Draw No.	Description
S-1	Index To Drawings, Abbreviations, Summary Of Estimated Quantities, Drilled Shaft Load Test Detail
S-2	Structural Notes
S-3	Bridge Plan, Longitudinal Section / Elevation
S-4	Typical Bridge Deck Section, Existing Bridge Demolition Plan
S-5	Foundation Plan, Drilled Shaft Detail, Abutment Section
S-6	Abutment No. 1 And 2 Elevations, Wing Wall Detail
S-7	Plank Layout Plan, Plank Details
S-8	Normal Deck Section, Threaded Insert Detail
S-9	End Post Type "a" Detail
S-10	End Post Type "b" And "c" Detail
S-11	Aesthetic Bridge Railing Detail
S-12	Metal Railing Details, Expansion Joint Details
S-13	Pedestrian Walkway Ramp Sections, Approach Slab Sections

Summary Of Estimated Quantities

Contract Item

Structure Excavation For Bridge Abutments And Wingwalls

Concrete In Approach Slabs (including Pedestrian Walkway Ramps)

Reinforcing Steel In Bridge (including Wing Walls, Approach Slabs, And Pedestrian Walkway Ramps)

Structure Backfill For Bridge Abutments And Wingwalls

Concrete In Abutments (including Wing Walls)

Aesthetic Bridge Railing (including End Post)

Concrete Jersey Barrier (including End Post)

Furnishing Drilled Shaft Drilling Equipment

Removal Of Existing Bridge

Filter Material

Metal Railing

Obstructions

Load Test

Trial Shaft

Concrete In Topping

18-inches Prestressed Planks

Standard Shaft Excavation

Drilled Shaft - (48-inch Diameter)

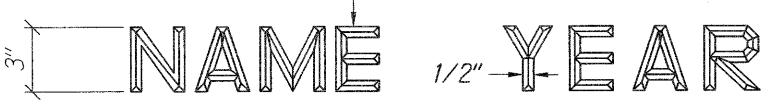
Coring For Integrity Testing

Special Shaft Excavation

<u>ABBREVIATIONS</u>	IONS	A		V	E	R		B	
----------------------	------	---	--	---	---	---	--	---	--

ø #	Diameter Number Or Pound	Lb., Lbs. L.f.	Pound, Pounds Linear Feet
A A	Anchor Bolt	L.s.	Lump Sum
A.b.	Asphalt Concrete		·
A.c.	Aspirati Concrete Azimuth	Max.	Maximum
Az.	AZIMOTT	Min.	Minimum
Bot., Bott., B	Bottom	No., #	Number
C.j.	Construction Joint	N.f.	Near Face
¢	Centerline	N.t.s.	Not To Scale
C.g.	Center Of Gravity		
Cir., Ci.	Clear	0.c.	On Center
Conc.	Concrete	<b>5</b> 5	David Day Outin Fort
Cont.	Continuous	Pcf	Pounds Per Cubic Feet
C.y.	Cubic Yard	PI, PL	Plate
U.y.	Cabic Tard	<i>Psf</i>	Pounds Per Square Fee
Dbl.	Double	<i>Psi</i>	Pounds Per Square Inc
Det.	Detail	Pvc.	Polyvinyl Chloride
D.i.	Ductile Iron		·
Dia.	Diameter	R, Rad.	Radius
Dn.	Down	Rebar	Reinforcing Bar
Dwg.	Drawing	Ref.	Reference
_	Ench Ench	Reinf.	Reinforced, Reinforcing,
E.f.	Each Face		Reinforcement
E.j.	Expansion Joint	R.o.w.	Right-of-way
Elev., El.	Elevation		
E.w.	Each Way	Sht.	Sheet
Exp.	Expansion	<i>S1</i> .	Slope
F.b.	Flat Bar	Sta.	Station
F.f.	Far Face	Std.	Standard
		Stirr.	Stirrup
G	Girder	Sym., Symm.	Symmetrical Symmetrical
Galv.	Galvanized	S.s.	Stainless Steel
G.j.	Grooved Joint		
		Thk., Th.	Thick
Horiz., H	Horizontal	Typ.	Typical
In.	Inch	Vert., V	Vertical
Jt.	Joint	2 Az 1 de 2	, o, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
V	Vina	W/	With
K	Kips		
Ksi	Kips Per Square Inch		

Depressed "V" Letters 3/8" Deep ——



Use Correct Name Of Bridge

Year Built

Unit

*l.s.* 

c.y.

c.y.

c.y.

1.5.

l.s.

l.s.

1.5.

l.f.

l.f.

l.f.

1.5.

hour

each

each

l.f.

i.f.

l.f.

*l.s.* 

Quantity

l.s.

410

110

I.S.

l.s.

l.s.

1.5.

90

134

67

1.5.

40

320

240

*560* 

110

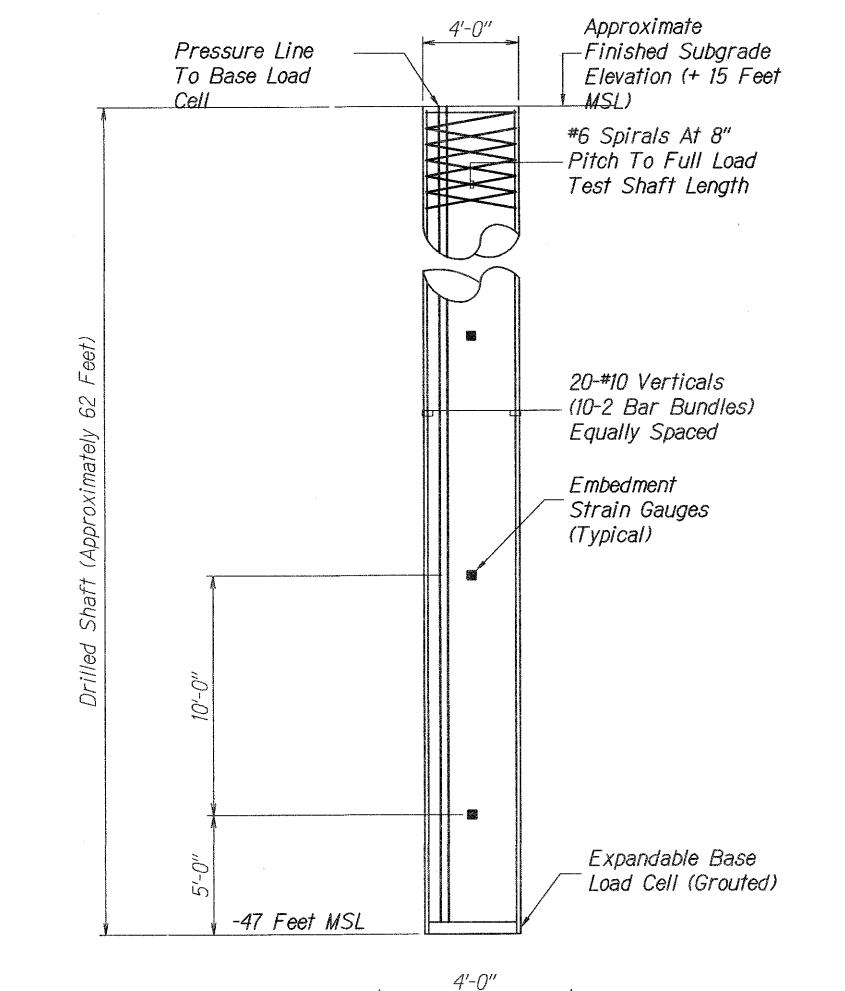
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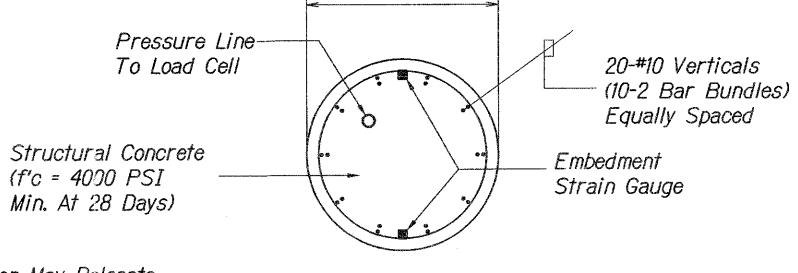
- 1. Name And Date Shall Be Placed At The Trailing End Post On Each Side Of The Roadway.
- 2. Exact Details And Spacing Of Letters And Figures And Location Shall Be As Directed By The Engineer. Gothic Letters And Figures Approximating Dimensions Shown Will Be Acceptable If Approved By The Engineer.
- 3. The Name Of Bridge Shall Be "Kokololio Stream Bridge."

TYPICAL DETAIL OF LETTERS AND FIGURES AT CONCRETE END POST

Not To Scale

FED. ROAD STATE FISCAL SHEET FED. AID YEAR NO. PROJ. NO. SHEETS DIST. NO. BR-083-1(50) 2005 HAW.

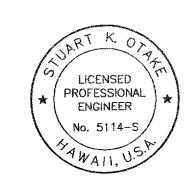




The Engineer May Relocate The Expandable Base Load Cell In The Field, As Required.

SECTION





DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION INDEX TO DRAWINGS, ABBREVIATIONS, SUMMARY OF ESTIMATED QUANTITIES

STATE OF HAWAII

KAMEHAMEHA HIGHWAY REPLACEMENT OF KOKOLOLIO STREAM BRIDGE Federal Aid Project No. BR-083-1(50)

Scale: As Noted Date: August 2004 SHEET No. 8-1 OF **13** SHEETS

QUANTITIES BY No. CHECKED BY	ORIGINAL SURVEY PLOTTED BY	SURVEY PLOTTED BY DRAWN BY TRACED BY DESIGNED BY QUANTITIES BY CHECKED BY

Item No.

202.0440

206.6100

206.7200

206.8000

503.1090

503.1091

503.1092

504.7400

507.1501

507.7000

507.7001

511.0100

511.0200

511.0300

511.0400

511.0500

511.0600

511.0700

511.0800

602.0090

NOTE: Contractor to check and verify dimensions at job before proceeding with work.

- General Specifications: Hawaii Standard Specifications For Road, Bridge, And Public Works Construction, 1994, Together With Special Provisions Prepared For This Contract.
- 2. Design Specifications: Aashto Lrfd Bridge Design Specifications, 2nd Edition, 1998, Including Subsequent Interim Revisions.
- 3. Loads:
  - Dead Loads:
    - 1. An Allowance Of 25 Psf (from Curb-to-curb) Has Been Provided For In The Design For Future Wearing Surface.
    - 2. An Allowance Of 150 Plf (at Each Side Of The Bridge) Has Been Provided For In The Design For Future Utilities.
  - Live Load: HI-93
  - Seismic Load: Acceleration Coefficient = 0.17

seismic Performance Zone = 2 importance Category = Critical Bridge soil Profile Type I (s = 1.0)

D. Railing Test Level = TI-2

4. Materials:

- A. Minimum Concrete Compressive Strength (at 28 Days):
  - Concrete Topping = 5,000 Psi
  - 2. All Other Concrete = 4,000 Psi. Concrete Shall Have 0.45 Maximum Water Cement
- All Reinforcing Steel Shall Conform To Astm A615, Grade 60, Unless Otherwise Noted.
- All Pipe Sections Shall Conform To Astm A53 (type E), Grade B, And Be Hot-dip Galvanized After Fabrication.
- D. All Stainless Steel Plates, Bars, Rods, Anchor Bolts And Shapes Shall Be Type 316 Or 3161. Welding Of Stainless Steel Shall Be In Accordance With The Latest Edition Of Aws D1.6 "structural Welding Code - Stainless Steel."
- Stainless Steel Reinforcing Bars Shall Be Aisi 3161 Grade Conforming To Astm A955 Grade
- F. Stainless Steel Clad Reinforcing Bars Shall Be Aisi 3161 Grade Conforming To Astm A955 Grade 60 And Have An Average Cladding Thickness Of 0.04 Inch. The Ends Of Stainless Steel Clad Dowels Shall Be Sealed With Epoxy As Per Section 11 Of Astm A755.
- Tetraguard As20 Shrinkage Reducing Admixture, Eclipse Plus Shrinkage Reducing Admixture, Or An Approved Equal, Shall Be Included In The Concrete Mix For The Concrete Topping, Concrete Jersey Barrier, And Aesthetic Bridge Railing. The Required Dosage Shall Be 96 Ounces Per Cubic Yard Of Concrete. Addition Of Shrinkage Reducing Admixture Shall Be As Recommended By The Manufacturer.
- H. For Materials Of Prestressed Planks, See Applicable Prestressed Plank Notes.
- Thru Bolts For Guardrail Connection Shall Conform To Aashto M164 (astm A325), Unless Otherwise Noted.
- A Corrosion Inhibiting Admixture Shall Be Included In The Concrete Mix For All Concrete Except The Drilled Shafts. The Corrosion Inhibiting Admixture Shall Contain A Minimum Of 30% Calcium Nitrate By Mass And Shall Be Added At A Dosage Rate Of 4.0 Gallons Per Cubic Yard Of Concrete. The Admixture Shall Be Rheocrete Cni Calsium Nitrite-based Corrosion Inhibitor, Dci S Corrosion Inhibitor, Or An Approved Equal. Addition Of Corrosion Inhibiting Admixture Shall Be As Recommended By The Manufacturer.
- 5. Reinforcement:

E. . . . .

SURVEY PLOTTE
DRAWN BY
TRACED BY
DESIGNED BY
QUANTITIES BY
CHECKED BY

- Unless Otherwise Noted, The Covering Measured From The Surface Of The Concrete To The Face Of Any Reinforcing Bars Shall Be As Follows:
  - Deck Top Bars = 2" Clear (with Tolerances Of -0 Inch And + 3/8" Inch)
  - Deck Bottom Bars = 1-1/4" Clear
  - Railings And Parapets = 2" Clear Formed Surfaces Exposed To Earth And Weather = 2" Clear
  - Bottom And Sides Of Footings And Where Concrete Deposited On Grade = 3" Clear
  - 6. Drilled Shafts = 3" Clear (to Spirals)
- B. Minimum Clear Spacing Between Parallel Bars Shall Be 1-1/2 Times The Diameter Of The Bar (for Non Bundled Bars) Or 1-1/2 Times The Diameter Derived From The Equivalent Total Area Of The Bars (for Bundled Bars), But In No Case Shall The Clear Distance Between The Parallel Bars Be Less Than 1-1/2 Times The Maximum Size Of The Coarse Aggregate Or 1-1/2 Inches.
- All Dimensions Relating To Reinforcing Bars (e.g. Spacing Of Bars, Etc.) Are To Center Of Bars, Unless Otherwise Noted. Reinforcing Bars Shall Be Detailed In Accordance With Aashto Lrfd Bridge Design Specifications, 2nd Edition, 1998, Including Subsequent Interim Revisions, Unless Otherwise Noted.
- Reinforcing Bars Shall Be Securely Tied At All Intersections And Lap Splices Except Where The Spacing Of The Intersections Is Less Than 12 Inches In Each Direction, In Which Case Alternate Intersections Shall Be Tied.
- Deformed Reinforcing Bars To Be Welded Shall Conform To Astm A706, Grade 60. Welding Of Deformed Reinforcing Bars Shall Be In Accordance With The Latest Edition Of Aws D1.4 "structural Welding Code - Reinforcing Bars."

## STRUCTURAL NOTES

- 6. General Construction Notes:
  - See Standard Specifications And Special Provisions.
  - All Items Noted Incidental Will Not Be Paid For Separately.
  - Standard Detail Drawings Refer To All Structures In General, Except For Modifications As May Be Required For Special Conditions. For Such Modifications, Refer To The Corresponding Detailed Drawings.
  - D. The Contractor Shall Comply With All Construction Permits For This Project. In Addition, The Contractor Shall Comply With All Applicable Laws Of The Federal, State And County
  - E. Unless Otherwise Noted, All Vertical Dimensions Are Measured Plumb.
  - F. The Contractor Shall Verify All Site Conditions Before Commencing With Work.
  - G. The Contractor Shall Verify The Location Of All Underground Utility Lines And Notify The Respective Owners Before Commencing The Work Of Excavation Or Drilled Shafts.
  - H. For Concrete Finish, See Standard Specifications.
- I. Unless Otherwise Noted, All Exposed Concrete Edges Shall Be Chamfered 3/4" X 3/4". 7. Prestressed Plank Bearing Surface:
  - A. Prestressed Plank Bearing Surface Shall Be Smooth And Sloped To Match The Finish Roadway Slope.
  - Prestressed Planks Shall Be Set On A Fresh Layer Of Mortar To Insure Full Bearing.
  - Prestressed Plank Shelf Elevations Shall Be Verified By The Contractor. Shelf Elevations Shall Take Into Consideration The Concrete Topping Thickness, Prestressed Plank Thickness, Roadway Slope, And The Calculated, Or If Available, The Actual Camber Of The Prestressed Planks.
- Foundation Excavation And Dewatering:
- A. Temporary Shoring (such As Interlocking Steel Sheetpile Cofferdams) And Dewatering May Be Required For Foundation Excavations. The Contractor Shall Refer To The Geotechnical Engineering Exploration Report By Geolabs, Inc. Dated July 25, 2003for Its Recommendations.
- B. Temporary Shoring And Dewatering, If Required, Shall Be Considered Incidental To Structure Excavation.
- 9. Foundation:

these Foundation Notes Were Based On Recommendations Contained In A Geotechnical Engineering Exploration Report By Geolabs, Inc. Dated July 25, 2003. The Report Shall Be Considered As Part Of The Construction Documents And Its Recommendations Shall Be Implemented Unless Otherwise Directed By The Geotechnical Engineer. Contractor May Obtain A Copy Of The Report At The State Of Hawaii, Department Of Transportation - Highways Division Upon Request.

- A. 4'-0" Diameter Drilled Shafts:
  - The Compressive Load Capacity For The Drilled Shafts Are From Friction Between The Concrete Shaft And The Surrounding Soils And Rock Formation. The Following Compressive Load Capacity Were Used For Design Of The Abutments: - Strength Limit State = 450 Kips
    - Extreme Event Limit State = 750 Kips
  - 2. The Uplift Load Capacity For The Drilled Shafts Are From A Combination Of Friction Between The Concrete Shaft And The Surrounding Soils And Rock Formation And From The Self Weight Of The Shaft. The Following Uplift Load Capacity Were Used For Design Of The Abutments:
    - Strength Limit State = 350 Kips
    - Extreme Event Limit State = 630 Kips
  - The Lateral Load Resistance For The Drilled Shafts Is Based On The Stiffness Of The Surrounding Soil, The Stiffness Of The Shaft, Allowable Deflection At The Top Of Shaft, And The Induced Moment In The Shaft. The Foundation Loads, Lateral Deflection And Maximum Induced Moments For The Drilled Shafts, Based On A Fixed Against Rotation Boundary Condition At The Top Of The Drilled Shaft, Are As Follows:
    - A) Longitudinal Lateral Load:
      - Strenath Limit State:
        - Longitudinal Shear Load At Top Of Shaft = 80 Kips
        - Longitudinal Moment At Top Of Shaft = 1,840 Ft-kips
        - Longitudinal Lateral Deflection = 0 Inches
        - Maximum Induced Moment = 1,005 Ft-kips
      - 2) Extreme Event Limit State:
        - Longitudinal Shear Load At Top Of Shaft = 100 Kips
        - Longitudinal Moment At Top Of Shaft = 1,680 Ft-kips - Longitudinal Lateral Deflection = 2.6 Inches
        - Maximum Induced Moment = 1,850 Ft-kips
      - B) Transverse Lateral Load:

      - 1) Extreme Event Limit State:
        - Transverse Shear Load At Top Of Shaft = 38 Kips
        - Transverse Lateral Deflection = 0.3 Inches
        - Maximum Induced Moment = 319 Ft-kips

- FED. ROAD STATE FISCAL SHEET TOTAL FED. AID YEAR DIST. NO. PROJ. NO. NO. SHEETS HAW. BR-083-1(50) 2005 55
- The Drilled Shaft Estimated Tip Elevations Shown On The Plans Are Based On The Boring Data. The Actual Tip Elevations Could Change Due To Varying Subsurface Conditions. The Geotechnical Engineer Of Record Shall Be Present During The Drilling Operation To Determine That The Actual Subsurface Conditions Are Consistent With The Conditions Assumed For Design. Based On The Geotechnical Engineers Recommendations, The Tip Elevations Could Change. The Contractor Shall Make Provisions For Extension Of The Reinforcing Steel Cages For The Drilled Shafts To Account For Variations In The Final Tip Elevations.
- 5. The Contractor Shall Exercise Care In Drilling The Shaft Holes And In Placing Concrete Into The Holes. Cobbles And Boulders Were Encountered In Several Of The Borings Drilled For This Project. Therefore, Difficult Drilling Conditions Likely Will Be Encountered At The Site And Should Be Expected. The Drilled Shaft Contractor Will Need To Have The Appropriate Equipment And Tools To Drill Through These Types Of Natural Obstructions, Where Encountered. Appropriate Measures Will Also Be Needed To Avoid Dislodging Boulders Into The Drilled Shaft Hole During The
- Drilling And Shaft Installation Process. 6. The Existing Bridge Footings May Be Encountered During Drilling Operations. The Drilled Shaft Contractor Will Need To Have The Appropriate Equipment And Tools To Drill Through The Existing Bridge Footings If Encountered.
- The Drilled Shafts Are Designed To Be Embedded Into The Basalt Formation Encountered At Greater Depths. Therefore, Coring Into The Dense Basalt Formations Encountered In The Borings Will Be Required.
- Due To The Potential For Presence Of Shallow Groundwater Zones And Loose To Medium Dense Coralline Materials Encountered In The Borings, Cave-in Of The Material May Occur During The Drilling Operation. To Reduce The Potential For Caving In Of The Holes, Temporary Casing Will Likely Be Needed.
- 9. Drilling Shall Not Be Conducted By Methods Utilizing Drilling Fluids Unless Approved By The Engineer.
- 10. Concrete Placement By Tremie Methods Is Recommended During Construction Of The Drilled Shafts. The Concrete Shall Be Placed Promptly After Completion Of Drilling (within 24 Hours) To Reduce The Potential Of Caving In Of The Sidewalls. The Concrete Shall Be Placed In A Suitable Manner By Displacing The Water In An Upward Fashion From The Bottom Of The Drilled Hole.
- 11. The Geotechnical Engineer Of Record Shall Be Present During The Trial Shaft Program To Evaluate The Contractor's Method Of Drilled Shaft Installation And To Evaluate The Subsurface Materials Encountered; During The Installation And Performance Of The Instrumented Load Test On The Drilled Shaft; And At The Site To Observe The Drilling And Installation Of Drilled Shafts During Construction.
- B. Abutment And Wing Walls:
  - 1. Static Lateral Earth Pressures:
    - Active Earth Pressure (level Backfill) = 40 Pcf
  - At-rest Earth Pressure (level Backfill) = 58 Pcf
- 2. Dynamic Lateral Earth Forces:
- Level Backfill = 7 X H Psf (rectangular Distribution)
- 3. Lateral Load Resistance Of Abutment Fills:
- Extreme Event Limit State = 4 Ksf/inch (1.25 Inches Maximum Deflection) Structural Backfill:
- 1. Structural Backfill Shall Be In Accordance With The
- Requirements For Structure Backfill Material A As Indicated In Section 703.20 Of The Hawaii Standard Specifications For Road, Bridge, And Public Works Construction, 1994 And The Special Provisions.
- 2. Placement Of The Structural Backfill Shall Be In Accordance With The Standard Specifications And The Special Provisions.
- Structural Backfill On The Backface Of The Abutment Shall Not Be Placed Until The Bridge Concrete Topping Has Been Poured And Has Attained It's 28-day Concrete Compressive Strength.



MITSUNAGA & ASSOCIATES, INC. EXP. DATI

NOTE: Contractor to check and verify

STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

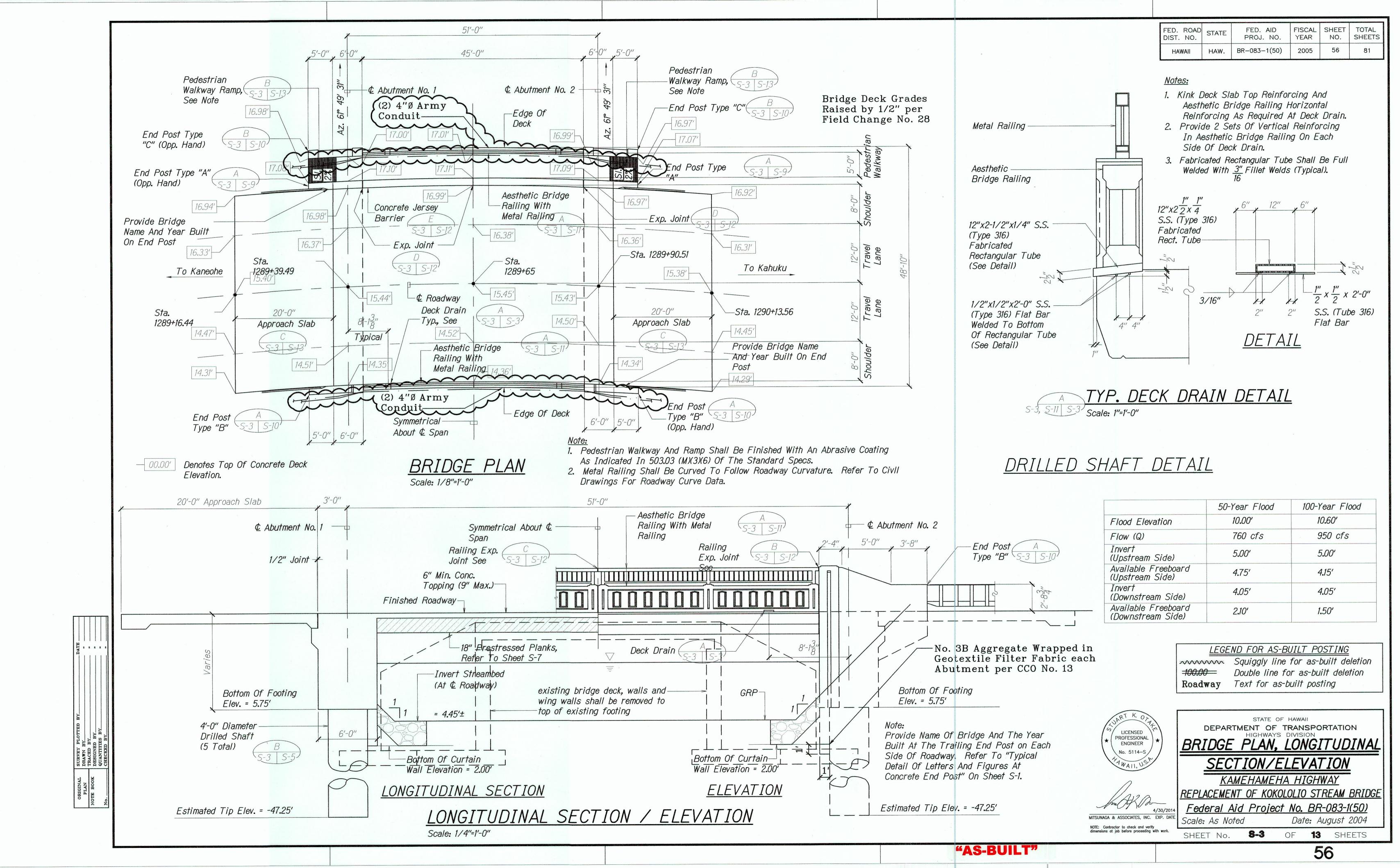
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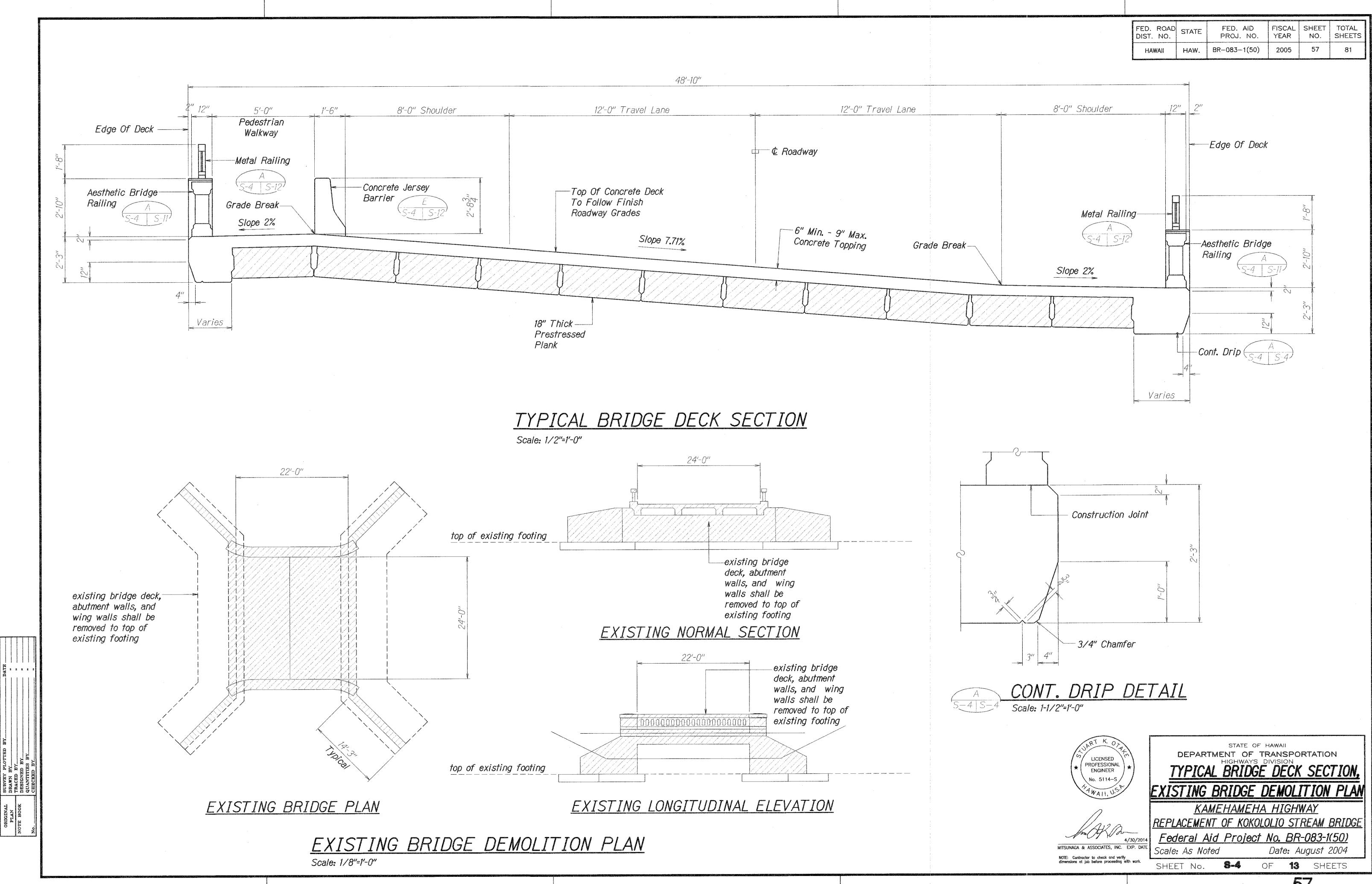
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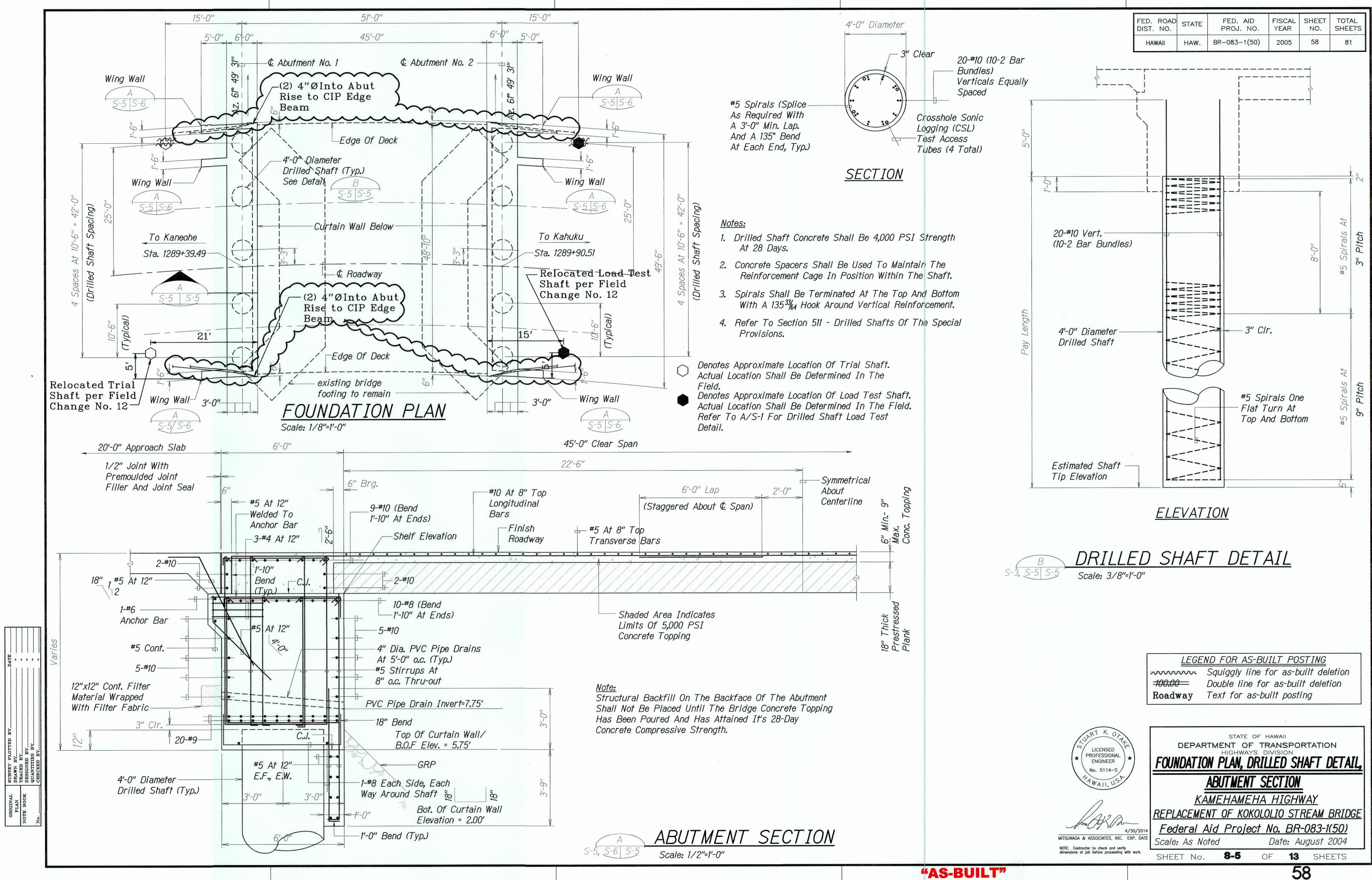
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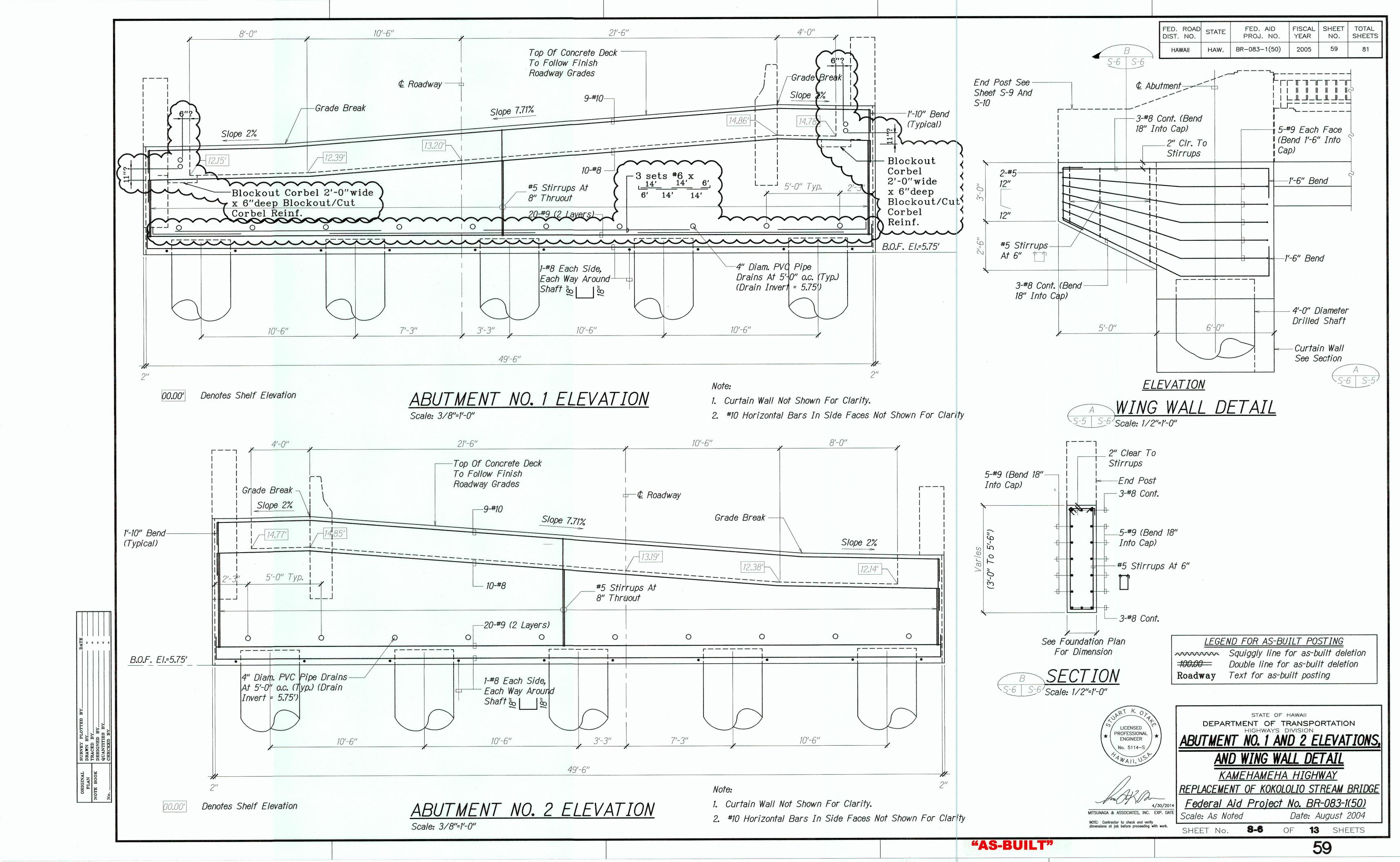
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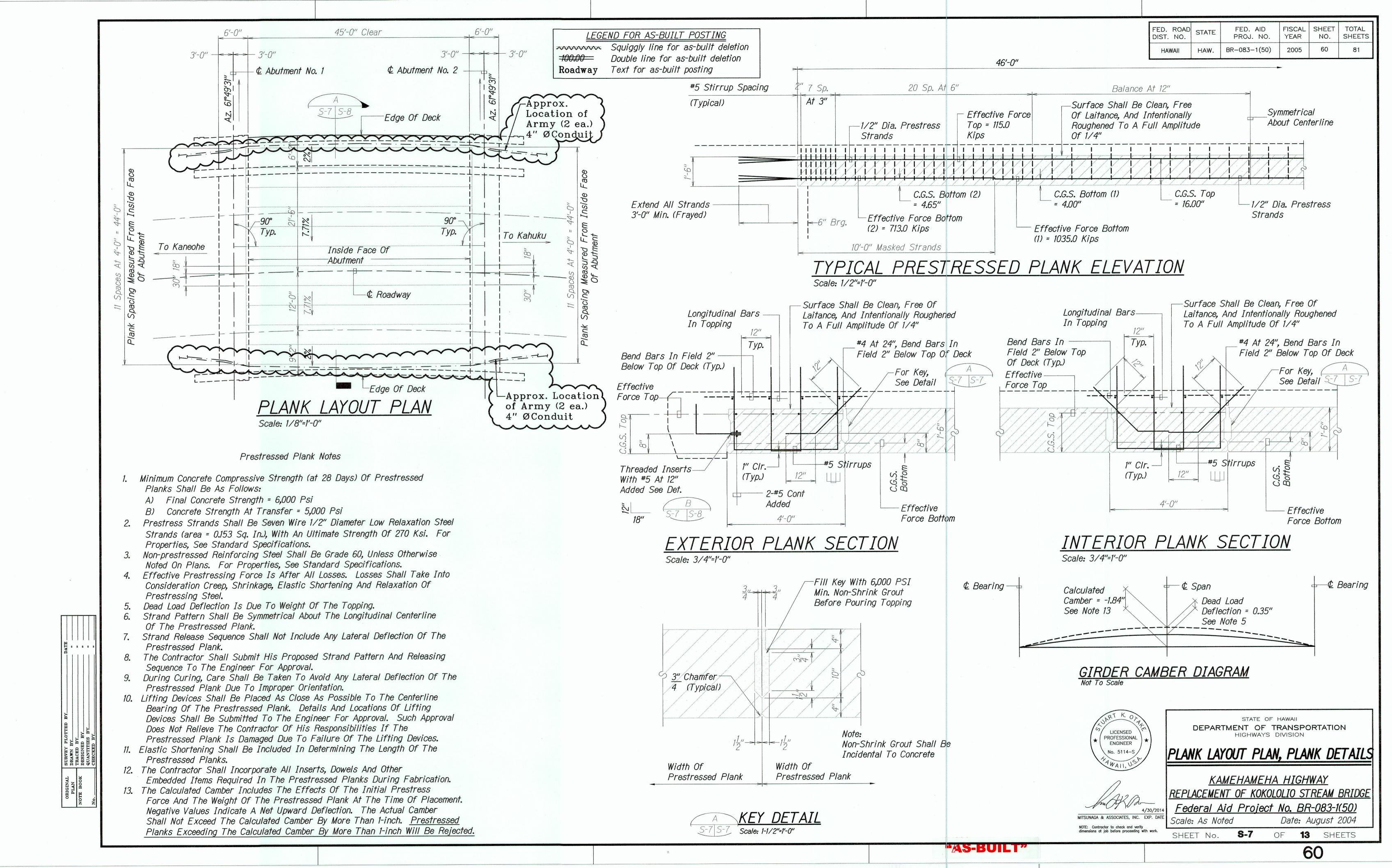
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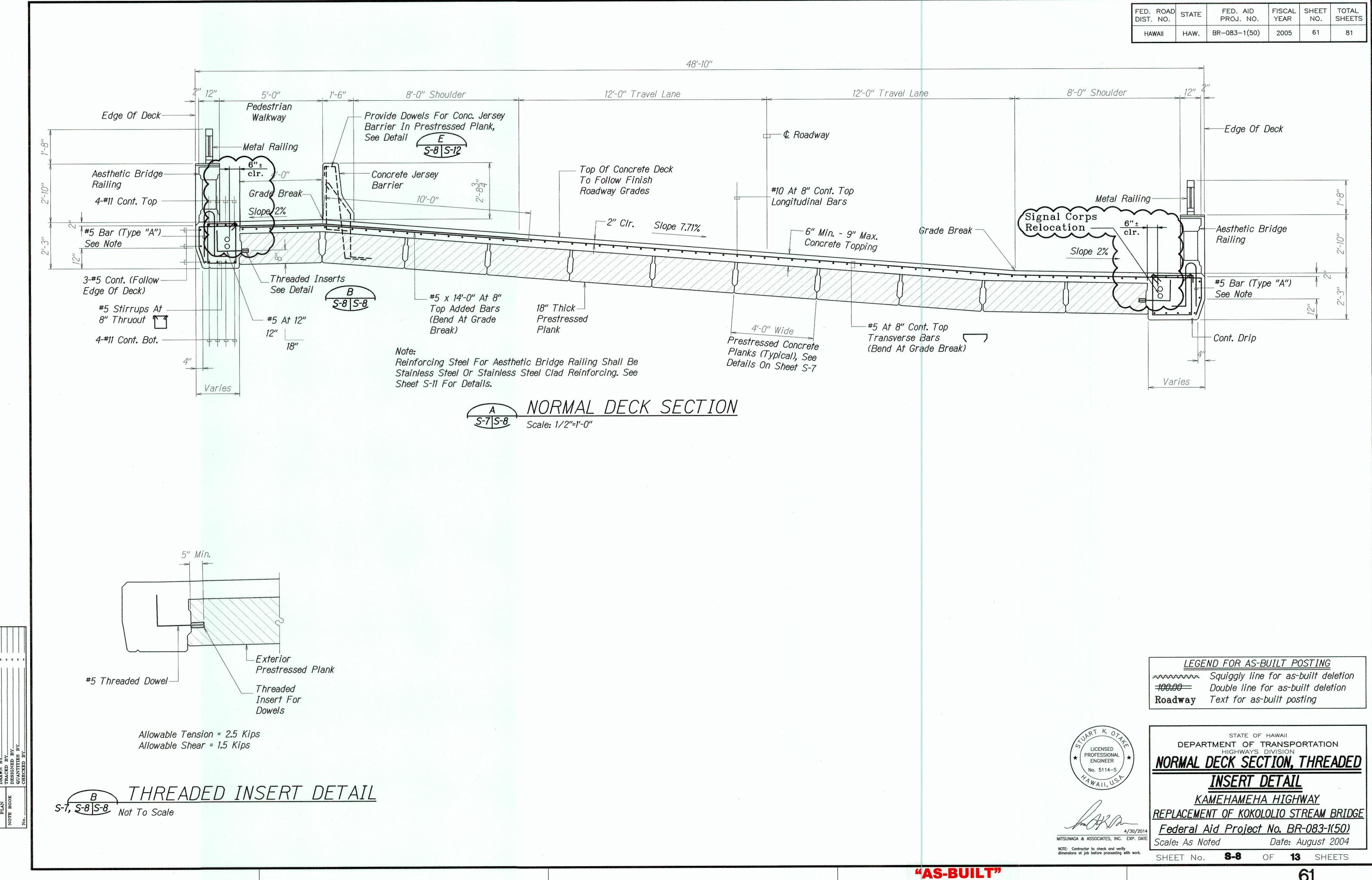


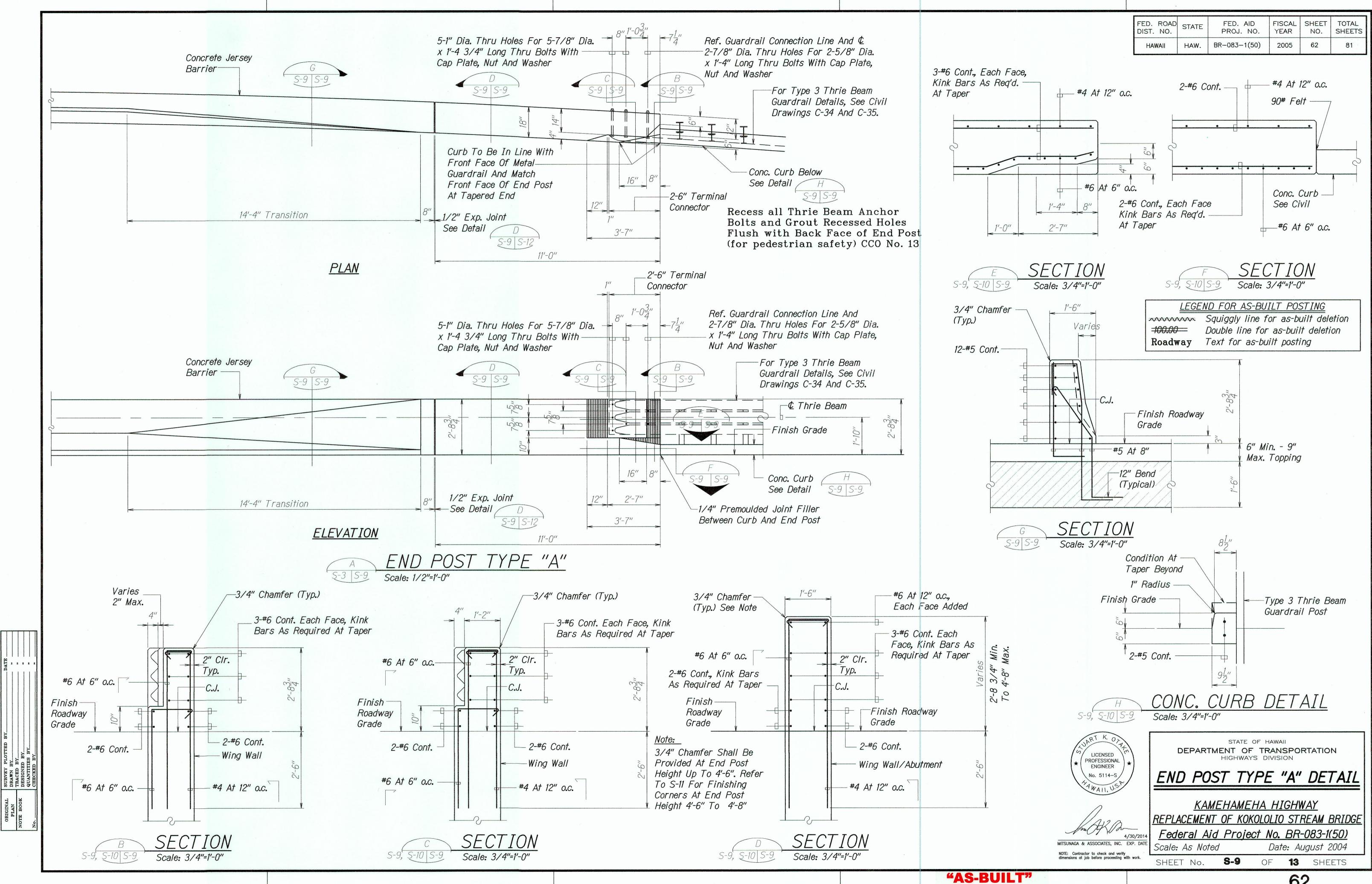


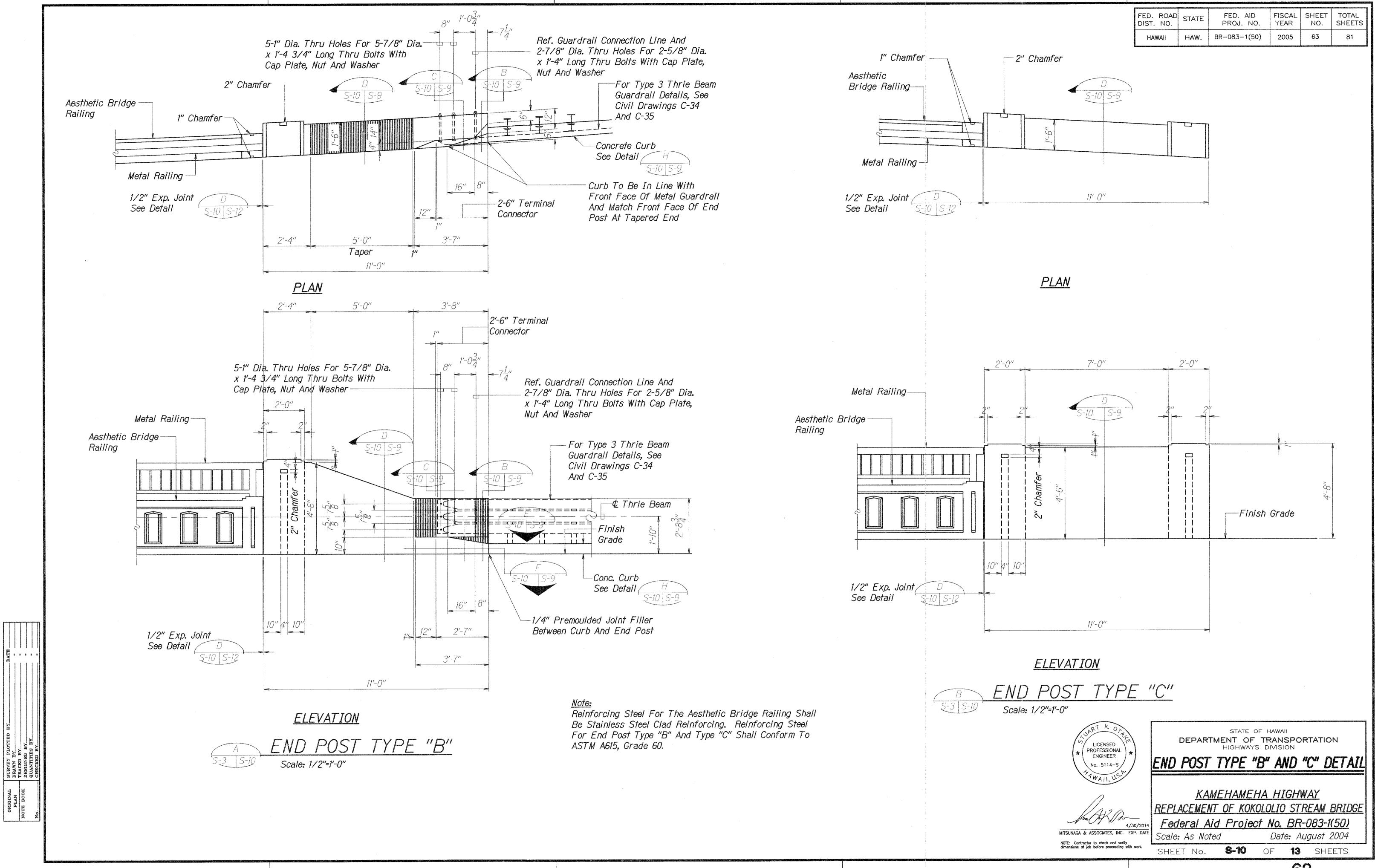


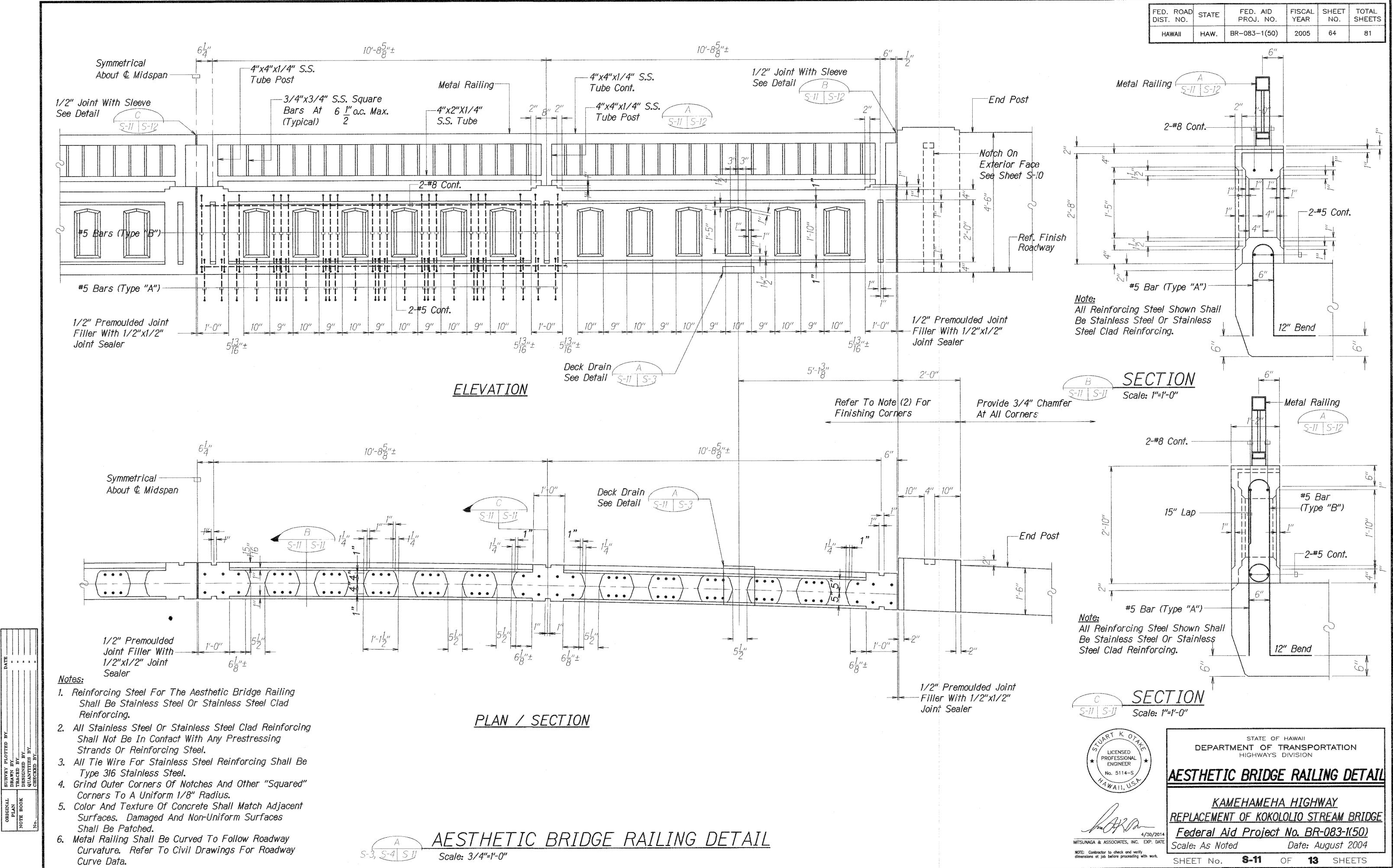


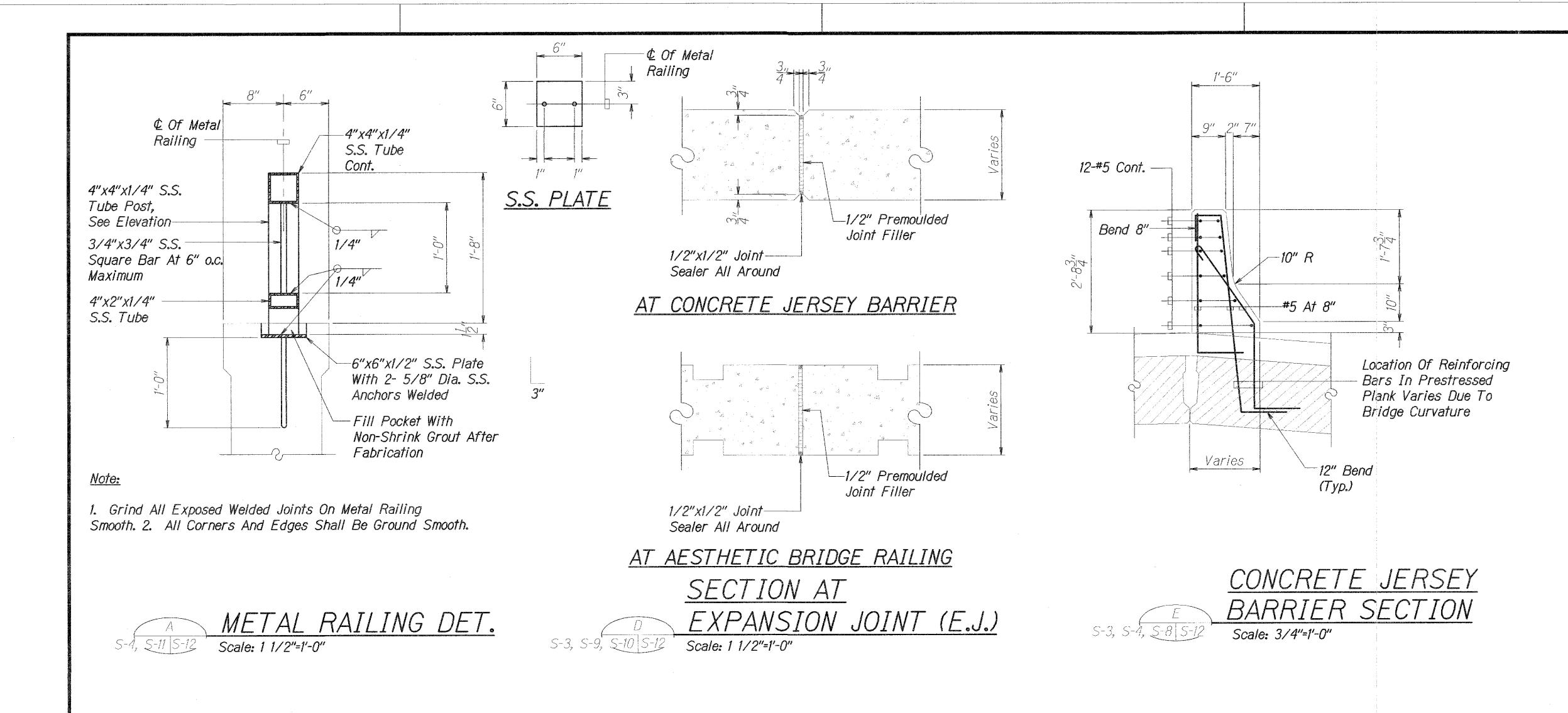


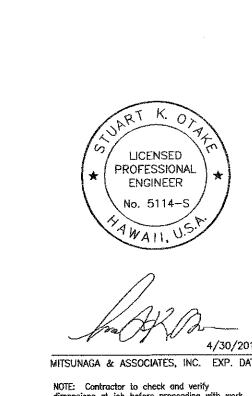












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FED. AID PROJ. NO.

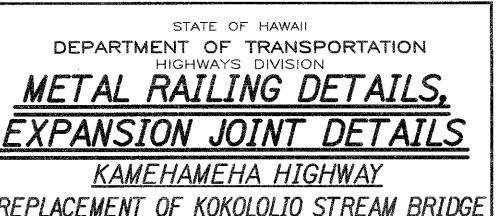
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FISCAL SHEET YEAR NO.

65

2005

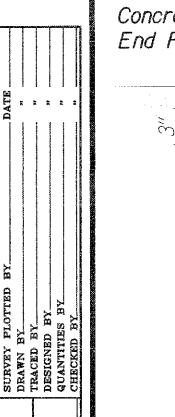
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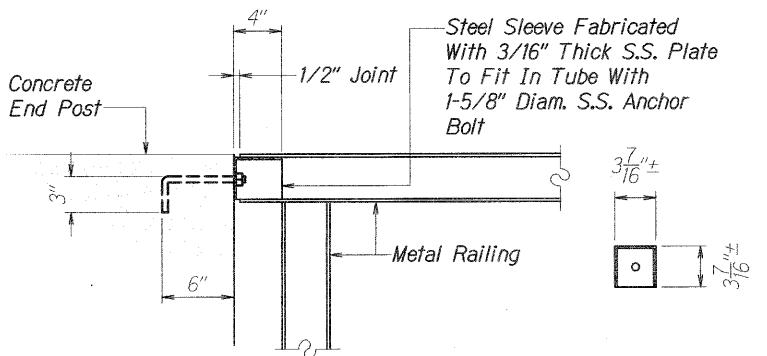


REPLACEMENT OF KOKOLOLIO STREAM BRIDGE Federal Aid Project No. BR-083-1(50)

Date: August 2004 Scale: As Noted

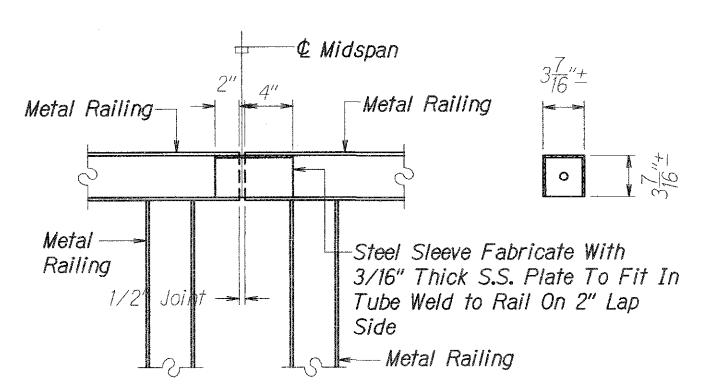
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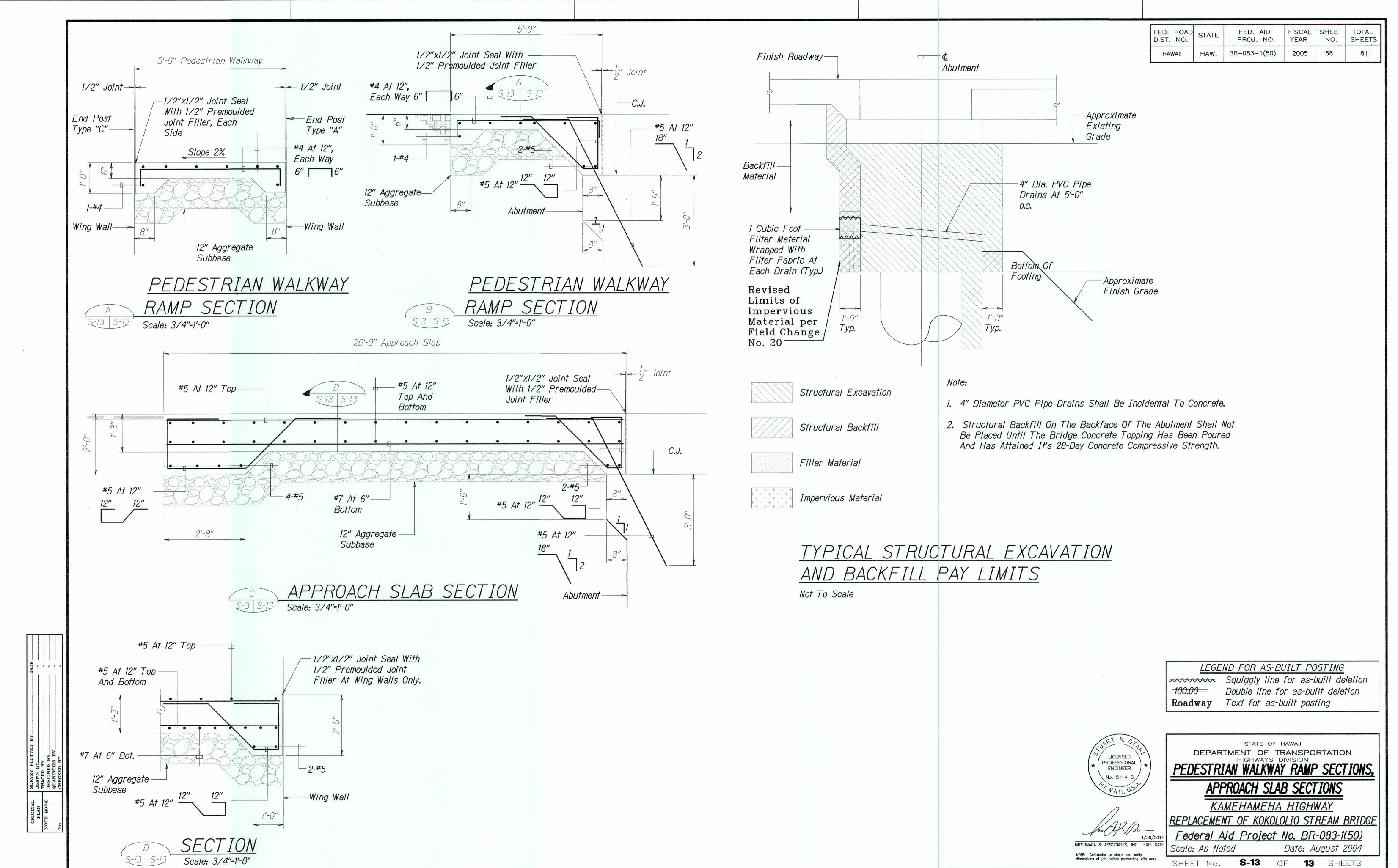
AT END POST

METAL RAILING EXPANSION JOINT DETAIL Scale: 1 1/2"=1'-0"

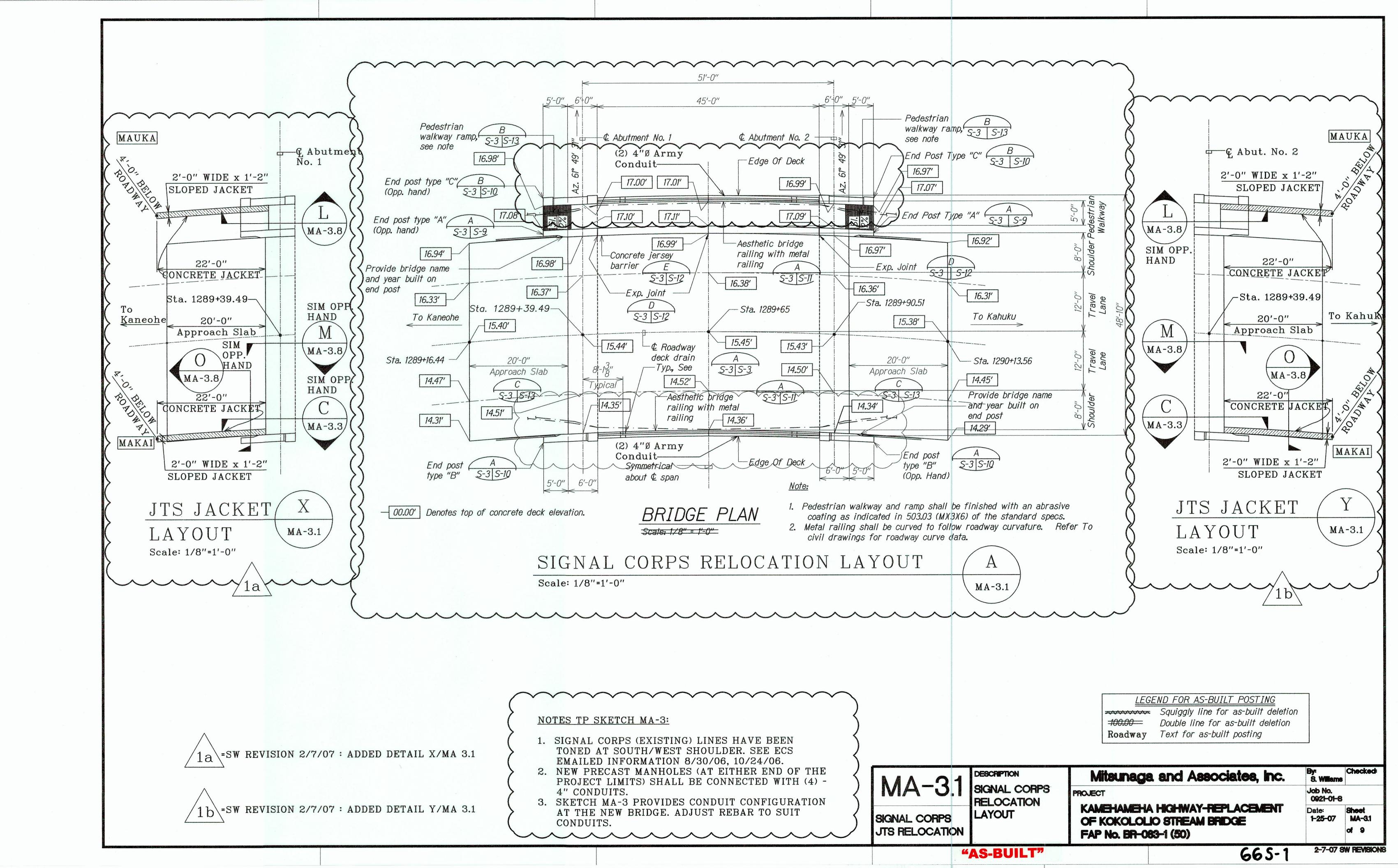


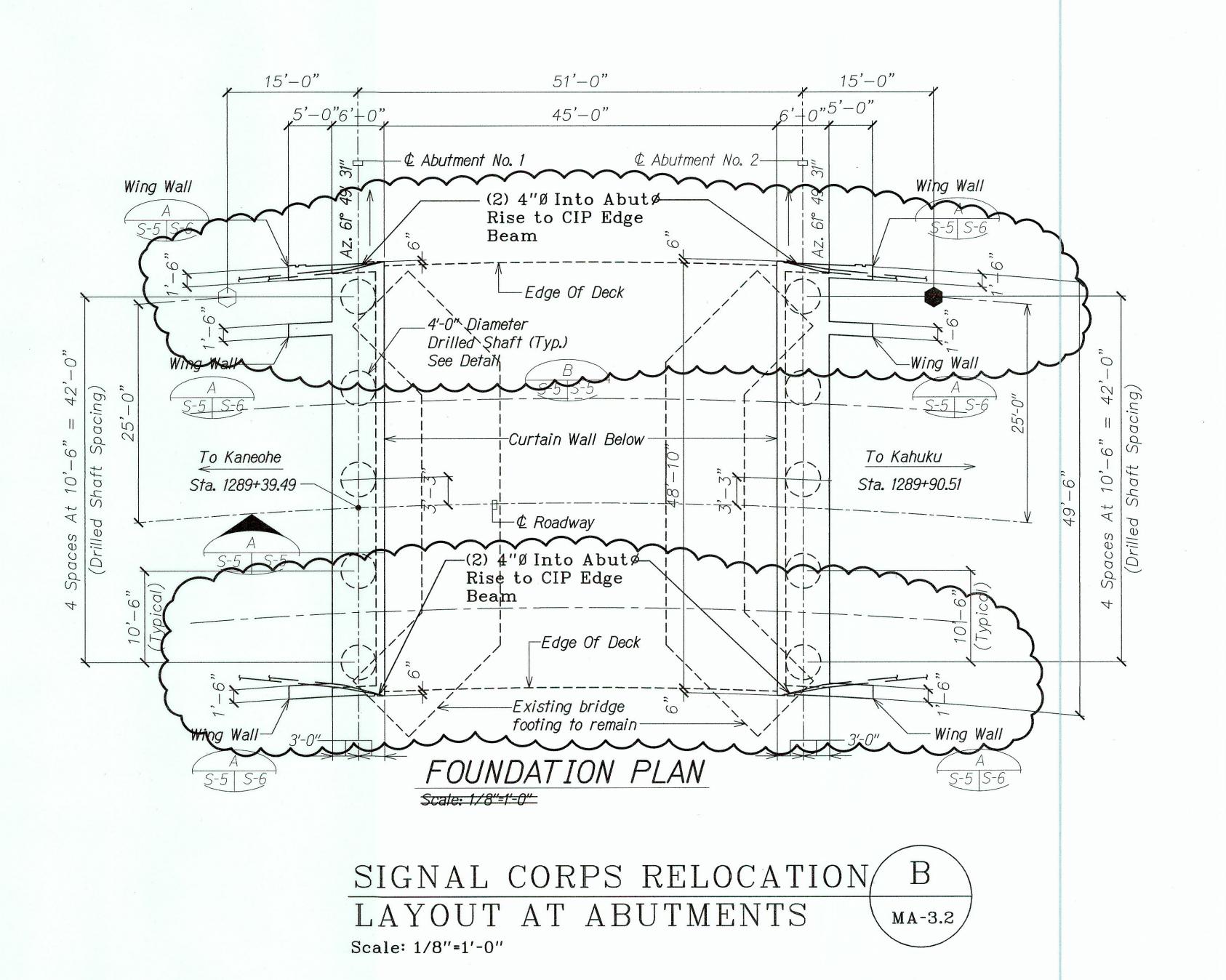
AT MIDSPAN

METAL RAILING EXPANSION JOINT DETAIL Scale: 1 1/2"=1'-0"



"AS-BUILT"





LEGEND FOR AS-BUILT POSTING Squiggly line for as-built deletion Double line for as-built deletion

Roadway Text for as-built posting

SIGNAL CORPS JTS RELOCATION

MA-3.2 SIGNAL CORPS RELOCATION LAYOUT AT ABUTMENTS Mitsunaga and Associates, Inc.

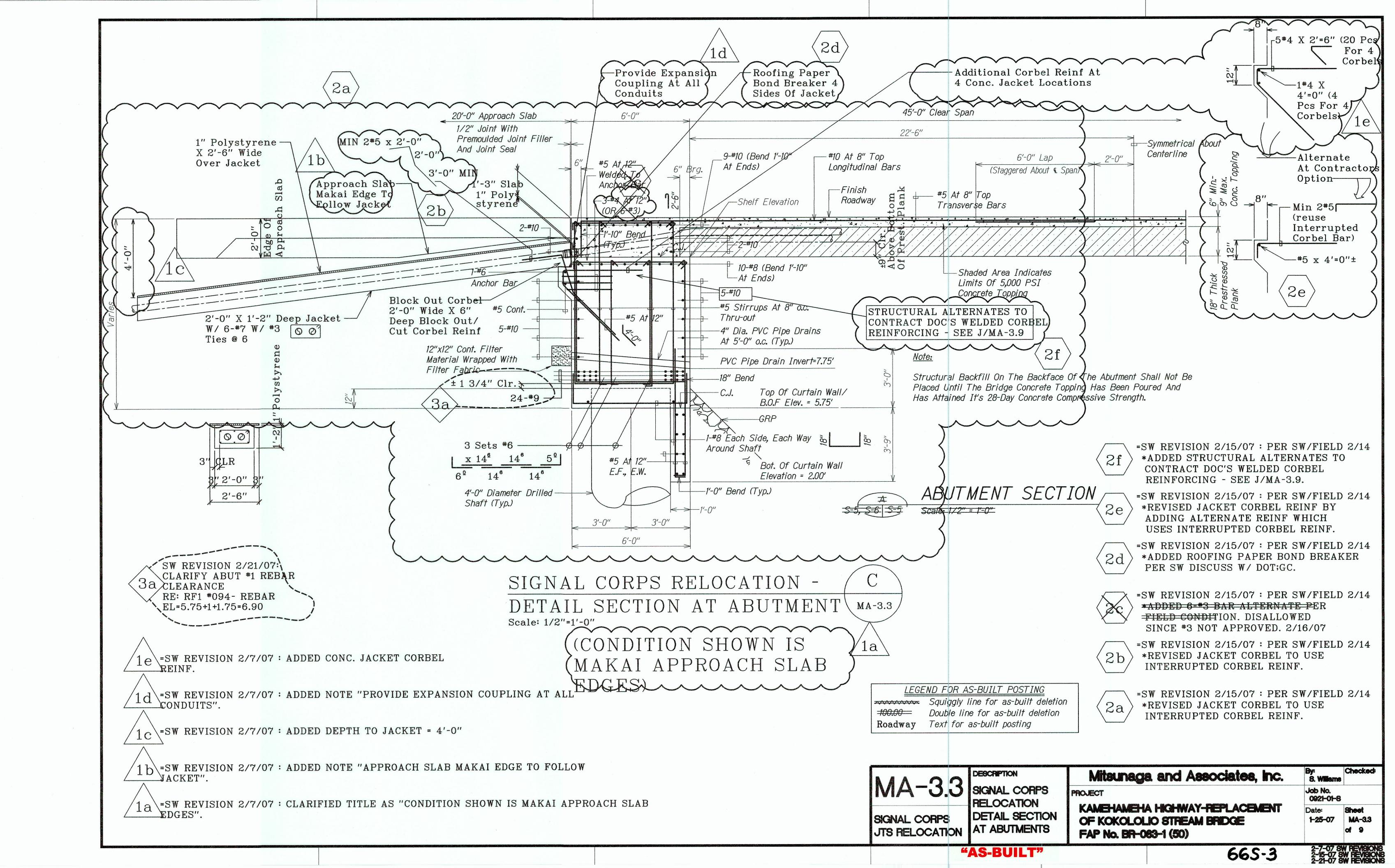
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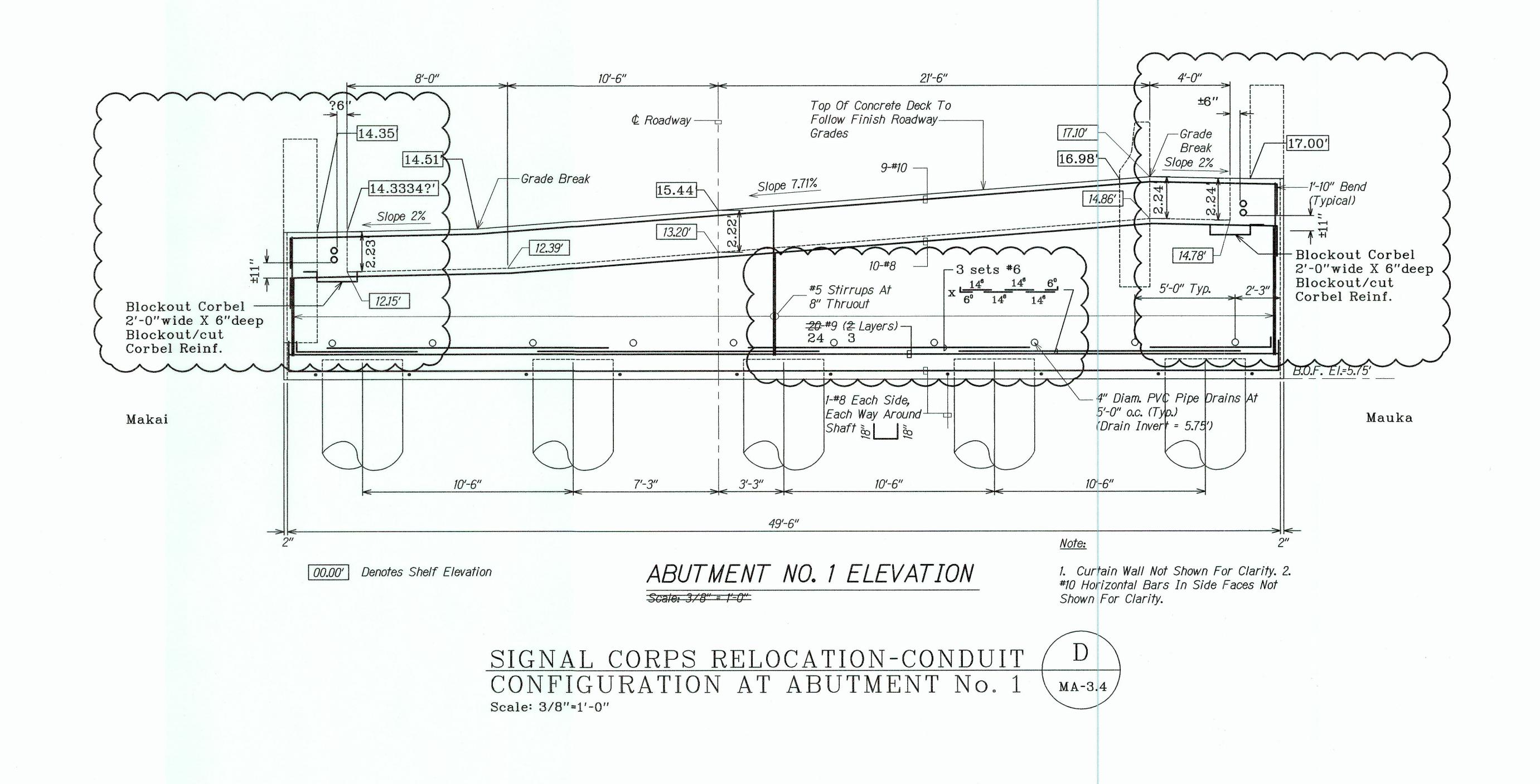
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"AS-BUILT"

Job No. 0921-01-8

Checked





LEGEND FOR AS-BUILT POSTING

Squiggly line for as-built deletion 100.00 Double line for as-built deletion Roadway Text for as-built posting

DESCRIPTION SIGNAL CORPS RELOCATION-CONDUIT SIGNAL CORPS CONFIGURATION AT JTS RELOCATION ABUTMENT No. 1

Mitsunaga and Associates, Inc. PROJECT

KAMEHAMEHA HIGHWAY-REPLACEMENT OF KOKOLOLIO STREAM BRIDGE FAP No. BR-083-1 (50)

2-7-07 SW REVISIONS

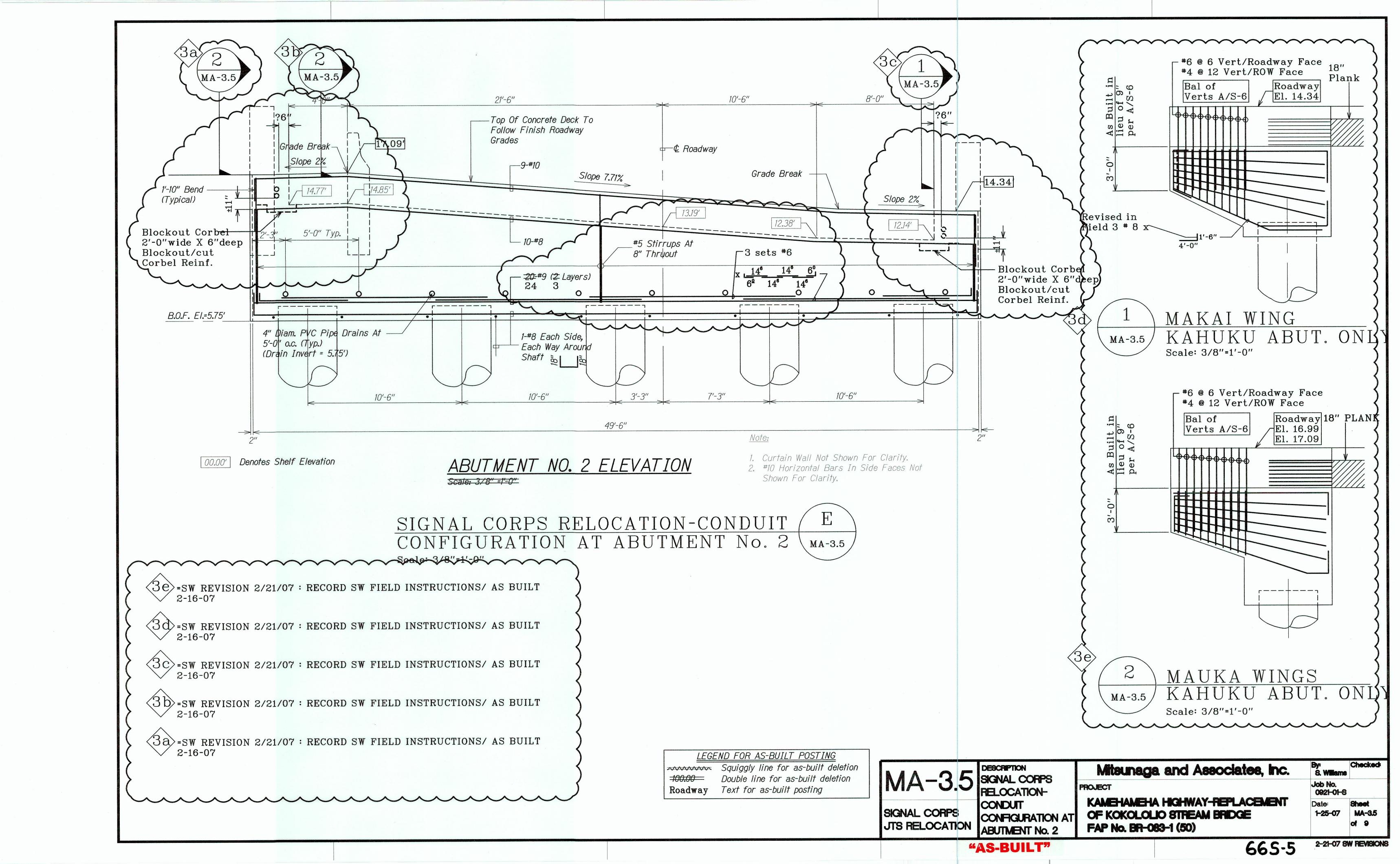
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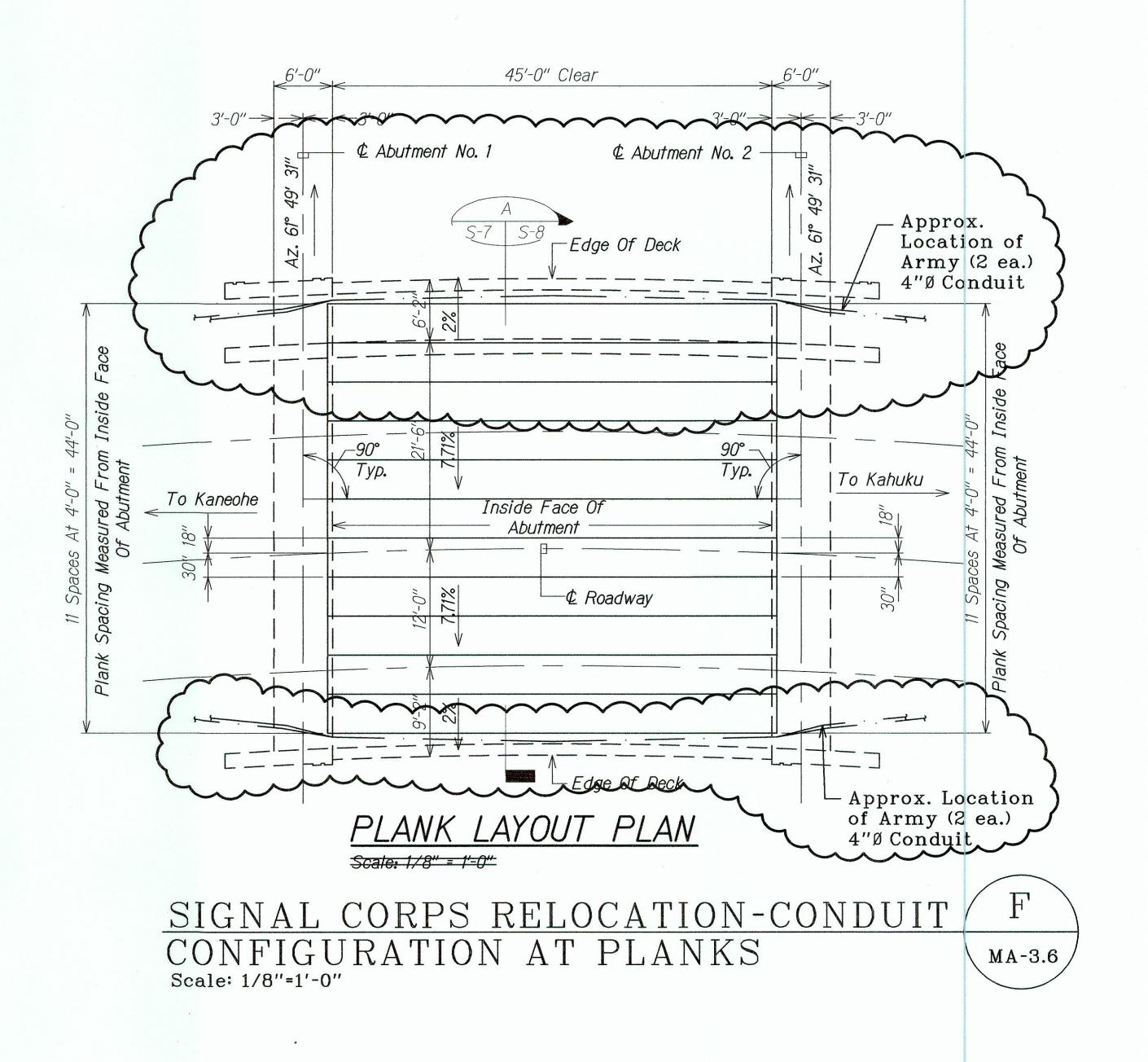
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Checked

MA-3.4





LEGEND FOR AS-BUILT POSTING Squiggly line for as-built deletion

Double line for as-built deletion Roadway Text for as-built posting

MA-3.6
SIGNAL CORPS
RELOCATION—
CONDUIT
CONFIGURATION
AT PLANKS

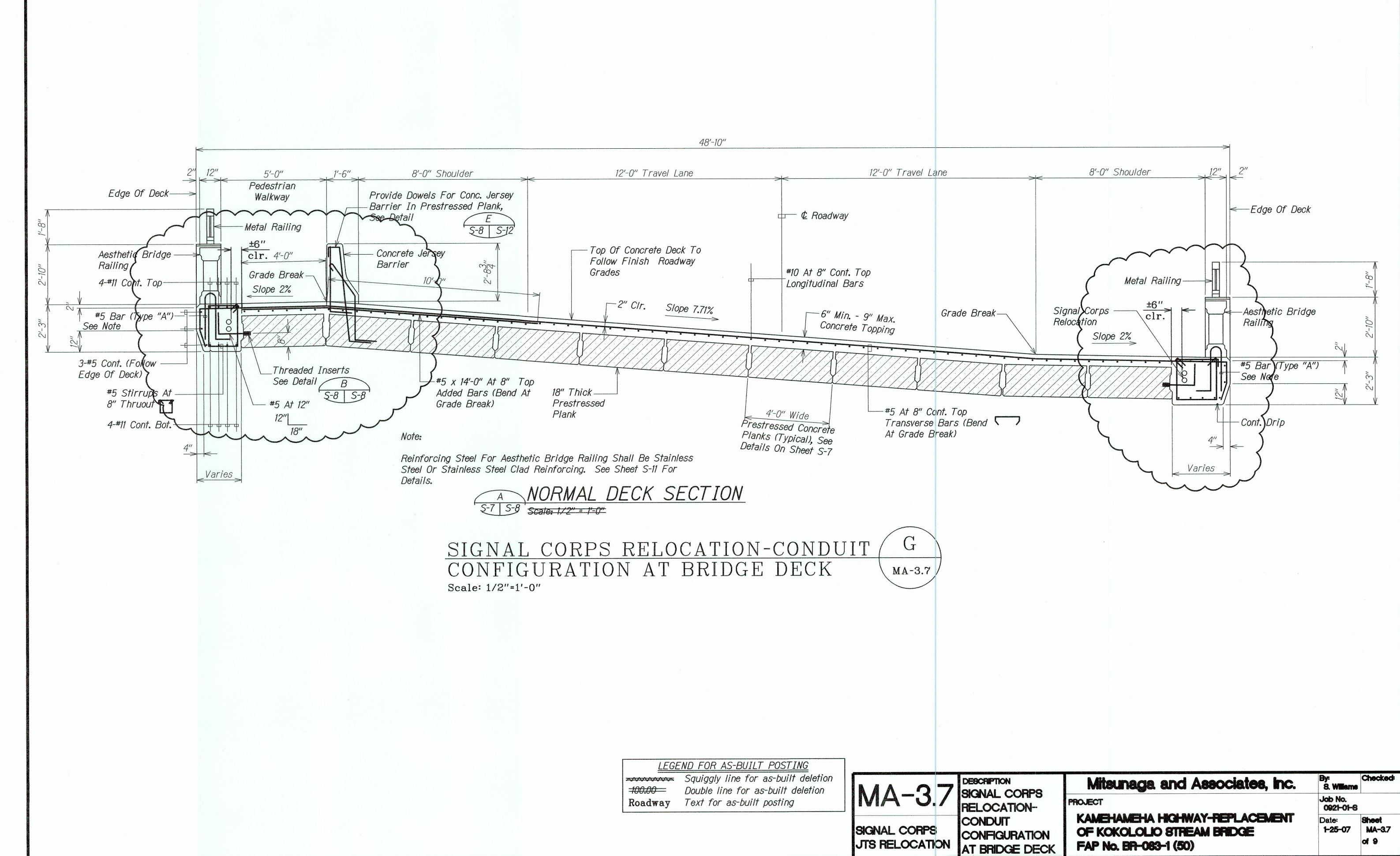
Mitsunaga and Associates, Inc. PROJECT

KAMEHAMEHA HIGHWAY-REPLACEMENT OF KOKOLOLIO STREAM BRIDGE FAP No. BR-083-1 (50)

Checked

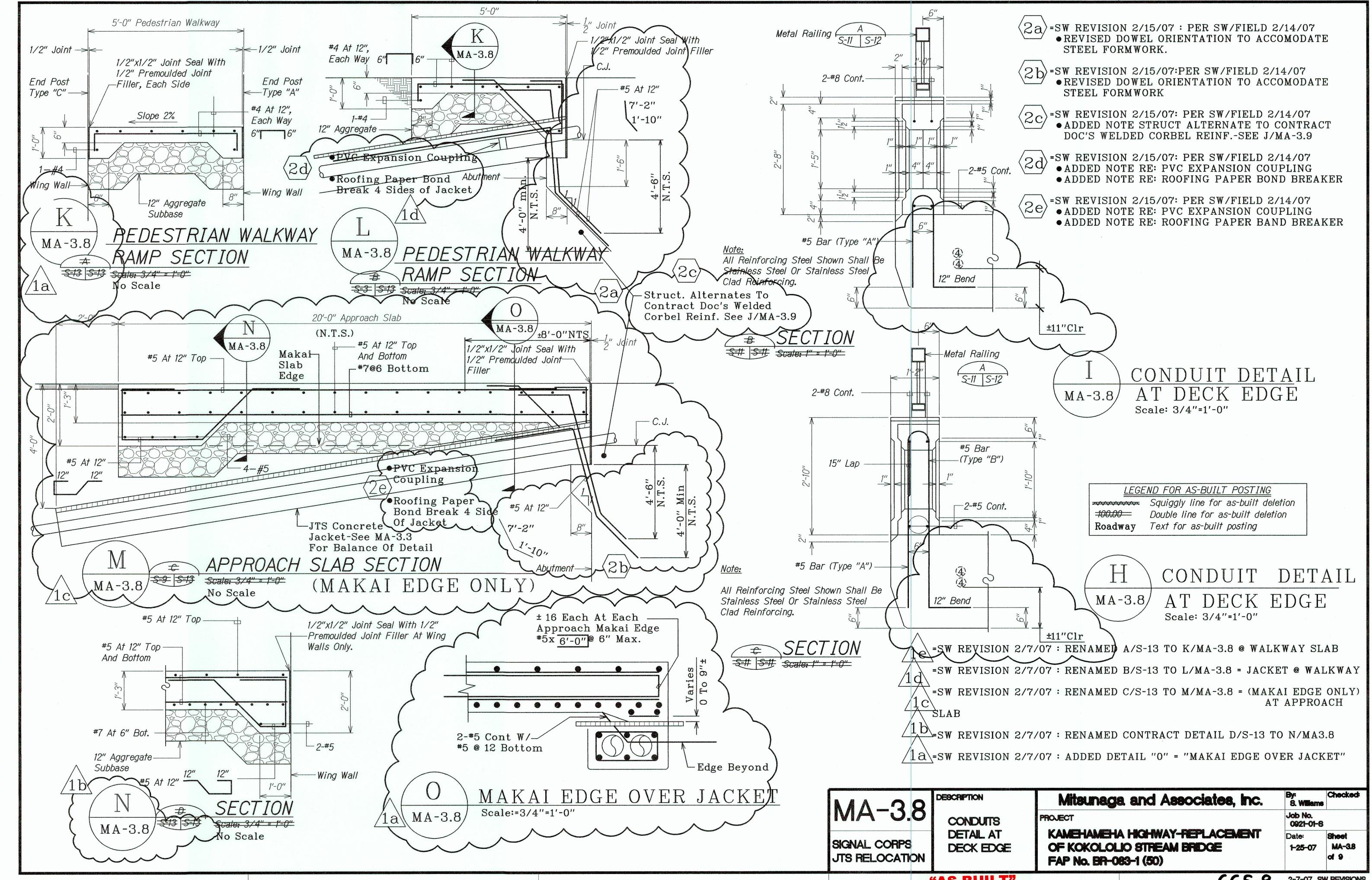
By: 8. Williams

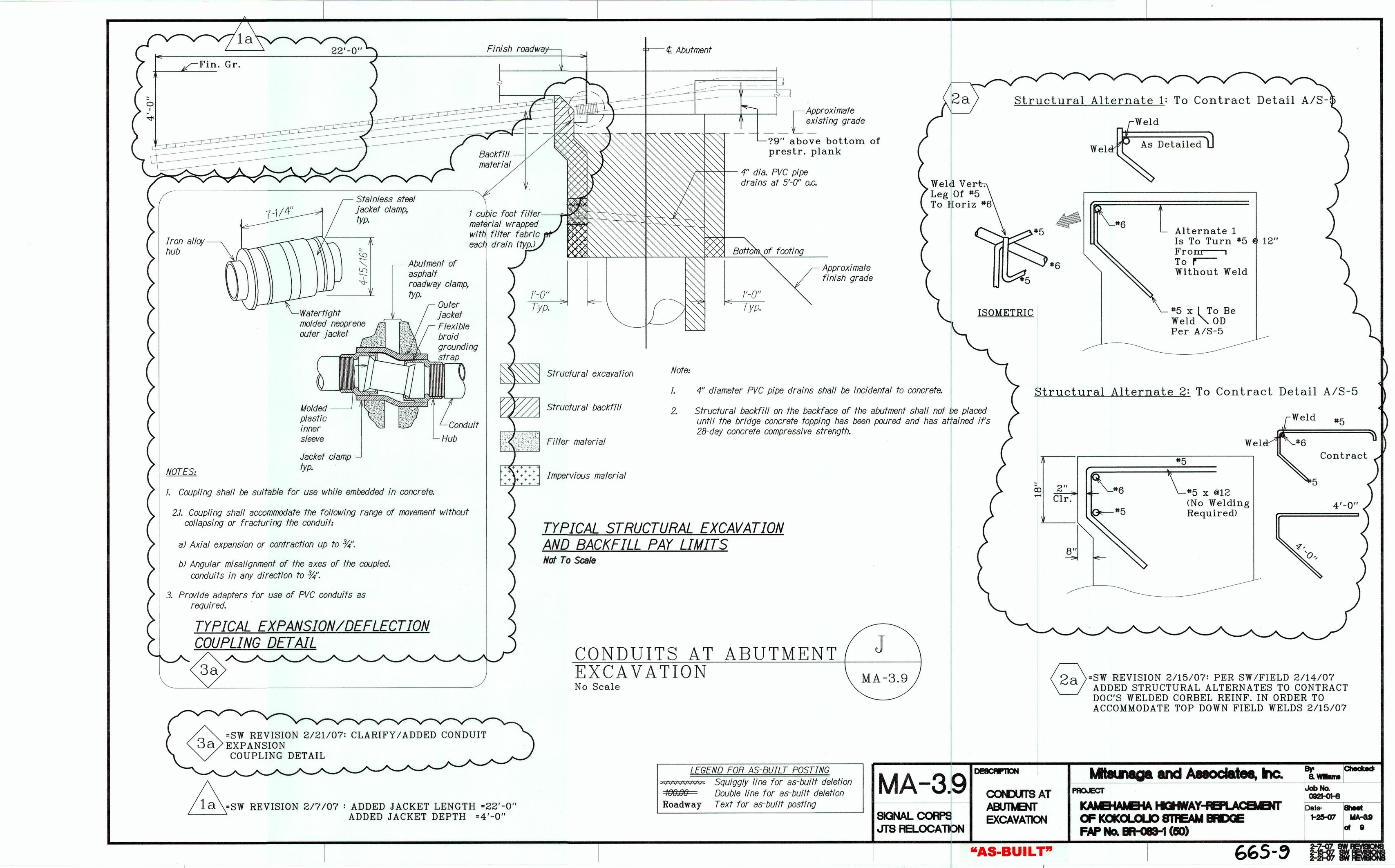
Job No. 0921-01-8

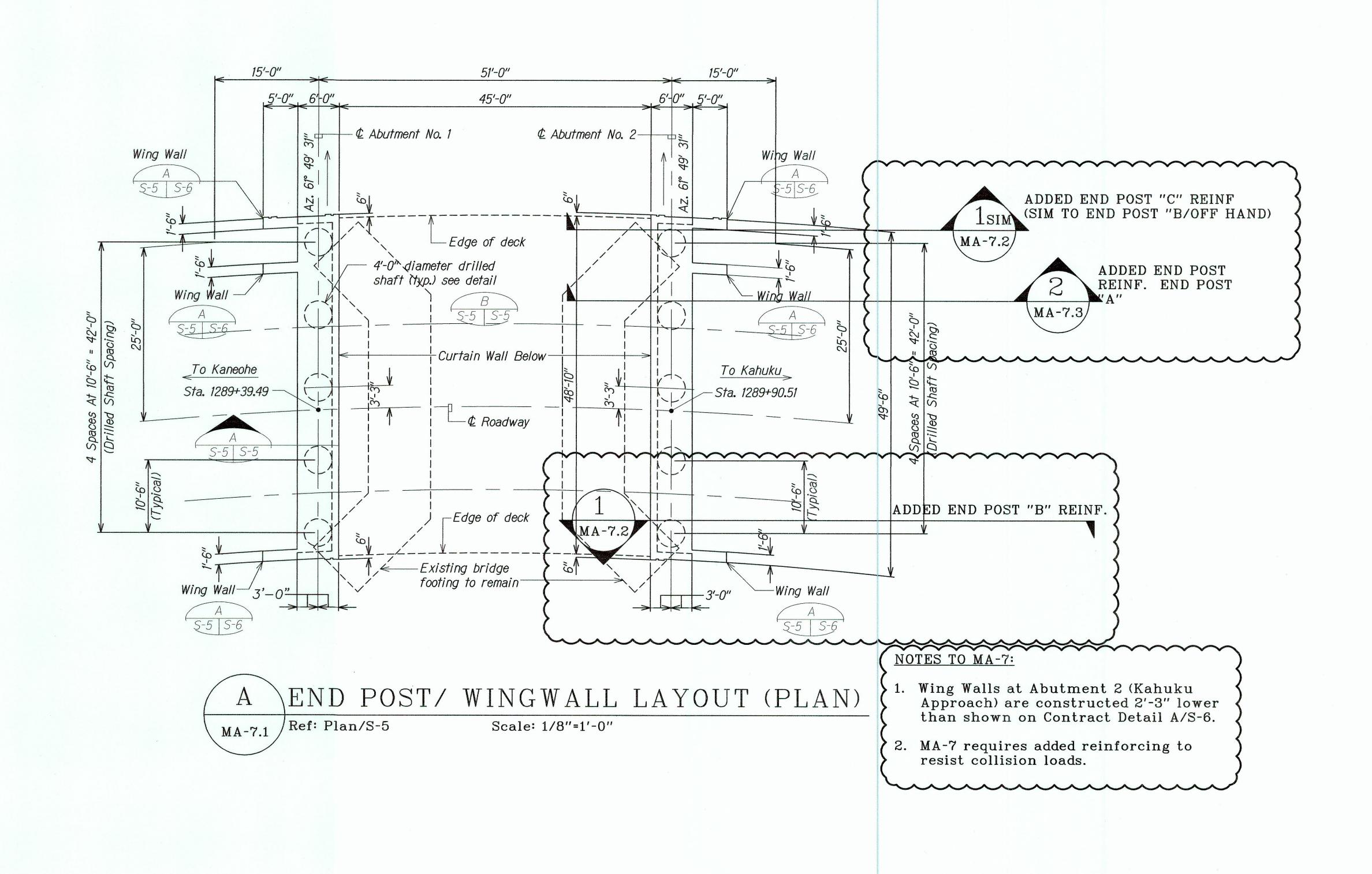


665-7

"AS-BUILT"







LEGEND FOR AS-BUILT POSTING Squiggly line for as-built deletion Double line for as-built deletion Roadway Text for as-built posting

DESCRIPTION REVISE END POST RENF LAYOUT (PLAN) FOR AS-BUILT WING

WALLS AT ABUTMENT 2

END POST/ WINGWALL

Mitsunaga and Associates, Inc. PROJECT KAMEHAMEHA HIGHWAY-REPLACEMENT OF KOKOLOLIO STREAM BRIDGE

FAP No. BR-083-1 (50)

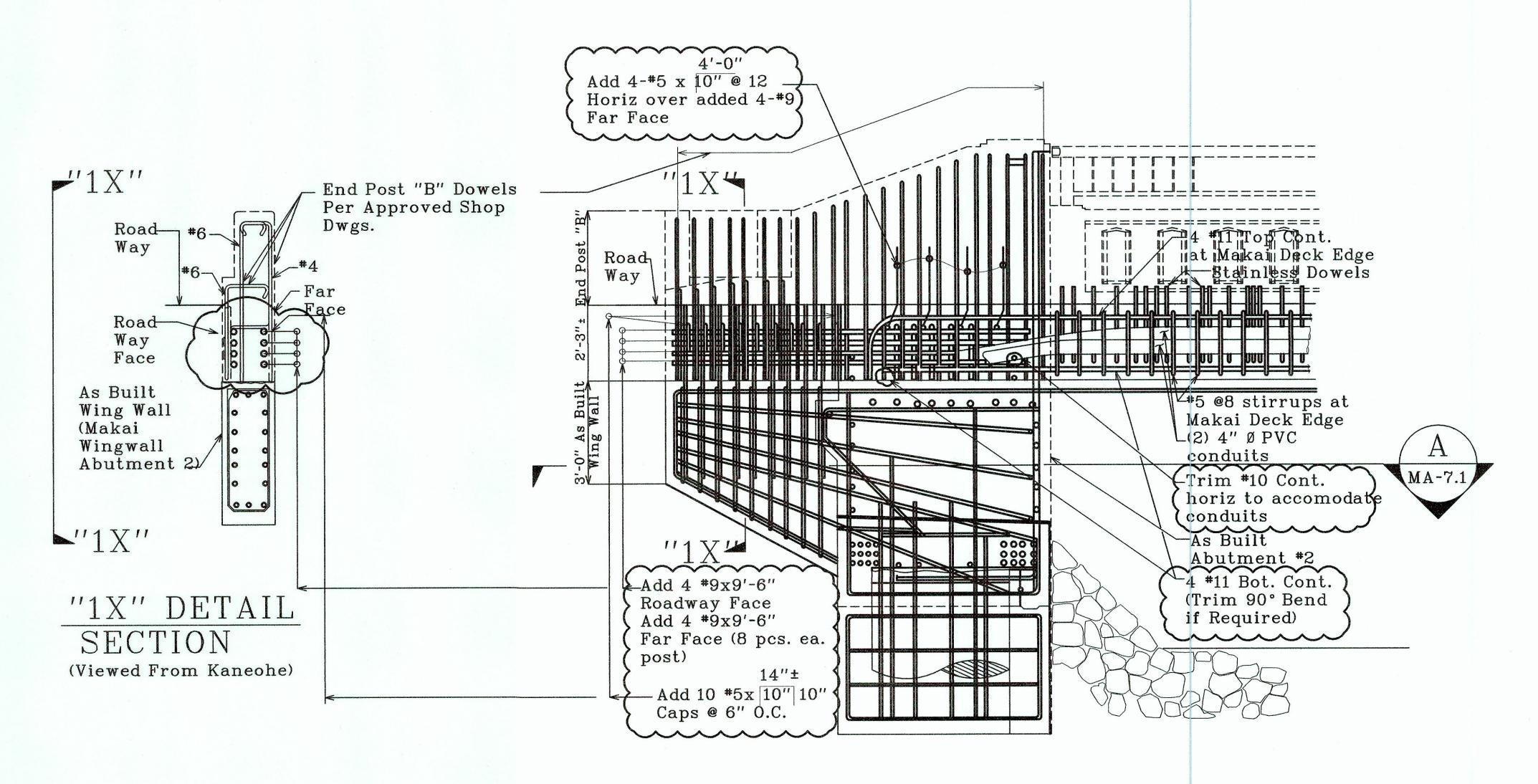
"AS-BUILT"

By: S. Williams

Job No. 0921-01-8

3/15/07

Checked



ADDED END POST REINF. - SECTION MA-7.2 MAKAI END POST "B"-ABUTMENT 2 (Kahuku Apprch)
Scale: 1/2"=1'-0"

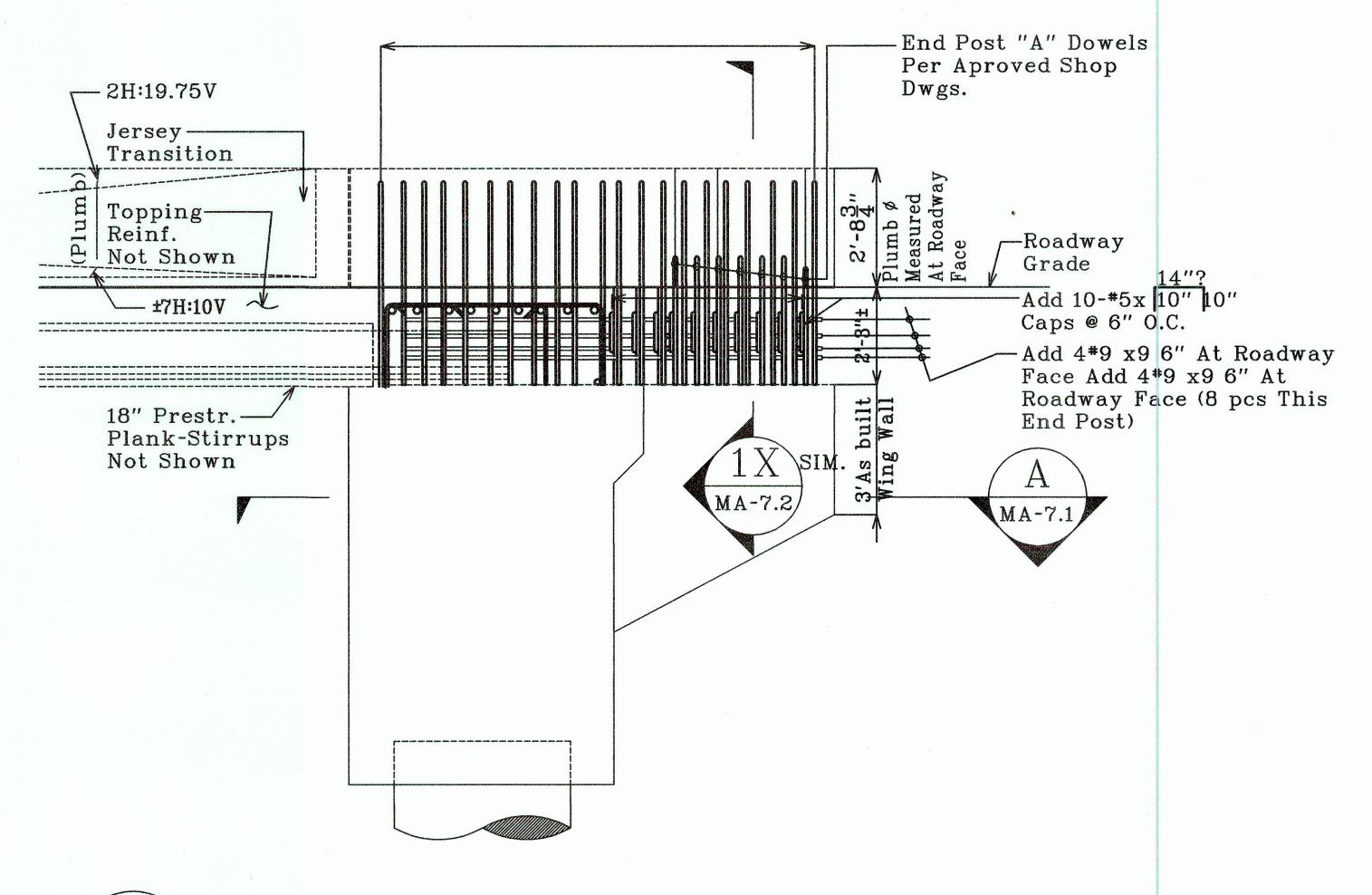
> LEGEND FOR AS-BUILT POSTING Squiggly line for as-built deletion 100.00 Double line for as-built deletion Roadway Text for as-built posting

DESCRIPTION FENSE BND POST FEEN F. FOR AS BULT WINGWALLS 'B' ABUTMENT 2

AT ABUTMENT 2

ADDED END POST PROJECT REINF.-SECTION MAKAI END POST

Checked Mitsunaga and Associates, Inc. By: 8. Williams Job No. 0921-01-8 KAMEHAMEHA HIGHWAY-REPLACEMENT OF KOKOLOLIO STREAM BRIDGE 3/15/07 MA-7.2 FAP No. BR-083-1 (50)



ADDED END POST REINF. - SECTION

MA-7.3 JERSEY BARRIER END POST "A" ØABUTMENT 2 (Kahuku Approach)

LEGEND FOR AS-BUILT POSTING

Squiggly line for as-built deletion Double line for as-built deletion Roadway Text for as-built posting

PEVSE END POST RENT:

FOR AS-BULT WINGWALLS

AT ABUTMENT 2

DESCRIPTION
ADDED END POST
REINF. - SECTION
PROJECT JERSEY BARRIER END POST 'A'

Mitsunaga and Associates, Inc.

KAMEHAMEHA HIGHWAY-REPLACEMENT OF KOKOLOLIO STREAM BRIDGE FAP No. BR-083-1 (50)

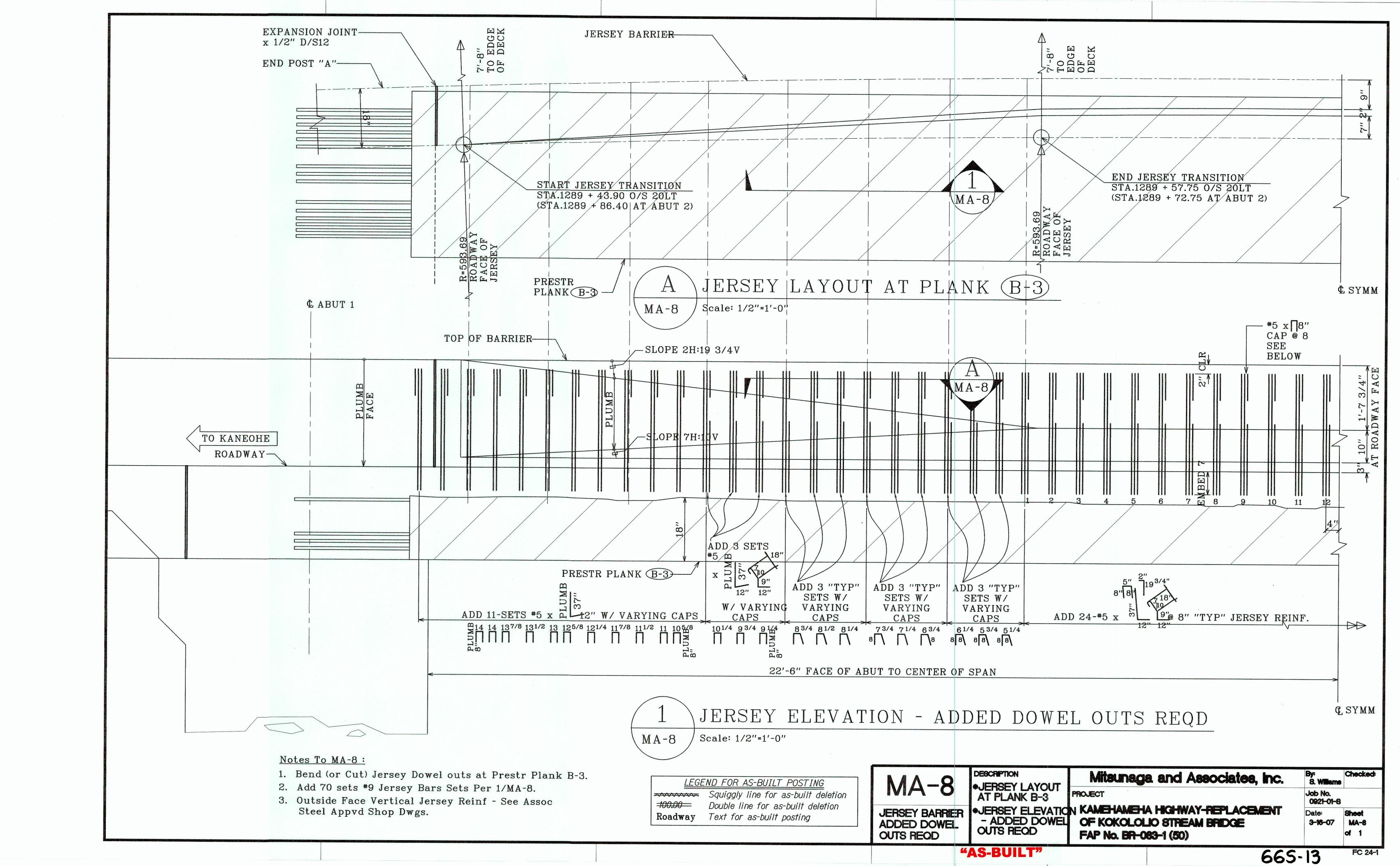
Job No. 0921-01-8 3/15/07 MA-7.3 FC 23-3

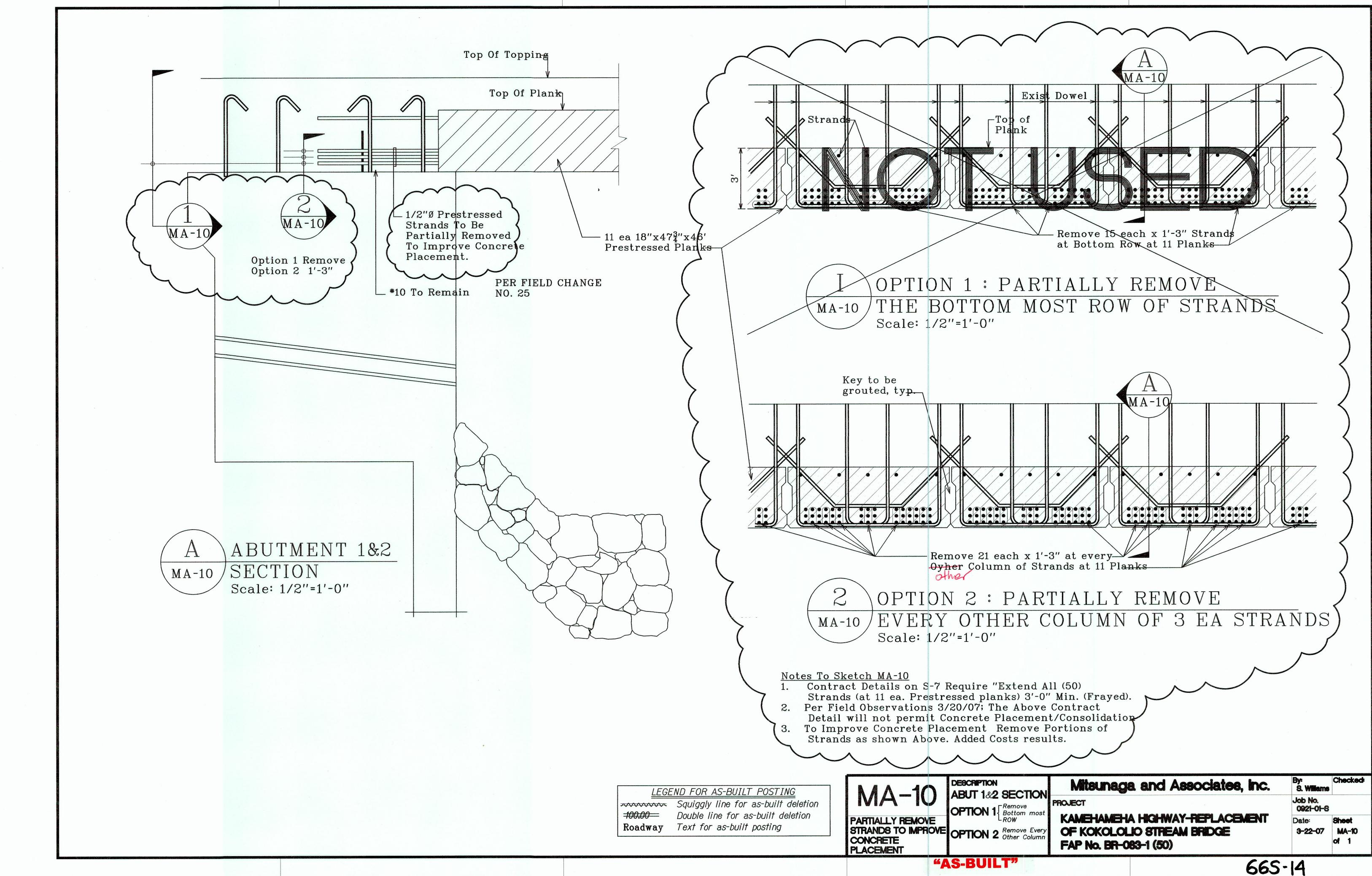
By: S. Williams

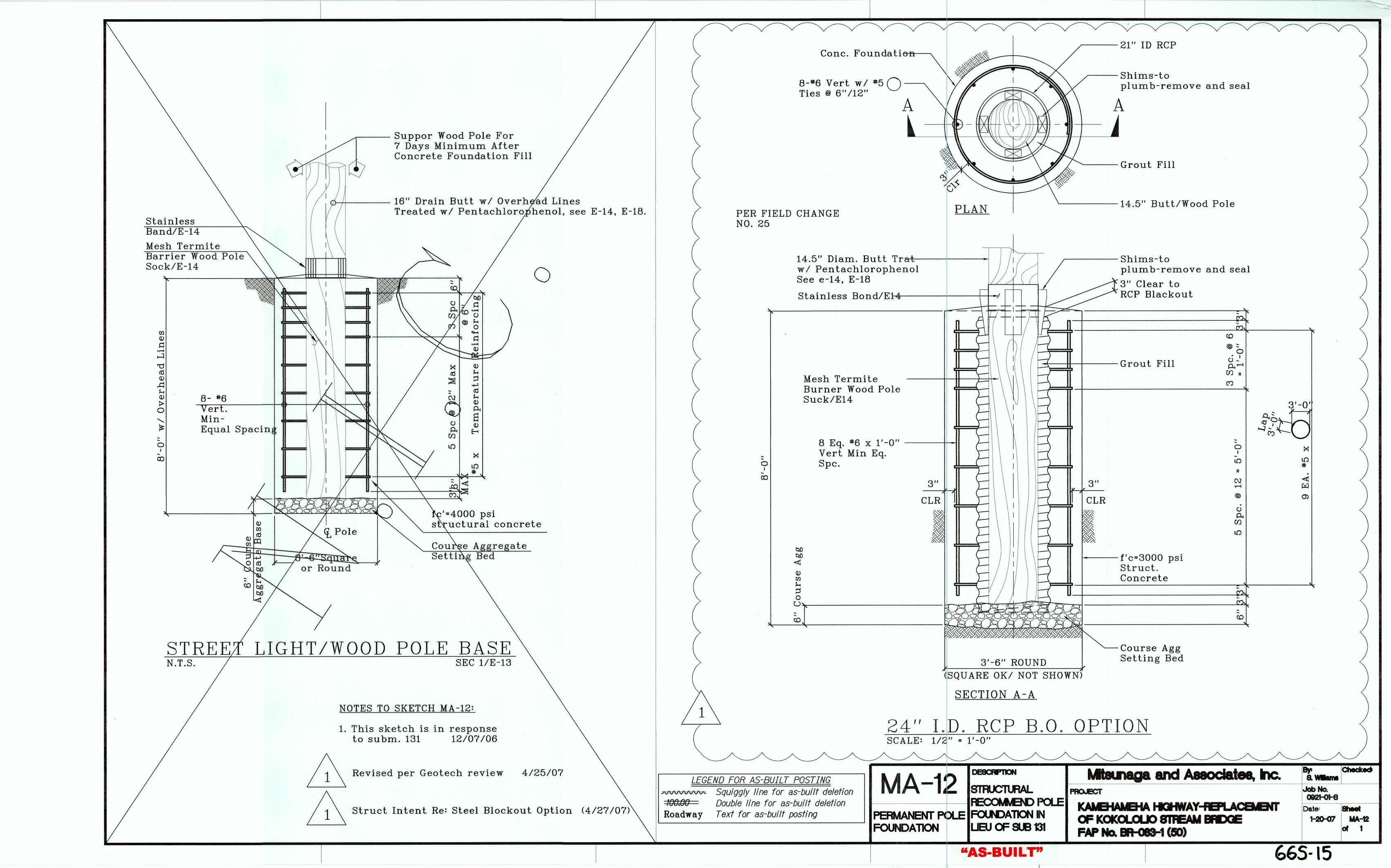
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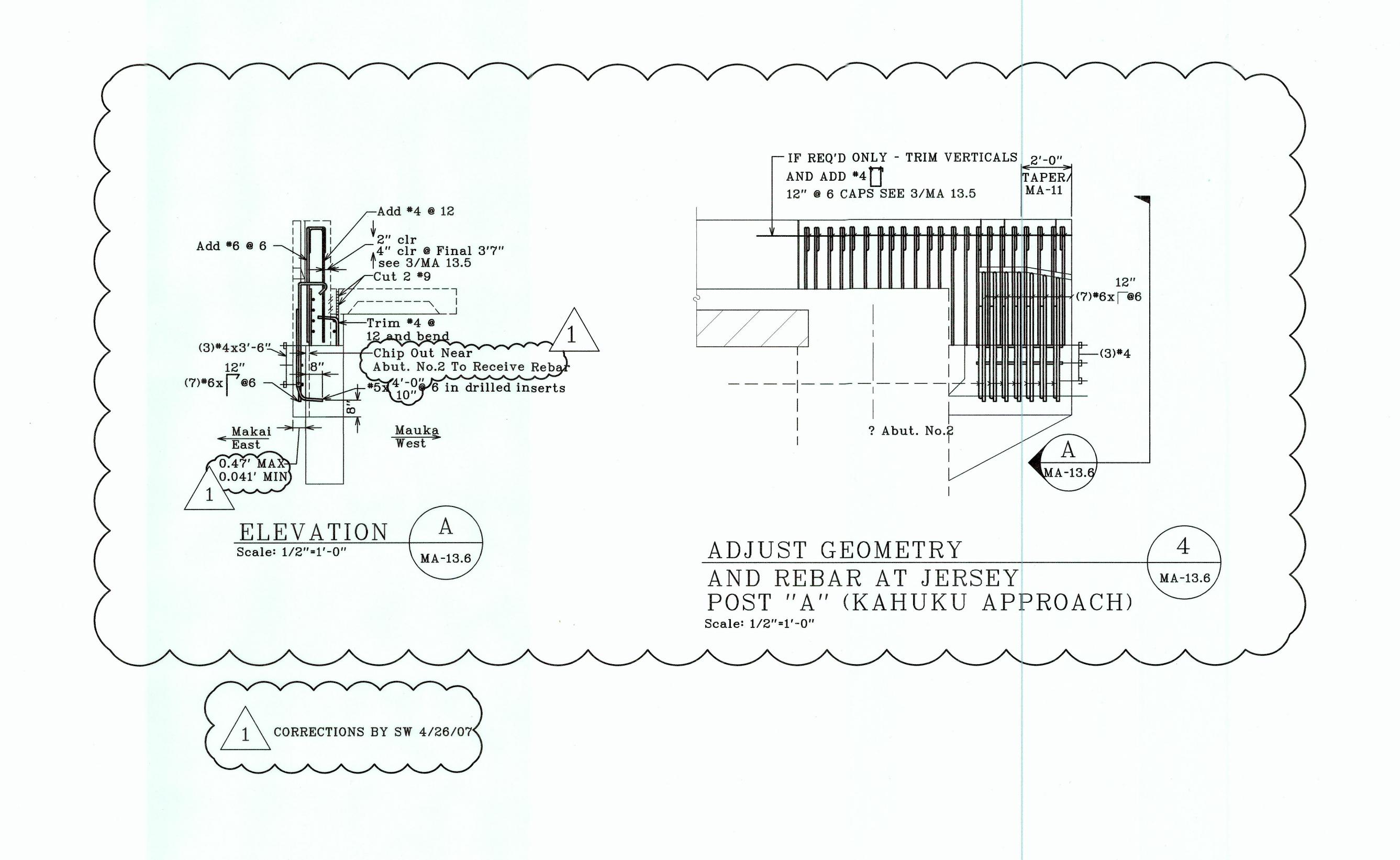
"AS-BUILT"

ABUTMENT 2









LEGEND FOR AS-BUILT POSTING Squiggly line for as-built deletion Double line for as-built deletion Roadway Text for as-built posting

DESCRIPTION MA-13.6 ADJUST GEOMETRY PROJECT JERSEY POST 'A' REBAR ADJUST-(KAHUKU MENTS FOR RFI

Mitsunaga and Associates, Inc.

By: S. **Williams** Job No. 0921-01-8 KAMEHAMEHA HIGHWAY-REPLACEMENT OF KOKOLOLIO STREAM BRIDGE 4-24-07 FAP No. BR-083-1 (50)

APPROACH) 102 AND RFI 103 "AS-BUILT" Checked

MA-13.6

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