## CANE HAUL ROAD PEDESTRIAN BRIDGE GENERAL NOTES:

- <u>General Specifications:</u> Hawaii Department of Transportion, Standard Specifications for Road and Bridge Construction, 1994, together with Special Provisions prepared for this contract.
- 2. <u>Design Specifications:</u> (A) AASHTO 2004 LRFD Bridge Design Specification (Third Edition) and its subsequent interim specification with interim supplements and modifications by the Highway Division, Department of Transportation,
  - (B) AASHTO/AWS D1.5M/D1.5 Bridge Welding Code, Latest Edition.

#### 3. <u>Loads:</u>

(A) Live Load: 100 psf

State of Hawaii.

(B) Seismec Loads: Aceleration coefficient - 0.18 Seismic Performance Zone - 2 Importance Category - Other Bridges

#### 4. Materials:

- (A) All concrete except Precast Prestressed Concrete Beam shall have a 28-day cylinder compressive strength of 4,000 psi.
- (B) For concrete strength of Precast Prestressed Concrete Beam, see Sht. S2-6.
- (C) Tetraguard AS20 shrinkage reducing admixture (SRA) shall be included in the concrete mix for the deck. The required dosage shall be 128 ounces per cubic yard of concrete and follow all manufacturer's recommendations
- (D) All reinforcing steel shall be ASTM A615 Grade 60 unless otherwise noted.
- (E) All structural steel shall be ASTM A36 hot dip galvanized after fabrication, unless otherwise noted.
- (F) All Type I Metal Bikeway Railings shall be steel structural Tubing conforming to ASTM A500 Grade B and shall be hot-dipped galvanized after fabrications.
- (G) All Type I Metal Bikeway Railing Anchor Bolts shall be steel bolts conforming to ASTM A307. Anchor bolts shall be hot-dipped galvanized.
- (H) All Type I Metal Bikeway Railing Anchor Bolt Nuts shall be steel nuts conforming to ASTM A563. Nuts shall be hot-dipped galvanized.
- (I) All Type I Metal Bikeway Railing Anchor Bolt washers shall be steel washers conforming to ASTM F436. Washers shall be hot-dipped galvanized.
- (J) All welds shall be made using E70XX electrodes. Welds shall be performed by a qualified welder.

#### Reinforcement:

- (A) The minimum covering measured from the surfaces of the concrete to the face of any reinforcing bars shall be as follows, except as otherwise shown:
  - (1) Deck
    - A. Top bars = 2" with a tolerances of 0 inch and +3/8 inch.
    - B. Bottom bars =  $1\frac{1}{2}$ " except as otherwise noted.

- (2) Abutment Walls and Wing Walls = 2" to ties or outermost reinforcing except as otherwise noted.
- (3) Concrete cast against and permanently esposed to earth = 3"
- (4) All other unless otherwise noted = 2"
- (B) Reinforcing bars shall be detailed in accordance with the latest edition of ACI Detailing Manual for reinforced concrete structures.
- (C) Minimum clear spacing between parallel bars shall be 1½ times the diameter of bars (for non bundled bars). In no case shall the clear distance between the bars be less than 11/2 times the maximum size of the coarse aggregate or  $1\frac{1}{2}$ ".
- (E) All dimensions relating to reinforcing bars are to centers of bars unless otherwise noted.
- (D) Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of intersections is less than one foot in each direction, in which case alternate intersections shall be tied.
- (F) Vertical wall bars be arranged in such manner as to avoid interference with girder and deck bars above as directed by the Engineer.

#### 6. Ginder Bearings:

- (A) Girder concrete bearing seat shall be poured monolithically with supporting structure. Top of concrete bearing seat shall be finished with steel trowel to a smooth level surface to the elevation shown on plans. Grind down high spots as needed to provide an even bearing surface to 1/16"± tolerance.
- (B) The concrete seat and elastomeric pad shall be placed as shown.

#### 7. <u>Construction Notes:</u>

- (A) See Standard Specifications and Special Provisions.
- (B) In general, top of concrete deck slab shall be constructed to follow the bridge vertical and horizontal curves and superelevations.
- (C) Except as otherwise noted, all vertical dimensions are measured plumb.
- (D) For concrete finish see Standard Specifications and Special Provisions.
- (E) Contruction joints may be relocated or additional ones added subject to the approval of the Engineer.
- (F) Unless otherwise noted, all exposed concrete edges shall be chamfered 3/4" x 3/4".
- (G) Contractor shall verify footing elevations before fabricating footing and wall reinforcing.
- (H) Elastomaric bearing pads shall be secured to the concrete bearing surface with adhesives or other means necessary as approved by the Engineer.

#### <u>General:</u>

(A) Standard detail drawings refer to all structures in general, except for modifications as may be requiered for special conditions. For such modifications refer to the corresponding detailed drawings.

## 9. <u>Foundation:</u>

Footings are embeded min. 5 feet below finish grade.

Footings should also be embeded such that a horizontal distance of 5 feet is maintained between the bottom edge of footing and slope face.

FED. ROAD

DIST. NO.

- (A) Allowable bearing value (net) = 4,000 psf for service limit state
  - = 7,200 psf for strength limit state

FEDERAL AID

PROJ. NO.

**FISCAL** 

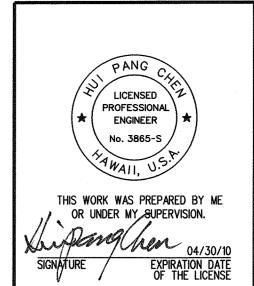
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SHEET TOTAL

- = 12,000 psf for extreme event limit
- (B) Coefficient of friction
- = 0.49 for strength limit state = 0.58 extreme event limit state
- (C) Passive earth pressure
- = 235 pcf for strength limit state = 470 pcf for extreme event limit state
- = 40 pcf (D) Active earth pressure
- (E) Internal friction angle = 30 degrees

### 10. Load Capacity Ratings:

- (A) Inventory Rating Design Load Rating
- = 2.19 X (Pedestrian LL = 100 PSF)
- (B) Operating Rating Design Load Rating
- = 3.85 X (Pedestrian LL = 100 PSF)



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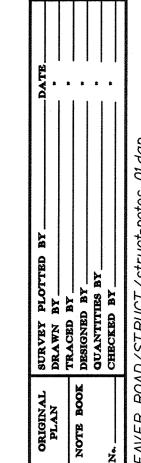
# CANE HAUL RD. PEDESTRIAN BRIDGE GENERAL NOTES

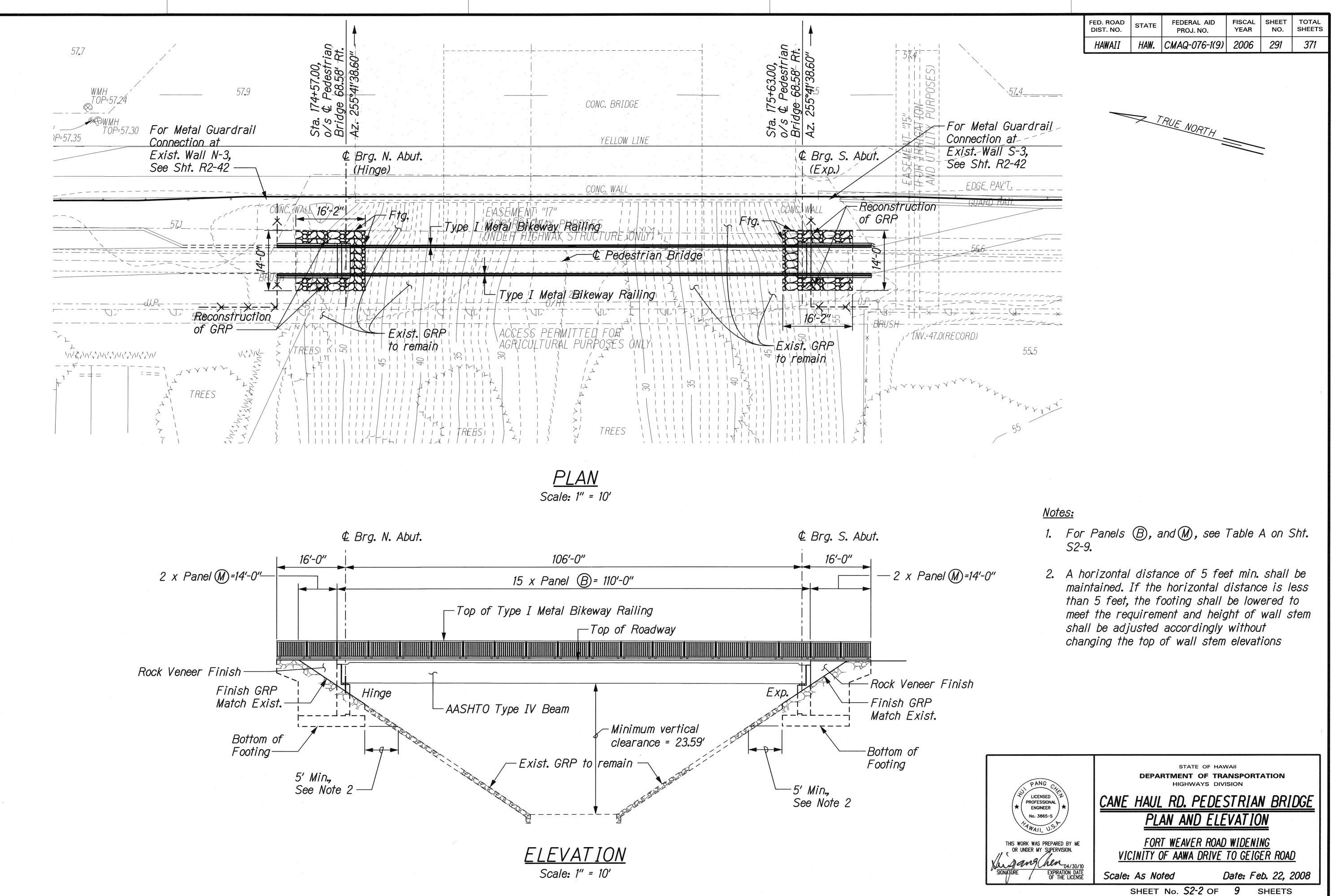
FORT WEAVER ROAD WIDENING VICINITY OF AAWA DRIVE TO GEIGER ROAD

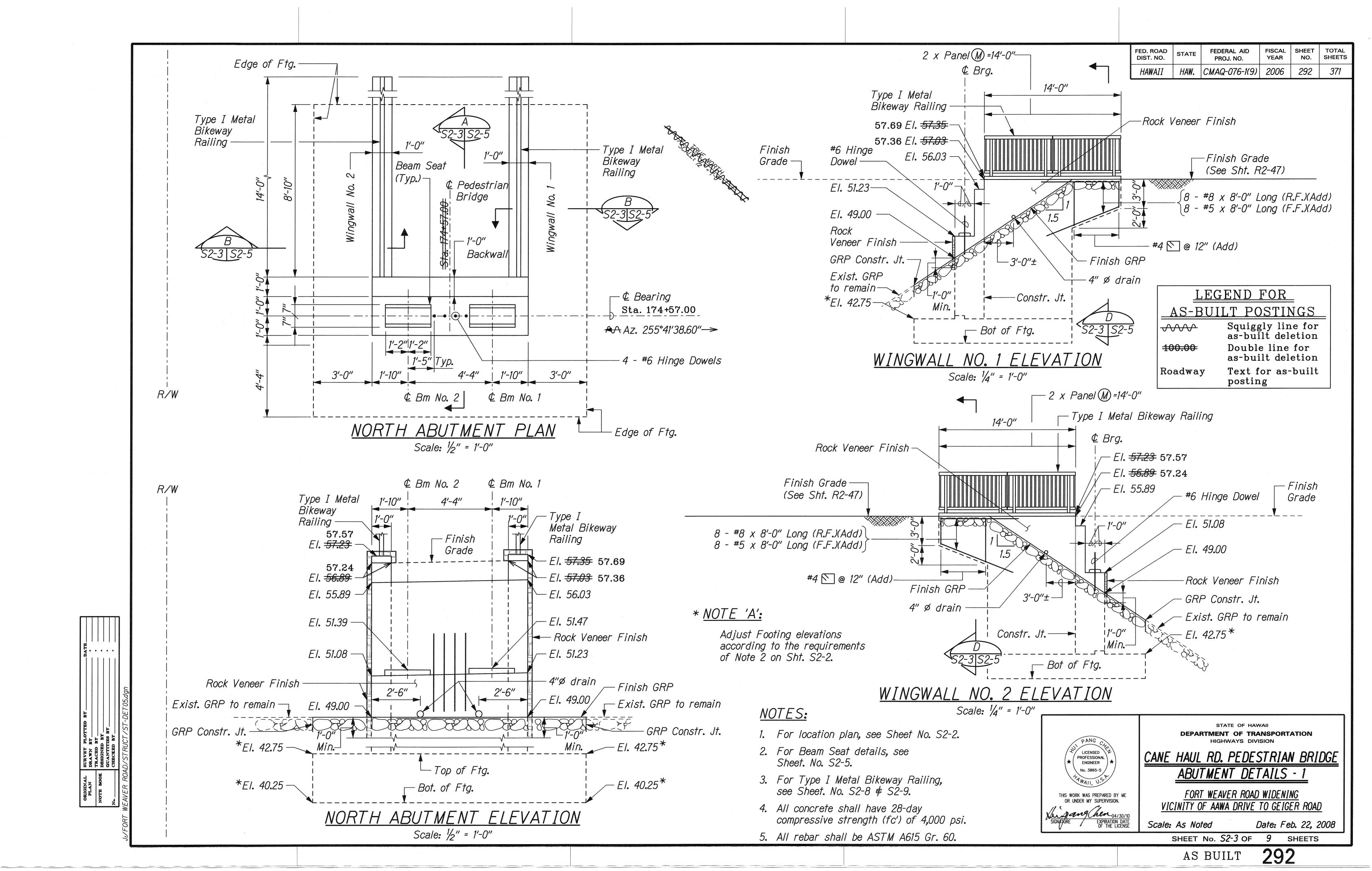
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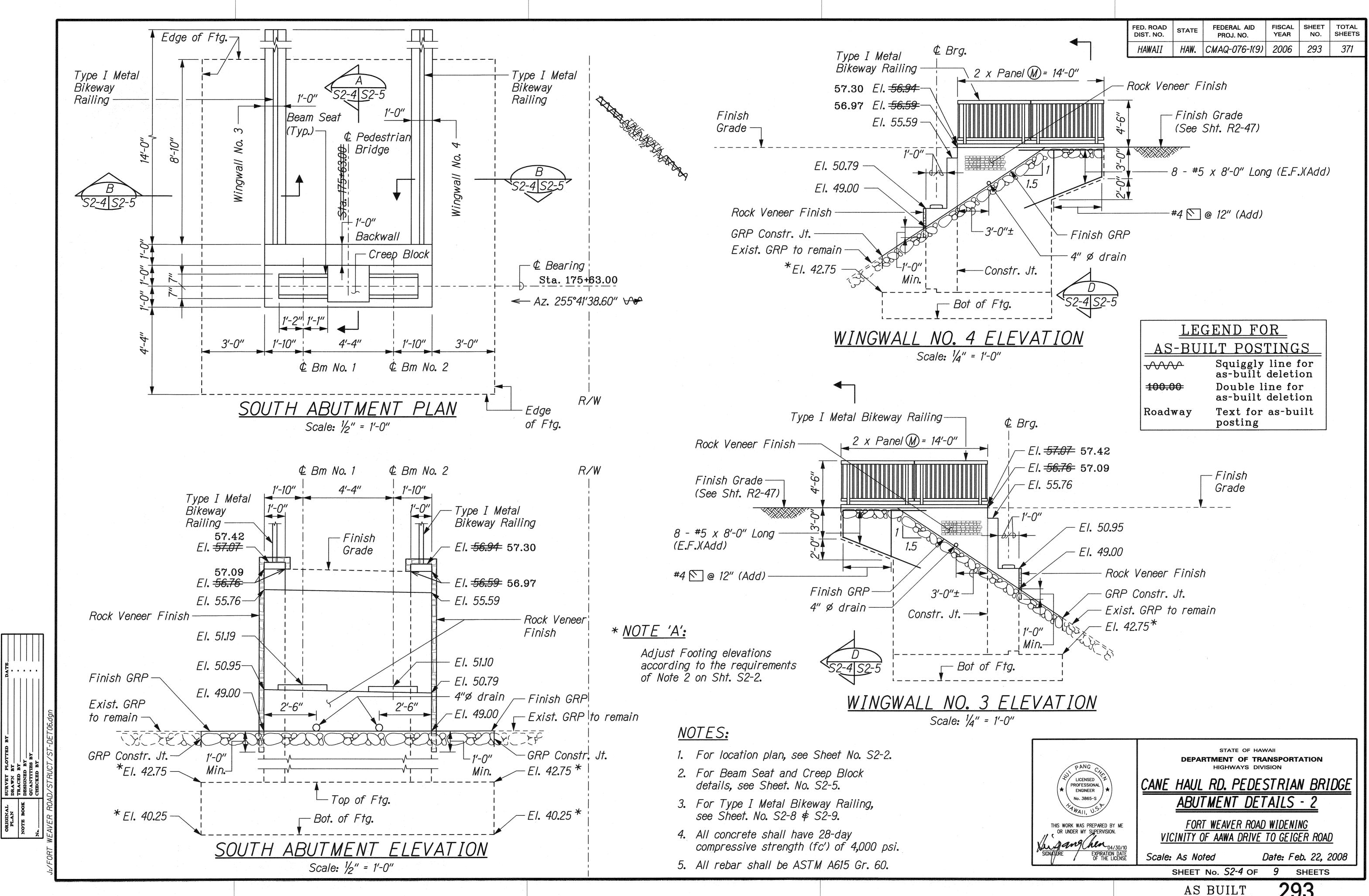
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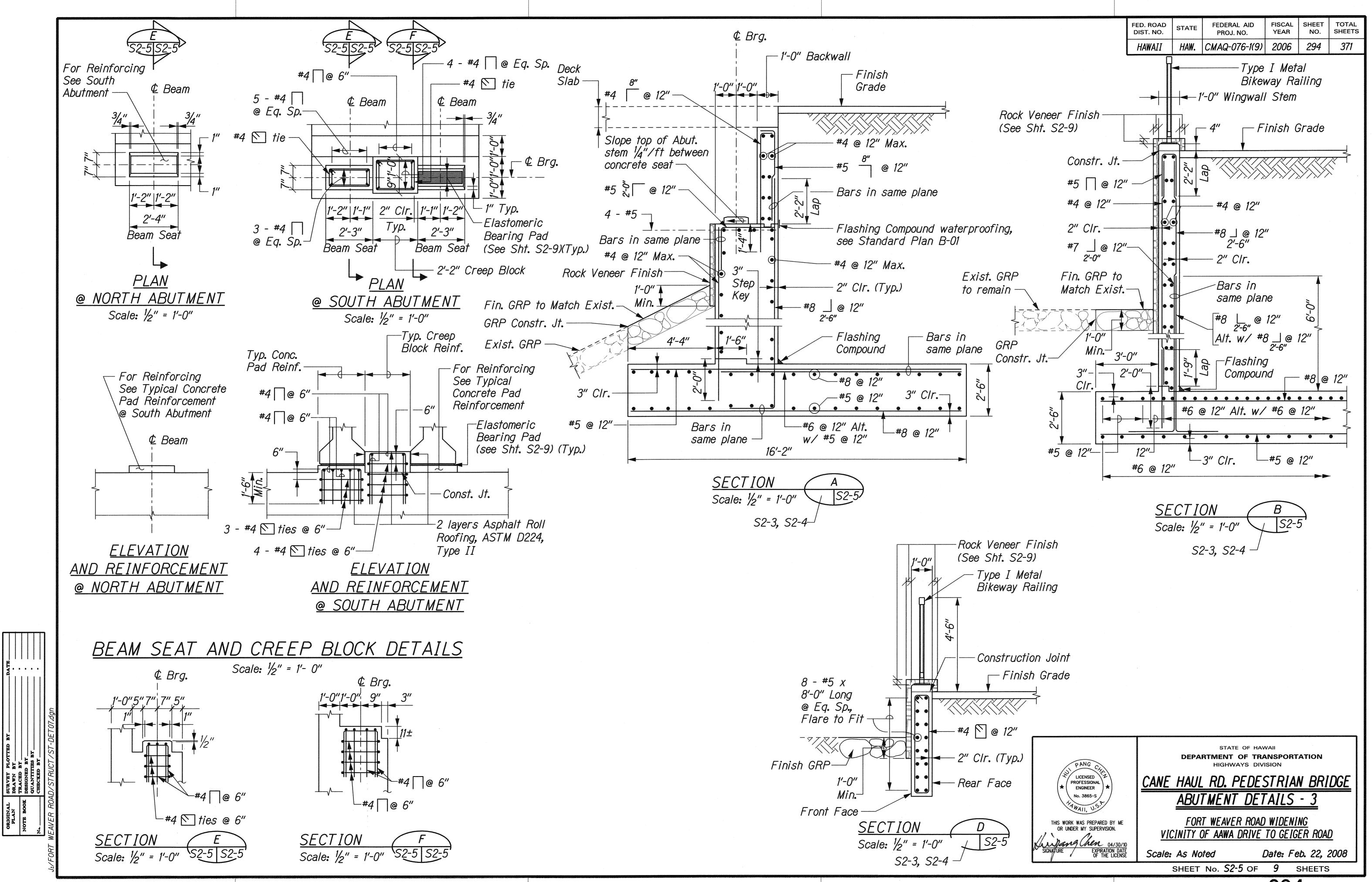
SHEET No. 52-1 OF 9 SHEETS

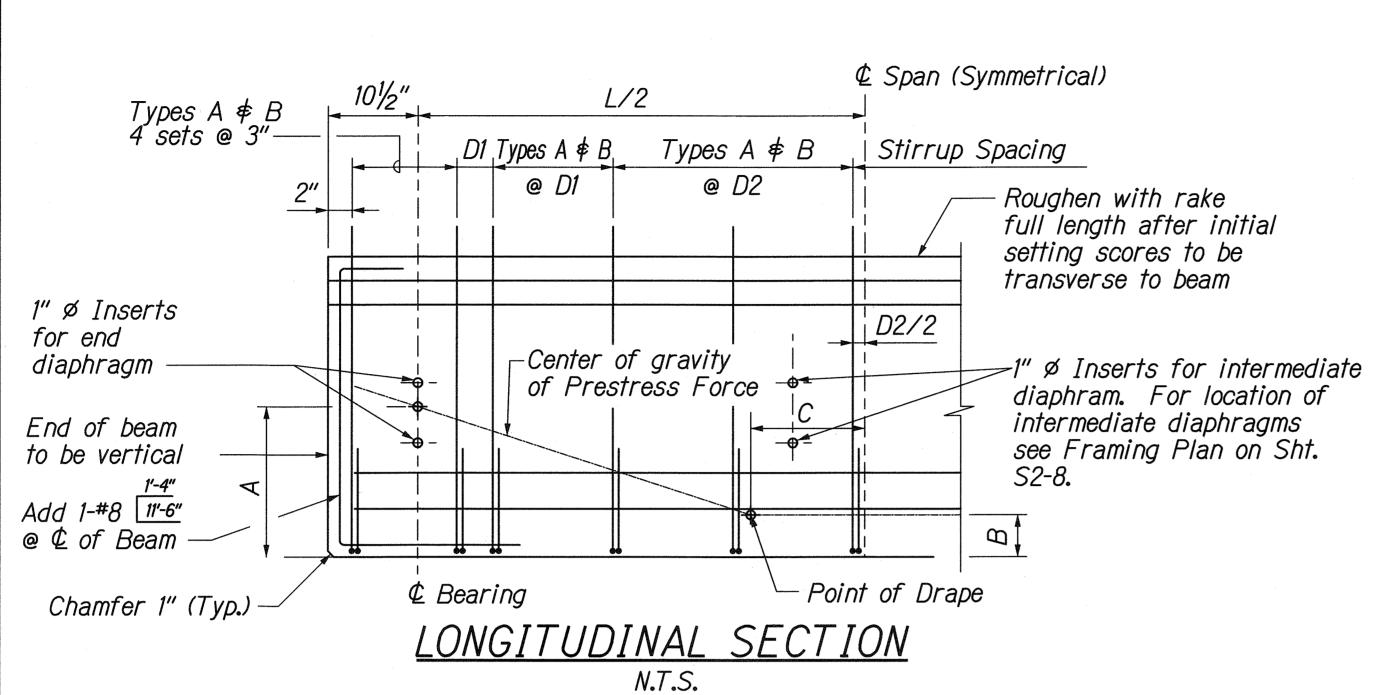


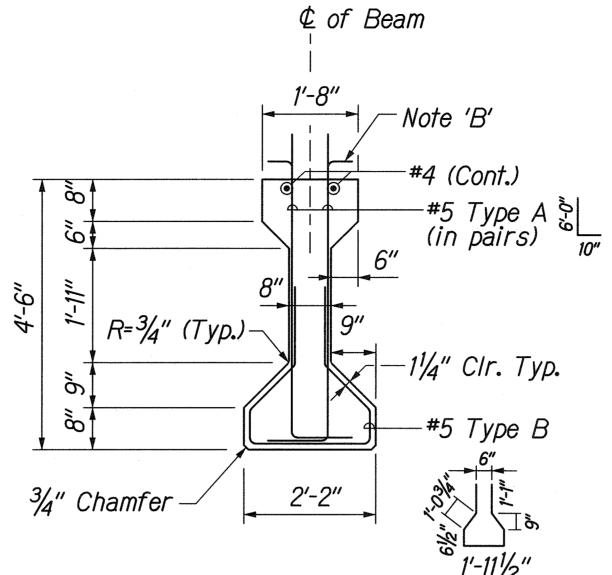












AASHTO TYPE IV TYPICAL BEAM SECTION

N.T.S.

## NOTE 'A':

For low relaxation strands, the stress immediately prior to transfer shall not exceed 0.75 fs', where fs' is the ultimate tensile strength of prestressing steel.

# NOTE 'B':

Bend bars in field 2" below top of deck. All reinforcing dimensions are out to out.

## PRESTRESSED BEAM NOTES:

- Minimum Concrete strength for precast/prestressed concrete beam shall be 6000 psi at 28 days.
- 2. Minimum compressive strength of concrete at transfer, see Table this Sheet.

FED. ROAD DIST. NO.

FEDERAL AID

PROJ. NO.

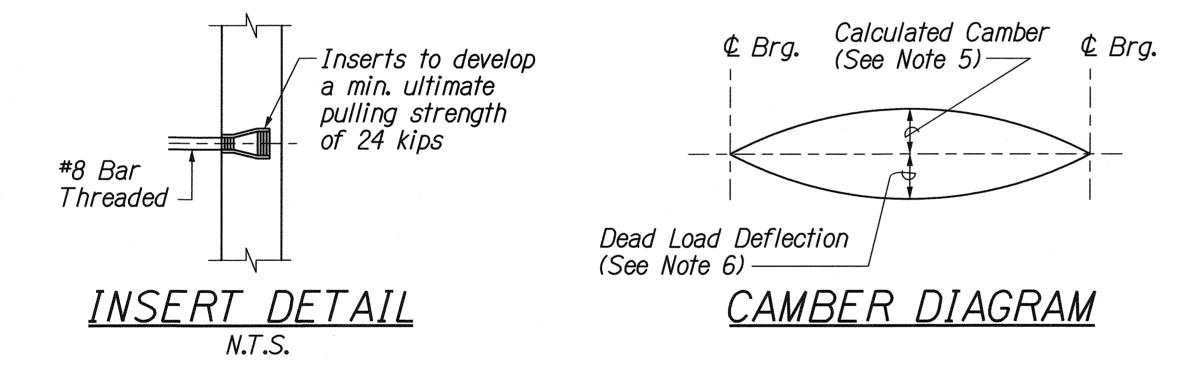
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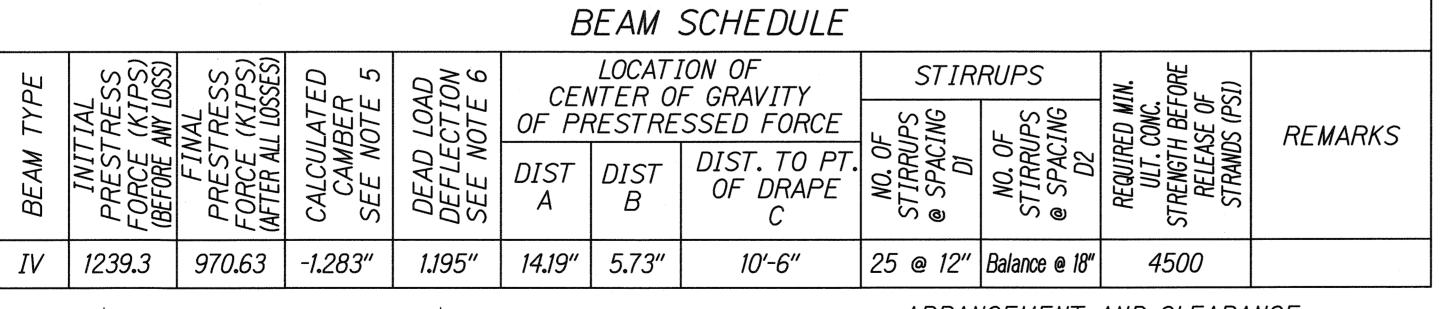
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TOTAL SHEETS

- 3. Prestressing strands shall be ½" diameter, 270 ksi, low-relax strands conforming to ASTM A 416. The estimated total losses due to creep, shrinkage, elastic shortening, and relaxation of steel are 43,900 psi.
- 4. Non-prestressed reinforcing steel in prestressed beam shall be Grade 60 unless otherwise noted. For properties, see State Standard Specifications.
- 5. The calculated camber includes the effect of the initial prestress force and the weight of the beam after removal from the bed. Negative values shown for calculated camber indicate a net upward deflection. The calculated camber values shown do not account for any camber growth of the beam. The actual camber shall not exceed the calculated camber shown on the plans by more than 100% at time of installation.
- 6. The dead load deflection includes the combined effects of the weight of beam, slab, haunch, diaphragm, and concrete railing.
- 7. Strand release sequence shall not induce any lateral deflection of the beam. For Strand release procedures, see Special Provisions.
- 8. The Contractor shall submit his proposed strand releasing sequence to the Engineer for acceptance.
- 9. During curing, care shall be taken to avoid any lateral deflection of the beam due to improper orientation.
- 10. Lifting devices shall be placed as close as possible to the centerline of bearings of the beam. Details and locations of lifting devices shall be submitted to the Engineer for acceptance. Such approval does not relieve the Contractor of his responsibilities if beam is damaged due to the failure of the lifting device.
- 11. For details of end and intermediate diagrams see Sheet No. S2-7.





Small space permitted CASE I

within group

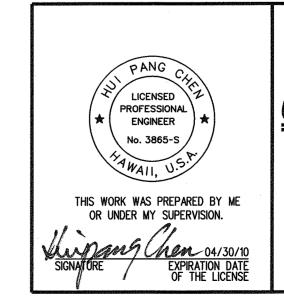
#4 @ 18" between \$ 6'-0" ea. side of harp pts. at top of each bundle

Max. of 6 vertically stacked strands in any one group

## ARRANGEMENT AND CLEARANCE FOR PRETENSIONED STRANDS

- Strands may be bundled in groups as shown.
- 2. The min. distance "a" between groups or individual strands is 2" for 1/2" \( \psi \) Strands.
- 3. "a" is measured between centers of adjacent strands.

CASE II BUNDLING OF PRESTRESSED REINFORCEMENT

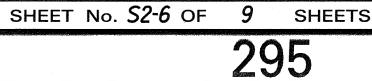


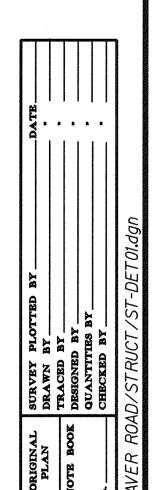
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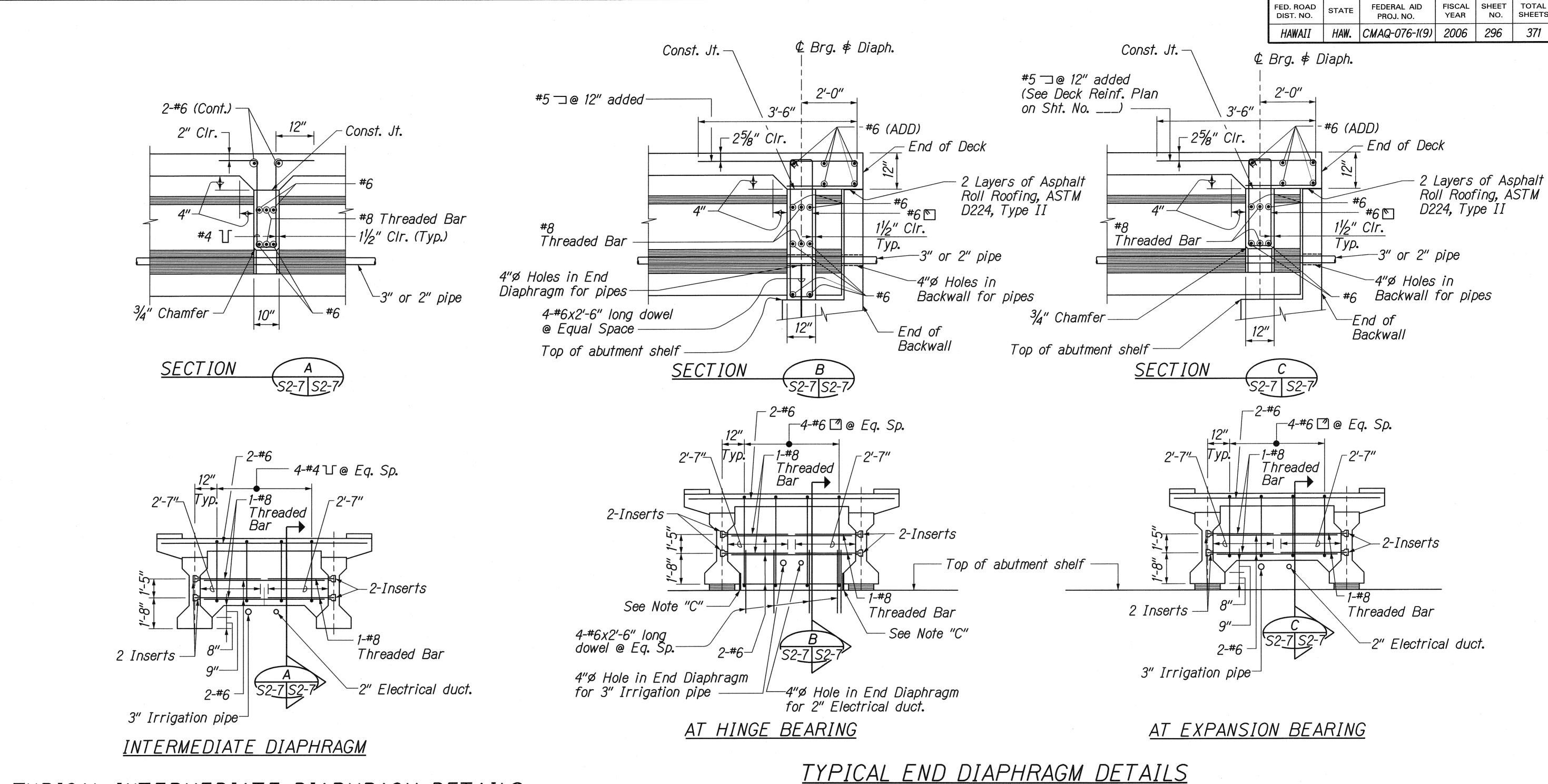
## CANE HAUL RD. PEDESTRIAN BRIDGE BEAM DETAILS - 1

FORT WEAVER ROAD WIDENING VICINITY OF AAWA DRIVE TO GEIGER ROAD

Date: Feb. 22, 2008 Scale: Not to Scale







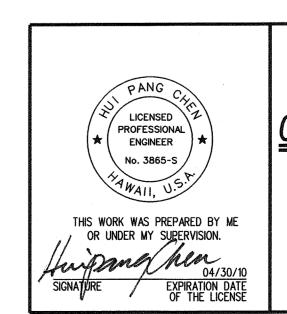
# TYPICAL INTERMEDIATE DIAPHRAGM DETAILS

N.T.S.

NOTE 'C':

Provide styrofoam to protect the elastomeric pads from concrete encroachment prior to placing end diaphragm concrete at the hinge bearing. Provide two layers Asphalt Roll Roofing ASTM D224, Type II.

N.T.S.

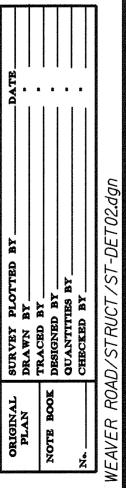


STATE OF HAWAII **DEPARTMENT OF TRANSPORTATION** HIGHWAYS DIVISION

# CANE HAUL RD. PEDESTRIAN BRIDGE BEAM DETAILS - 2

FORT WEAVER ROAD WIDENING VICINITY OF AAWA DRIVE TO GEIGER ROAD

Scale: As Shown Date: Feb. 22, 2008 SHEET No. S2-7 OF 9 SHEETS

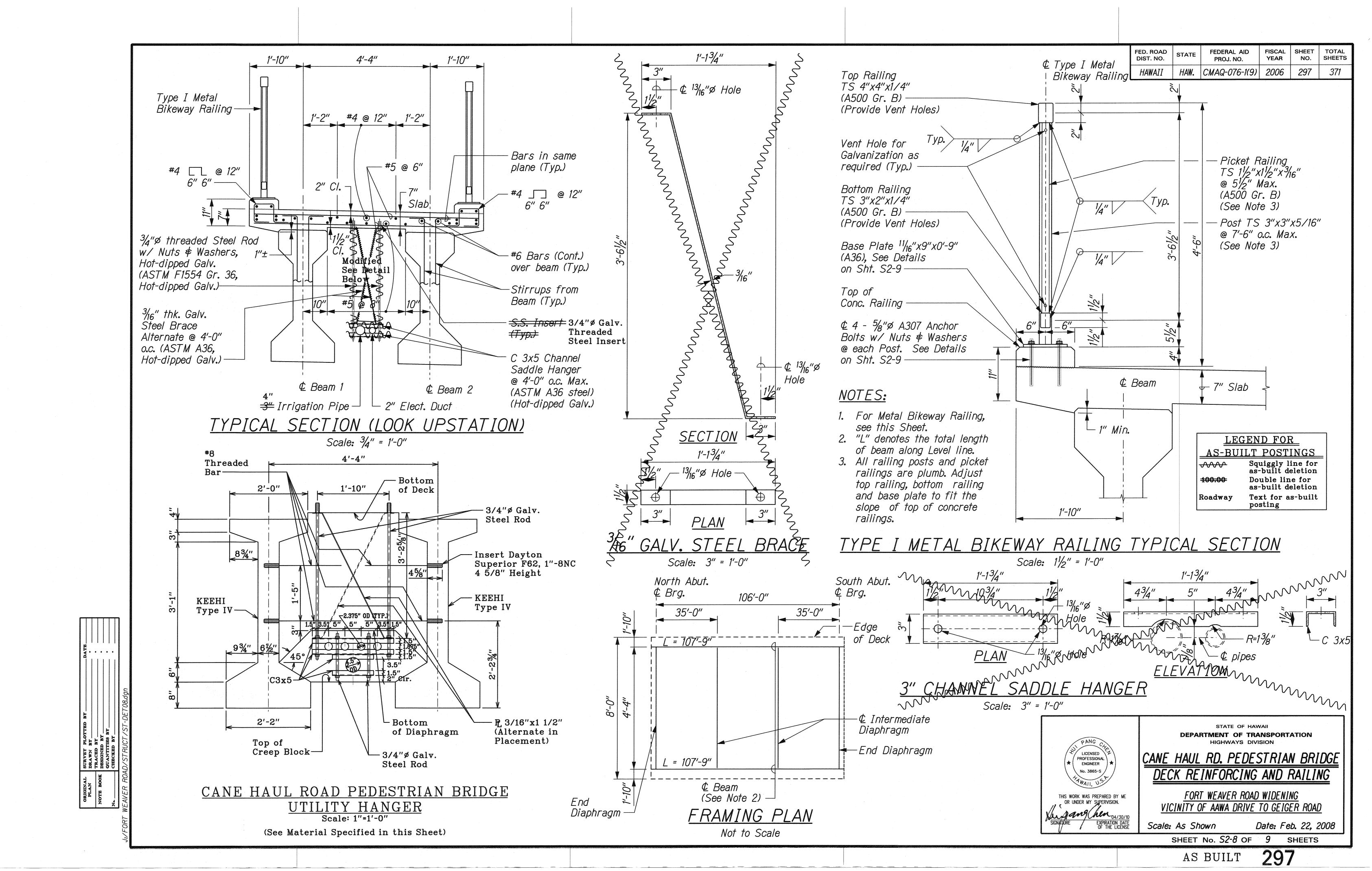


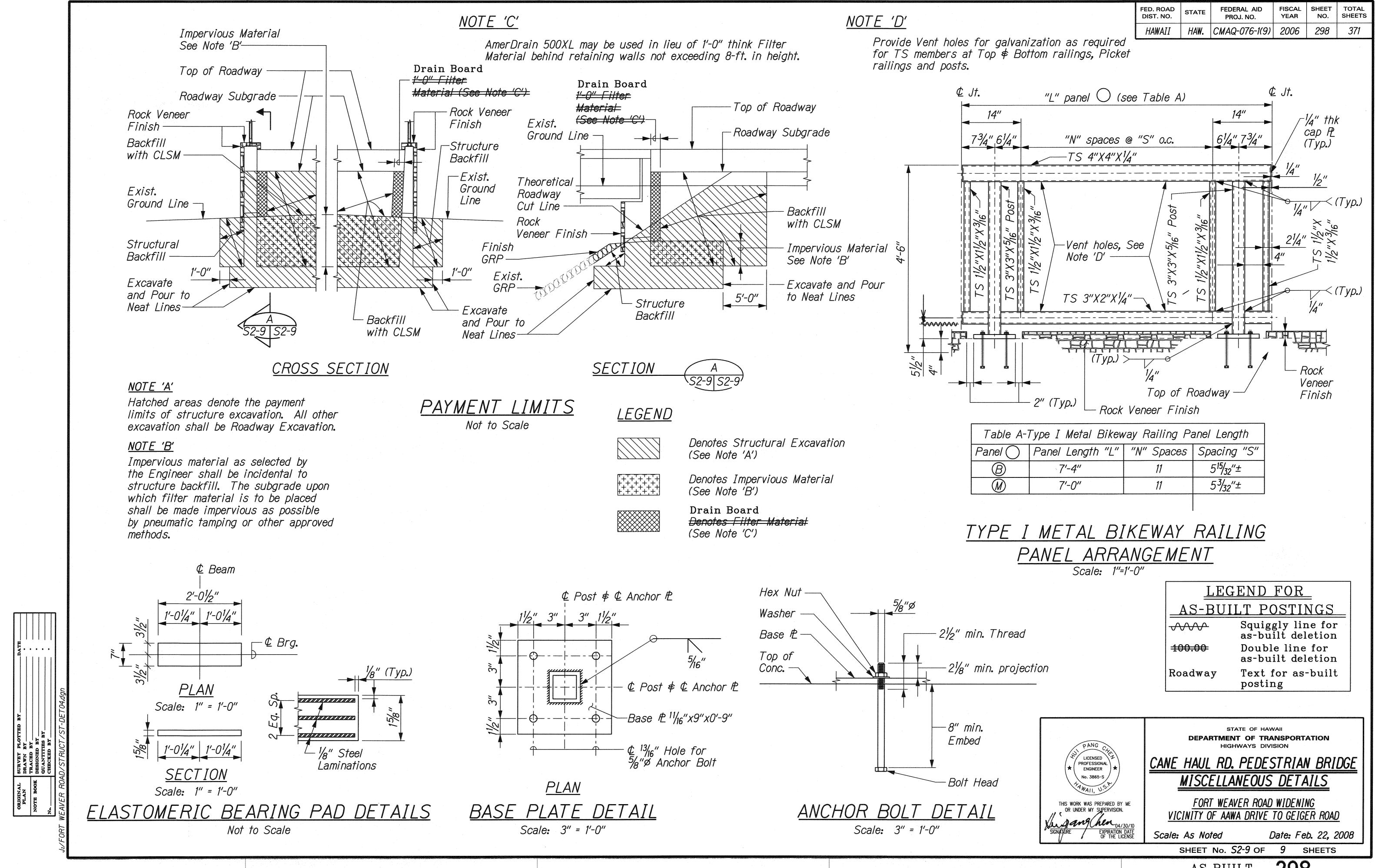
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SHEET NO.

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