## STRUCTURAL GENERAL NOTES

- All work shall conform to applicable portions of the Hawaii Standard Specification for Road and Bridge Construction, 2005, of the State of Hawaii unless otherwise indicated.
- 2. The contractor shall verify all repair quantities with the Engineer before proceeding with the repair work.
- The contractor shall verify all field dimensions, existing elevations and conditions against the contract drawings prior to starting work. All discrepancies shall be reported to the Engineer.
- The Contractor is responsible for visiting and inspecting the site in order to ascertain site conditions; to properly gage the extent of concrete damage; to determine appropriate construction methods; to anticipate difficulties in the sequencing and lay out construction operations; and to prepare accurate material, labor, and cost estimates. Any claims for additional payment, based upon work difficulties that arise from failure of the Contractor to fully inspect the site or to closely read the available plans, shall be denied by the Engineer.
- 5. All work specified in the contract but not listed separately shall be considered incidental and will not be paid for separately.
- During construction, the contractor shall be responsible for jobsite safety. The contractor shall assume responsibility for the design and provision of all temporary bracing, shoring, guys, etc. in accordance with all national, state and local safety ordinances.
- 7. All omissions or conflicts between the various elements of the contract drawings and/or specifications shall be brought to the attention of the Engineer before starting any work.
- 8. Notes and details on drawings shall take precedence over general notes and typical details. Should there be conflicts between the requirements of the drawings or specifications, the more stringent requirement shall apply.
- Details shown on the drawings shall be typical for all similar conditions. Modify details for special conditions as directed by the Engineer.
- Contractor shall protect existing surfaces and objects to remain from damage. Any item to remain that is damaged by the contractor shall be replaced or repaired to match existing adjacent surfaces at no additional cost.
- 11. The contractor shall provide all necessary measures to protect the new work and existing structures during the construction.
- 12. All existing utilities to remain shall be protected from damages during construction. Any damages by the contractor shall be repaired at no additional cost to the State.
- Information shown on the drawings has been obtained from field observations. The accuracy and completeness of the drawings are not guaranteed. Dimensions of the structure shown on the drawings may not be exact. Contractor shall verify all existing conditions and structure dimensions before commencing work. Notify the Engineer of any discrepancies.

14. No debris shall be allowed to enter the water. The contractor shall provide a temporary platform or other suitable positive means of capturing debris from construction and demolition operations. These facilities shall be in place prior to starting demolition work.

### DESIGN CRITERIA (For New Work Only)

- AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Seventh Edition, 2014, including Interim revisions.
- Design Criteria for Bridges and Structures, August 8, 2014, State of Hawaii, Department of Transportation, Highways Division.

#### DESIGN LOADS (For New Work Only)

- 1. Dead Loads:
  - A. An allowance of 25 PSF (from curb-to-curb) for future A.C. wearing surface has been provided for in the design.
  - B. An allowance of 150 PLF for future utilities in addition to known or existing utility loads on the structure.
- Live Load: HL-93 Design Truck or Design Tandem, and Design Lane Load.
- 3. Median Areas:

Live Load: 250 PSF.

### EPOXY MATERIAL NOTES

- Refer to drawings for structures that require epoxy embedded connections. Refer to Standard Specs Section 656 - "Drilling Holes and Installing Dowel Reinforcing Bars." Cost shall be incidental to Section 602 Reinforcing Steel.
- Epoxy shall be installed in strict conformance to manufacturer installation instructions. Holes shall be properly cleaned by air hose and/or manufacturer air nozzles.
- Holes shall be drilled with a rotary impact drill or rock drill. Coring of holes is not permitted. Do not over drill depth of hole.
- Confirm epoxy expiration date shown on packaging.
- 5. Contractor shall submit product data of proposed product including International Code Council Evaluation reports for Engineer's review prior to construction.
- To prevent damage, contractor shall locate existing steel reinforcing by non-destructive methods prior to drilling holes. Contractor is responsible for repair of existing reinforcing if damage is caused.
- Epoxy adhesive shall be Simpson Set-XP Epoxy, Hilti HIT-RE 500-SD, or approved equal. Epoxy for epoxied reinforcing steel dowels shall be fully cured prior to pouring concrete.

### SITE CAST CONCRETE

- High early strength concrete shall be a mixture of cement, fine aggregate, coarse aggregate, plasticizing admixture, corrosion inhibitor admixture, fiber, and water. It shall have a minimum compréssive strength of 3,000 PSI when opened to traffic and no later than 2 hours after casting. Minimum compressive strength of concrete shall be 6,000 PSI at 7 days.
- Aggregates shall be basalt, no larger than 3/4" and otherwise conform to the State Standard Specifications and Special Provisions.
- Unless otherwise indicated, plasticizer admixtures shall be used at the Contractor's option subject to approval of the Engineer.
- Concrete mix design shall be submitted to the Engineer for review.
- Minimum clear cover of concrete over outer reinforcing bars or ties shall be as shown on drawings, unless otherwise noted. See Standard Specification Table 602.03-2 for additional information.
- Concrete admixtures containing chloride salts shall not be used.
- 7. Do not feather edge repairs.
- All exposed rebars shall be cleaned of all scale, rust, dirt, oil and other deleterious materials.
- All forms shall be water tight, concrete and water with cementitious particles shall not overflow formwork. Formwork and joints shall be sealed to prevent concrete and water with cementitious particles from leaking.
- Unless otherwise noted, existing deck reinforcing steel shall not be removed or damaged. Reinforcing steel shall be re-used in the new deck and placed in the same orientation and location as originally shown on the existing drawings.
- All roughened surfaces in concrete shall be made with a minimum amplitude of 1/4".
- Unless otherwise noted on drawings, all exterior corners and re-entrant angles 90 dégrees or less in concrete work shall be chamfered 3/4"x3/4".
- Contractor shall hire a certified independent testing lab to take and test a minimum of seven 4"x8" test cylinders per 300 SF of deck per day. Three cylinders shall be tested before opening deck to traffic, three cylinders tested at 7 days, and one held as reserve. Cost is incidental.
- 14. Contractor option to use Maturity Method per ASTM C1074 prior to opening deck to traffic.
- Clean and coat new concrete and existing concrete surfaces within 5 feet of all new work with a concrete penetrating sealer such as Sika Ferrogard 903, or pre-reviewed equal. Clean and apply per sealant manufacturer literature.
- <u>Precast Concrete Plank Notes:</u> Refer to sheet S-17.

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# NON-SHRINK GROUT

1. Non-Shrink grout shall have a minimum 28 day compressive strength of 7,000 PSI.

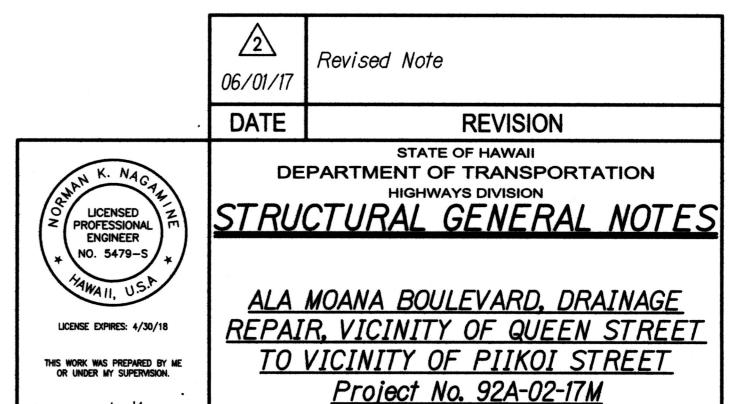
#### REINFORCING STEEL FOR SITE CAST CONCRETE

- Reinforcing steel bars shall be AASHTO M31 (ASTM A-615) Grade 60, unless otherwise noted. Provide ASTM A706 Grade 60 where noted on drawings.
- Reinforcing steel bars shall be uncoated, unless otherwise noted.
- Splices in reinforcing steel shall not be permitted, unless otherwise noted.
- 4. All reinforcing steel bars, anchor bolts, dowels and other embedded items shall be securely tied in place before concrete pour.
- All reinforcing steel bar bends shall be made cold. Field bending of reinforcing steel bar larger than #5 shall not be permitted. Refer to Standard Specifications Section 602.03 (C) (2).
- 6. Welding of reinforcing steel shall not be permitted, unless otherwise noted.

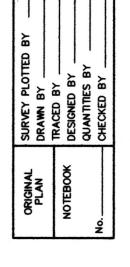
# INSPECTION REQUIREMENTS

- Contractor shall refer to Standard Specifications Section 105.11 - "Inspection of the Work and Materials."
- The work items that will require inspection by the Engineer shall be, but not be limited to, fhe following items:
  - A. Reinforcing steel / Prestress Strands Concrete
- Epoxy embedded reinforcing steel dowels

Contractor shall notify the Engineer at least 7 working days prior to the above inspections.
Inspection items listed are applicable to both site cast and precast concrete.

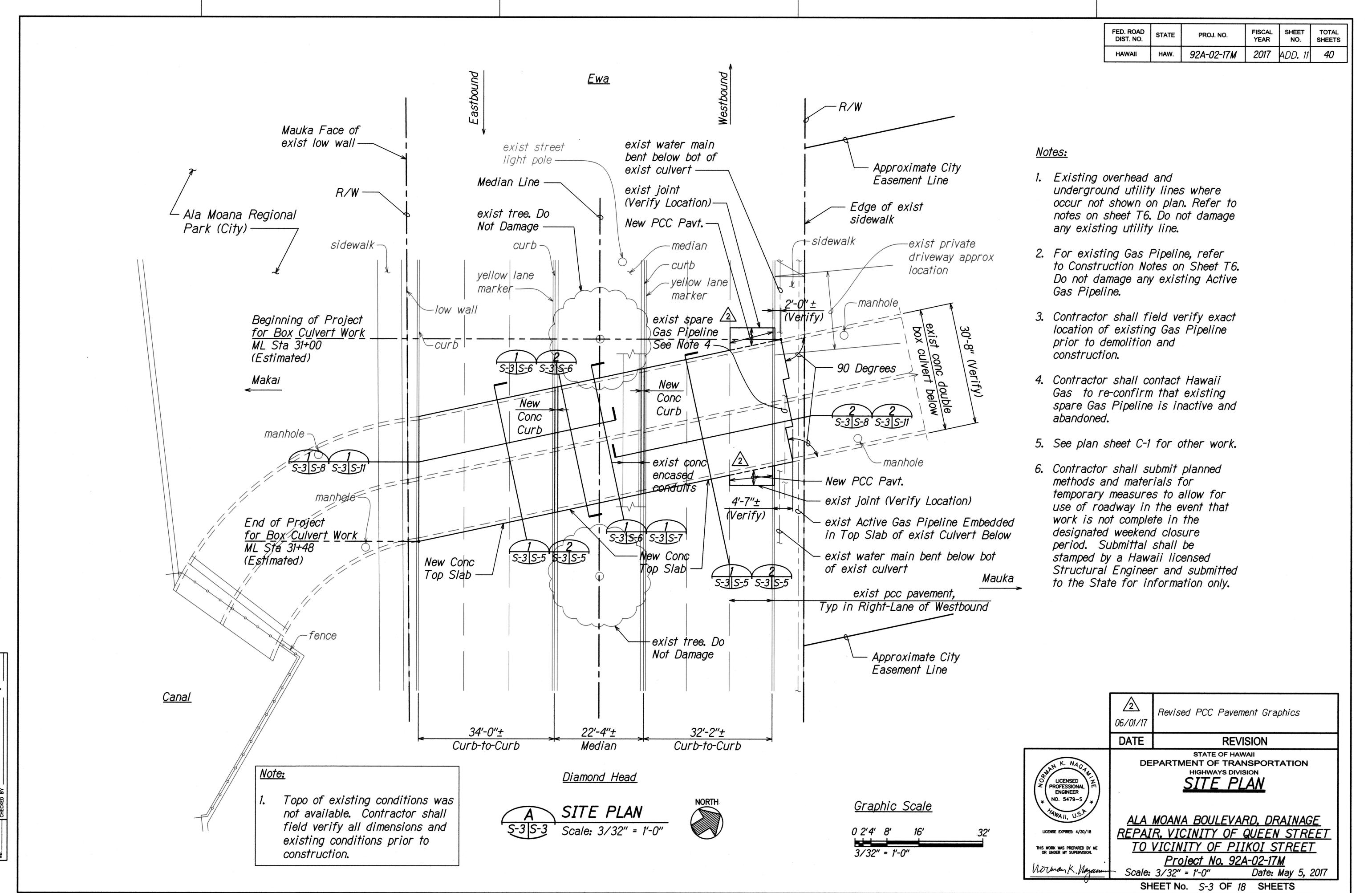


Norman K. Nazamu Scale: None SHEET No. S-1 OF 18 SHEETS

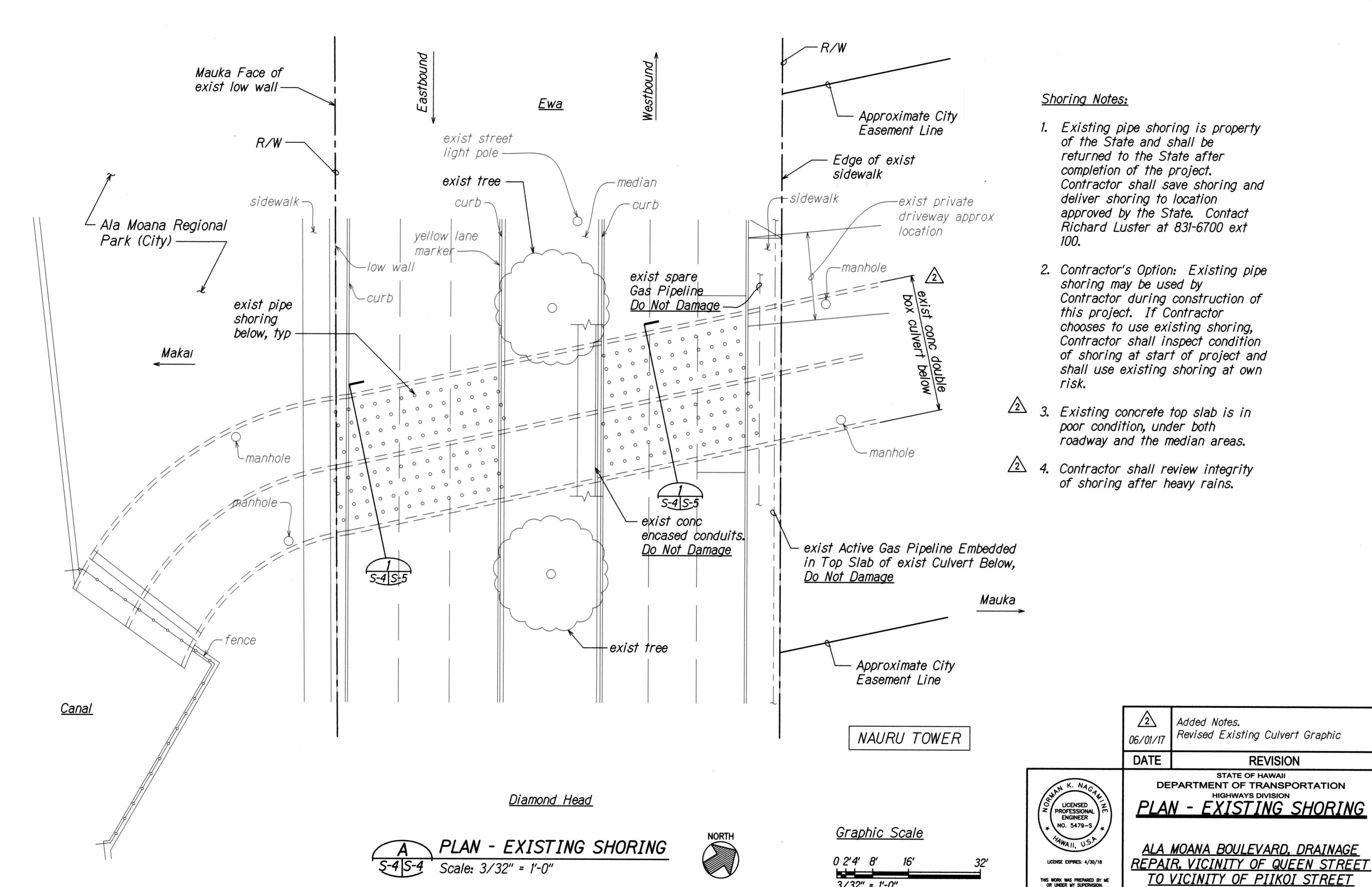


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Date: May 5, 2017



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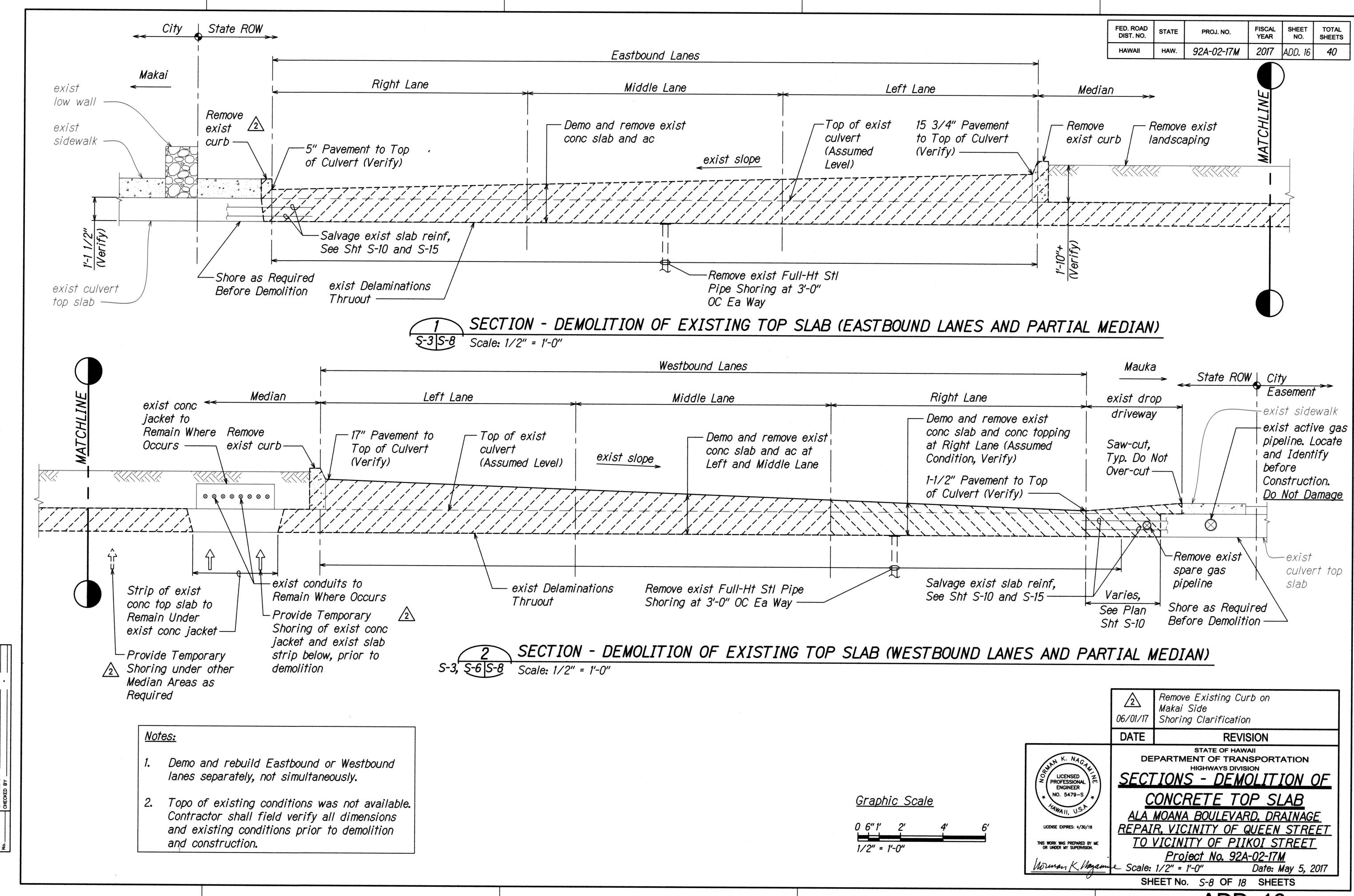
SHEET No. S-4 OF 18 SHEETS

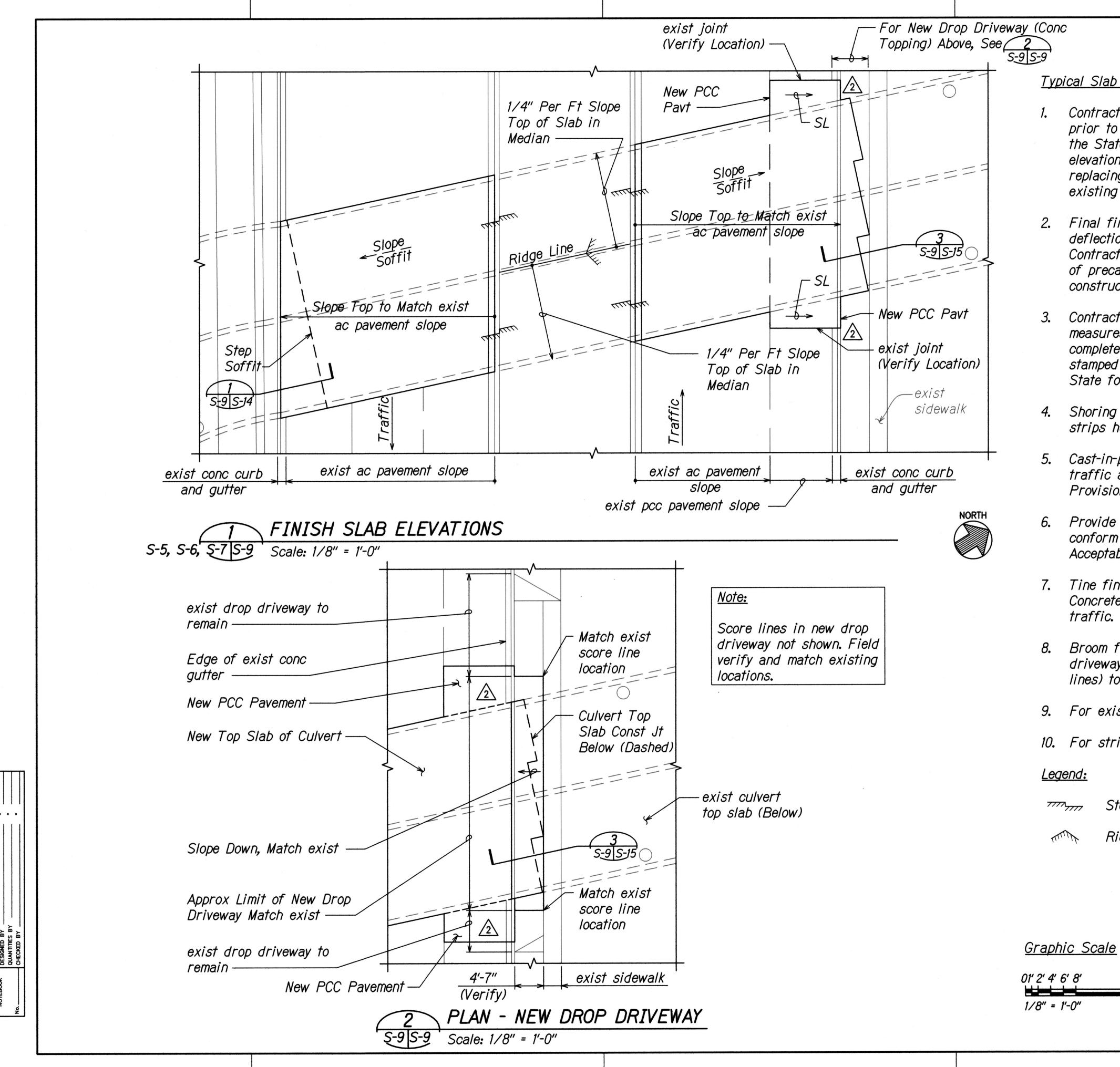
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THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

Normank, Magamin

3/32" = 1'-0"





Typical Slab Notes:

Contractor shall measure and record existing top of pavement elevations prior to demolition. Contractor shall provide these existing elevations to the State and Engineer prior to beginning demolition. New top of slab elevations shall match the top elevations for the pavement that it is replacing. Provide smooth transitions throughout and smooth transitions to existing AC roadway approach at each end of culvert.

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- 2. Final finish slopes and elevations of slab shall account for dead load deflection of slab and precast planks self-weight after removal of shores. Contractor shall submit final finish slopes, top of topping elevation and top of precast plank elevations to State and Engineer for review prior to construction.
- 3. Contractor shall submit planned methods and materials for temporary measures to allow for use of roadway in the event that work is not complete in the designated weekend closure periods. Submittal shall be stamped by a Hawaii licensed Structural Engineer and submitted to the State for information only.
- Shoring shall not be removed until concrete topping and cast-in-place pour strips have cured for 14 days minimum. See Standard Specs Section 503.
- Cast-in-place concrete shall be cured 1 hour minimum prior to opening to traffic and attain 3000 PSI minimum concrete strength. Refer to Special Provisions Section 503 for further requirements.
- 6. Provide curing compound after casting concrete. Curing Material shall conform to ASTM C1315 and ASTM C 309 Type II, Class B White pigment. Acceptable product is Unitex K.C. White Acrylic Cure, or approved equal.
- 7. Tine finish concrete surface in roadway. See Standard Specs Section 503 -Concrete Structures. Tine direction shall be transverse to direction of
- Broom finish concrete surface in new drop driveway to match existing drop driveway finish. Provide transverse and longitudinal control joints (score lines) to match existing score lines.
- For existing Gas Pipeline, refer to Construction Notes on Sheet T6.
- 10. For striping plan, see Civil drawings.

Step in Concrete

Ridge in Concrete

Revised PCC Pavement and Gutter/Curb Replacement Extent 06/01/17

DATE **REVISION** STATE OF HAWAII DEPARTMENT OF TRANSPORTATION

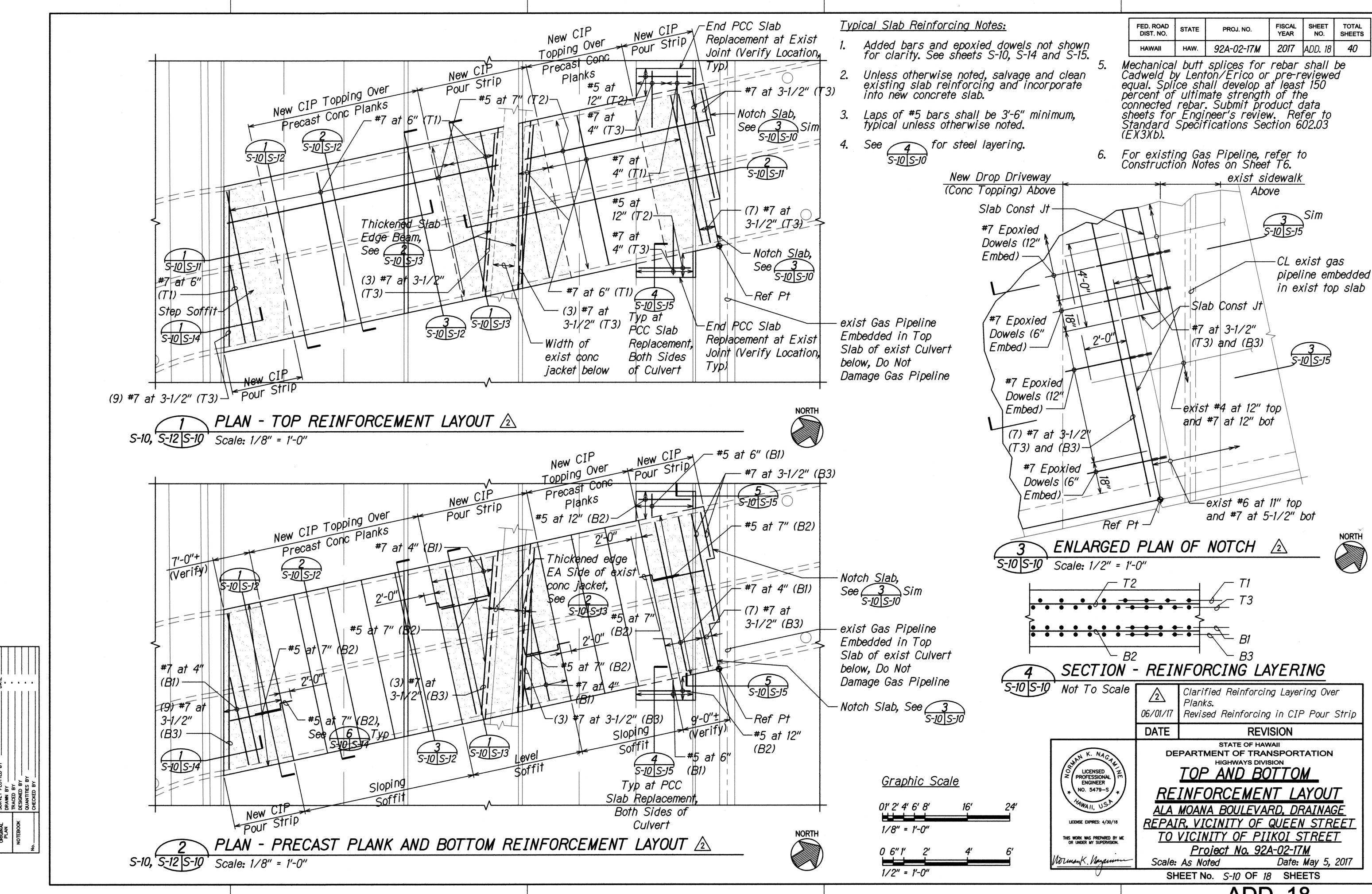
HIGHWAYS DIVISION LICENSED PROFESSIONAL ENGINEER FINISH SLAB ELEVATIONS NO. 5479-S

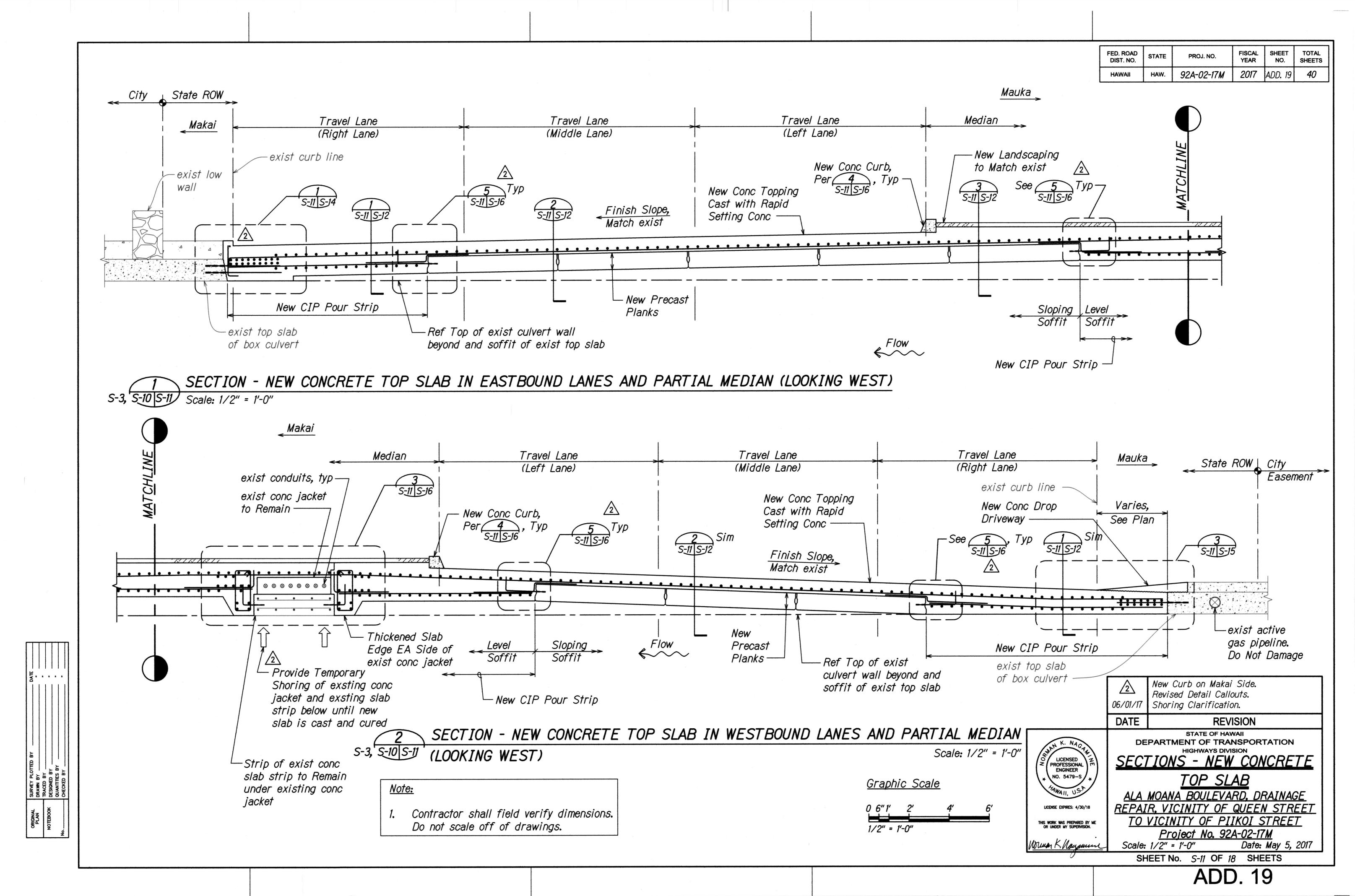
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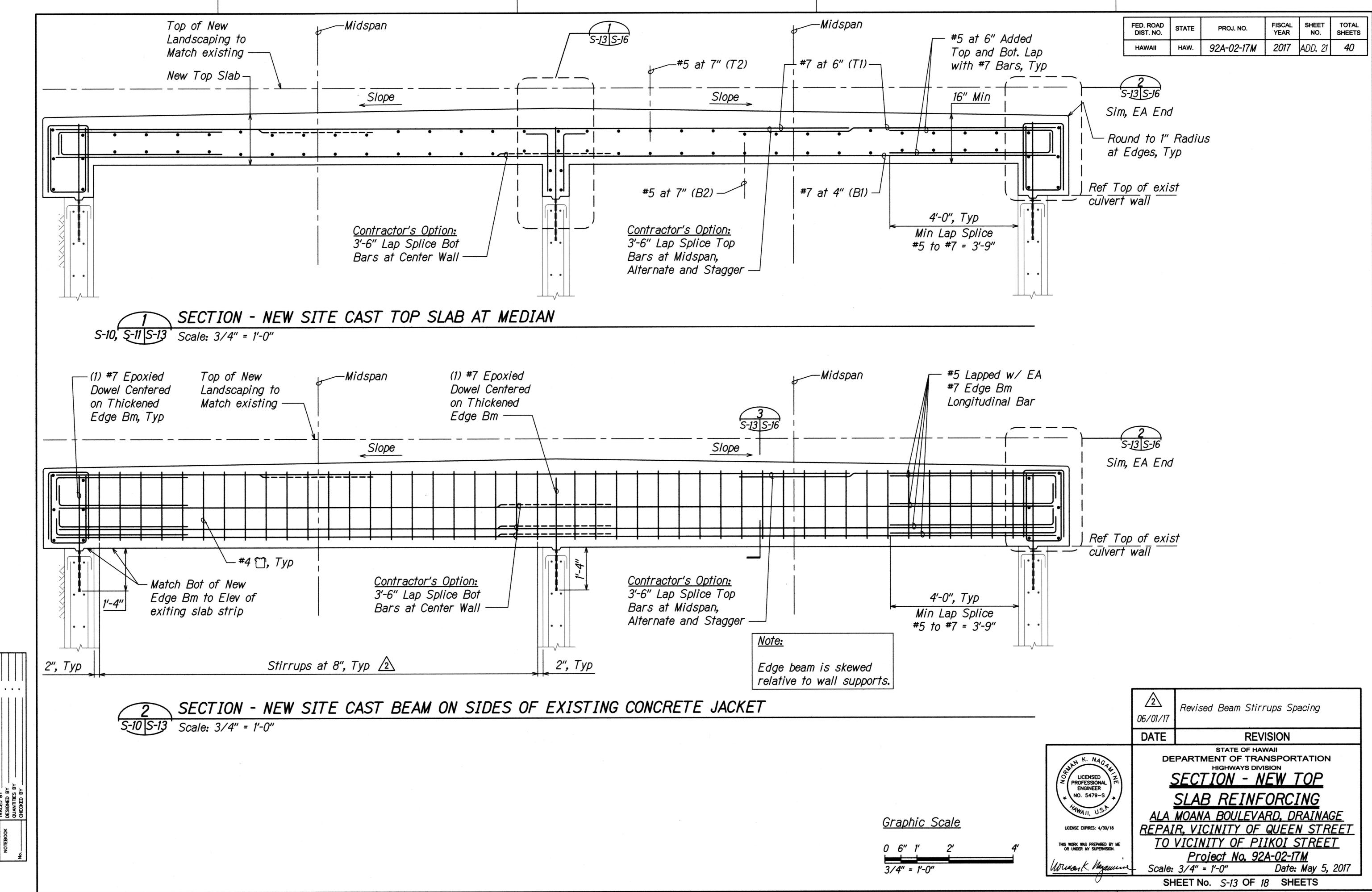
Worman K. Nogamm Scale: 1/8" = 1'-0"

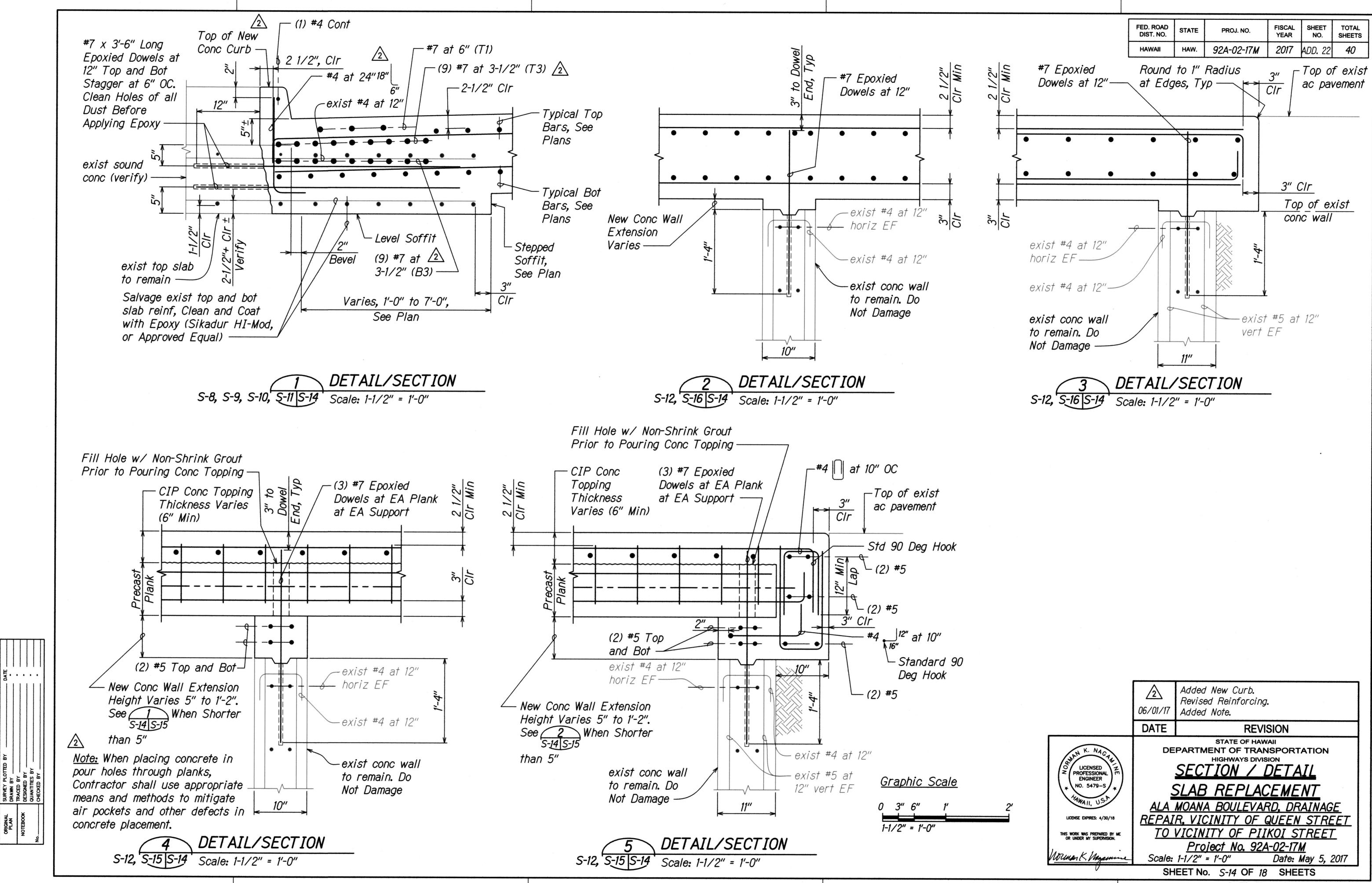
ALA MOANA BOULEVARD, DRAINAGE REPAIR, VICINITY OF QUEEN STREET TO VICINITY OF PIIKOI STREET Project No. 92A-02-17M Date: May 5, 2017

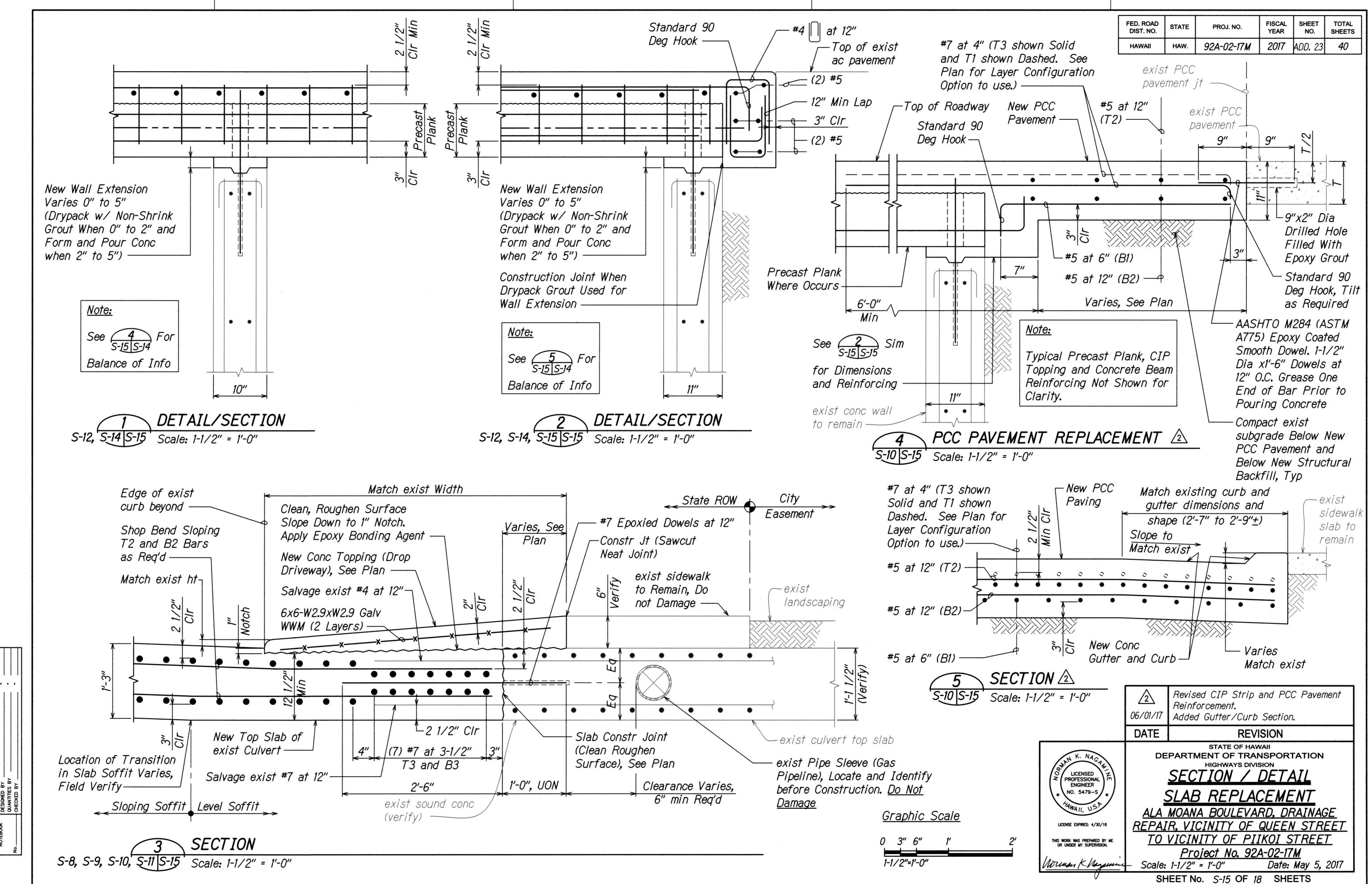
SHEET No. S-9 OF 18 SHEETS

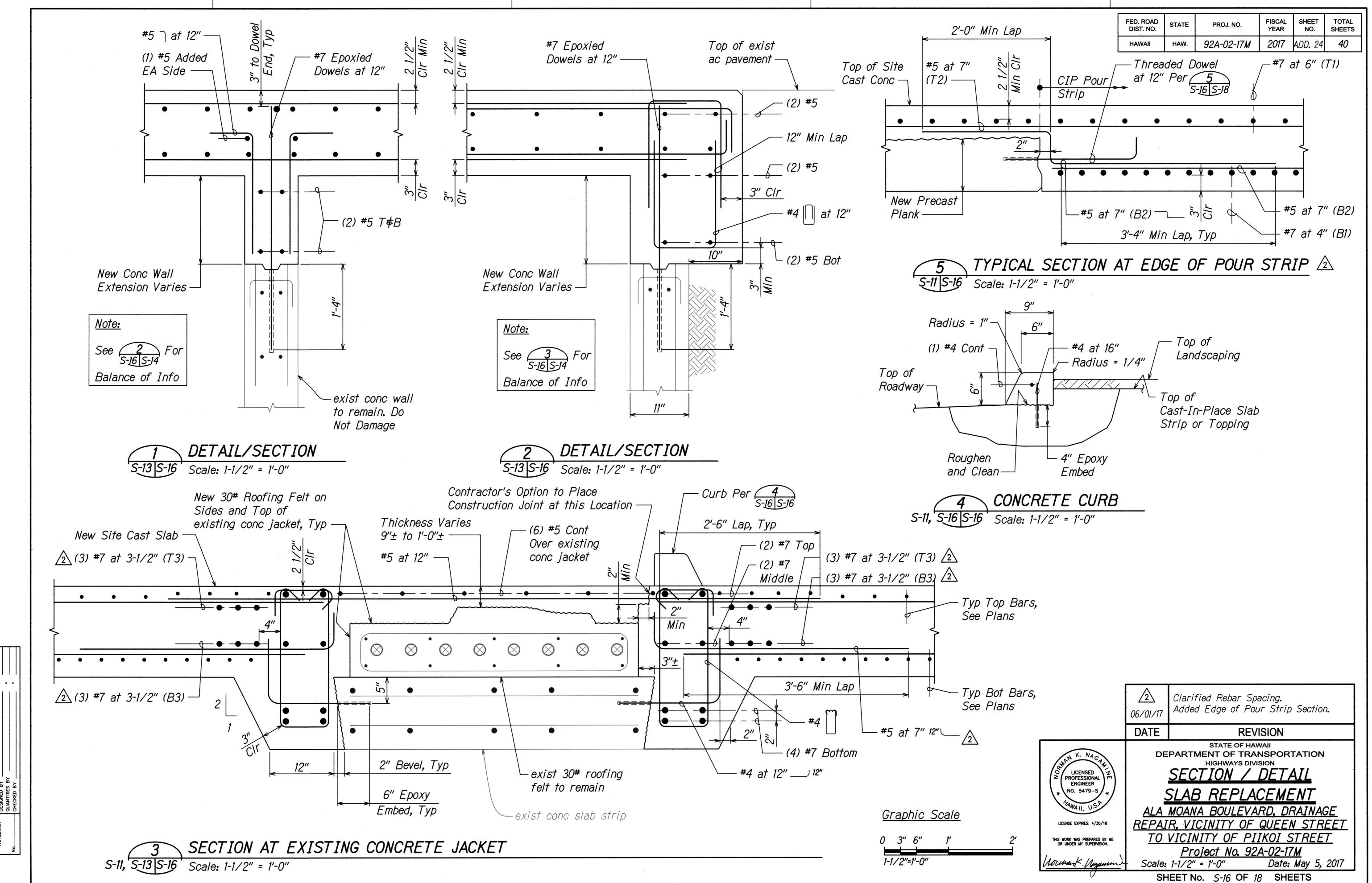


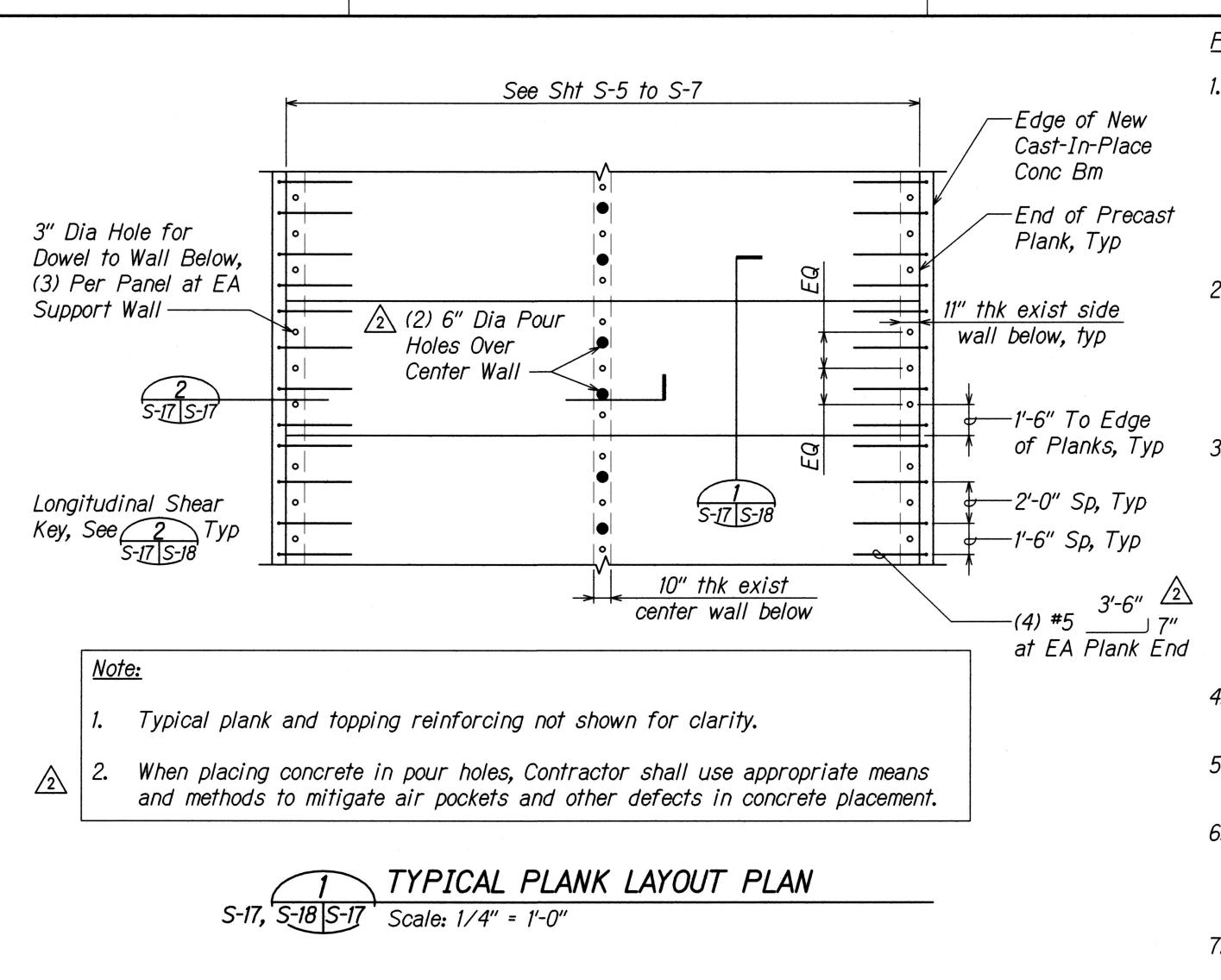












Precast Concrete Plank Notes:

1. Minimum concrete compressive strength of prestressed planks shall be as follows:

A. Final Concrete Strength

= 6,000 PSI at 28 days
B. Concrete Strength at Transfer
= 4,000 PSI

Prestressed strands shall be seven wire 1/2" diameter low relaxation steel strands (Area = 0.153 Sq In) with ultimate strength of 270 KSI. For properties, see standard specifications.

3. Non-prestressed reinforcing steel shall be AASHTO M31 (ASTM A615) grade 60, unless otherwise noted on plans. Provide ASTM A706 grade 60 where noted on drawings. For properties, see standard specifications. Cost of non-prestressed reinforcing steel shall be incidental to precast concrete work.

A. Reinforcing steel bars shall be uncoated, unless otherwise noted.

Splices in reinforcing steel shall not be permitted, unless otherwise noted.

All reinforcing steel bars, anchor bolts, dowels and other embedded items shall be securely tied in place before concrete pour.

7. All reinforcing steel bar bends shall be made cold. Field bending of steel shall not be permitted.

· (3) Spaces at 6"

8. Welding of reinforcing steel shall not be permitted, unless otherwise noted.

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9. Effective prestressing force is after all losses. Losses shall take into consideration creep, shrinkage, elastic shortening and relaxation of prestressing steel.

10. Dead load deflection is due to weight of topping.

11. Strand pattern shall be symmetrical about the longitudinal centerline of prestressed plank.

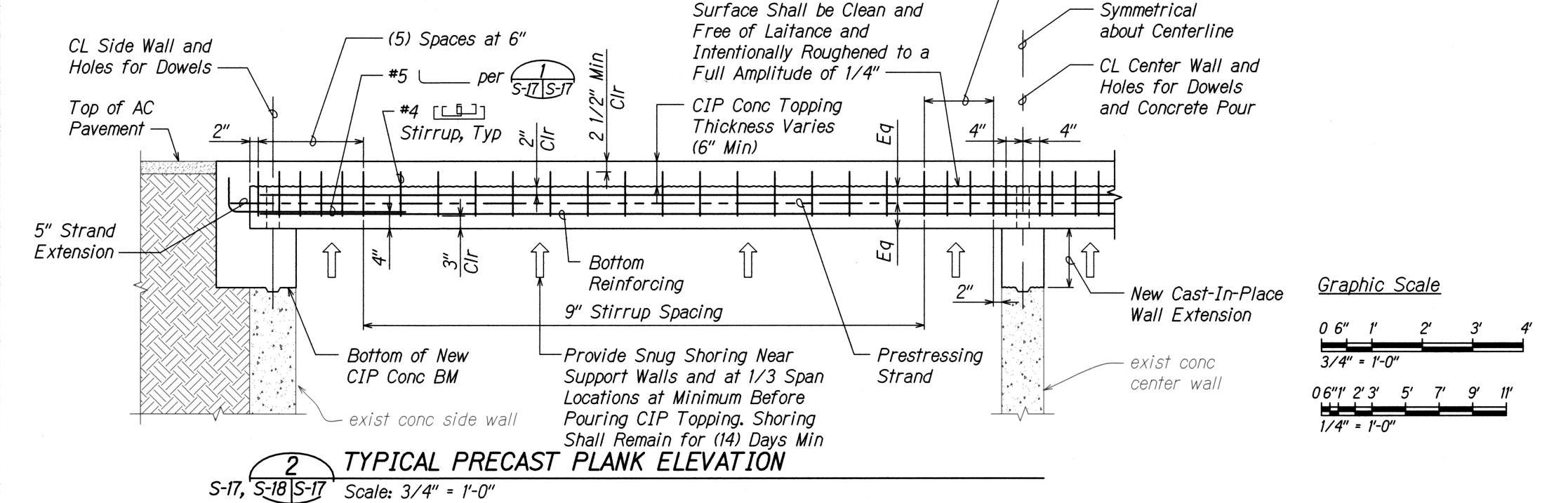
12. Strand release sequence shall not include any lateral deflection of the prestressed plank.

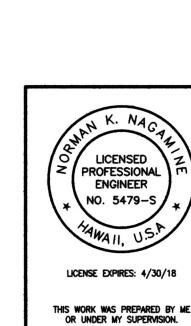
13. The Contractor shall submit his proposed strand pattern and releasing sequence to the engineer for review.

14. During curing, care shall be taken to avoid any lateral deflection of the prestressed plank due to improper orientation.

15. Elastic shortening shall be included in determining the length of the prestressed planks.

16. The Contractor shall incorporate all inserts, dowels and other embedded items required in the prestressed planks during fabrication.





Norman K. Mayan

Typical Plank Layout Plan - Revised Hole Locations.
Revised Hooked Rebar Locations.
Added Note 2.

DATE REVISION

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

PRECAST PLANK

DETAILS

ALA MOANA BOULEVARD, DRAINAGE

REPAIR, VICINITY OF QUEEN STREET

TO VICINITY OF PIIKOI STREET

Project No. 92A-02-17M

Scale: As Noted Date: May 5, 2017

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