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 HAWAII	HAW.	BR-093-1(21)	2015	50	99

## ABBREVIATION LIST:

Ø	Diameter			NO., #	Number
#	Number Or Pound	E.F.	Each Face	N.F.	Near Face
AASHTO	American Association Of	E.J.	Expansion Joint	N.T.S.	Not To Scale
	State Highway And	Elev., El.	Elevation		
	Transportation Officials	E.W.	Each Way	O.C.	On Center
A.B.	Anchor Bolt	Exp.	Expansion	OD	Outside Diameter
A.C.	Asphalt Concrete	_,,,,,			
ASTM	American Society For	F.B.	Flat Bar	PCF	Pounds Per Cubic Feet
ASTW	Testing And Materials	F.F.	Far Face	PL., P	Plate
AWS	American Welding Society	/ •/ •	1 41 1 400	PLF	Pounds Per Linear Foot
AZ.	Azimuth	G	Girder	PSF	Pounds Per Square Foot
AZ.	AZIIIUIII	Galv.	Galvanized	PSI	Pounds Per Square Inch
Dot Dott D	Dottom		Grooved Joint	PVC.	•
Bot., Bott., B	Bottom	<i>G.J.</i>	GI OOVEU JOITH	FVC.	Polyvinyl Chloride
C.J.	Construction Joint	$H^{\prime}$	Height	R, Rad.	Radius
<u>C</u>	Centerline	Horiz., H	Horizontal	Ŕebar.	Reinforcing Bar
C.G.	Center Of Gravity	•		Ref.	Reference
C.G.S.	Center Of Gravity Of Steel	In.	Inch	Reinf.	Reinforced, Reinforcing,
					Reinforcement
CIr., CI.	Clear	Jt.	Joint	R.O.W.	Right Of Way
Conc.	Concrete				
Cont.	Continuous	K	Kips	Sht.	Sheet
C.Y.	Cubic Yard	KSI	Kips Per Square Inch	SI.	Slope
<b>.</b>	Cabic raid		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Sta.	Station
Dbl.	Double	Lb., Lbs	Pound, Pounds	Std.	Standard
Det.	Detail	L.F.	Linear Feet	Stirr.	Stirrup
D.I.	Ductile Iron	LRFD	Load And Resistance	Sym., Symm.	Symmetrical
Dia.	Diameter	LIVI D	Factor Design	5.S.	Stainless Steel
	Down	L.S.	Lump Sum	<b>3.3.</b>	Statilioss Stool
Dn.		L	Lump Sum	Thk., Th.	Thick
Dwg.	Drawing	Max.	Maximum		
•			Minimum	Тур.	Typical
		Min.	WITHUIH	Vort V	Vertical
	Depressed 'V' letters			Vert., V	VOITICAL
·	3/8" deep			W/	With
				VV /	VV

Use Correct Name Of Bridge

<u>Year Built</u>

## NOTES:

- 1. Name and date shall be placed at the trailing end post on each side of the rideway.
- 2. Exact details and spacing of letters and figures and location shall be as directed by the engineer. Gothic letters and figures approximating dimensions shown will be acceptable if approved by the engineer.
- 3. The name of bridge shall be "Maipalaoa Bridge"

TYPICAL DETAIL OF LETTERS AND FIGURES AT CONCRETE END POST Not To Scale

The 2011 4/30/16

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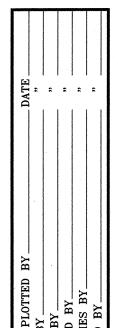
STATE OF HAWAII DEPARTMENT OF TRANSPORTATION

STRUCTURAL INDEX TO DRAWINGS AND ABBREVIATIONS

FARRINGTON HIGHWAY

Replacement of Maipalaoa Bridge

Federal Aid Project No. BR-093-1(21) OF THE LICENSE Scale: AS NOTED Date: JUNE 2015 SHEET No. S-1 OF 26 SHEETS



FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	BR-093-1(21)	2015	51	99

### STRUCTURAL NOTES:

- General specifications: Hawaii Standard Specifications for Road And Bridge Construction, 2005, together with special provisions prepared for this contract.
- Design specifications: AASHTO LRFD Bridge Design Specifications, 2012, including 2013 interim revisions.
- LOADS:
  - Dead loads:
  - An allowance of 25 psf (from curb-to-curb) has been Provided for in the design for future wearing surface.
  - An allowance of 150 plf (at each side of the bridge) has been provided for in the design for future utilities.
  - Live Load: HL-93
  - Seismic load:

Acceleration coefficient = 0.18 Seismic performance zone = 3 Importance category = Essential Bridge Site Class: E

Railing Test Level = TL-3

### MATERIALS:

- Minimum concrete compressive strength (at 28 days):
- Concrete topping, end beams and abutments = 5,000 psi
- Drilled shaft = 4,500 psi
- Prestressed concrete box girder = 7,000 psi.
- (4.)All other concrete = 4,000 psi. Concrete shall have' 0.45 maximum water cement ratio.
- All reinforcing steel shall conform to ASTM A615, Grade 60, unless otherwise noted.
- All stainless steel plates, bars, rods, anchor bolts and shapes shall be type 316 or 316L. welding of stainless steel shall be in accordance with the latest edition of AWS D1.6 -Structural welding code stainless steel. - 8.
- Tetraguard AS20 shrinkage reducing admixture, eclipse plus shrinkage reducing admixture, or an approved equal, shall be included in the concrete mix for the concrete topping, Bridge railing. The required dosage shall be 128 ounces per cubic yard concrete. Addition of shrinkage reducing admixture shall be as recommended by the manufacturer.

- For materials of prestressed girders, see applicable prestressed girder notes.
- A corrosion inhibiting admixture shall be included in the concrete mix for all concrete except the drilled shafts. the corrosion inhibiting admixture shall contain a minimum of 30% calcium nitrate by mass and shall be added at a dosage rate of 4.0 gallons per cubic yard of concrete. The admixture shall be Rheocrete CNI calcium nitrite-based corrosion inhibitor, DCI S corrosion inhibitor, or an approved equal. Addition of corrosion inhibiting admixture shall be as recommended by the manufacturer.

### 5. REINFORCEMENT:

- Unless otherwise noted, the covering measured from the surface of the concrete to the face of any reinforcing bars shall be as follows:
- Deck bars = 3" clear
- Railings and parapets = 3" clear
- Formed surfaces exposed to earth and weather = 3" clear
- Bottom and sides of footings and where concrete is deposited on grade = 3" clear
- Drilled shafts = 4" clear (to spirals)
- Minimum clear spacing between parallel bars shall be 1-1/2 times the diameter of the bar (for non bundled bars) or 1-1/2 times the diameter derived from the equivalent total area of the bars (for bundled bars), But in no case shall the clear distance between the parallel bars be less than 1-1/2 times the maximum size of the coarse aggregate or 1-1/2 inches.
- All dimensions relating to reinforcing bars (eg. spacing of bars, etc.) are to center of bars, unless otherwise noted.
- Reinforcing bars shall be detailed in accordance with AASHTO LRFD bridge design specifications, 2012, including 2013 interim revisions, unless otherwise noted.
- Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of the intersections is less than 12 inches in each direction, in which case alternate intersections shall be tied.
- Deformed reinforcing bars to be welded shall conform to ASTM A706, Grade 60. Welding of deformed reinforcing bars shall be in accordance with the latest edition of AWS D1.4 -Structural Welding code - Reinforcing bars.

### 6. GENERAL CONSTRUCTION NOTES:

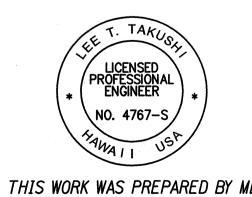
- See standard specifications and special provisions.
- All items noted incidental will not be paid for separately.
- Standard detail drawings refer to all structures in general, except for modifications as may be required for special conditions. For such modifications, refer to the corresponding detailed drawings.
- The Contractor shall comply with all construction permits for this project. In addition, The Contractor shall comply with all applicable laws of the federal, state and county Governments.
- All temporary construction items shall be in conformance with the AASHTO Guide Design Specifications for Bridge Temporary Works, 1995, with 2008 Interim Revisions and the Construction Handbook for Bridge Temporary Works, 1995, with 2008 Interim Revisions.
- Unless otherwise noted, all vertical dimensions are measured plumb.
- The Contractor shall verify all site conditions before commencing with work.
- The Contractor shall verify the location of all underground utility lines and notify the respective owners before commencing the work of excavation or drilled shafts.
- For concrete finish, see standard specifications.
- Unless otherwise noted, all exposed concrete edges shall be chamfered 3/4" x 3/4".

### 7. PRESTRESSED GIRDER BEARING SURFACE:

- Prestressed girder bearing surface shall be smooth and sloped to match the finish roadway slope.
- Prestressed girder shelf elevations shall be verified by the contractor. Shelf elevations shall take into consideration the concrete topping thickness, prestressed girder thickness, roadway slope, and the calculated, or if available, the actual camber of the prestressed girders.

### 8. FOUNDATION EXCAVATION AND DEWATERING:

- Temporary shoring and dewatering may be required for foundation excavations.
- Temporary shoring and dewatering, if required, shall be considered incidental to structure excavation.



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4/30/16

STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

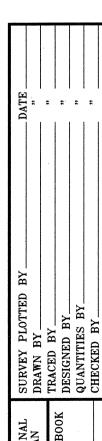
STRUCTURAL NOTES

FARRINGTON HIGHWAY

Replacement of Maipalaoa Bridge Federal Aid Project No. BR-093-1(21)

EXPIRATION DATE | Scale: AS NOTED Date: JUNE 2015

SHEET No. S-2 OF 26 SHEETS



# FED. ROAD DIST. NO. STATE FED. AID PROJ. NO. FISCAL SHEET TOTAL SHEETS HAWAII HAW. BR-093-1(21) 2015 52 99

## STRUCTURAL NOTES (CONT.):

### 9. FOUNDATION:

These foundation notes were based on recommendations contained in a Geotechnical Engineering Exploration report by Geolabs, Inc. Dated September 12, 2014. Contractor may obtain a copy of the report at the State of Hawaii, Department Of Transportation - Highways Division upon request.

- A. 4'-0- Diameter Drilled Shafts:
- (1). The compressive load capacity for the drilled shafts are from friction between the concrete shaft and the surrounding soils and coral formation, the following

Compressive load capacity were used for design of the abutments:

- Strength limit state = 730 kips
- Extreme event limit state = 1620 kips
- (2). The uplift load capacity for the drilled shafts are from a combination of friction between the concrete shaft and the surrounding soils and coral formation and from the self weight of the shaft. The following uplift load capacity were used for design of the abutments:
  - Strength limit state = 427 kips (Abutment #1) 418 kips (Abutment #2)
  - Extreme event limit state = 1,220 kips (Abutment #1) = 1,195 kips (Abutment #2)
- (3). The lateral load resistance for the drilled shafts is based on the stiffness of the surrounding soil, the stiffness of the shaft, allowable deflection at the top of shaft, and the induced moment in the shaft. The foundation loads, lateral deflection and maximum induced moments for the drilled shafts are as follows:
  - A) Longitudinal Lateral Load:
    - 1) Strength limit state:
      - Axial Loads = 5,100 kips (Both Abutments)
      - Longitudinal Horizontal Loads = 630 kips (Both Abutments)
      - Longitudinal lateral deflection = 0.42 inches (Abutment #1)
        0.88 inches (Abutment #2)
      - Maximum induced moment = 1,085 ft-kips (Abutment #1)
        1,545 ft-kips (Abutment #2)
    - 2) Extreme Event limit state:
      - Axial Loads = 4,280 kips (Both Abutments)
      - Longitudinal horizontal loads = 0 kips (Both Abutments)
      - Longitudinal lateral deflection = 0.21 inches (Abutment #1)

        0.81 inches (Abutment #2)
      - Maximum induced moment = 320 ft-kips (Abutment #1) 580 ft-kips (Abutment #2)

- B) Transverse Lateral Load (Based on a fixed against rotation boundary condition at the top of the drilled shaft):
  - 1) Extreme event limit state:
    - Transverse Horizontal Loads = 738 kips (Both Abutments)
    - Transverse lateral deflection = 0.25 inches (Abutment #1)
      0.38 inches (Abutment #2)
    - Maximum induced moment = 900 ft-kips (Abutment #1) 820 ft-kips (Abutment #2)
- 4). The drilled shaft estimated tip elevations shown on the plans are based on the boring data. The actual tip elevations could change due to varying subsurface conditions. The Geotechnical Engineer of record shall be present during the drilling operation to determine that the actual subsurface conditions are consistent with the conditions assumed for design. The Contractor shall make provisions for extension of the reinforcing steel cages for the drilled shafts to account for variations in the final tip elevations.
- (5). The contractor shall exercise care in drilling the shaft holes and in placing concrete into the holes. Cobbles and boulders may be encountered in the fill deposit at the replacement bridge foundation locations. The drilled shaft contractor will need to have the appropriate equipment and tools to drill through these types of natural obstructions, where encountered. Appropriate measures will also be needed to avoid dislodging boulders into the drilled shaft hole during the drilling and shaft installation process.
- (6). The abutment locations are underlain by sandy deposits. There is a strong potential for caving-in of the materials during the drilling operations. To reduce the potential for significant caving-in of the drilled holes, temporary casing will be required during drilled shaft installation. Care should be exercised during removal of the temporary casing to reduce the potential for "necking" of the drilled shaft concrete...
- (7). Concrete placement by tremie methods is recommended during construction of the drilled shafts. The concrete shall be placed promptly after completion of drilling (within 24 hours) to reduce the potential of caving in of the sidewalls. The concrete shall be placed in a suitable manner by displacing the water in an upward fashion from the bottom of the drilled hole
- (8). The Geotechnical Engineer of record shall be present during the trial shaft program to evaluate the contractor's method of drilled shaft installation and to evaluate the subsurface materials encountered; during the installation and performance of the instrumented load test on the drilled shaft; and at the site to observe the drilling and installation of production drilled shafts during construction.
- (9). Drilling of shafts within a horizontal distance of 3.0 times the shaft diameter shall not commence until a minimum of 24 hours after the drilled shaft has been completed by placement of concrete to the top of shaft elevation.

#### 10. ABUTMENT AND WING WALLS:

- A. Static lateral earth pressures:
  - Active earth pressure (Level Backfill) = 36 Pcf
  - At-rest Earth Pressure (Level backfill) = 55 Pcf
  - Active earth pressure (Max. 2H:1V Sloping Backfill)

Horizontal = 47 Pcf Vertical = 23 Pcf

- At-rest Earth Pressure (Max. 2H:1V Sloping Backfill) Horizontal = 64 Pcf

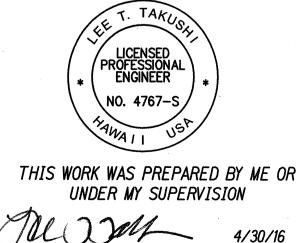
Vertical = 32 Pcf

B. Dynamic lateral earth forces:

- Level Backfill = 6.9 X H Psf (Rectangular Distribution)

### 11. STRUCTURAL BACKFILL:

- 1. Structural Backfill shall be in accordance with the requirements for Structure Backfill Material as indicated in Section 703.20 of the Hawaii Standard Specifications for Road and Bridge Construction, 2005 and the Special Provisions
- 2. Placement of the structural backfill shall be in accordance with the standard specifications and the special provisions.
- Structural backfill on the backface of the abutment shall not be placed until the bridge concrete topping has been poured and has attained it's 28-day concrete compressive strength.



STATE OF HAWAII
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HIGHWAYS DIVISION

## STRUCTURAL NOTES

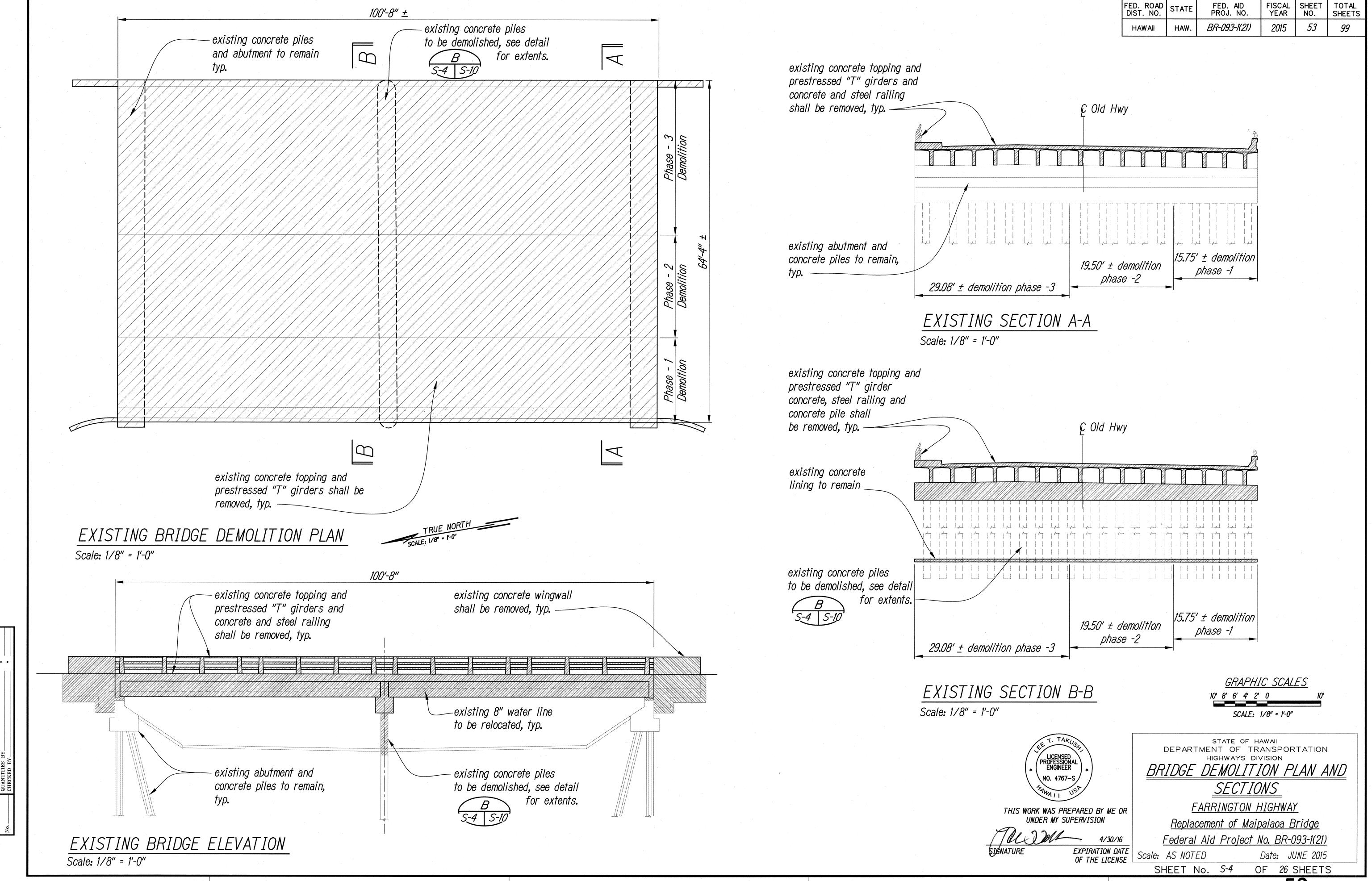
FARRINGTON HIGHWAY

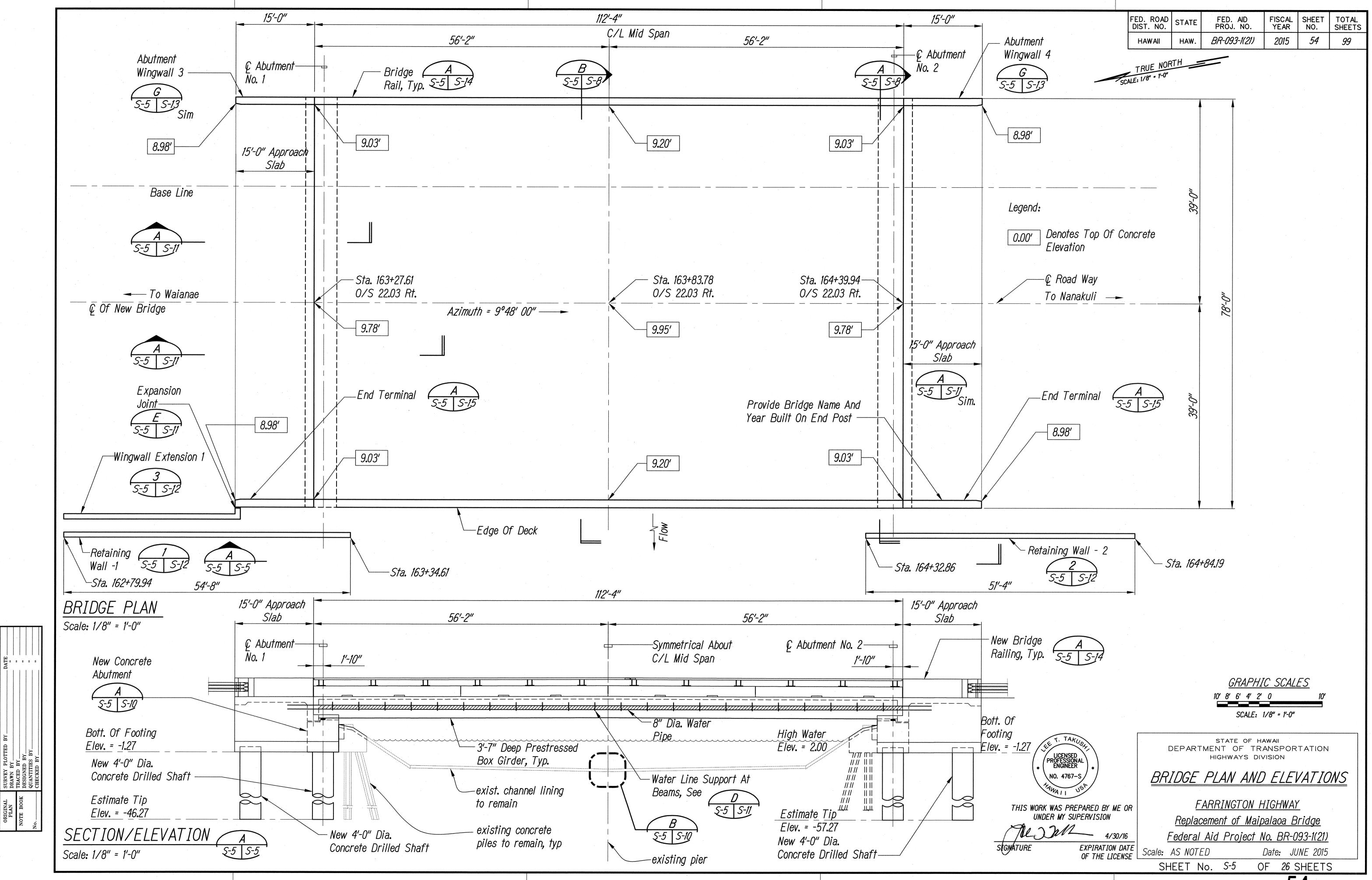
Replacement of Maipalaoa Bridge

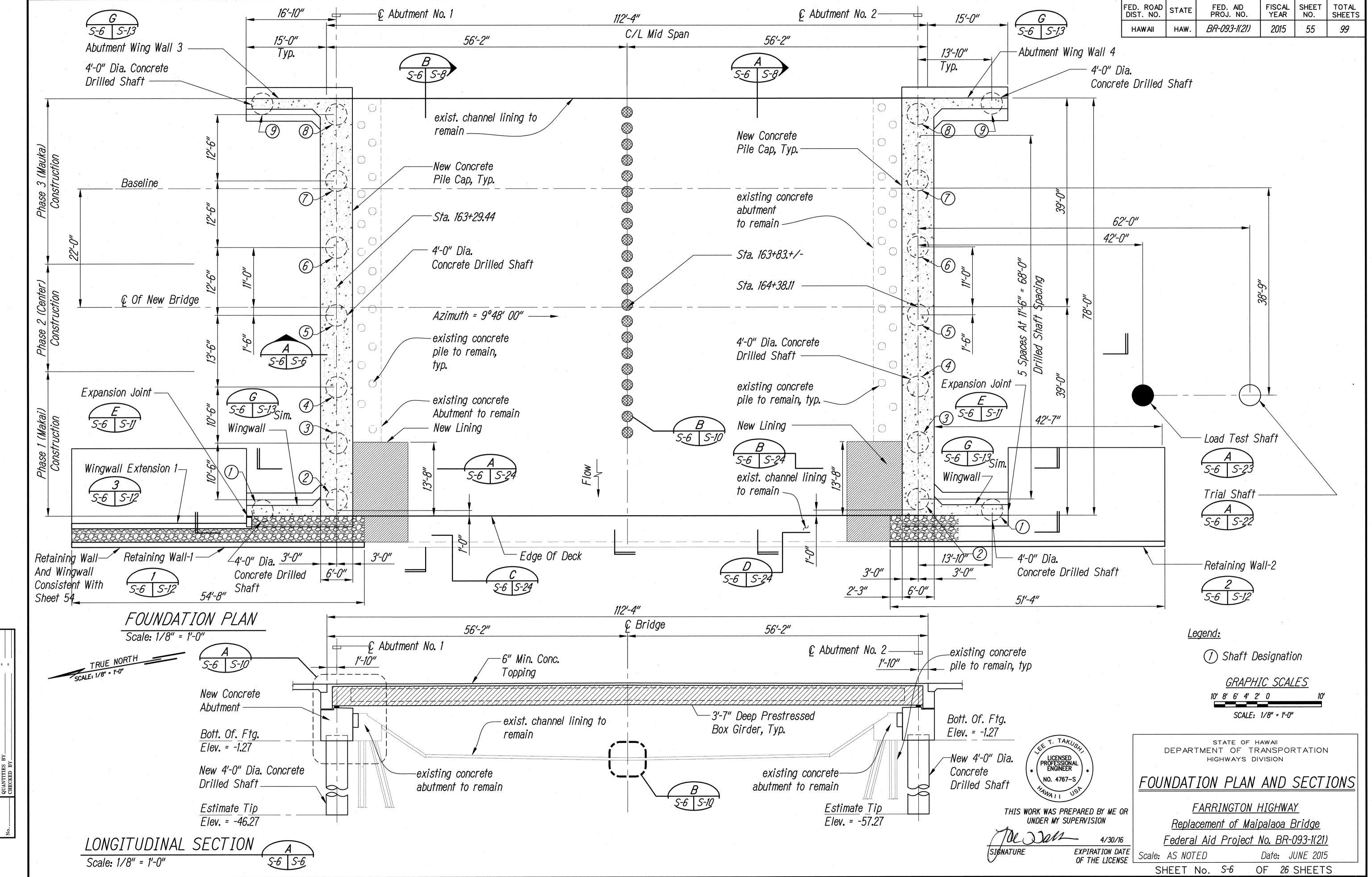
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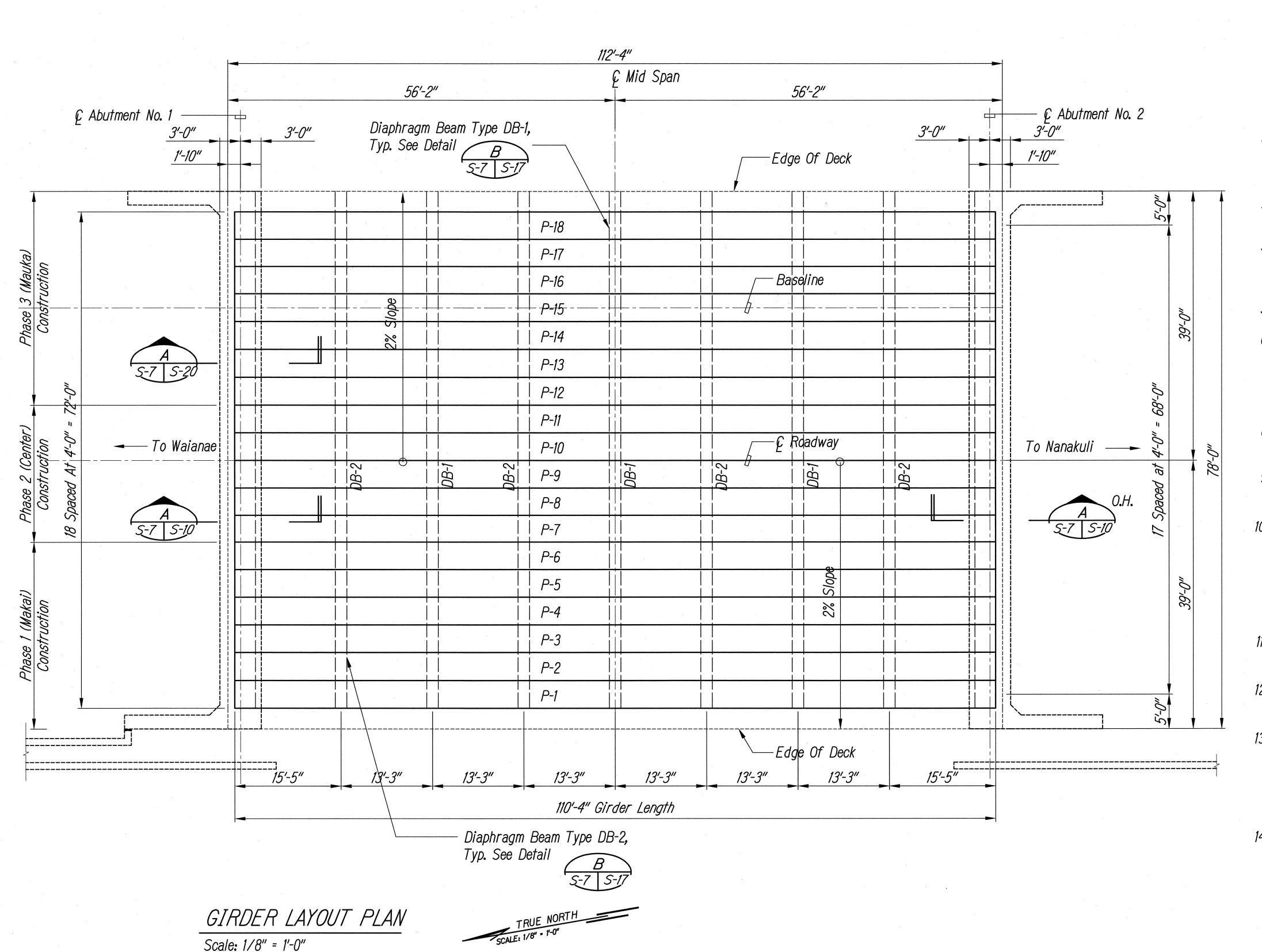
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SHEET No. S-3 OF 26 SHEETS





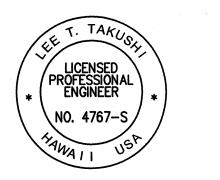




FED. ROAD DIST. NO. FISCAL SHEET YEAR NO. TOTAL SHEETS FED. AID PROJ. NO. STATE 2015 BR-093-1(21) *56* HAW. HAWAII

# PRESTRESSED GIRDER NOTES:

- 1. Minimum concrete compressive strength (at 28 days) of prestressed girders shall be as follows:
- A) Final concrete strength = 7,000 Psi B) Concrete strength at transfer = 6,000 Psi
- 2. Prestress strands shall be seven wire 1/2" diameter low relaxation steel strands (area = 0.153 sq. In.), with an ultimate strength of 270 Ksi. For properties, see standard specifications.
- 3. Non-prestressed reinforcing steel shall be Grade 60, unless otherwise noted on plans. For properties, See Standard Specifications.
- 4. Effective prestressing force is after all losses. Losses shall take into consideration creep, shrinkage, elastic shortening and relaxation of prestressing steel.
- 5. Dead load deflection is due to weight of the topping.
- 6. Strand pattern shall be symmetrical about the longitudinal centerline of the prestressed girder.
- 7. Strand release sequence shall not include any lateral deflection of the prestressed girder.
- 8. The contractor shall submit his proposed strand pattern and releasing sequence to the engineer for approval.
- 9. During curing, care shall be taken to avoid any lateral deflection of the prestressed girder due to improper orientation.
- 10. Lifting devices shall be placed as close as possible to the centerline bearing of the prestressed girder. Details and locations of lifting devices shall be submitted to the engineer for approval. Such approval does not relieve the contractor of his responsibilities if the prestressed girder is damaged due to failure of the lifting devices.
- 11. Elastic shortening shall be included in determining the length of the prestressed girders.
- 12. The Contractor shall incorporate all inserts, dowels and other embedded items required in the prestressed girders during fabrication.
- 13. The calculated camber includes the effects of the initial prestress force and the weight of the prestressed girder at the time of placement. Negative values indicate a net upward deflection. The actual camber shall not exceed the calculated camber by more than 1-inch.
- 14. Contractor shall provide actual camber measurements prior to delivery and after delivery of prestressed girders to site. Prestressed girders exceeding the calculated camber by more than 1-inch will be rejected.



GRAPHIC SCALES

SCALE: 1/8" = 1'-0"

10' 8' 6' 4' 2' 0

4/30/16

EXPIRATION DATE
OF THE LICENSE

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FARRINGTON HIGHWAY

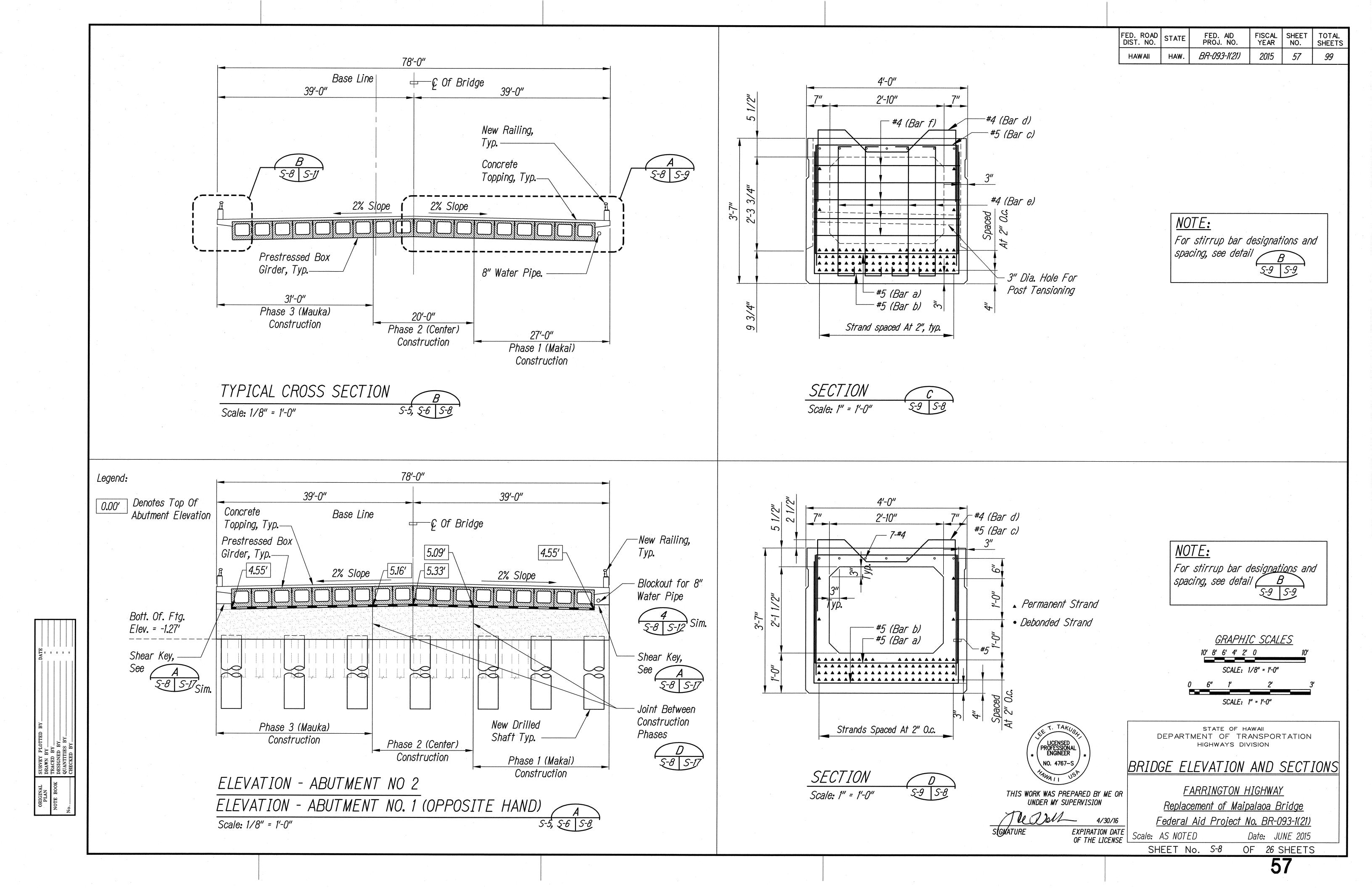
Replacement of Maipalaoa Bridge Federal Aid Project No. BR-093-1(21)

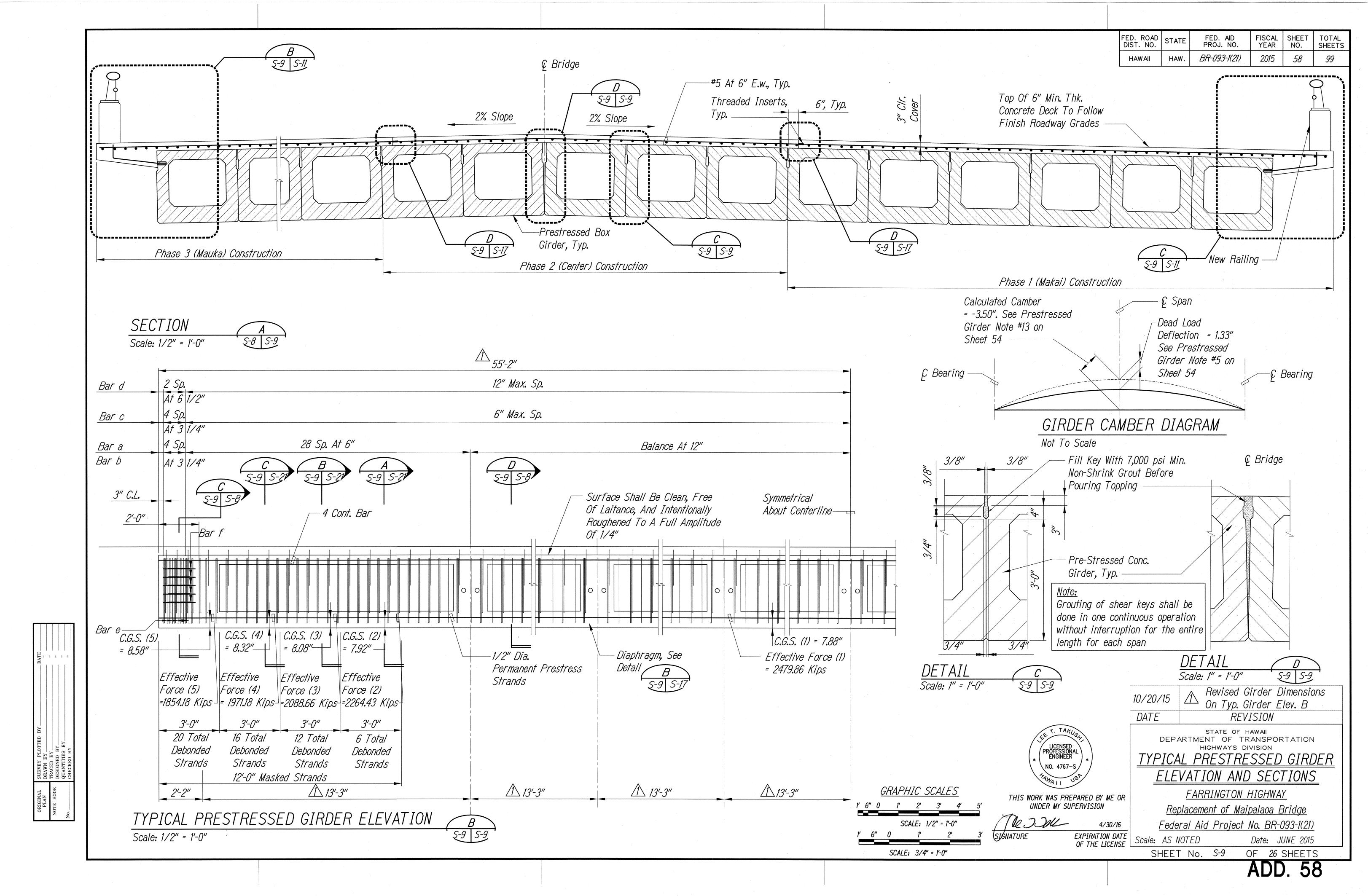
STATE OF HAWAII

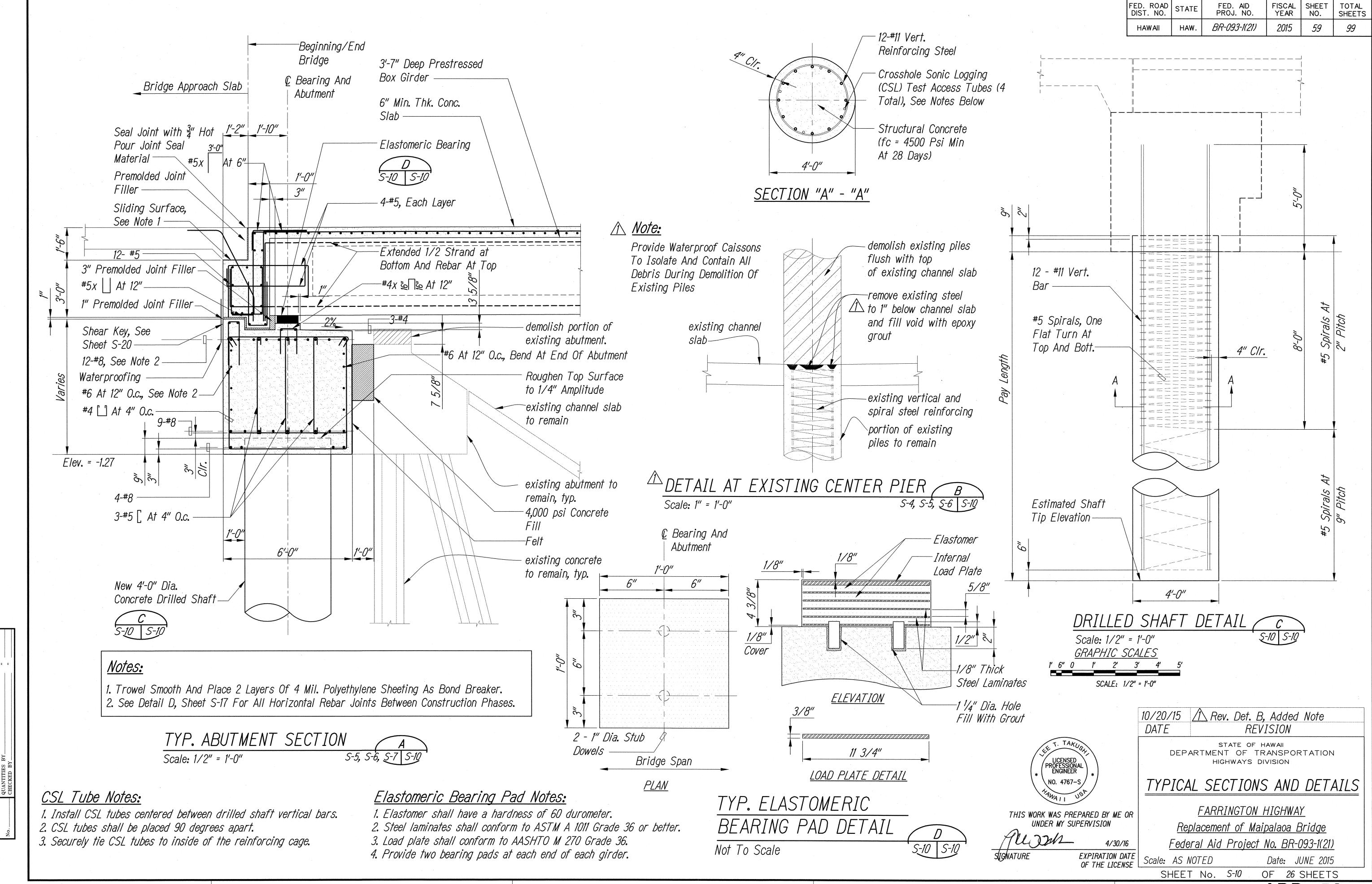
DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

GIRDER LAYOUT PLAN AND NOTES

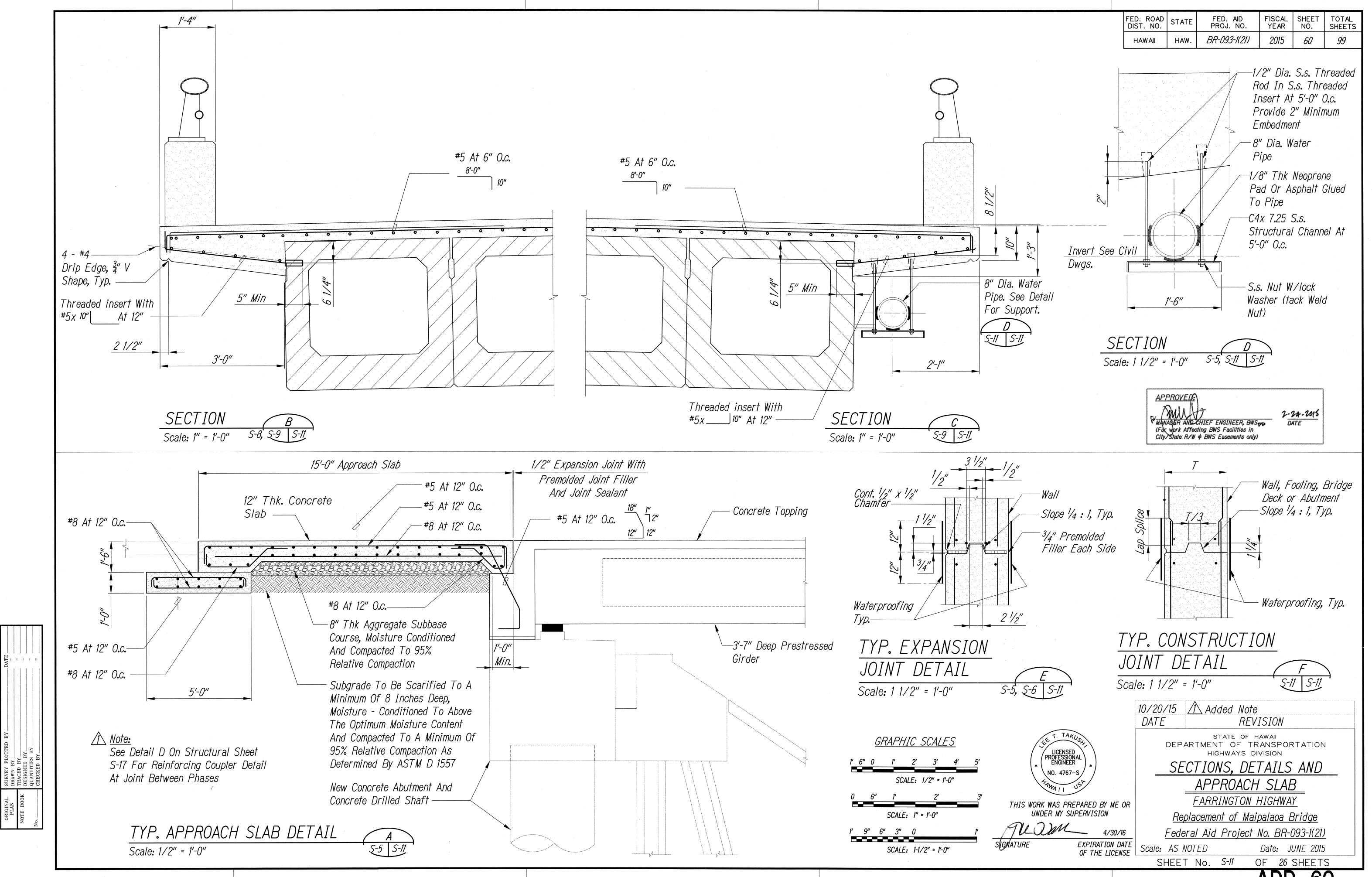
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