MEMORANDUM

TO:	HWY-DS			
	ATTN:	Shawn M. Clarke		
FROM:	HWY	-PH [}		
		·		

The following data are submitted for your information:

NAME OF PROJECT: Pali Highway Resurfacing, Waokanaka St to Kamehameha Hwy

Districts of Koolaupoko and Honolulu, Oahu

Route 61 (2.64 to 7.69 mp)

PROJECT NO.:

SECTION:	Pali Highway, Waokanaka Street to Kamehameha Highway Route 61 (2.64 to 7.69 mp)
TRAFFIC DATA:	Tomas Young Town
2013 ADT	49,100
2023 ADT	51,800
DHV	4,400
Design K	8.5
Design D	65/35
Design T	3.0
T24	3.0

	24-HOUR TRUCK COMPOSITION
CLASSIFICATION	<u>PERCENT</u>
BUS	32.57
2D	51.93
3X	5.23
4X	0.26
2S1, 3S1, 2S2	4.13
3S2, 3-2, 2-3	4.78
6/6+X S-TLR	0.88
5X M-TLR	0.02
6X M-TLR	0.01

0.20

REF. NO. TA 13-19

7X M-TLR

TRAFFIC INDEX DETERMINATION

Project:

Rehabilitation of Pali Highway Waokanaka Street to Kamehameha Highway

Honolulu to Kaneohe, Oahu, Hawaii

Street Na	me: Pali Highway (Waokanaka Street	to Kamehameha High	ıway)	
(1)	Design Period (years)			10
(2)	Current Average Daily Traffic (ADT) Per Direct	ction		31915
(3)	Future Average Daily Traffic (ADT) Per Direct	tion		33670
(4)	Average ADT Per Direction Over Design Peri	od		32792.5
(5)	Design Lane Factor			1
	Number of Lanes In One Direction Factor 1 1 1 2 1 1 3 0.8 4 0.75	_		
(6)	24-Hour Truck Traffic, T_{24} (%) Truck Traffic Distribution: 2-axle = 3-axle = 4-axle = 5-axle = 6-axle = 7-axle =	84.50% 5.23% 4.39% 4.80% 0.89% 0.20%		3
(7)	Average Daily Truck Traffic Per Direction, AD	TT		984
(8)	Equivalent 18-kip Single Axle Loads, ESAL 2-axle: % of 2-axle trucks x No. trucks x 65 3-axle: % of 3-axle trucks x No. trucks x 525 4-axle: % of 4-axle trucks x No. trucks x 116 5-axle: % of 5-axle trucks x No. trucks x 146 6-axle: % of 6-axle trucks x No. trucks x 968	62 62	= = = = =	54034 27012 50184 69037 10380
	Annual ESAL :		=	210647
	Total ESAL For Design Period		=	2106474
	TRAFFIC INDEX (TI) = 9 (ESAL/1,000,000)E	EXP(0.119)		9.83
			SAY	10.0

Project: Rehabilitation of Pali Highway

Waokanaka Street to Kamehameha Highway

Honolulu to Kaneohe, Oahu, Hawaii

Street: Milepost 2.64 to Milepost 5.33

Design Parameters

Traffic Index	10.0
R value of ACB	90
R value of ASB	60
R value of Subgrade	15

Pavement Section using Asphalt Concrete Base and Aggregate Subbase

<u>Trial Thickness of AC + ACB</u> 9 Inches

(1)	Asphalt Concrete (AC)	
	GE required	

GE required	•				0.320	
GE with Tolerance =	0.320	+	0.240	=	0.560	
Gf of AC					2.013	
GE/Gf	=	3.34		SAY	3.500	Inches
				USF	4 000	Inches

(2) Asphalt Concrete Base (ACB) GE required =

GE required	=			1.280	
GE of AC	=			0.431	
GE required of ACB	=			0.849	
Gf of ACB				1.912	
GE/Gf	=	5.33	SAY	5.50	Inches
			USE	6.00	Inches

(3) Calculate New Gf of AC

Thickness of AC + Thickness of ACB	0.833
New Gf of AC	2.084
New Gf of ACB	1.980

(4) Aggregate Subbase (ASB)

GE required	=			2.720	
GE of AC	=			0.455	
GE of ACB	=			0.990	
GE required of ASB	=			1.275	
GE less tolerance	=			1.035	
Gf of ASB	=			1.000	
GE/Gf	=	12.42	SAY	13.00	Inches
			USE	13.00	Inches

Design Pavement Section

4.0	Inches	AC	
6.0	Inches	ACB	
13.0	Inches	ASB	
23.0	Inches	Total Thickness	•

W.O. 6782-00(A) **GEOLABS, INC.** PLATE D-3.1

Project: Rehabilitation of Pali Highway

Waokanaka Street to Kamehameha Highway

Honolulu to Kaneohe, Oahu, Hawaii

Street: Milepost 5.33 to Milepost 6.59

Design Parameters

Traffic Index	10.0
R value of ACB	90
R value of ASB	60
R value of Subgrade	15

Pavement Section using Asphalt Concrete Base and Aggregate Subbase

Trial Thickness of AC + ACB Inches

(1)	Asphalt Concrete (AC)
	GE required

GE required	•				0.320	
GE with Tolerance =	0.320	+	0.240	=	0.560	
Gf of AC					2.013	
GE/Gf	=	3.34		SAY	3.500	Inches
				USF	4 000	Inches

Asphalt Concrete Base (ACB) (2) GF required

OL roquirou	_			1.200	
GE of AC	=			0.431	
GE required of ACB	=			0.849	
Gf of ACB				1.912	
GE/Gf	=	5.33	SAY	5.50	Inches
			USE	7.00	Inches

1 280

(3) Calculate New Gf of AC

Thickness of AC + Thickness of ACB	0.917
New Gf of AC	2.151
New Gf of ACB	2.043

(4) **Aggregate Subbase (ASB)**

GE required	=			2.720	
GE of AC	=			0.477	
GE of ACB	=			1.192	
GE required of ASB	=			1.051	
GE less tolerance	=			0.811	
Gf of ASB	=			1.000	
GE/Gf	=	9.73	SAY	10.00	Inches
			USE	10.00	Inches

Design Pavement Section

4.0	Inches	AC
7.0	Inches	ACB
10.0	Inches	ASB
21.0	Inches	Total Thickness

W.O. 6782-00(A) PLATE D-3.2 GEOLABS, INC.

Project: Rehabilitation of Pali Highway

Waokanaka Street to Kamehameha Highway

Honolulu to Kaneohe, Oahu, Hawaii

Street: Milepost 6.59 to Milepost 7.68

Design Parameters

Traffic Index	10.0
R value of ACB	90
R value of ASB	60
R value of Subgrade	15

Pavement Section using Asphalt Concrete Base and Aggregate Subbase

Trial Thickness of AC + ACB 9 Inches

(1)	Asphait Concrete (AC)
	GE required

GE required					0.320	
GE with Tolerance =	0.320	+	0.240	=	0.560	
Gf of AC					2.013	
GE/Gf	=	3.34		SAY	3.500	Inches
				USF	4 000	Inches

(2) Asphalt Concrete Base (ACB) GE required =

•	, ,				
GE required	=			1.280	
GE of AC	=			0.431	
GE required of ACB	=			0.849	
Gf of ACB				1.912	
GE/Gf	=	5.33	SAY	5.50	Inches
			USE	9.50	Inches

(3) Calculate New Gf of AC

Thickness of AC + Thickness of ACB	1.125
New Gf of AC	2.301
New Gf of ACB	2.186

(4) Aggregate Subbase (ASB)

GE required	=			2.720	
GE of AC	=			0.527	
GE of ACB	=			1.731	
GE required of ASB	=			0.462	
GE less tolerance	=			0.222	
Gf of ASB	=			1.000	
GE/Gf	=	2.67	SAY	6.00	Inches
			USE	3.00	Inches

Design Pavement Section

4.0	Inches	AC	
9.5	Inches	ACB	
3.0	Inches	ASB	
16.5	Inches	Total Thickness	