"	48"
1"	6'-6"	2	15"	48"
1"	7'-0"	2	15"	54"
1"	7'-6"	2	18"	54"
1"	8'-0"	2	18"	60"
1"	8'-6"	2	20"	62"
1"	9'-0"	2	22"	64"
1"	9'-6"	2	23"	68"
1"	10'-0"	2	24"	72"
1"	10'-6"	2	24"	78"
1"	11'-0"	2	24"	84"
1"	11'-6"	2	27"	84"
1"	12'-0"	2	30"	84"
1"	12'-6"	2	30"	90"
1"	13'-0"	2	30"	96"
1"	13'-6"	2	30"	102"
1"	14'-0"	2	30"	108"
1"	14'-6"	2	36"	102"
1"	15'-0"	2	36"	108"
2.5"	15'-6"	2	36"	114"
2.5"	16'-0"	2	39"	114"
2.5"	16'-6"	2	39"	120"
2.5"	17'-0"	2	39"	126"
2.5"	17'-6"	2	42"	126"
2.5"	18'-0"	2	42"	132"
2.5"	18'-6"	2	45"	132"
2.5"	19'-0"	2	45"	138"
2.5"	19'-6"	2	45"	144"
2.5"	20'-0"	2	48"	144"
2.5"	20'-6"	2	48"	150"
2.5"	21'-0"	2	51"	150"
2.5"	21'-6"	2	51"	156"
2.5"	22'-0"	2	51"	162"
2.5"	22'-6"	2	54"	162"
2.5"	23'-0"	2	54"	168"
2.5"	23'-6"	2	54"	174"
2.5"	24'-0"	2	57"	174"

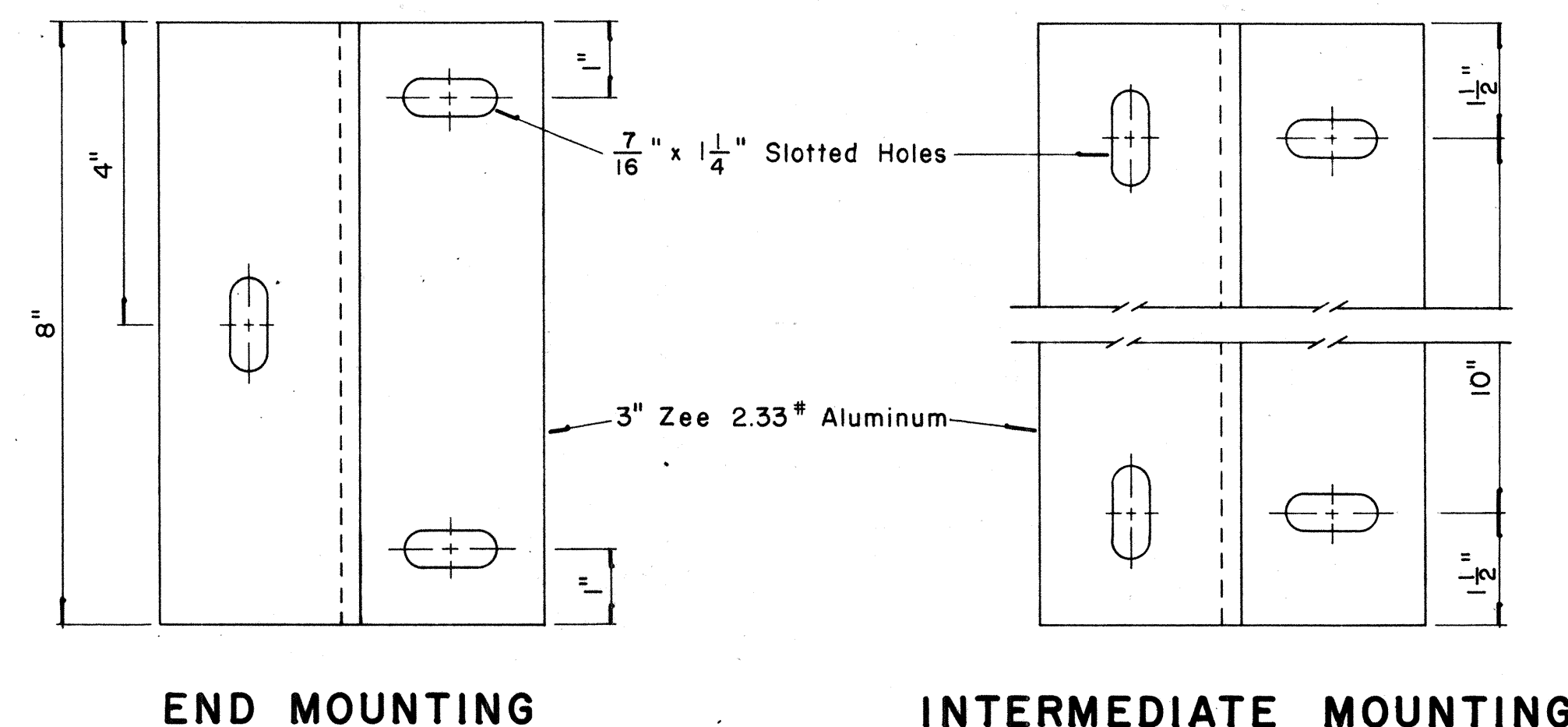


CLOSURE DETAIL
SECTION "C-C"
FULL SCALE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

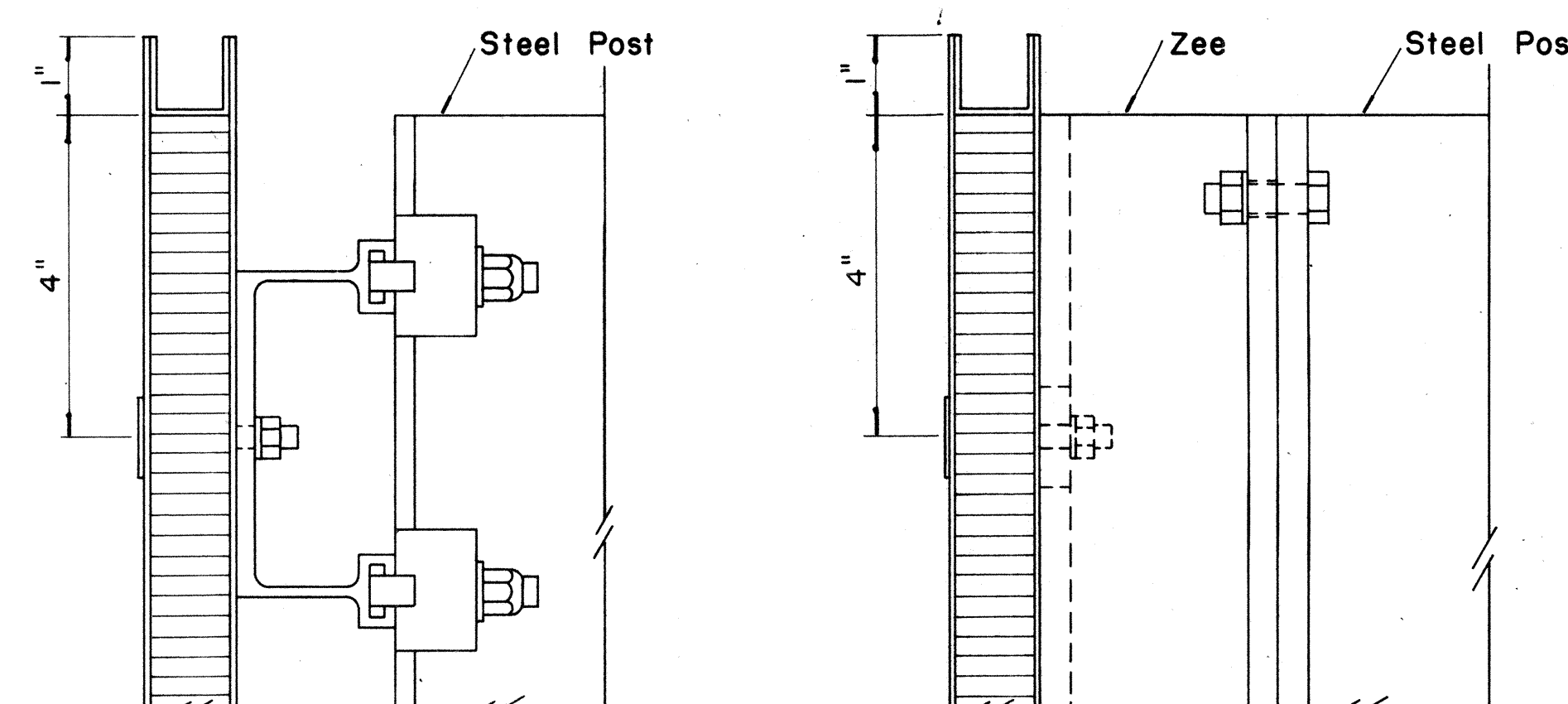
STANDARD DETAILS
LAMINATED ALUMINUM
SIGN PANELS
(GROUND MOUNTED)
Scale: As Shown

SHEET No. 5-33 OF 34 SHEETS DT 206



ZEE MOUNTING DETAILS

Scale: $\frac{1}{2}$ " = 1"



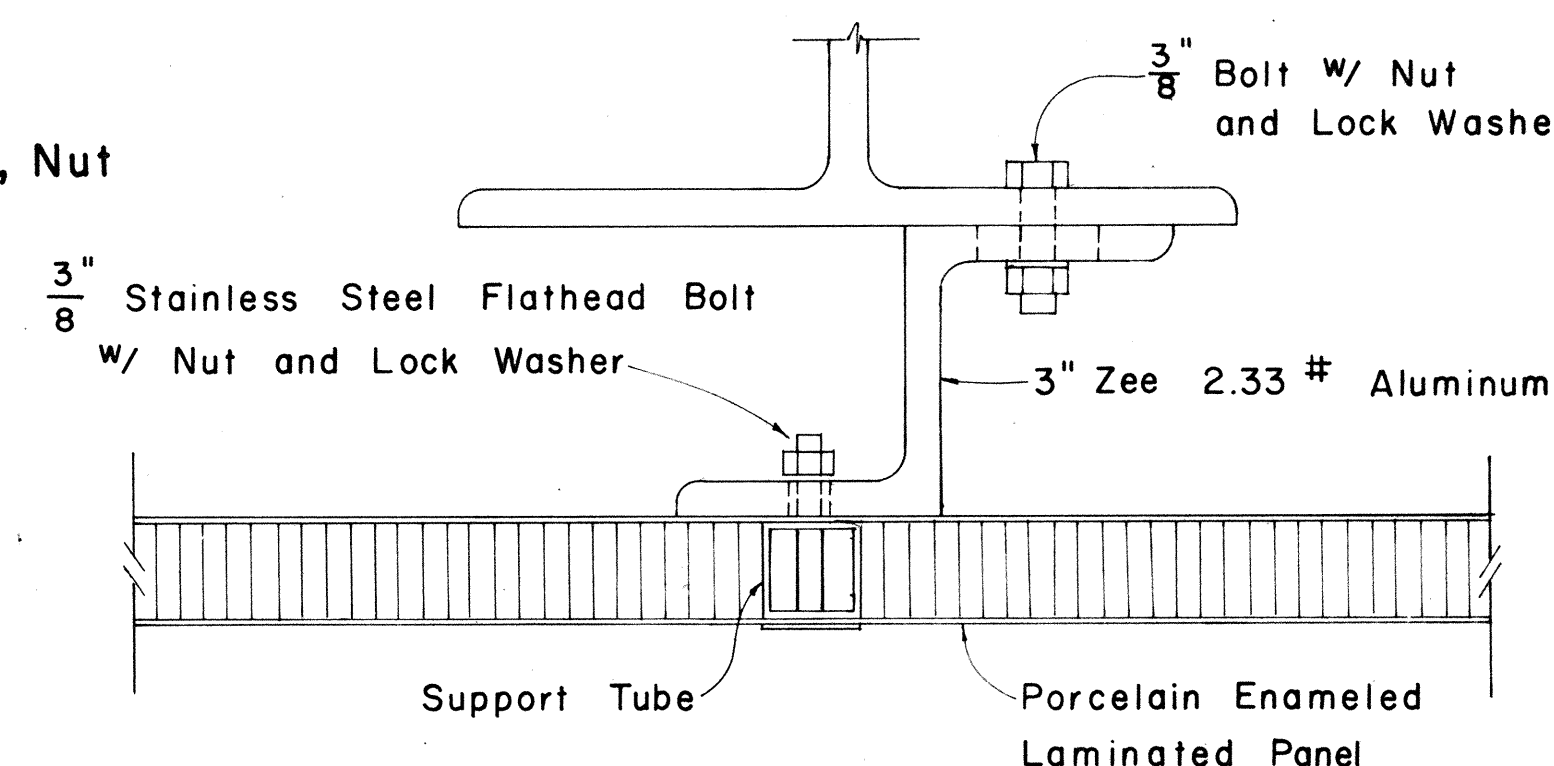
SECTION "B-B"

Scale: $\frac{1}{2}$ " = 1"

SECTION "B-B"

(Alternate)

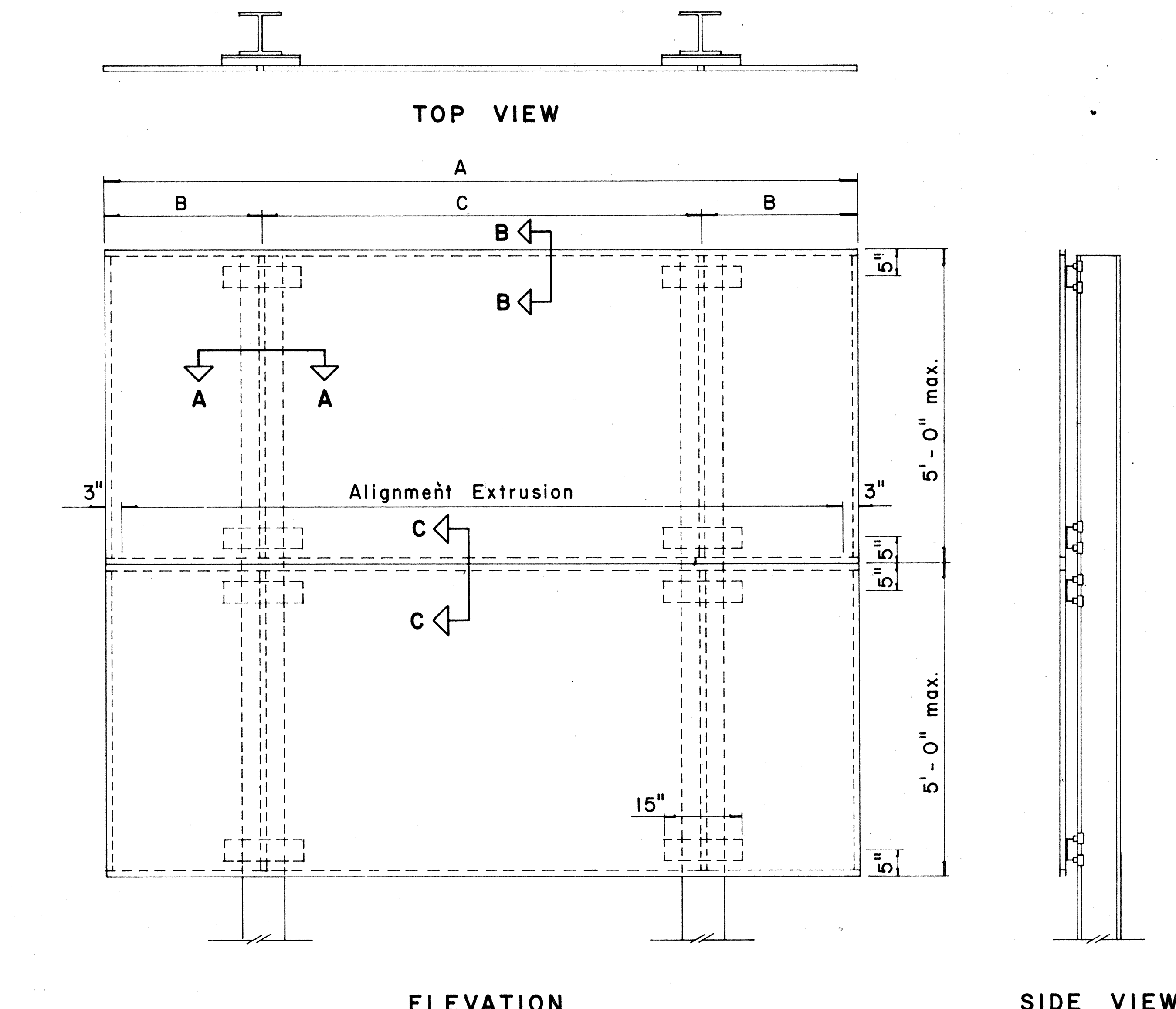
Scale: $\frac{1}{2}$ " = 1"



SECTION "A-A"

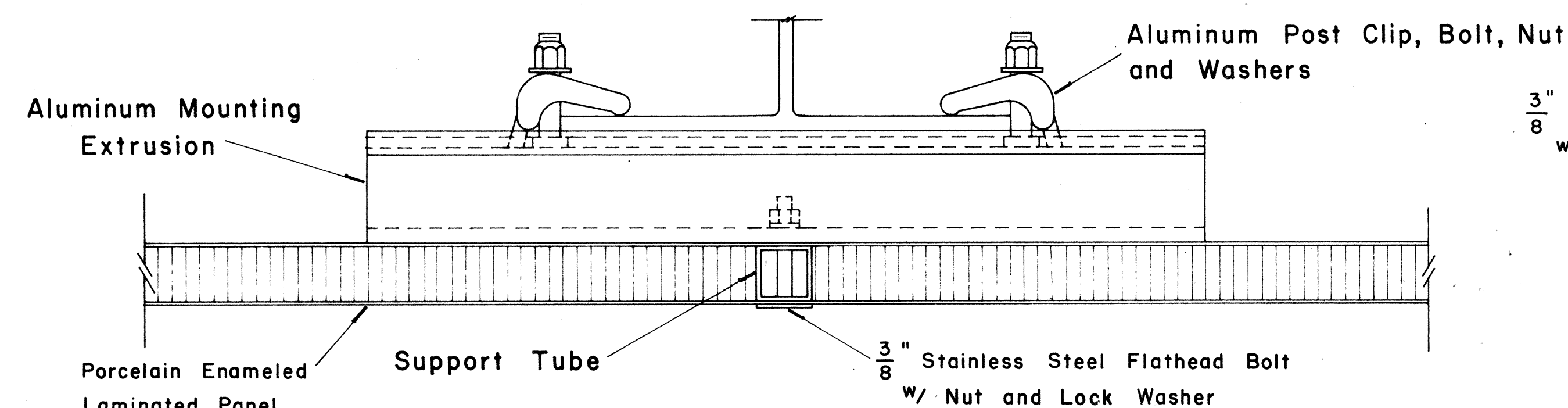
(Alternate)

Scale: $\frac{1}{2}$ " = 1"



TYPICAL SIGN ASSEMBLY

Scale: $\frac{3}{4}$ " = 1'-0"



SECTION "A-A"

Scale: $\frac{1}{2}$ " = 1"

APPROVAL RECOMMENDED:

Eishi Tanaka
TRAFFIC ENGINEER

12/29/69
DATE

APPROVED:

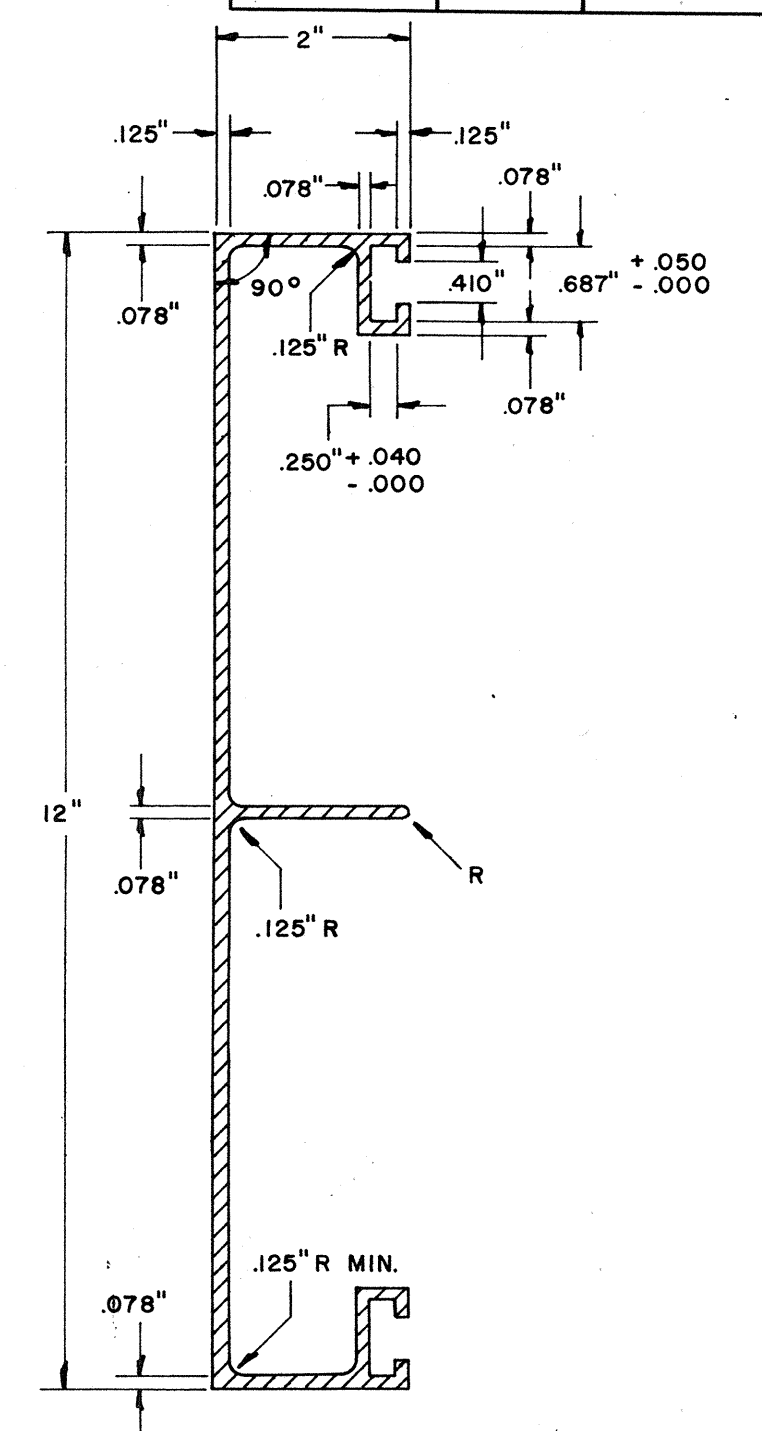
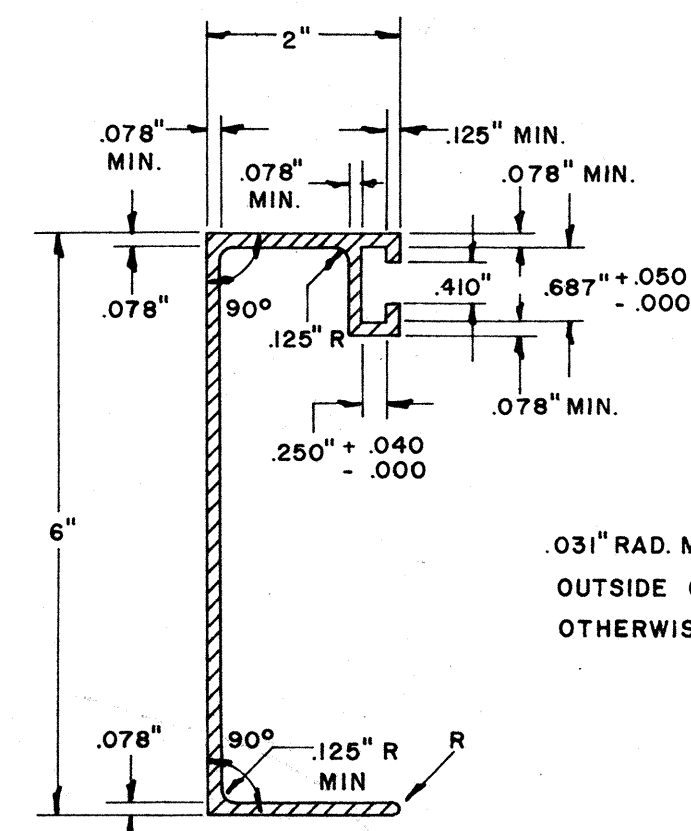
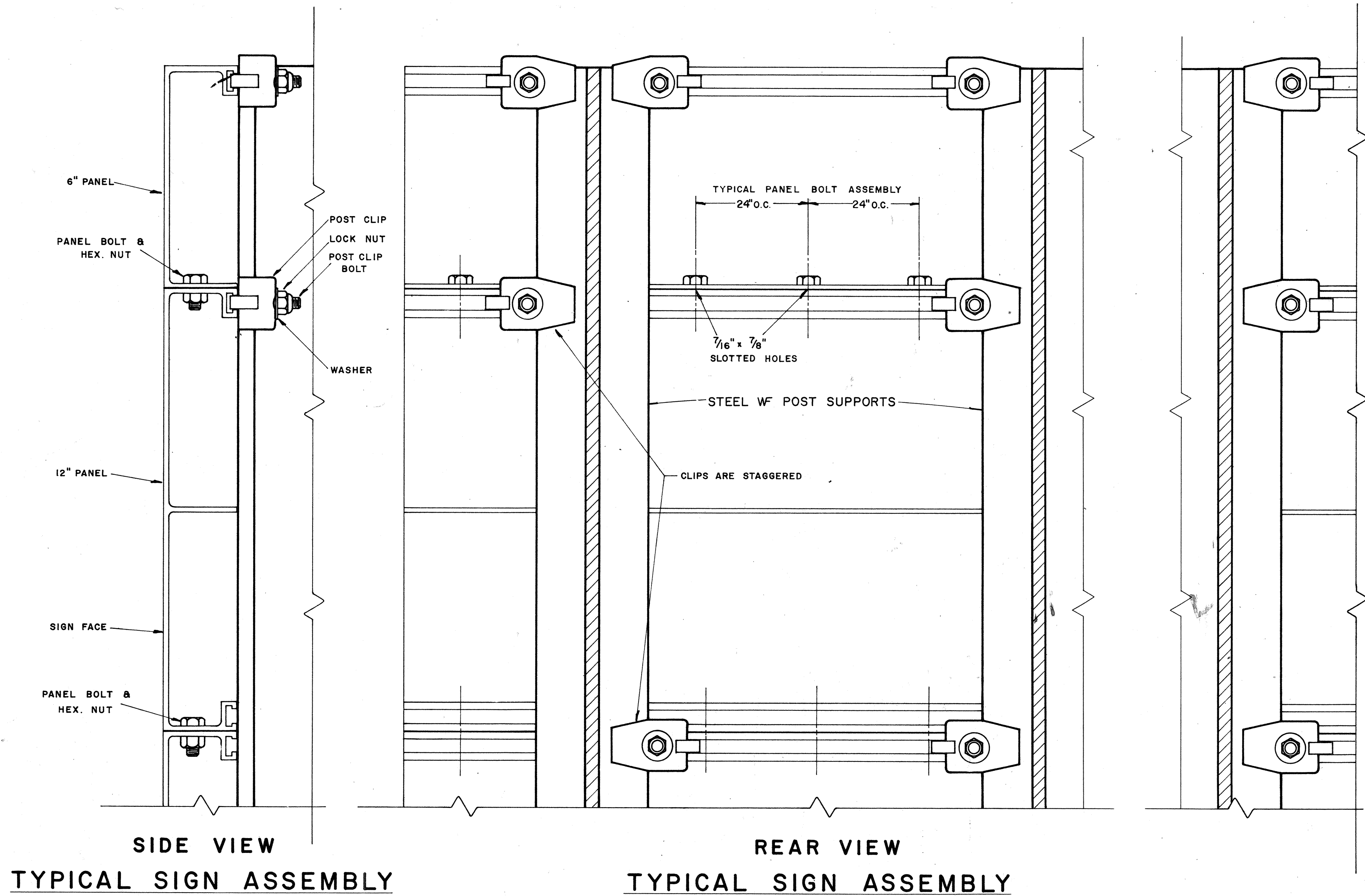
John J. S. S.
ASSISTANT CHIEF, ENGINEERING

12-30-69
DATE

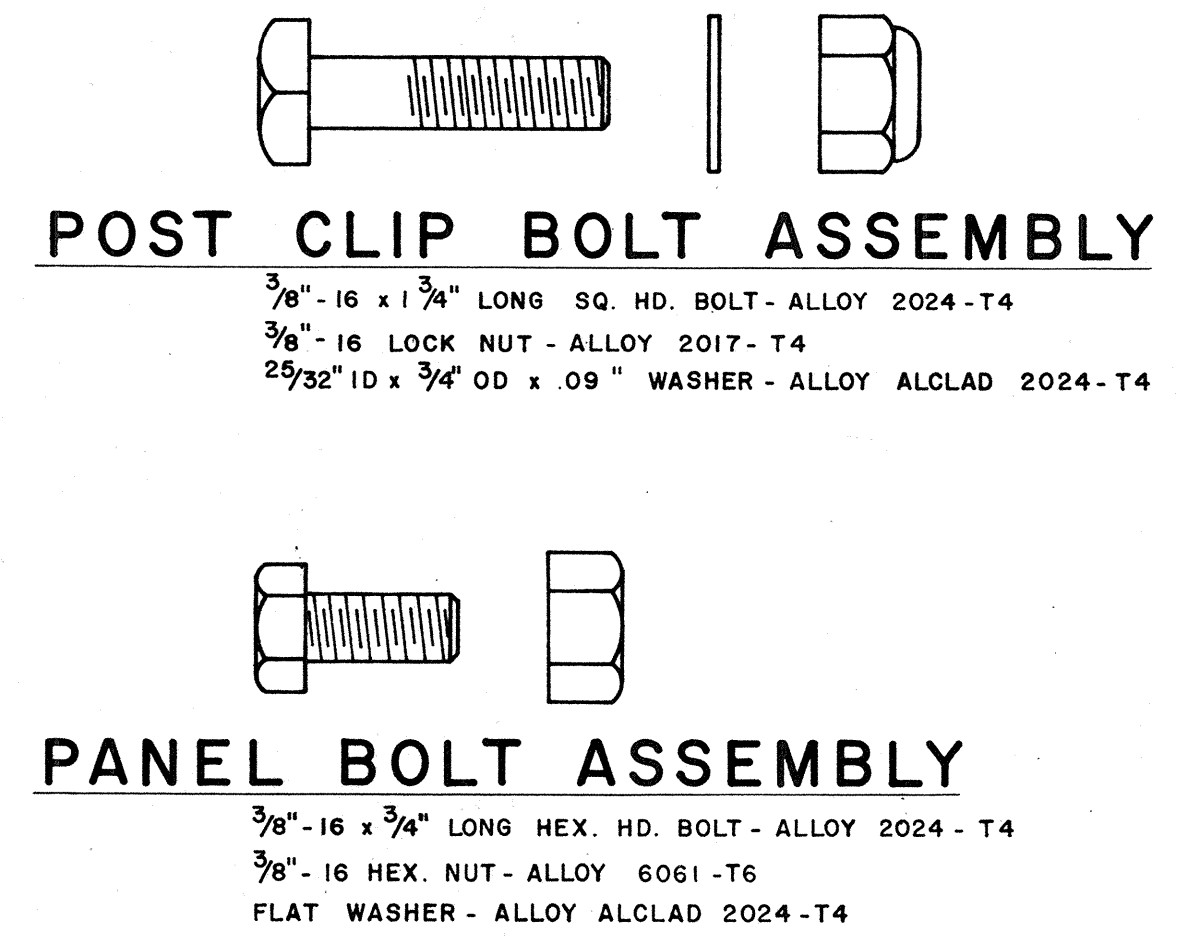
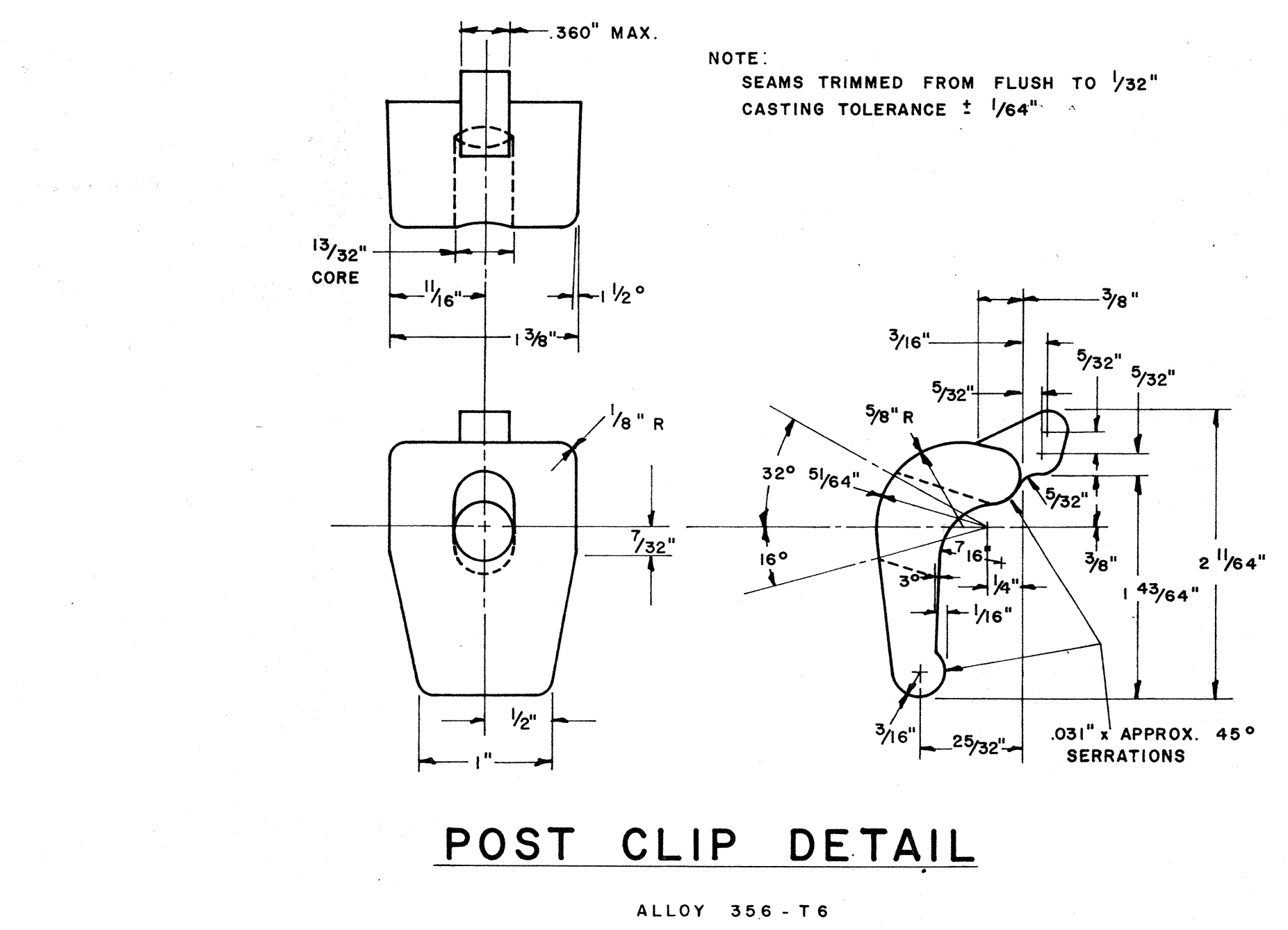
NO.	REVISION	APPROVED BY	DATE

SURVEY PLOTTED BY
DATE
DESIGNED BY
CHECKED BY
ORIGINAL PLAN
NOTE BOOK
No.

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	I-H-1(III)-14	1974	188	234



SOLID EXTRUDED SIGN
PANEL DETAILS



NOTE:
Extruded Aluminum Panels to be used for
Destination Signs with Reflective Sheeting Background

APPROVAL RECOMMENDED:
Erichi Tanaka 12/29/29
TRAFFIC ENGINEER DATE

APPROVED:
[Signature] 12-29-29
ASSISTANT CHIEF, ENGINEERING DATE

NO.	REVISION	APPROVED BY	DATE

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION
STANDARD DETAILS
SOLID ALUMINUM
EXTRUDED SIGN PANEL
AND
ACCESSORY DETAILS
SCALE: