

Design.

In accordance with AASHTO LRFD Bridge Design Specifications, 7th Edition, 2014 with current interims and Hawaii Department of Transportation Division of Highway Design Criteria for Bridges and Structures, 2014.

Construction

Federal Highway Administration Standard Specifications for Construction of Roads and Bridges on Federal Highway Projects, FP-14, U.S. customary units.

DESIGN LOADS:

Dead Loads:

CIP Concrete: 160 pcf

Future wearing surface allowance: 25 psf

Existing utilities = 150 plf

Future utilities allowance = 150 plf each side

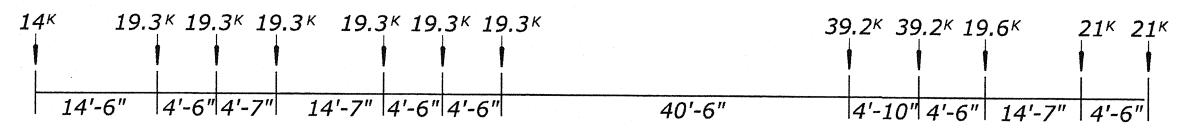
Lateral Earth Pressure: equivalent fluid unit weight of soil, 40 pcf (active)

Vertical earth: unit weight of existing soil = 115 pcf. unit weight of GRS backfill = 125 pcf.

Live Load:

HL-93. Maximum Dynamic Load Allowance (Impact), IM = 33%.

Permit truck requested by Hawaiian Electric, 12 axles:



#### DESIGN CRITERIA:

Serviceability stresses limited to current AASHTO specification values for reinforced concrete elements and precast prestressed concrete elements with the exception that no tension stress is allowed in the precompressed tensile zone of the prestressed concrete beams after all losses.

### SEISMIC DESIGN CRITERIA

In accordance with AASHTO LRFD Bridge Design Specifications, 7th Edition, 2014. Peak Ground Acceleration (PGA = 0.174g), modified by the Site Coefficient ( $F_{PGA}$  = 1.45) to give a spectrum acceleration,  $A_s$  = 0.253g. Short period acceleration at 0.2 seconds ( $S_s$  = 0.398g) modified by the Site Coefficient ( $F_s$  = 1.48) to give the short period spectrum acceleration,  $S_{DS}$  = 0.590g. Long period acceleration at 1.0 seconds ( $S_1$  = 0.109g) modified by the Site Coefficient ( $F_V$  = 2.36) to give the long period spectrum acceleration,  $S_{D1}$  = 0.258g. Site Class = D. Seismic Zone = 2.

### MATERIALS:

Concrete:

All superstructure (bridge deck, end diaphragm, fin walls, approach slab, and sleeper beam) cast-in-place concrete shall be structural class A with a minimum 28 day compressive strength f'c = 4,500 psi. Barriers, retaining walls, sidewalks and approach slabs shall be structural class A cast-in-place concrete with a minimum 28 day compressive strength f'c = 4,000 psi. All abutments, piers, and wingwalls shall be structural class A with a minimum 28 day compressive strength f'c = 4,000 psi. Type II low alkali cement shall be used. A shrinkage reducing admixture (SRA) (see SCR 711.03(b)) shall be added to the concrete mix for all cast-in-place concrete. The minimum dosage requirement shall be 128 ounces per cubic yard of concrete. A migrating corrosion inhibitor amine carboxylate water-based admixture (see SCR 711.03(a)) shall be added to the concrete mix for all cast-in-place concrete. The minimum dosage requirement shall be 1.5 pints per cubic yard of concrete. Concrete for the bridge deck, end diaphragm, fin walls, approach slab, and sleeper beam shall contain 13 lbs./cuyd of alkali resistant glass macrofiber. The fiber shall be  $1\frac{1}{2}$ " long minimum and have an aspect ratio of 67. The use of any calcium chloride in any concrete is prohibited. Chamfer exposed edges of all concrete  $\frac{3}{4}$ ", unless noted otherwise on the plans. Preformed expansion joint filler shall meet the requirements of AASHTO M213. Preformed flexible cellular joint filler shall meet the requirements of AASHTO M33.

Reinforcing Steel:

All reinforcing steel shall conform to AASHTO M31 or M322, grade 60 deformed. The minimum concrete cover to the face of any bar shall be 2", unless shown otherwise on the plans. Minimum splice length for all bars sizes shall be as shown on the plans. Bar splices other than those shown on the plans shall not be paid for.

Prestressing Steel:

Prestressing steel shall be grade 270, 0.6" dia., seven wire, uncoated, low-relaxation, prestressing strand conforming to AASHTO M203. Each strand shall be pretensioned to a total load of 43,943 lbs. at which f'si = 0.75 (f's) = 202,500 psi.

Prestressed Concrete Girders:

All concrete for prestressed concrete plank girders shall be Class P or Class P (AE), with a minimum 28-day strength, f'c = 8,500 psi and initial strength f'ci = 7,000 psi. A migrating corrosion inhibitor amine carboxylate water-based admixture (see SCR 711.03(a)) shall be added to the concrete mix for precast concrete plank girders. The minimum dosage requirement shall be 1.5 pints per cubic yard of concrete. the use of any calcium chloride in any concrete is prohibited. Structural metal for bearing plates shall conform to ASTM 36. Welding for welded anchors shall conform to ANSI/AASHTO/AWS D1.5, Chapter 7.

TATE	PROJECT	SHEET NO.	
Н	HI STP H1 (1)	S2	de de la companya de

	ESTIMATE			
Item No.	Item	Quantity:	Unit:	Notes.
15214-1000	Survey and staking, bridge	1	LPSM	
20304-2000	Removal of bridge	1	LPSM	
20435-2000	Backfill, granular	138	CUYD	(1)
20720-0300	Reinforcement geosynthetic, type 3	921	SQYD	(1)
20801-0000	Structure excavation	1,870	CUYD	(1)
20803-0000	Structural backfill	1,570	CUYD	(1)
20810-0000	Shoring and bracing	1	LPSM	(5)
20815-0000	Cofferdams	1	LPSM	(4)
25801-0300	Reinforced concrete retaining wall, 8 feet	198	SQFT	(1)
55101-0300	Precast prestressed concrete pile, in place	1,855	LNFT	
55104-1000	Dynamic pile load test	4	EACH	(1)
55115-1000	Preboring	820	LNFT	
55201-0100	Structural concrete, class A	350	CUYD	(1)(3)
55235-0000	Expansion joints	96	LNFT	(1)
55302-0300	Precast, prestressed concrete slabs, 48-inch non-voided	1,089	LNFT	(1)
55401-1000	Reinforcing steel	61,400	LB	(1)
55601-0500	Bridge railing, concrete	290	LNFT	(1)(2)
61707-0000	Structure transition railing	25	LNFT	(1)
61901-1300	Fence, chain link	19	LNFT	(1)
63610-1600	Conduit, 2-inch, PVC	100	LNFT	
63610-2200	Conduit, 3-inch, PVC	148	LNFT	
63610-2800	Conduit, 4-inch, PVC	283	LNFT	
64604-3000	Fixture, pedestrian railing	35	LNFT	(1)

### **ESTIMATE NOTES:**

- (1) Contract Quantity.
- (2) Includes cost of concrete, reinforcing steel, endblock concrete, and endblock reinforcing steel.
- (3) Includes cost of furnishing and installing bearing related items, and all joint fillers.
- (4) Cofferdams are shown on the "Suggested Construction Staging" sheets, but other means or methods may be used to divert half of the channel, such as a portadam. The contractor shall determine the means and method for channel diversion. This item includes all work required for channel diversion, including multiple installations if needed and any dewatering.
- (5) Shoring and bracing includes any shoring or bracing needed to construct the GRS backfill and retaining walls against H-1 and maintain the integrity and stability of the H-1 bridge.



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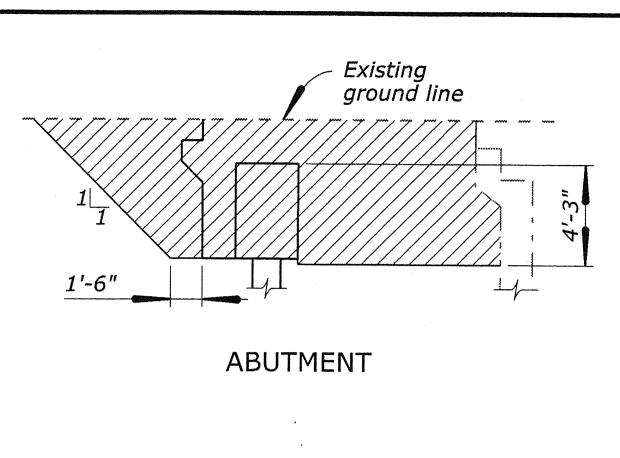
HALONA STREET BRIDGE

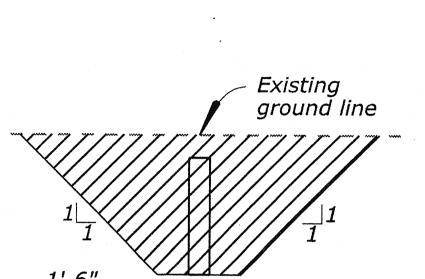
HALONA STREET
HONOLULU COUNTY, HAWAII

**GENERAL NOTES & ESTIMATE** 

REVISIONS DESIGNED BY CHECKED BY SCALE PROJECT TEAM LEADER BRIDGE DRAWING DATE DRAW	NO DATE DY	DEVICIONS										
A. PLANKIS G. McGINN B. LUEBBERS NO SCALE 1. ROHNER 2 of 35 SEPTEMBER 2016 RC	NO. DATE BY	REVISIONS	NO. DATE BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
S. KOLINEK ZOTO IKO	12/2				A. PLANKIS	G. McGINN	B. LUEBBERS	NO SCALE	J. ROHNER	2 of 35	SEPTEMBER 2016	RG3077-B

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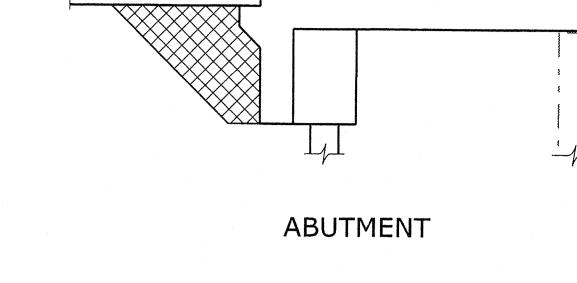
**FINWALL** 

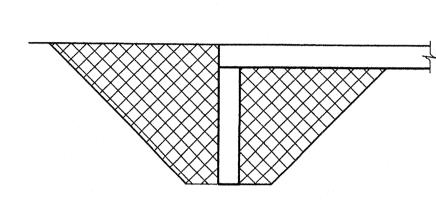
Existing

ground line

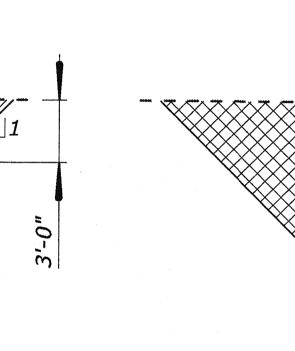
**EXISTING** 

PIERS 2 & 3



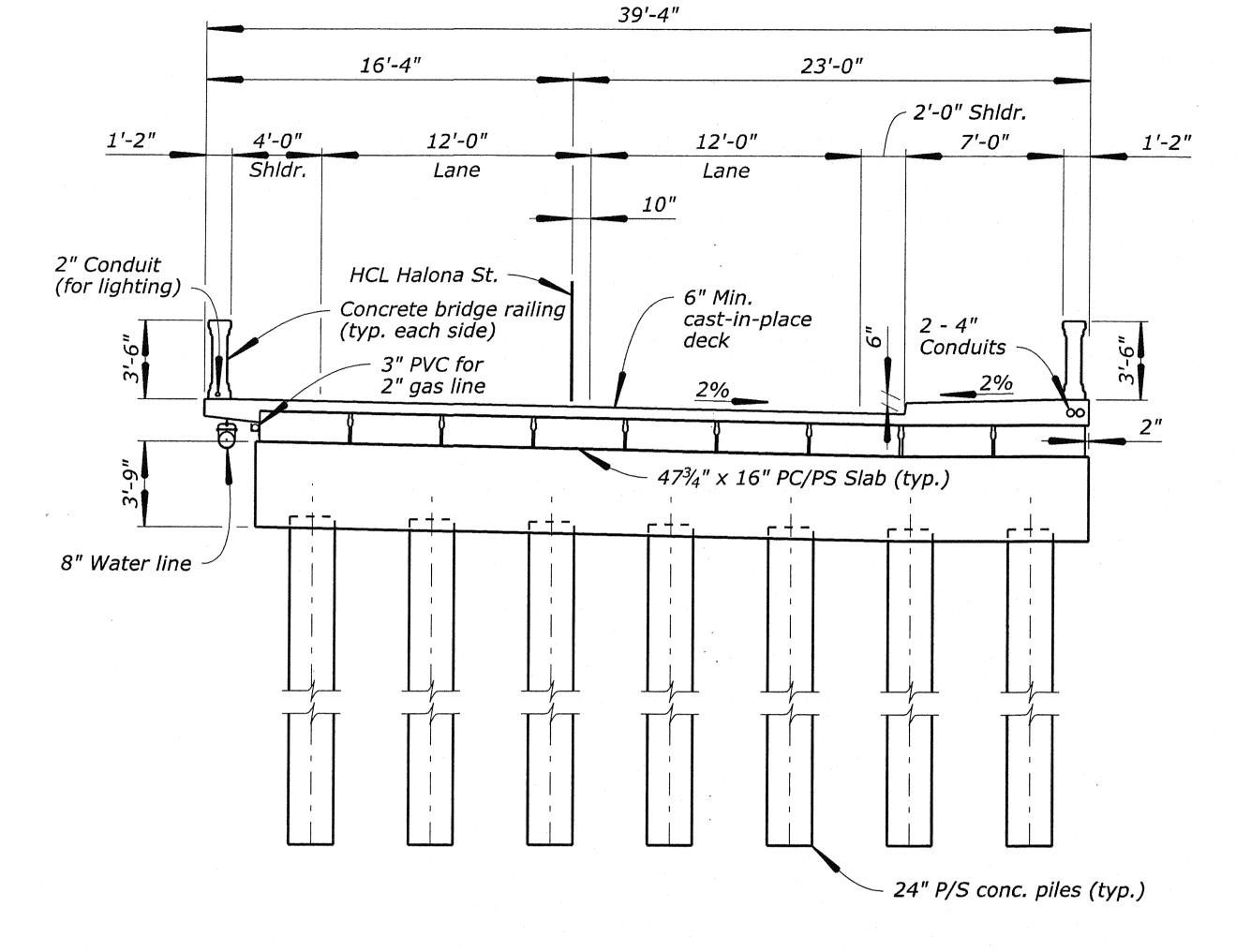


FINWALL





EXISTING PIERS 1 & 4 EXISTING PIERS 2 & 3



### TYPICAL SECTION AT THE PIER

Scale:  $\frac{1}{8}$ " = 1'-0"

## STRUCTURE EXCAVATION

Not to Scale

**EXISTING** 

PIERS 1 & 4

### **LEGEND**

1'-6"

(Typ.)

12'-0"

Pay limits of structure excavation

Pay limits of structural backfill

## STRUCTURAL BACKFILL

Not to Scale

### NOTES:

- 1. Structural backfill at existing piers does not need to meet the material requirements of specification 208 but still needs to follow the construction requirements. The material that was excavated to remove the piers may be used to backfill the same area.
- 2. If shoring is required to excavate for removal of existing piers, the cost of this shoring will not be paid for separately, but will be incidental to structure excavation.



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STATE

PROJECT

HI STP H1 (1)

SHEET NO.

**S3** 

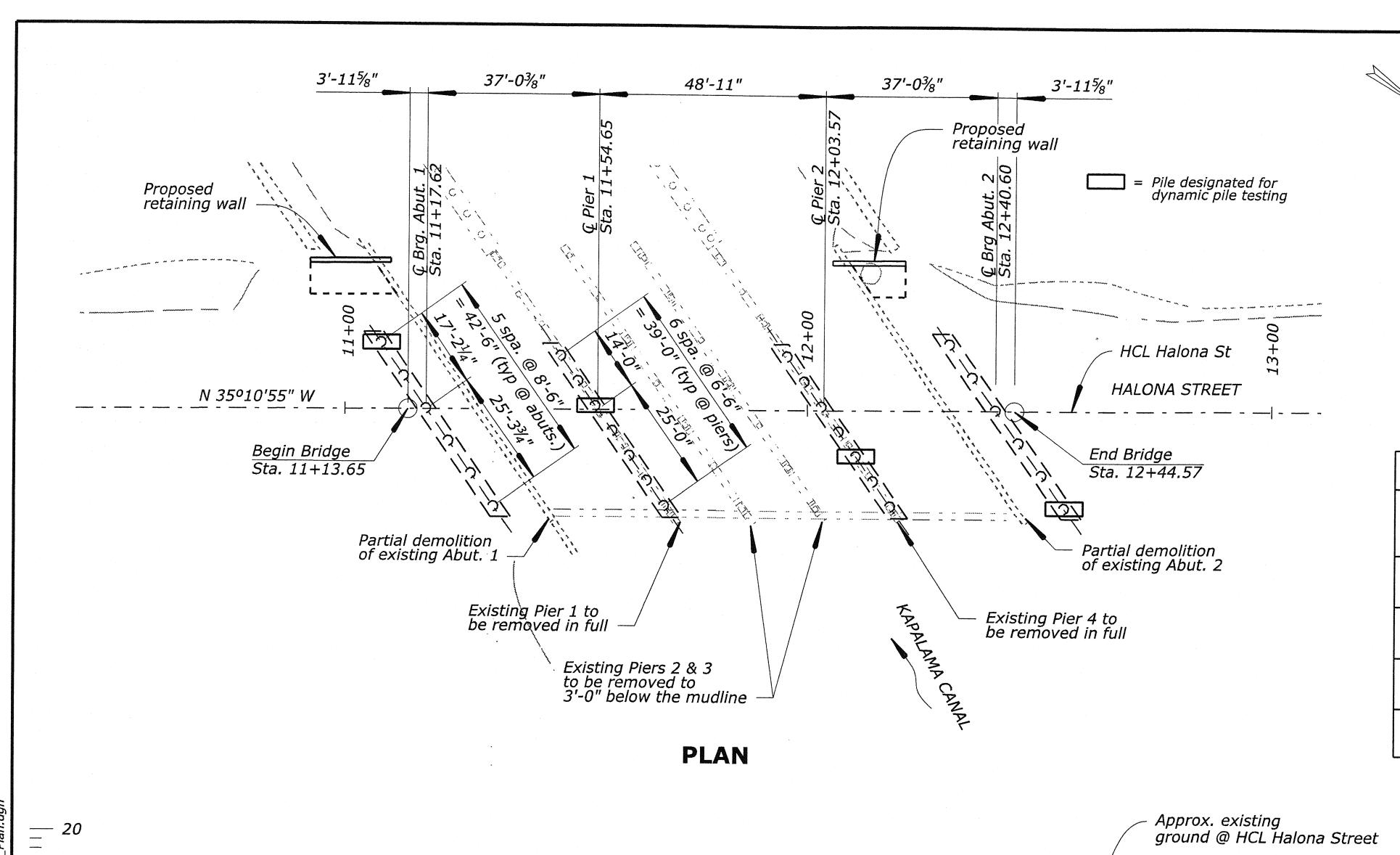
HALONA STREET BRIDGE

HALONA STREET

HONOLULU COUNTY, HAWAII

TYPICAL SECTION AND EXCAVATION AND BACKFILL LIMITS

NO. DATE BY REVISIONS NO. DATE BY REVISIONS DESIGNED BY DRAWN BY CHECKED BY PROJECT TEAM LEADER SCALE BRIDGE DRAWING DATE DRAWING NO. A. PLANKIS G. McGINN 1/8" = 1'-0" RG3077-C B. LUEBBERS J. ROHNER 3 of 35 SEPTEMBER 2016



Est. tip El. -70.00

**ELEVATION** 

PIER 2

Approx. & Kapalama Canal

Est. tip El. -62.00

ABUT. 2

El. 4.23 & Varies

Est. tip El. -62.00

ABUT. 1

24" P/S Conc. piles (typ.)

--- -10

Est. tip El. -70.00

PIER 1

STATE PROJECT SHEET NO.

HI HI STP H1 (1) S4

### NOTES:

- 1. Predrill holes using an auger with a 19"-22" diameter to elevation -27.0 for all pile locations.
- 2. The resistance factors (Ø) used to determine the pile capacity are 1.00 for Service I, 1.00 for Extreme Event I, and 0.65 for Strength I.
- 3. The strength resistance factor (Ø) of 0.65 assumes dynamic load tests will be performed to confirm pile axial resistance.
- 4. For further pile capacity and driving information, see final geotechnical report, Jan. 2016, prepared by Hirata & Associates, Inc.

			PILE DATA		
LOCATION	SERVICE 1 LOAD (k)	STRENGTH 1 LOAD (k)	EXTREME EVENT 1 LOAD (k)	NOMINAL RESISTANCE (k)	DESIGN TIP ELEVATION (ft)
ABUT. 1	147	211	94	330	-62.00 (a) -39.00 (b)
PIER 1	177	255	124	400	-70.00 (a) -62.00 (b)
PIER 2	177	255	124	400	-70.00 (a) -62.00 (b)
ABUT. 2	147	211	94	330	-62.00 (a) -43.00 (b)

Note: Design tip elevations are controlled by: (a) axial load or (b) lateral load. Test piles shall be driven to estimated tip elevation shown in the elevation view. Pile quantities on sheet S2 are based on lowest tip elevations given. It is anticipated that the piles will need to be driven to the estimated tip based on geotechnical report. No piles will be accepted with pile tips that are above the lateral load design tip elevation.

ALEX L FIRTA
* LICENSED PROFESSIONAL ENGINEER No. 16749-S
Equall U.S.F.

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04/30/2018 Expiration Date of the License

100	J-YR SCC	DUR DATA	TABLE
LOCATION	GROUND ELEV. (FT)	SCOUR DEPTH (FT)	SCOUR ELEV. (FT.)
PIER 1	-0.10	9.36	-9.46
PIER 2	-0.70	9.36	-10.06

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HALONA STREET BRIDGE

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HONOLULU COUNTY, HAWAII

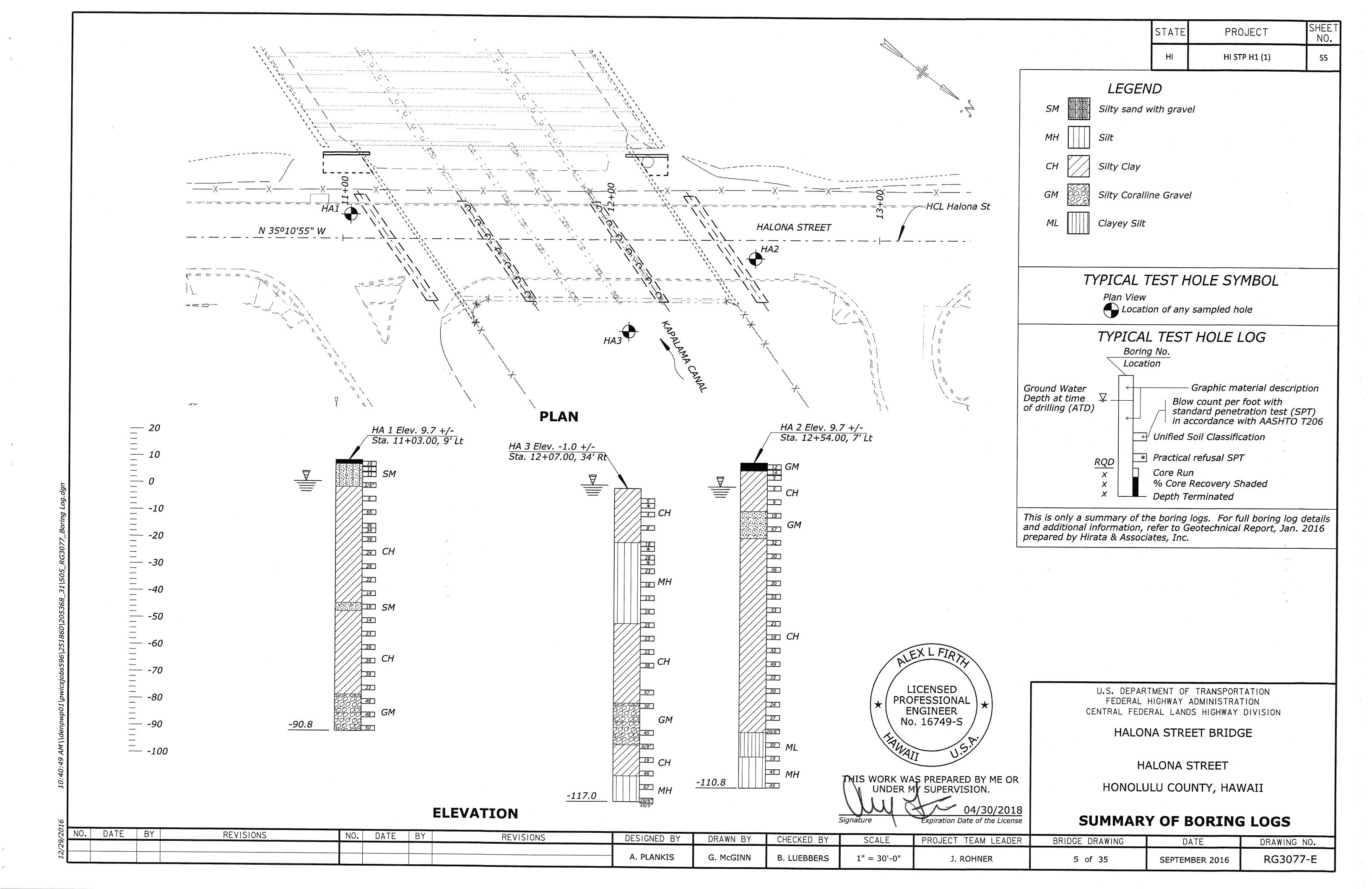
### **FOUNDATION PLAN**

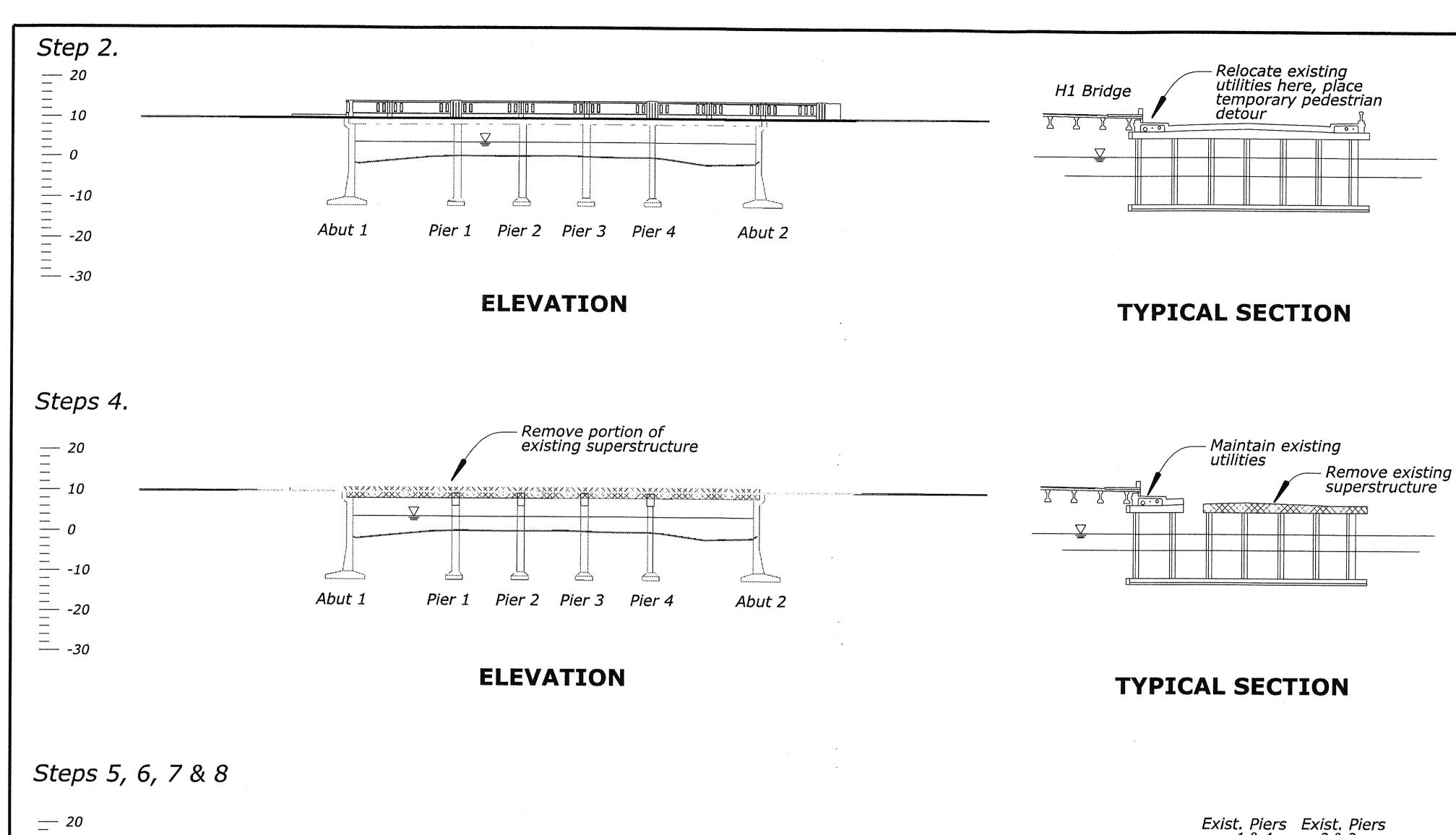
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12/.					A. PLANKIS	G. McGINN	B. LUEBBERS	1" = 30'-0"	J. ROHNER	4 of 35	SEPTEMBER 2016	RG3077-D

El. 4.10 & Varies

El. -27.00

Bottom of predrilled holes





Abut 2

Abut 2

Construct GRS backfill (typ)

Install piles and

construct pile cap

Pier 2 / Pier 3

**ELEVATION** 

Install piles and

construct pile cap

Pier 2

Abut 1

Abut 1

Pier :

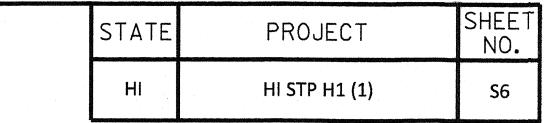
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**---** -10

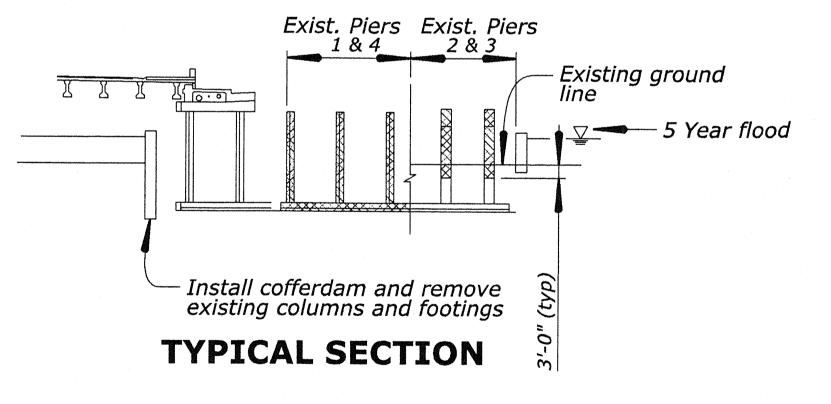
Steps 9, 10 & 11

Install cofferdam and remove existing columns and footings



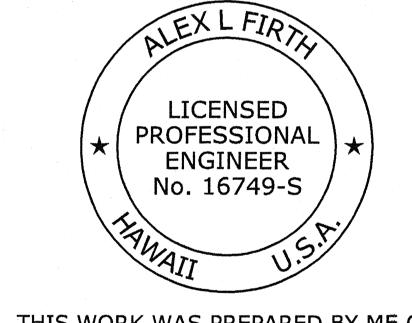
## SUGGESTED SEQUENCE OF CONSTRUCTION:

- Prepare and submit detailed demolition plan for approval.
- Temporarily relocate the 8" water line and the  $1\frac{1}{2}$ " gas line to the area under the existing bridge that has the abandoned 12" waterline, on the makai side of the bridge.
- Divert traffic to the approved detour route and establish the temporary pedestrian detour above the relocated utilities.
- Excavate behind the existing abutments, then remove the existing superstructure and top portion of existing abutment as shown, except for the portion of the bridge that is supporting the relocated utilities and pedestrians.
- Install cofferdam to divert half of the canal. Provide a minimum freeboard of 1 foot above the 5 year storm elevation. The cofferdam must accommodate the 5 year storm event without overtopping.
- Excavate and remove the pier columns and footings to the limits as shown on the plans. The columns and footings that support the portion of the bridge with the temporarily relocated utilities will be removed after the utilities have been permanently installed on the new structure.
- Restore the bottom of the stream channel to the original elevation after the columns and/or footing(s) have been removed.
- Remove and reinstall the cofferdam to block opposite half of canal and repeat steps 6 and 7 to excavate and remove pier columns and footings as indicated.
- Install piles and construct pile caps for the proposed piers.
- Install abutment piles and pile caps at each end of the bridge.
- Fill between existing abutment and new abutment with geosynthetic reinforced soil (GRS) to the limits indicated on the plans.



- Install piles and

construct pile cap



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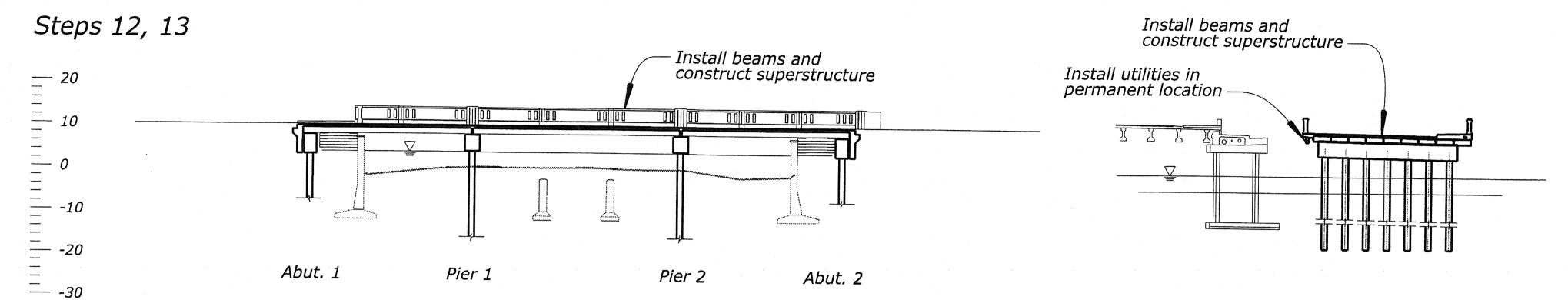
HALONA STREET BRIDGE

HALONA STREET

HONOLULU COUNTY, HAWAII

SUGGESTED CONSTRUCTION

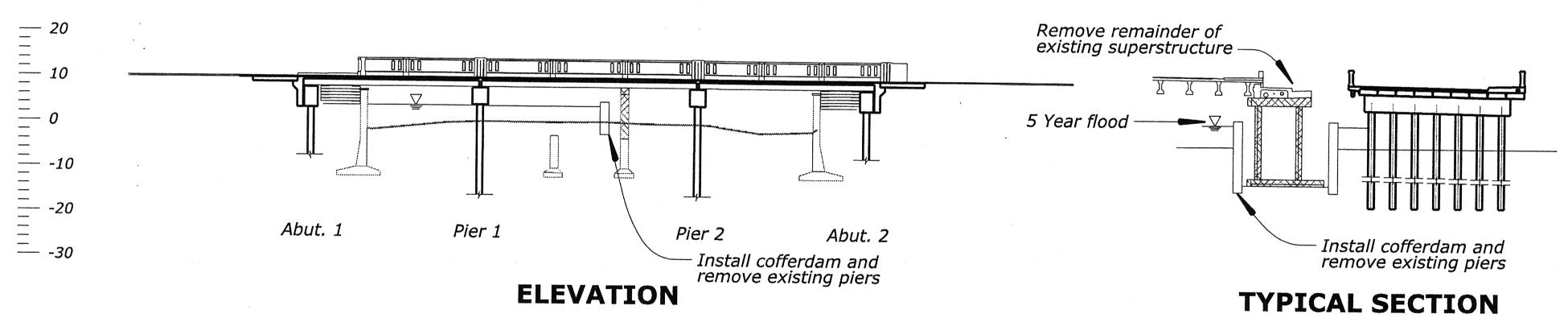
910 -30	ELEVAITON						L SECTION	<b>J</b>		PHASING PLAN 1 OF 2			
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12/2					A. PLANKIS	G. McGINN	B. LUEBBERS	NO SCALE	J. ROHNER	6 of 35	SEPTEMBER 2016	RG3077-F	



### **ELEVATION**

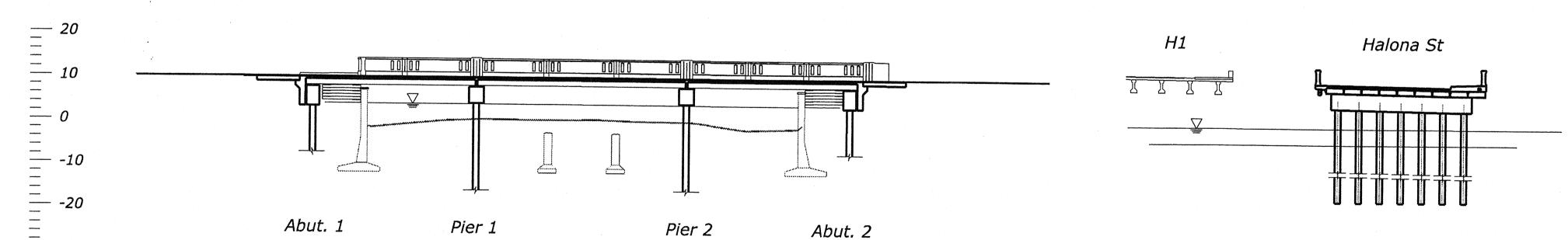
## **TYPICAL SECTION**

### Steps 14, 15 & 16



## Steps 17 & 18

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### **ELEVATION**

### TYPICAL SECTION

(Removal at Piers 1 and 4 shown, at Piers 2 and 3 only remove to 3' below mud line)

# SUGGESTED SEQUENCE OF CONSTRUCTION (CONTINUED):

- 12. Place beams and construct new superstructure.
- 13. Install utilities in permanent location on the new superstructure, and under approach roadways as needed to connect to existing facilities.
- 14. Construct approach slabs and sidewalks along the approach roadways. Reroute pedestrians onto new bridge.
- 15. Excavate behind the existing abutments between the new bridge and the H-1 bridge, then remove the portion of the existing superstructure adjacent to the H1 bridge that was used to support the temporary utility relocation.
- 16. Install cofferdam, as above, and remove the remainder of the existing pier columns and footings as indicated. Restore the bottom of the stream channel to the original elevation.
- 17. Construct the retaining walls at the H1 abutments. Replace existing fill material with GRS to the limits shown on the plans.
- 8. Grade soil on top of the GRS and restore the area between the two bridges.
- 19. Reopen Halona Street to normal traffic and remove the detour route.



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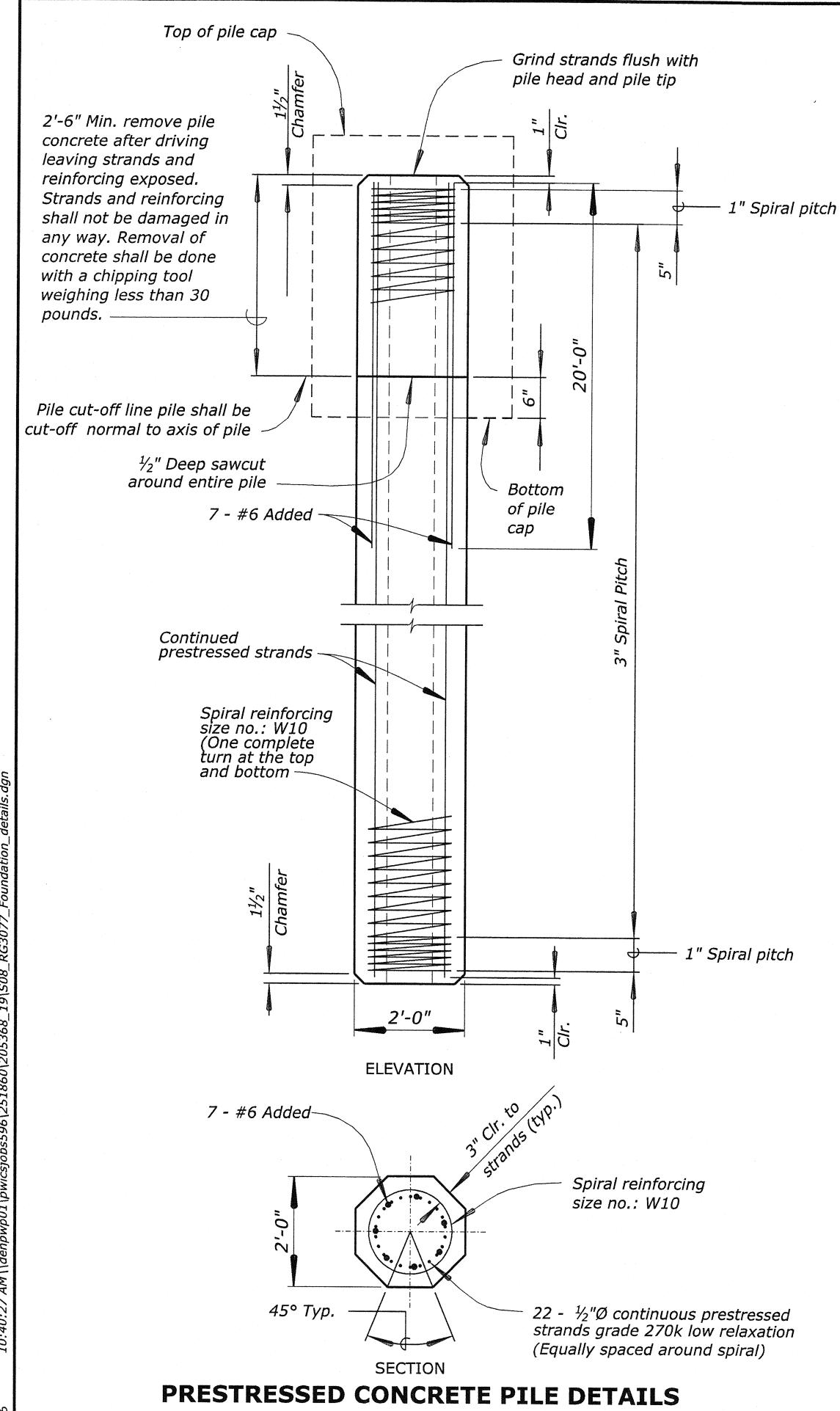
HALONA STREET BRIDGE

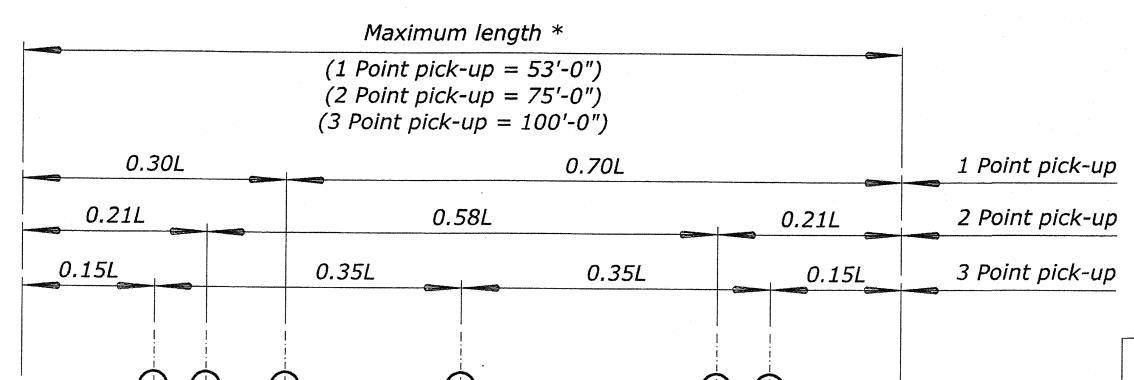
HALONA STREET

HONOLULU COUNTY, HAWAII

SUGGESTED CONSTRUCTION PHASING PLAN 2 OF 2

NO. DATE	BY REVISIONS	NO	DATE D	Y DEVICIONS						PHASI	ING PLAN 2 O	F 2
/30/	TILL VIOLOTO	I NO.	DATE	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
12,					A. PLANKIS	G. McGINN	B. LUEBBERS	NO SCALE	J. ROHNER	7 of 35	SEPTEMBER 2016	RG3077-G





\* The length "L" is the distance end to end of pile.

## PRESTRESSED CONCRETE PILE PICK-UP POINTS

### **NOTES:**

- 1. The 7 #6 bars shall have a minimum of 2'-6" embeded into the pile cap and extending a minimum of 10'-0" below the bottom of pile cap.
- Top end of strands, #6 bars, and spirals may be cut off to maintain minimum concrete cover.
- 3. The minimum effective force in the prestressed steel for each pile shall be 561 kips.
- 4. Piles accepted by the Contracting Officer (CO) shall not be damaged. Damaged piles shall be replaced or repaired as directed by the CO, at the Contractor's expense.
- Work of cutting off prestressed concrete piles or cast-in-place pile extensions shall be performed in such as to avoid spalling or damaging of the pile below cut-off. Damaged portions shall be removed and pile cut-off elevation lowered as directed by the CO. Additional costs resulting from this shall be borne by the Contractor.
- 6. Top of pile at cut-off line shall be comply with Specification Section 551.14.
- 7. Pile splice will not be permitted, except for pile length in excess of 100 feet, unless otherwise approved by the CO.
- 3. The in-place length of pile above the splice shall be approximately 50 percent of the total length of pile driven, unless location of splice is indicated in the plans or specification.
- 9. Concrete shall be Class P or Class P (AE). Piles and cast-in-place pile extensions may be driven after the concrete attained a compressive strength of 5,000 psi., but not earlier than 7 days after the concrete is poured.
- 10. See Specification 715.03 for handling and transportation of prestressed concrete piles.
- 11. For pile length  $\leq 75'-0''$ , place supports at 2 point pick-up locations. For pile lengths > 75'-0'' and  $\leq 100'-0''$ , place supports at 3 point pick-up locations.

PT	IF	NG	TH	DA	TA

STATE

PROJECT

HI STP H1 (1)

NO.

S8

			DAIF	<b>78.</b>	Post Paris
Pile Lo	cation	Top of	Est. Tip	Min. Pile	
Support	Offset	Pile Elev.	Elev.	Length (ft)	Actual Pile Tip
Abut 1	14.08′ L	7.60	-62.00	69.60	Elevation
	7.12' L	7.51	-62.00	69.51	-68.90
	0.16' L	7.41	-62.00	69.41	-65.01
	6.81' R	7.32	-62.00	69.32	-62.74
	13.77' R	7.22	-62.00	69.22	-64.67
	20.73' R	7.21	-62.00	69.21	-62.74
Pier 1	11.47' L	7.79	-70.00	77.79	-61.35
	6.15' L	7.70	-70.00	77.70	-63.88
	0.83' L	7.62	-70.00	77.62	-64.81
	4.50' R	7.53	-70.00	77.53	-64.79
	9.82' R	7.44	-70.00	77.44	-68.63
	15.15' R	7.36	-70.00	77.36	-63.71
	20.47' R	7.36	-70.00	77.36	-63.52
Pier 2	11.47' L	7.82	-70.00	77.82	-61.40
	6.15' L	7.70	-70.00	77.70	-61.40
	0.83' L	7.59	-70.00	77.59	-61.40
	4.50' R	7.48	-70.00	77.48	-62.40
	9.82' R	7.36	-70.00	77.36	-61.40
	15.15' R	7.26	-70.00	77.26	-63.40
	20.47' R	7.26	-70.00	77.26	-61.40
Abut 2	14.08' L	7.76	-62.00	69.76	-56.70
	7.12' L	7.60	-62.00	69.60	-56.55
	0.16' L	7.43	-62.00	69.43	-56.50
	6.81' R	7.27	-62.00	69.27	-60.94
·	13.77' R	7.11	-62.00	69.11	-55.80
-  -  -	20.73' R	7.09	-62.00	69.09	-55.69



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Signature Expiration Date of the License

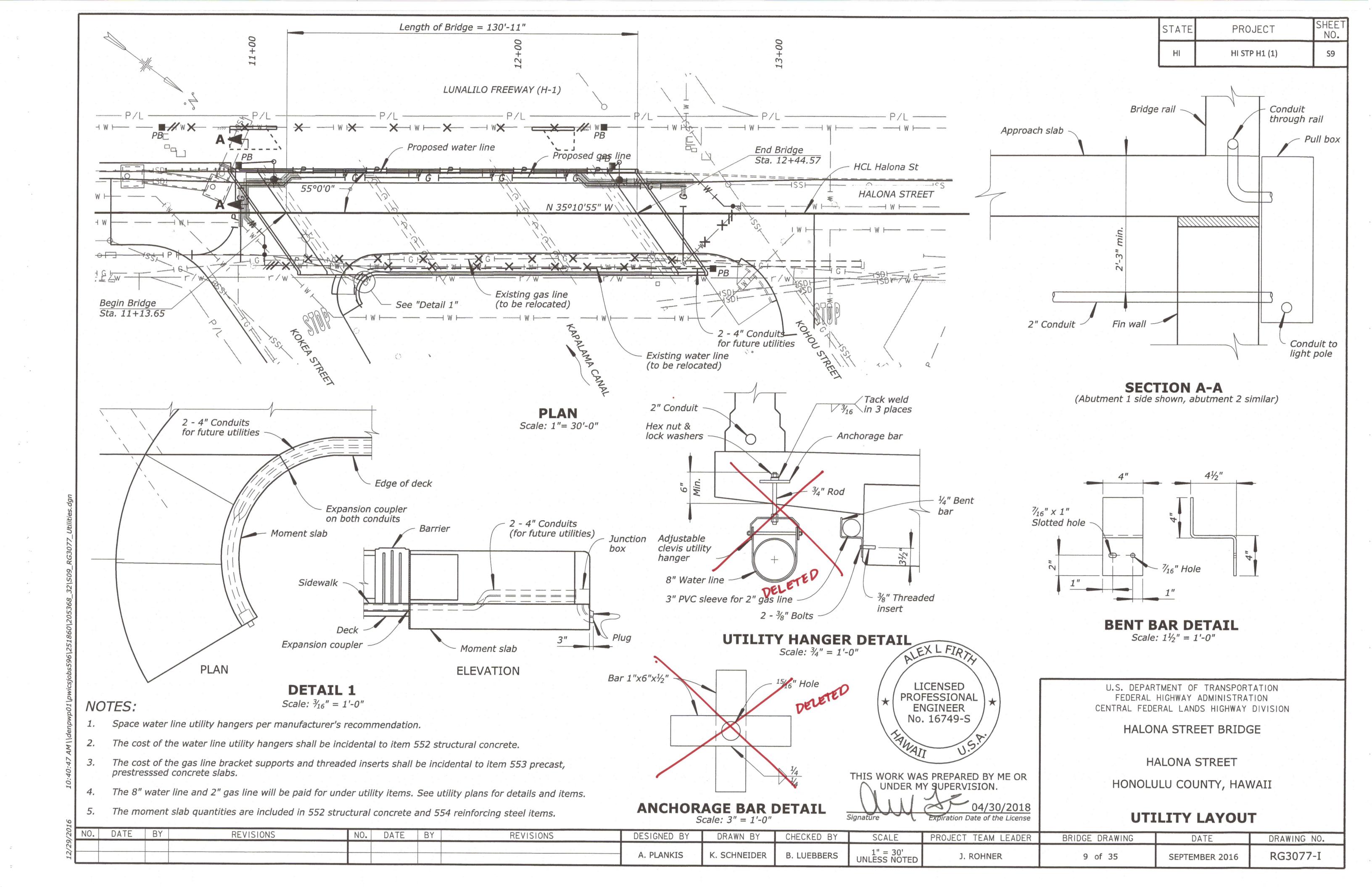
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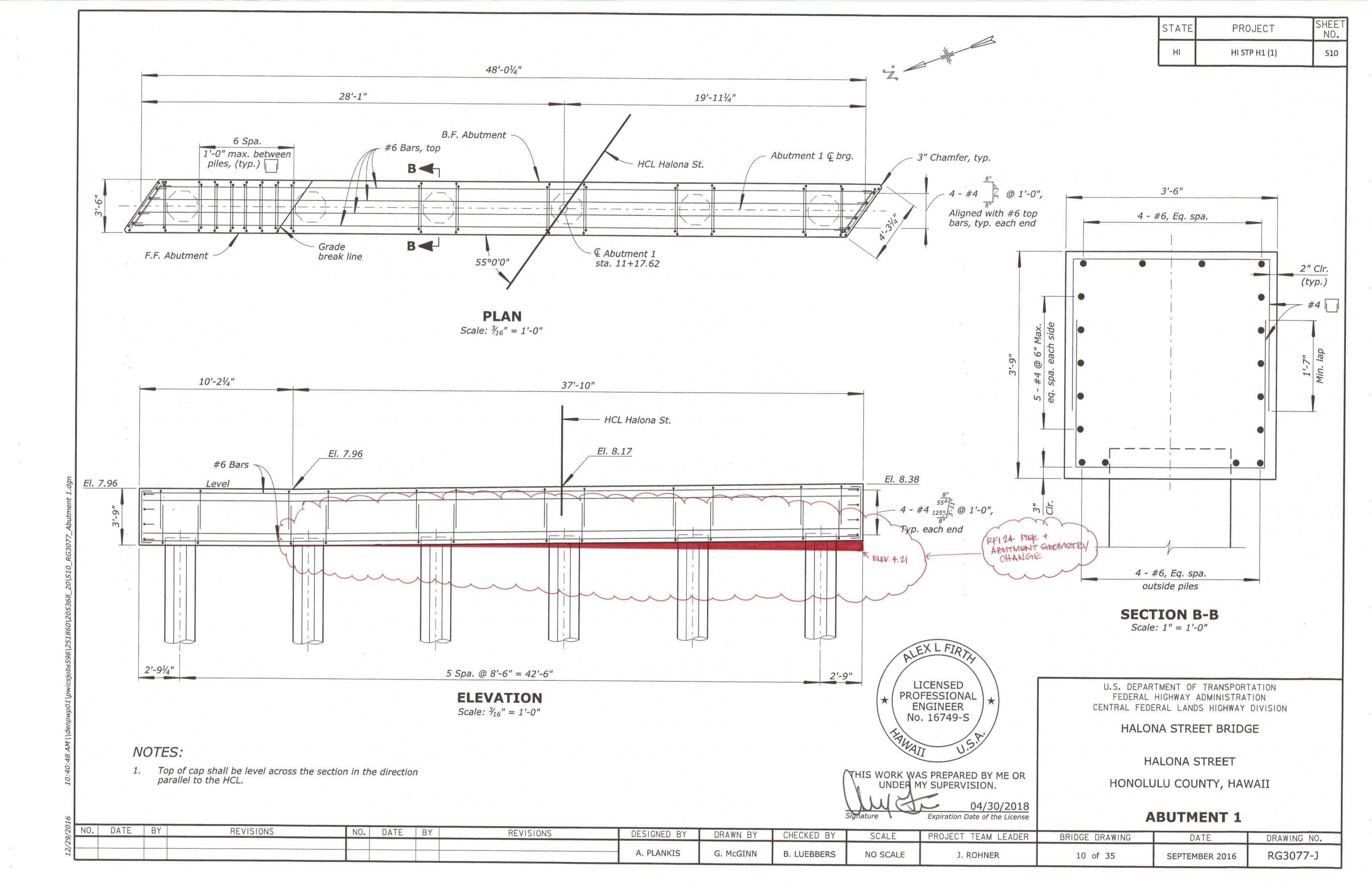
HALONA STREET BRIDGE

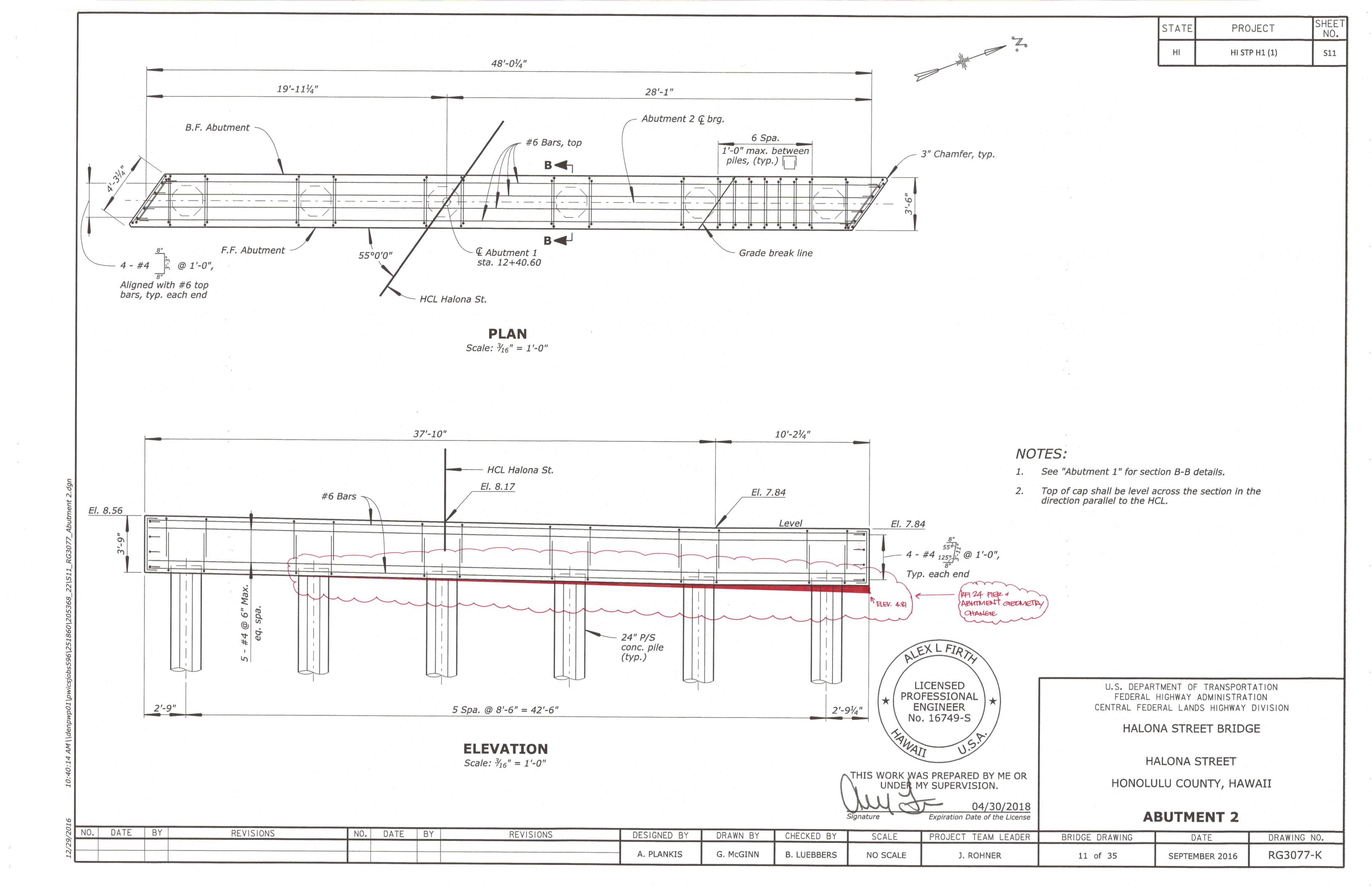
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HONOLULU COUNTY, HAWAII

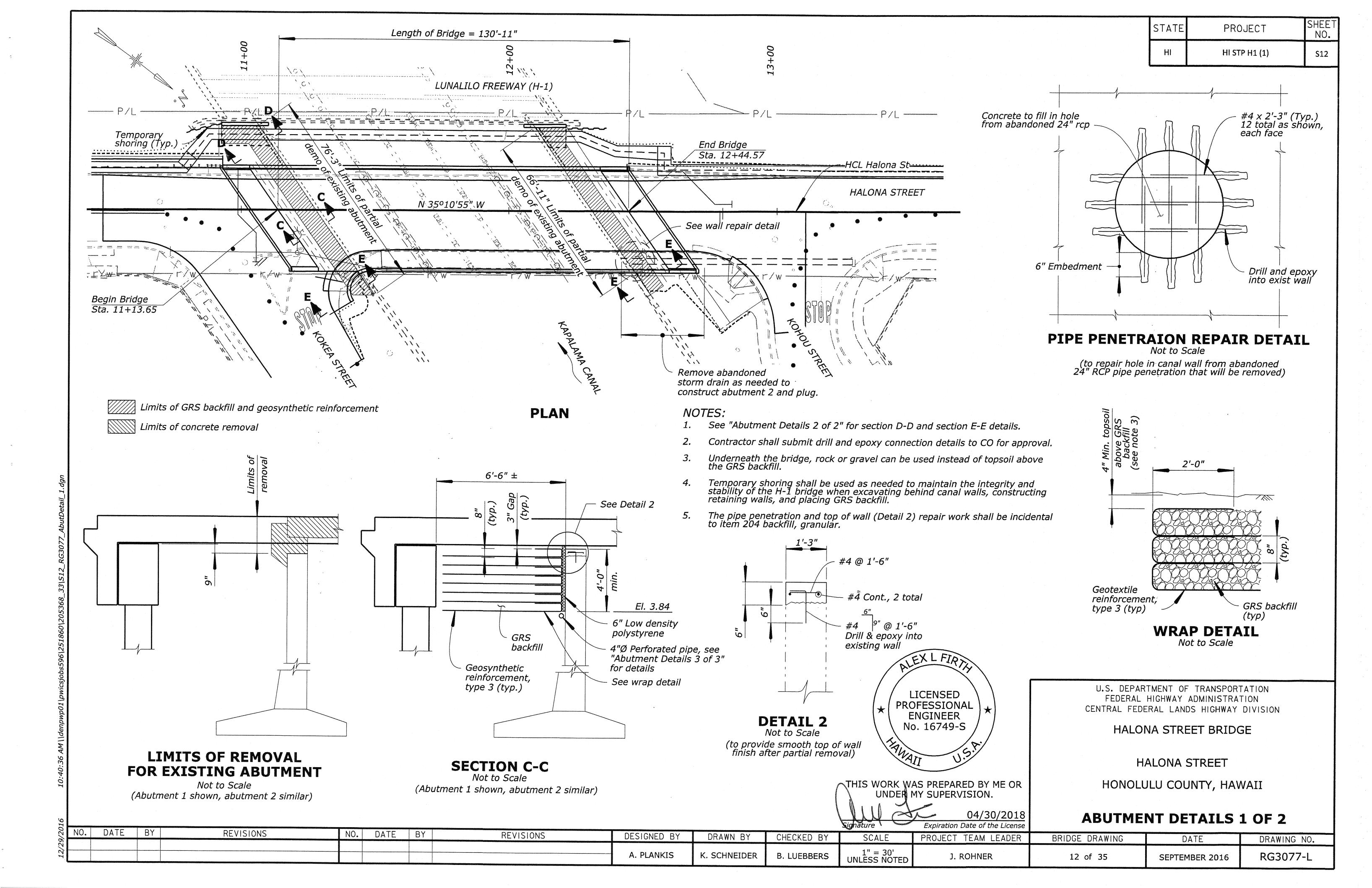
### FOUNDATION DETAILS

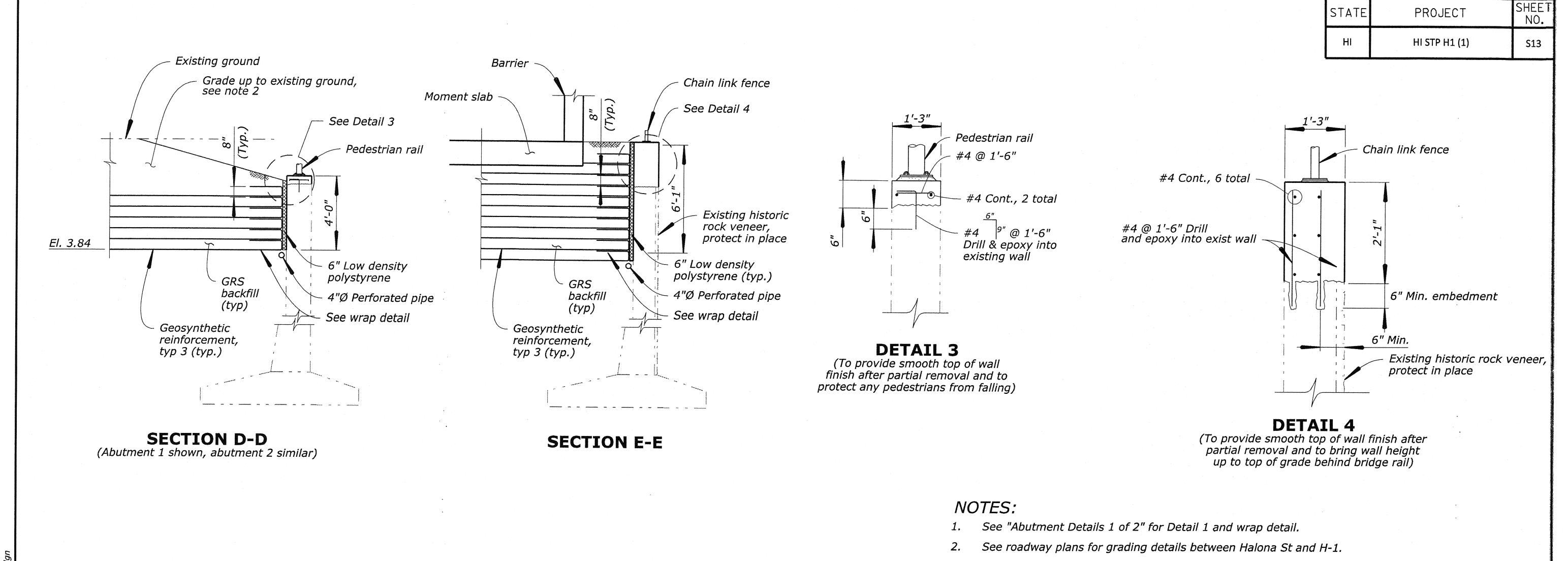
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12/2								A. PLANKIS	G. McGINN	B. LUEBBERS	NO SCALE	J. ROHNER	8 of 35	SEPTEMBER 2016	RG3077-H











- 3. See "Pedestrian Railing" and "Fence Chain Link" sheets for fence and railing connection details to the top of wall.
- 4. Canal walls upstream of the bridge have a historic rock veneer that must be protected in place. If veneer is damaged by the Contractor during construction, it must be repaired or replaced to its original state at the Contractor's expense.
- 5. Remove portions of the existing abutment and canal walls in accordance with Section 203 of the SCR's. Construct concrete sections on top of the existing abutment and canal walls in accordance with Section 258 of the SCR's.
- 6. Detail 3 work shall be incidental to item 646, pedestrian rail.
- 7. Detail 4 work shall be incidental to item 619, fence chain link.

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U.S. DEPARTMENT OF TRANSPORTATION

HALONA STREET BRIDGE

HALONA STREET
HONOLULU COUNTY, HAWAII

**ABUTMENT DETAILS 2 OF 2** 

NO. DATE BY	REVISIONS NO. DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
12//2				A. PLANKIS	K. SCHNEIDER	B. LUEBBERS	NO SCALE	J. ROHNER	13 of 35	SEPTEMBER 2016	RG3077-M

4"Ø Perforated pipe

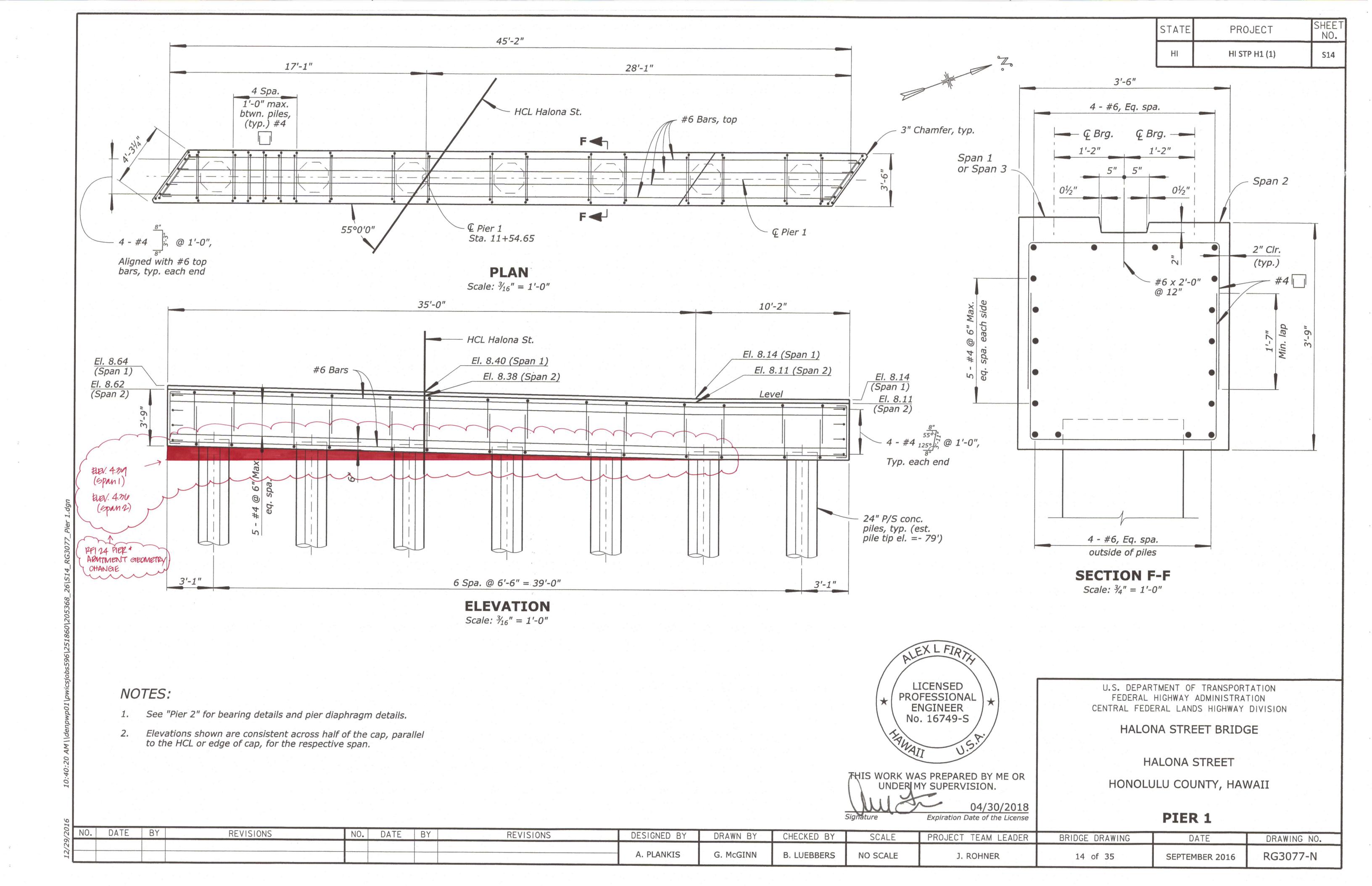
Existing historic

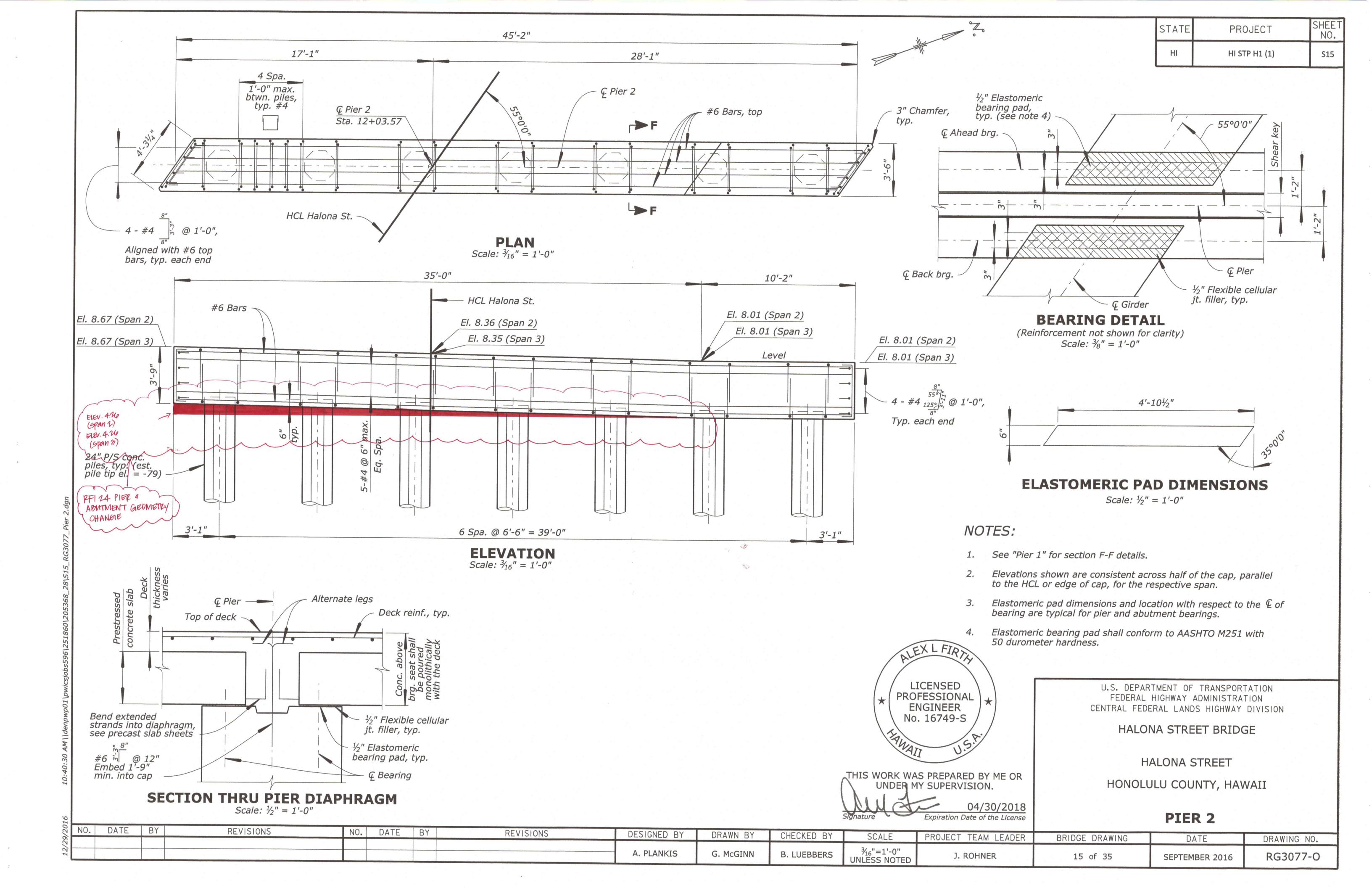
protect in place

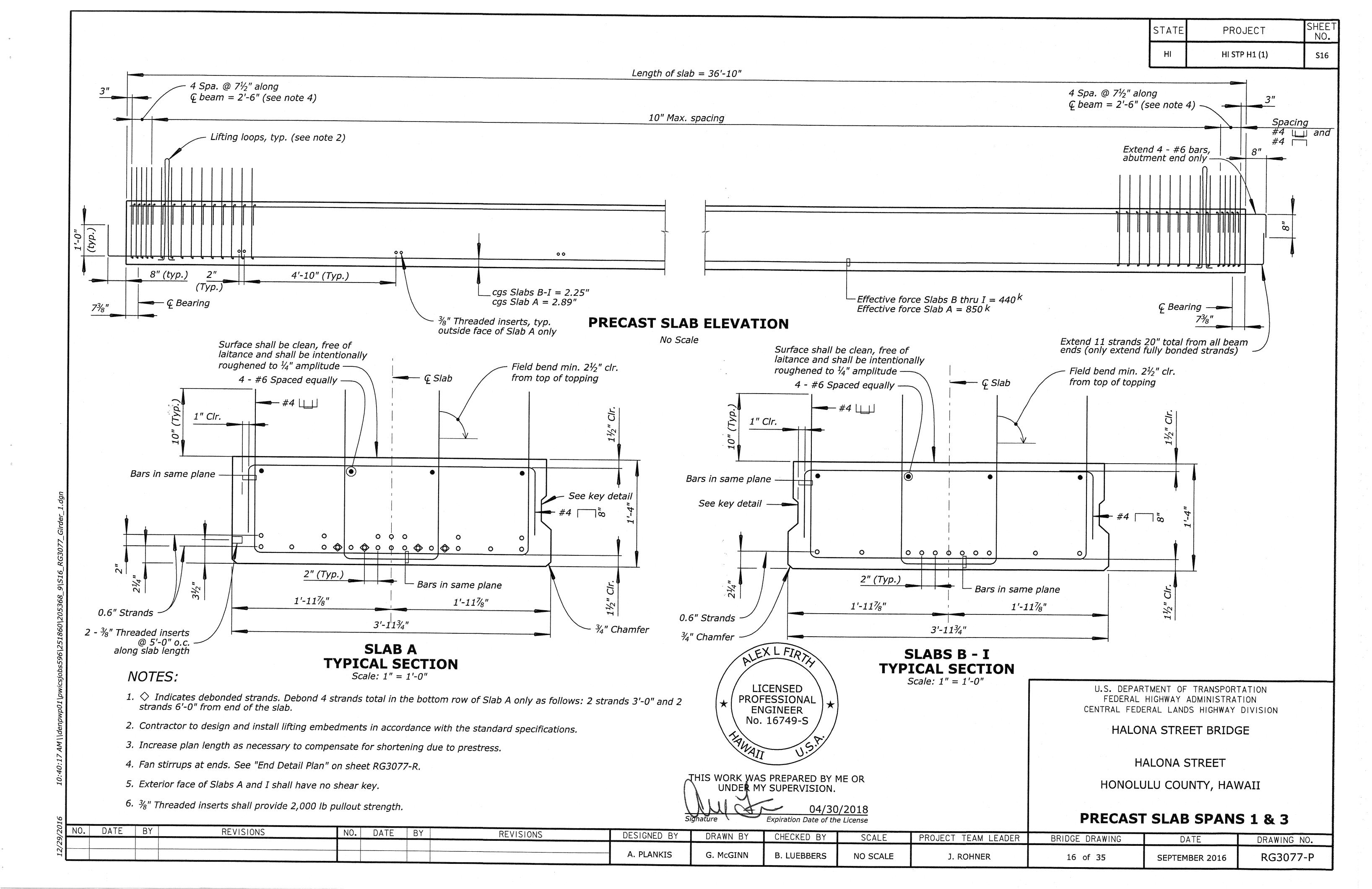
rock veneer,

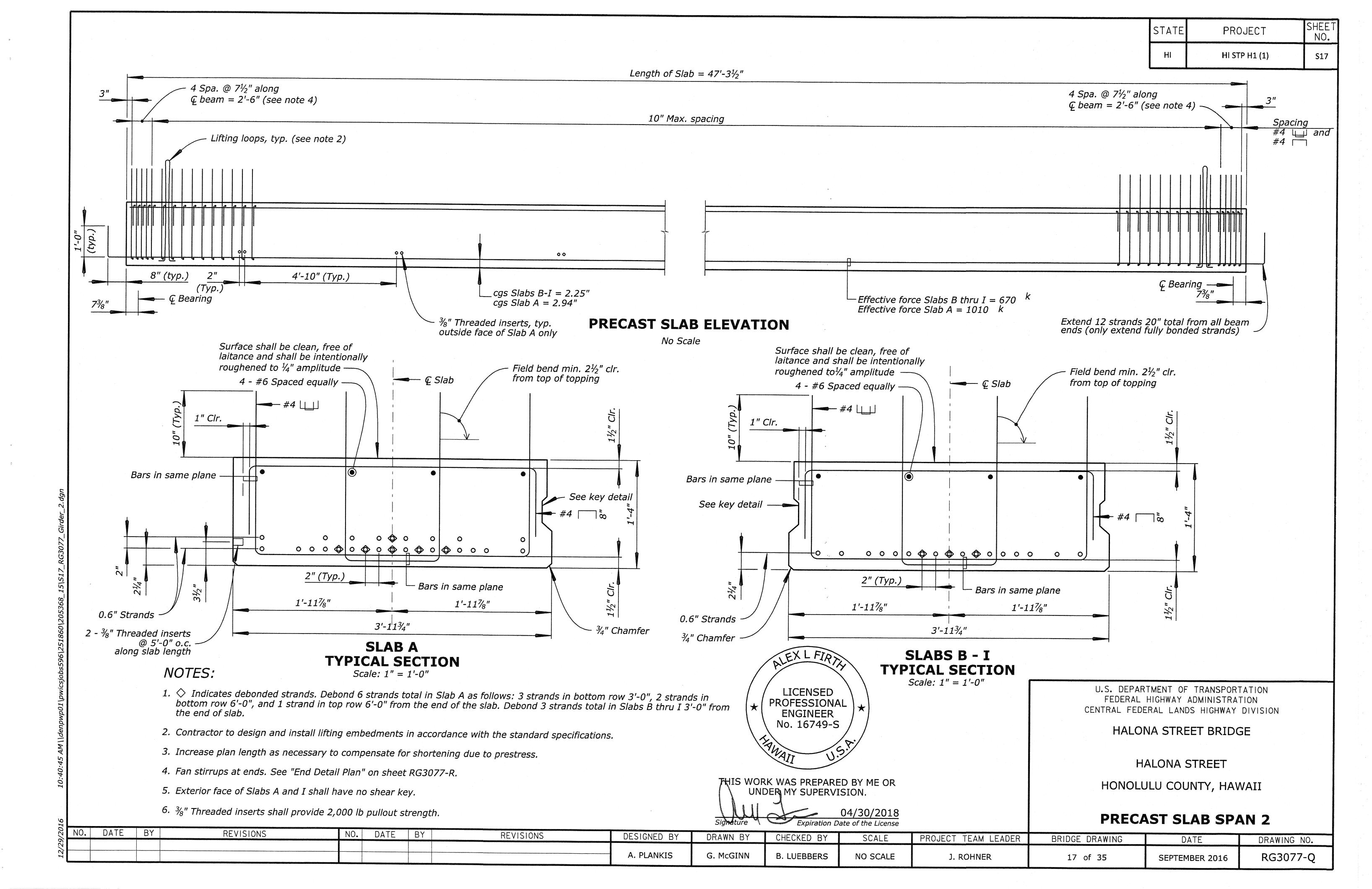
DRAINAGE DETAIL

(Fence not shown for clarity)









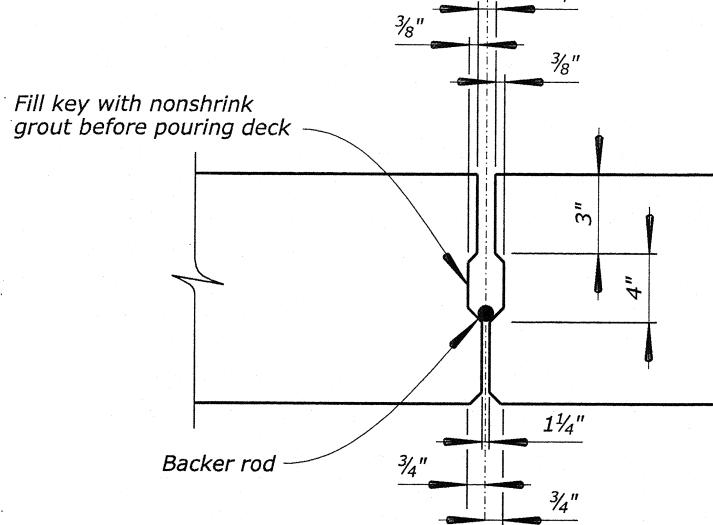
Estimated slab camber before placement of deck, sidewalk and rail concrete (see

camber & fillet table),

# CAMBER & FILLET TABLE Girder Est Camber Fi

Span	Girder	Est. Camber	Fillet
1	Α	2"	13/8"
<u>.</u>	B thru I	3/4"	1 <sup>3</sup> / <sub>8</sub> "
2	A	3"	13/4"
: <b>4-</b>	B thru I	13/4"	13/4"
2	A	2"	1 <sup>5</sup> / <sub>8</sub> "
3	B thru I	3/4"	15/8"

STATE PROJECT SHEET NO.
HI HISTPH1(1) S18



# TRANSVERSE SLAB CONSTRUCTION JOINT

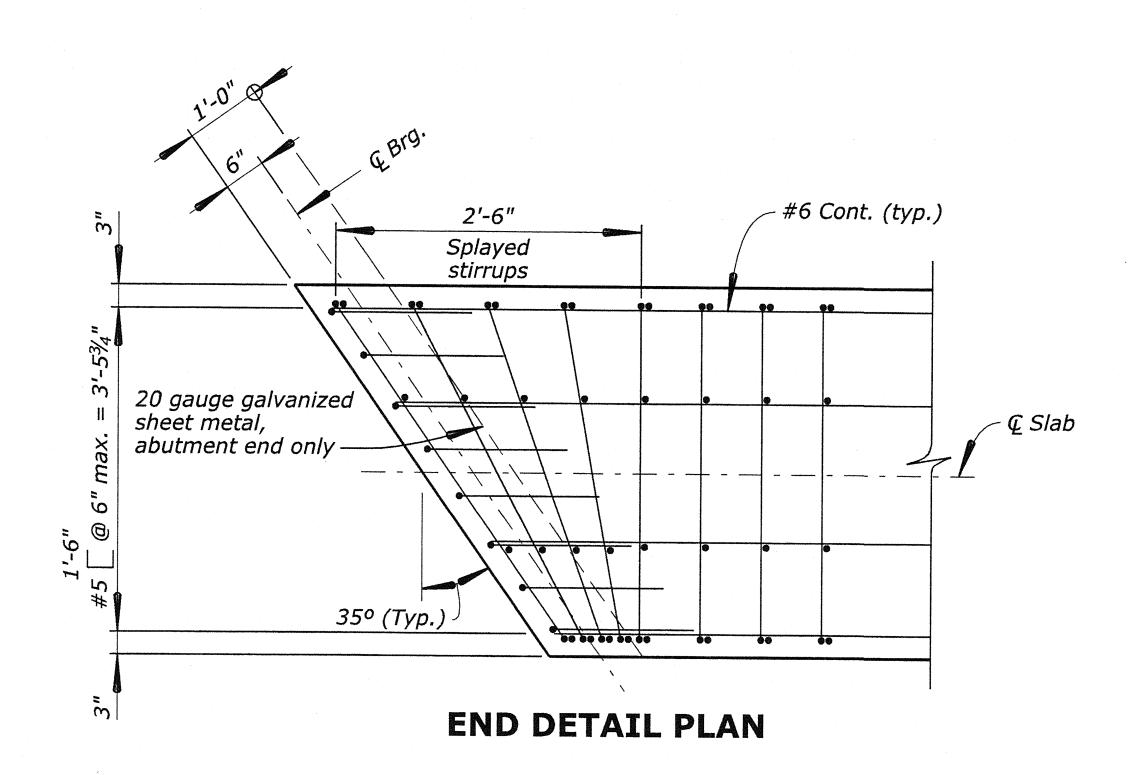
## REQUIRED ACTIONS:

Slab position before placement of deck, sidewalk, and rail concrete

Final Position of girder

- 1. Measure slab camber prior to setting deck forms. If it exceeds the estimated slab camber (see camber table) by more than 1", the fillet will have to be increased by raising profile grade as directed by the CO.
- 2. Set the deck forms and camber the deck machine screed rails to offset the slab deflections due to deck, sidewalk, and rail placement.

### **DECK FORM SETTING DIAGRAM**



### **Deflection Equation**

Longitudinal slab reinforcing continuous through joint (typ.)

$$\triangle = 0.25 - \frac{X^2}{1270}$$
 Spans 1 & 3  $\triangle = 0.75 - \frac{X^2}{707}$  Span 2

Where  $\triangle$  = Deflection, in inches, of girder at any point caused by the weight of deck. and X = Distance, in feet, measured from midspan (See diagram).

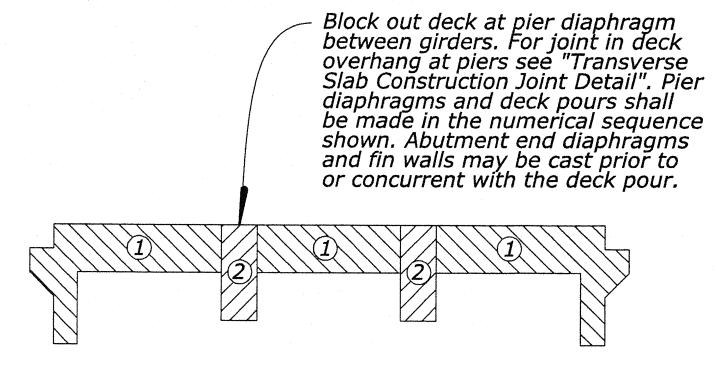
Note:  $\triangle$  max =  $\frac{1}{4}$ " at X=0 (midspan)  $\triangle$  min = 0 at 17'-9 $\frac{1}{2}$ " ( $\mathbb{Q}$  bearing)

Spans 1 & 3

 $\triangle$  max = $\frac{3}{4}$ " at X=0 (midspan)  $\triangle$  min = 0 at X= 23'-0 $\frac{1}{2}$ " ( $\mathcal{C}$  bearing) Span 2

## **KEY DETAIL**

Note: Maintain  $1\frac{1}{4}$ " gap at bottom between slabs, including where beam seat grade changes



Abut. 1 End Diaphragm

Pier 1 Diaphragm

n D

Pier 2 Abut. 2 Diaphragm End Diaphragm

## **DECK POURING DIAGRAM**

\*

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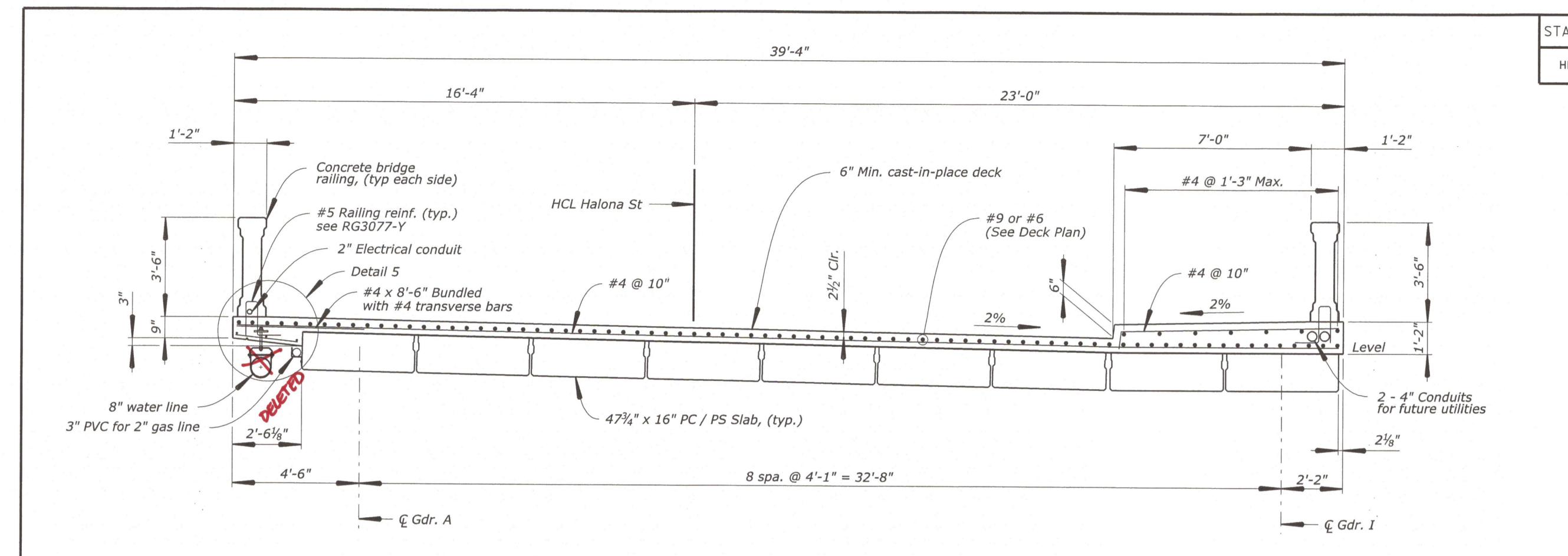
04/30/2018 ignature Expiration Date of the License U.S. DEPARTMENT OF TRANSPORTATION
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HALONA STREET BRIDGE

HALONA STREET
HONOLULU COUNTY, HAWAII

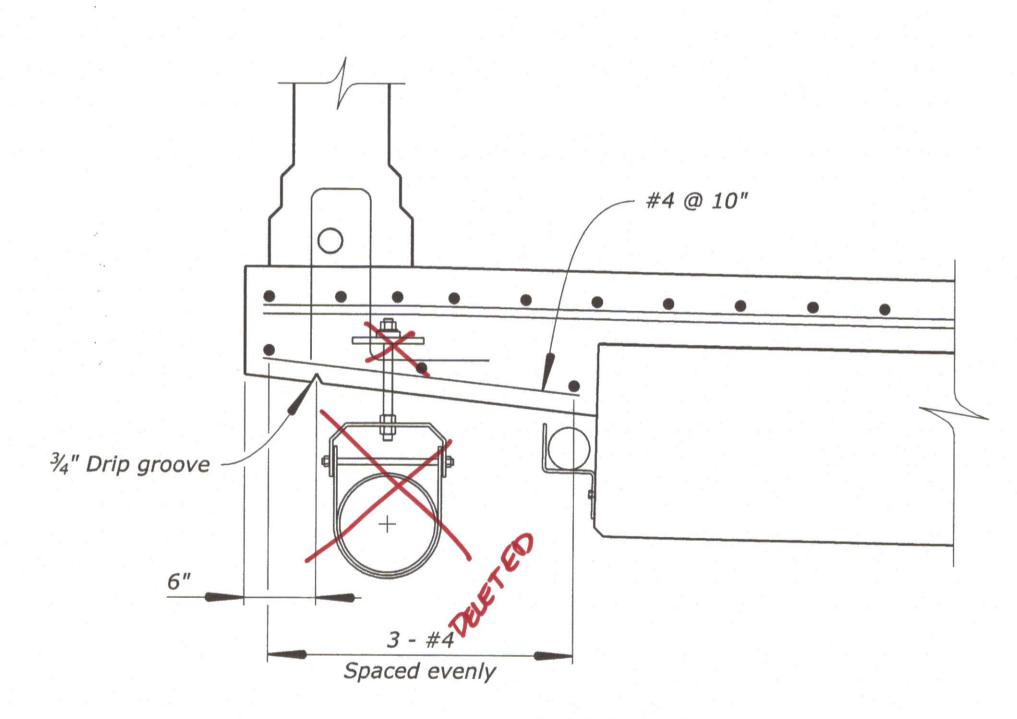
PRECAST SLAB DETAILS

NO. DATE BY	REVISIONS	NO DATE DV									
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175				A. PLANKIS	G. McGINN	B. LUEBBERS	NO SCALE	J. ROHNER	18 of 35	SEPTEMBER 2016	RG3077-R



## **TYPICAL SECTION**

(Looking ahead stationing)



## **DETAIL 5**

Scale:  $\frac{3}{4}$ " = 1'-0"



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SHEE NO.

**S19** 

PROJECT

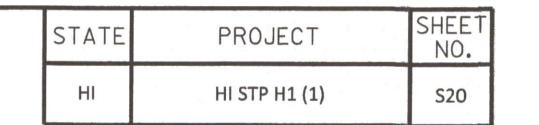
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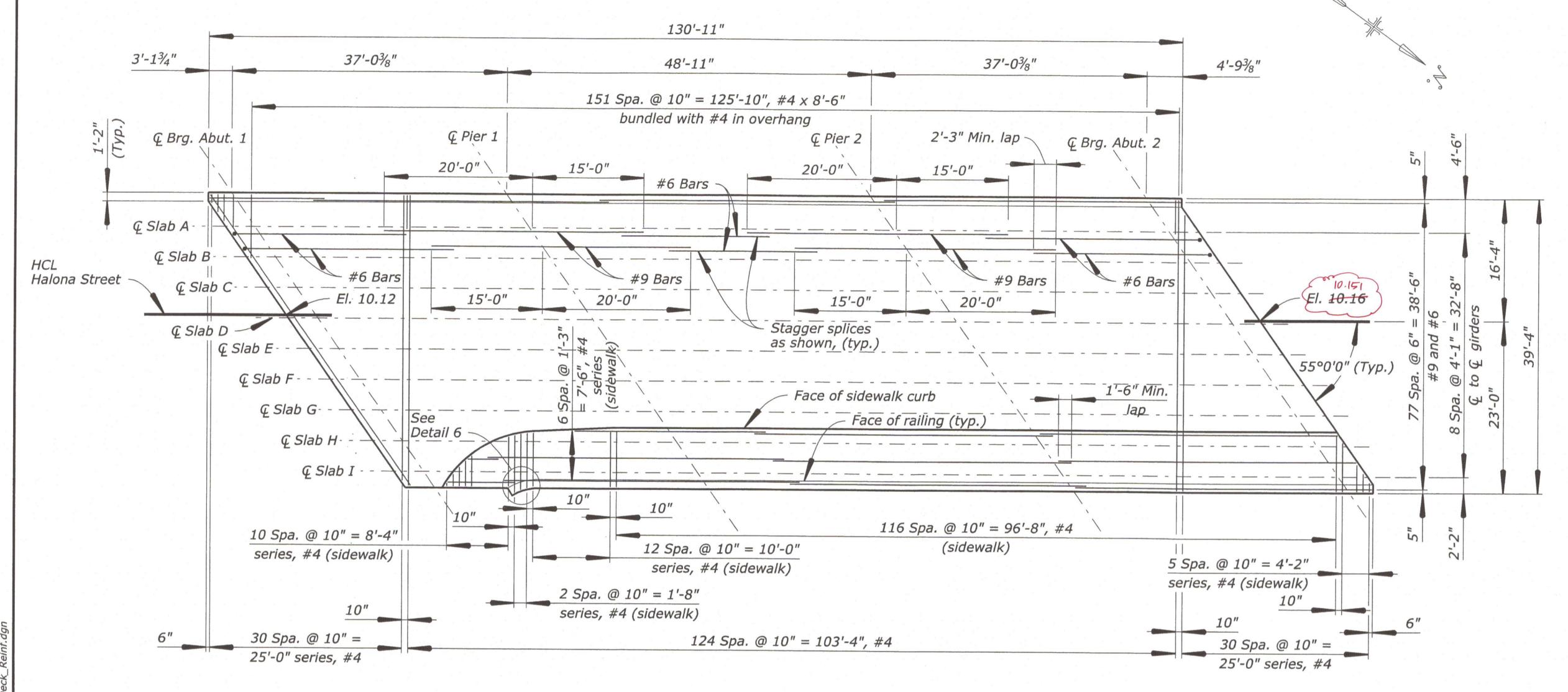
HALONA STREET BRIDGE

HALONA STREET
HONOLULU COUNTY, HAWAII

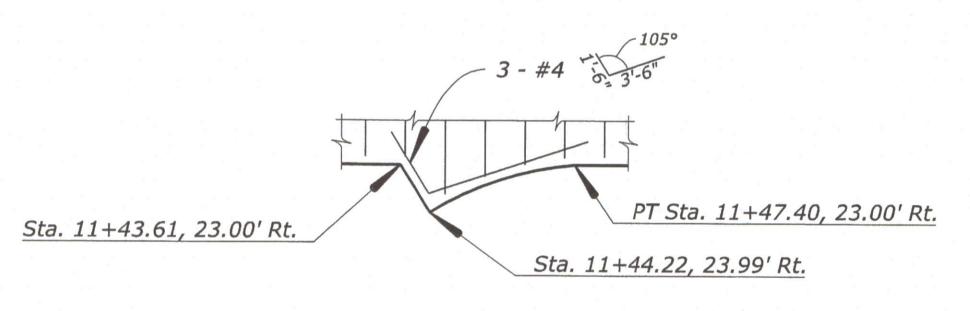
SUPERSTRUCTURE SECTION

NO. DATE BY REVISIONS	NO. DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
12/2								J. ROHNER	19 of 35	SEPTEMBER 2016	





## **DECK AND FRAMING PLAN**



DETAIL 6
No Scale

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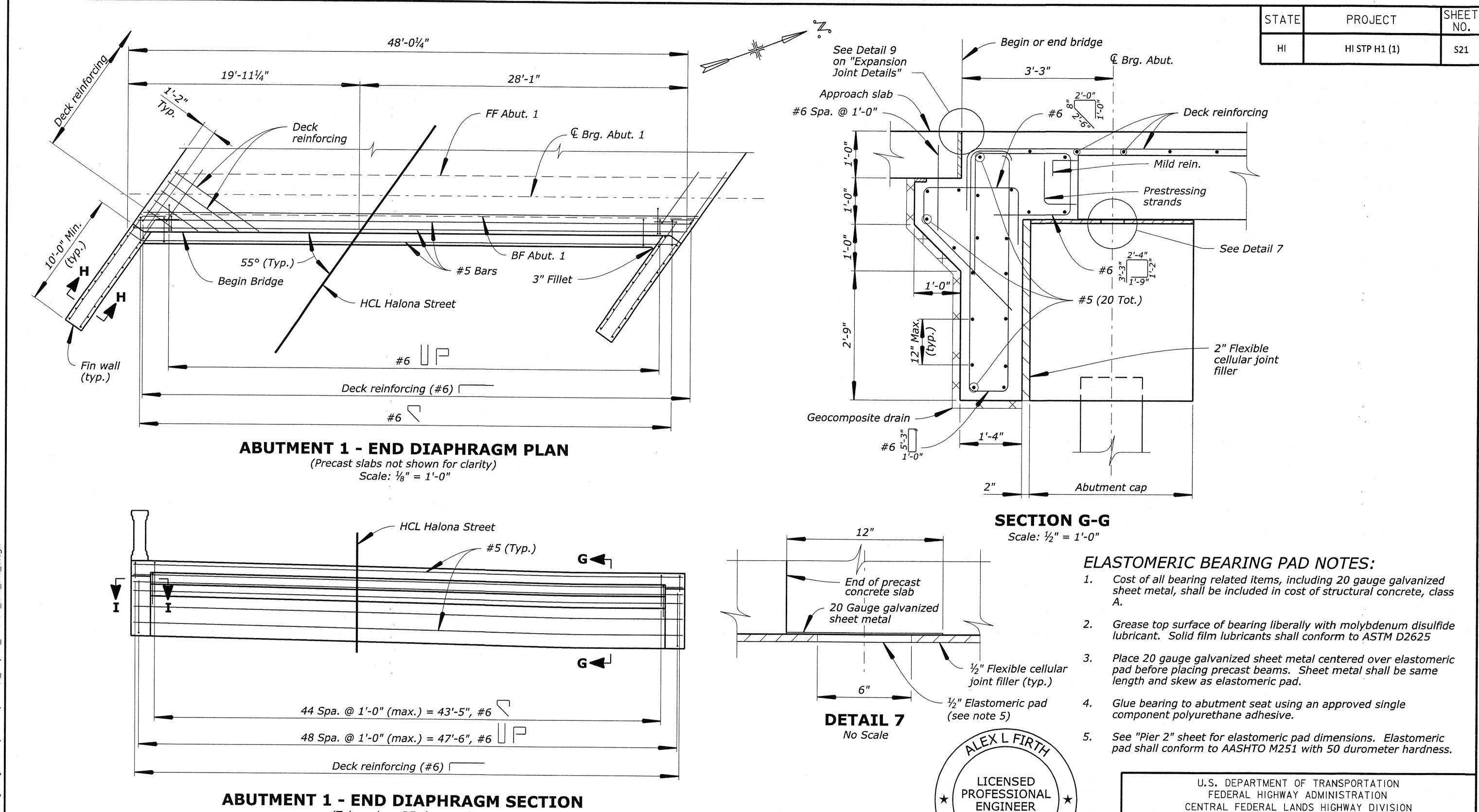
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HALONA STREET BRIDGE

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## **DECK AND FRAMING PLAN**

NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
12/2								and the second		B. LUEBBERS			20 of 35	SEPTEMBER 2016	



NOTES:

- 1. Fin wall reinforcing not shown for clarity on elevation views.
- See "End Diaphragm Details 2 of 3" for Section H-H details and fin wall elevation view.

(Taken along BF abutment)

Scale:  $\frac{1}{8}$ " = 1'-0"

See "End Diaphragm Details 3 of 3" for Section I-I.

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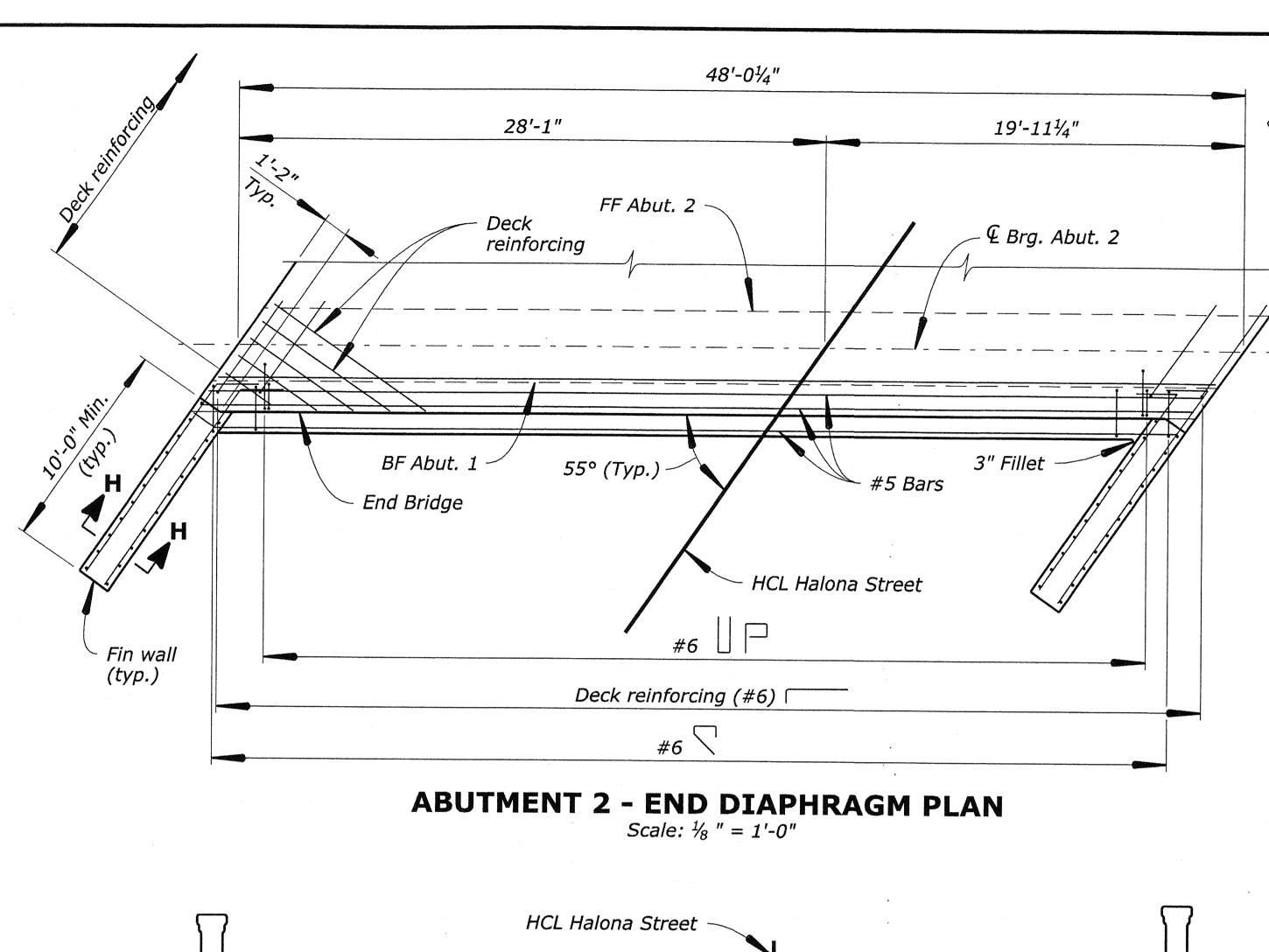
HALONA STREET BRIDGE

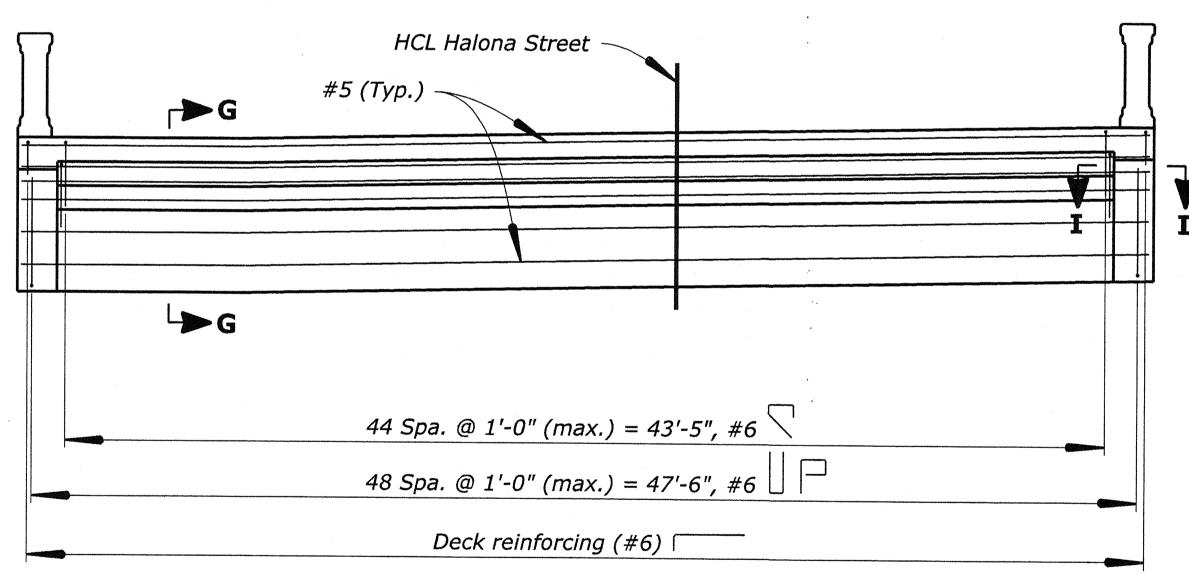
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HONOLULU COUNTY, HAWAII

END DIAPHRAGM DETAILS 1 OF 3

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12/2				A. PLANKIS	G. McGINN	B. LUEBBERS	$\frac{1}{8}$ " = 1'-0" UNLESS NOTED	J. ROHNER	21 of 35	SEPTEMBER 2016	RG3077-U



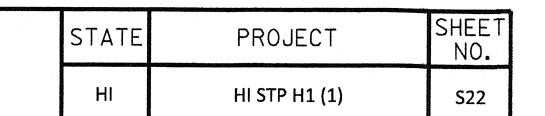


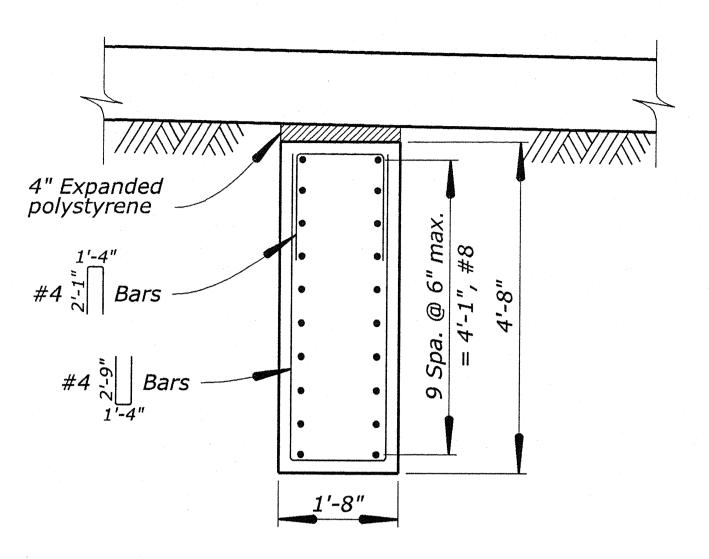
## **ABUTMENT 2 - END DIAPHRAGM SECTION**

(Taken along BF abutment) Scale:  $\frac{1}{8}$ " = 1'-0"

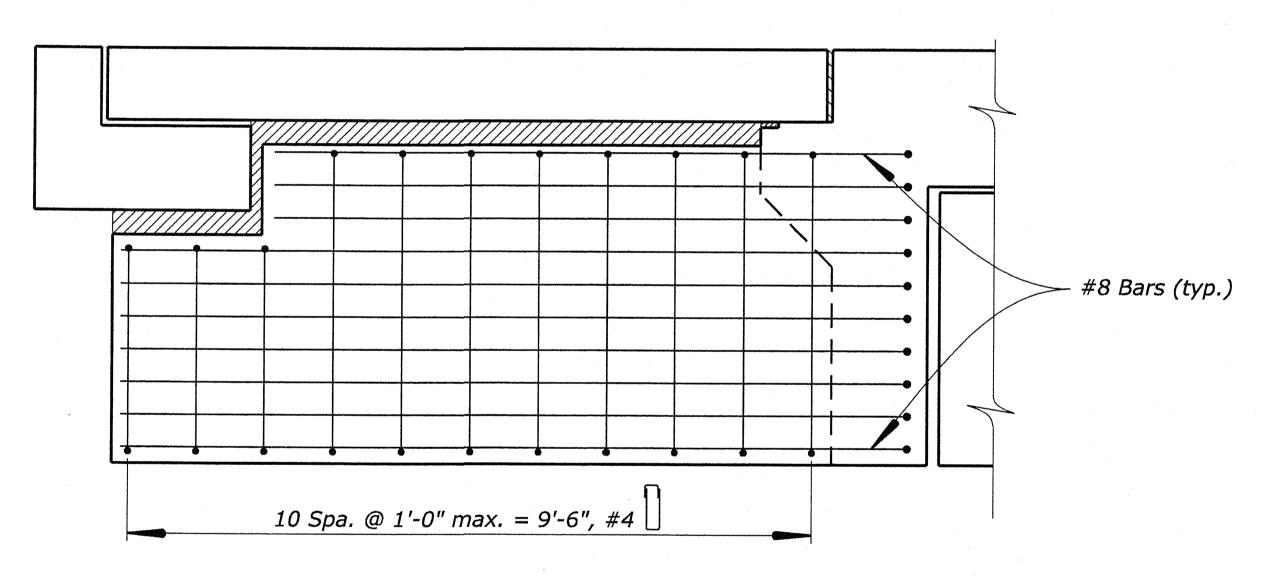
### NOTES:

- 1. Fin wall reinforcing not shown for clarity on elevation views.
- See "End Diaphragm Details 1 of 3" for Section G-G details and fin wall elevation view.
- See "End Diaphragm Details 3 of 3" for Section I-I.





# SECTION H-H (FINWALL SECTION) Scale: $\frac{3}{8}$ " = 1'-0"



### FIN WALL ELEVATION

Scale:  $\frac{3}{8}$ " = 1'-0"



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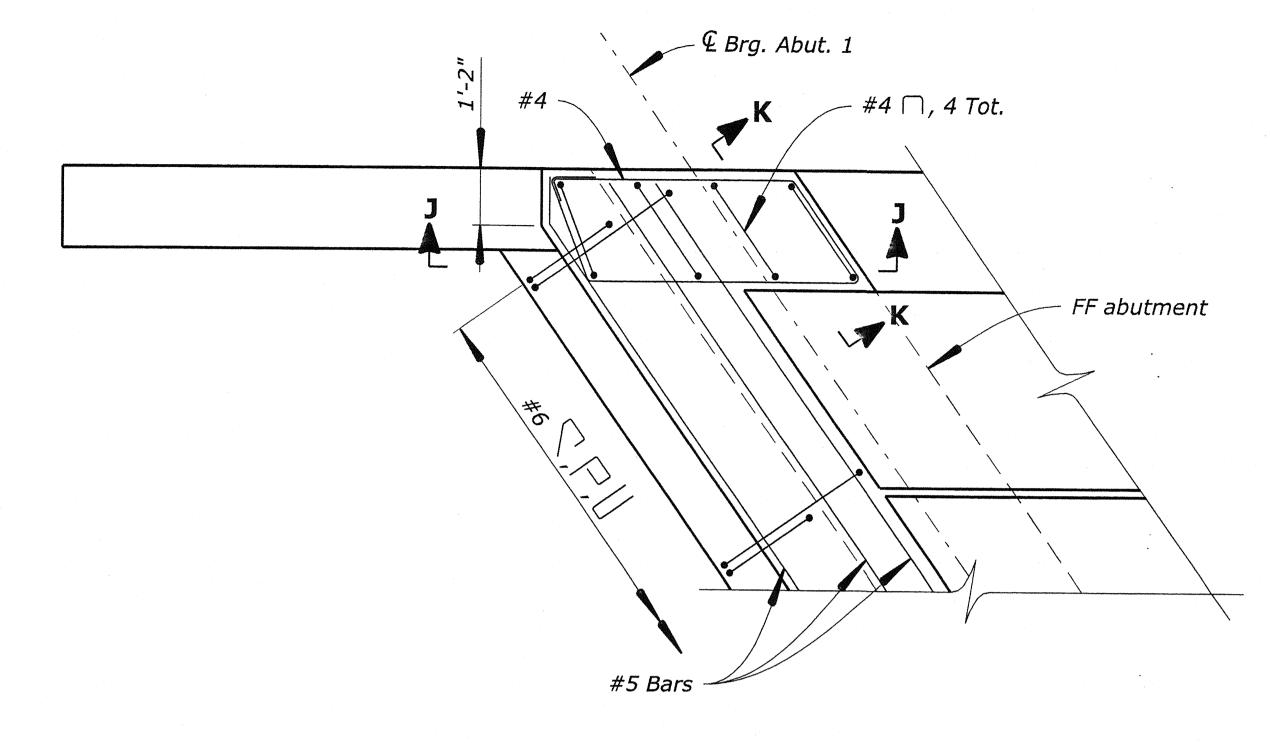
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HALONA STREET

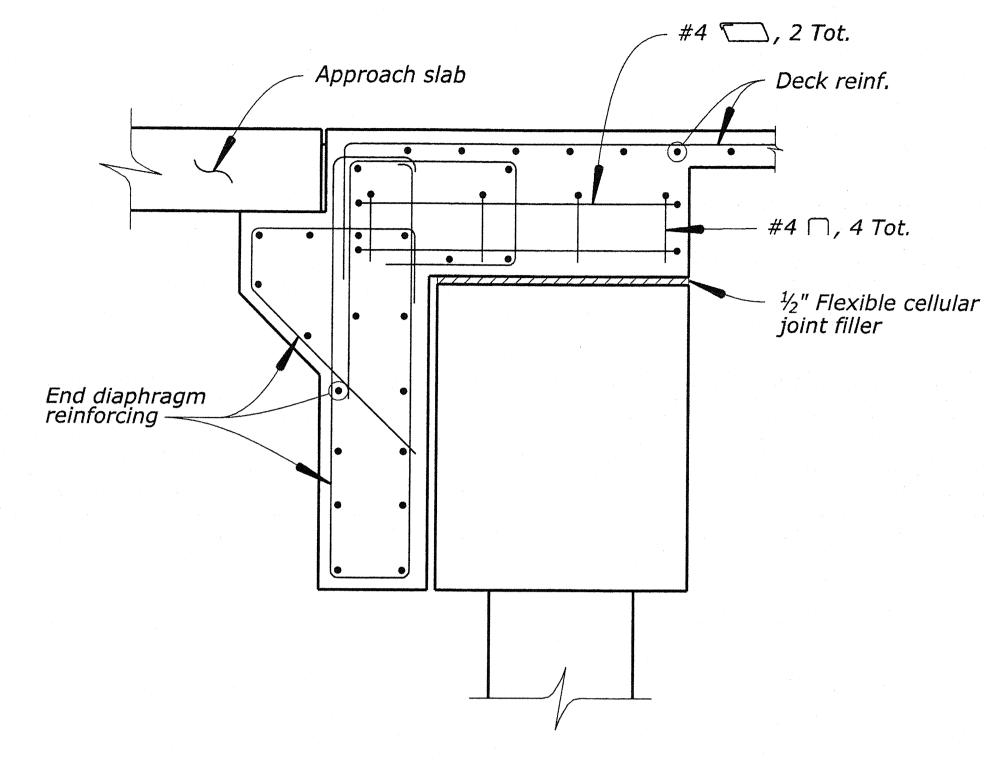
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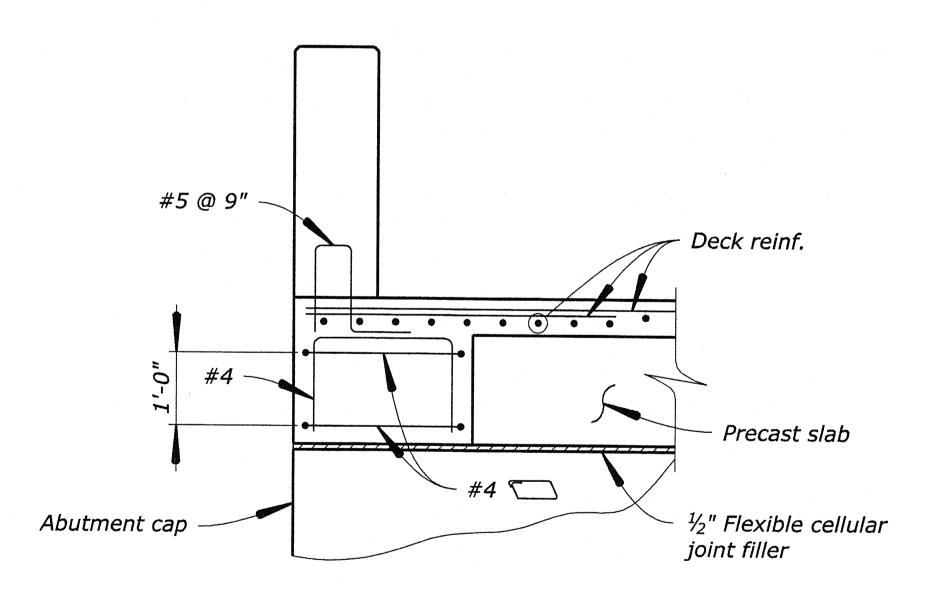
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12/2					A. PLANKIS	G. McGINN	B. LUEBBERS	$\frac{1}{8}$ " = 1'-0" UNLESS NOTED	J. ROHNER	22 of 35	SEPTEMBER 2016	RG3077-V



**SECTION I-I**Scale: \( \frac{1}{4}'' = 1' - 0'' \)



**SECTION J-J** *Scale:*  $\frac{3}{8}$ " = 1'-0"



**SECTION K-K**Scale: 3/8" = 1'-0"



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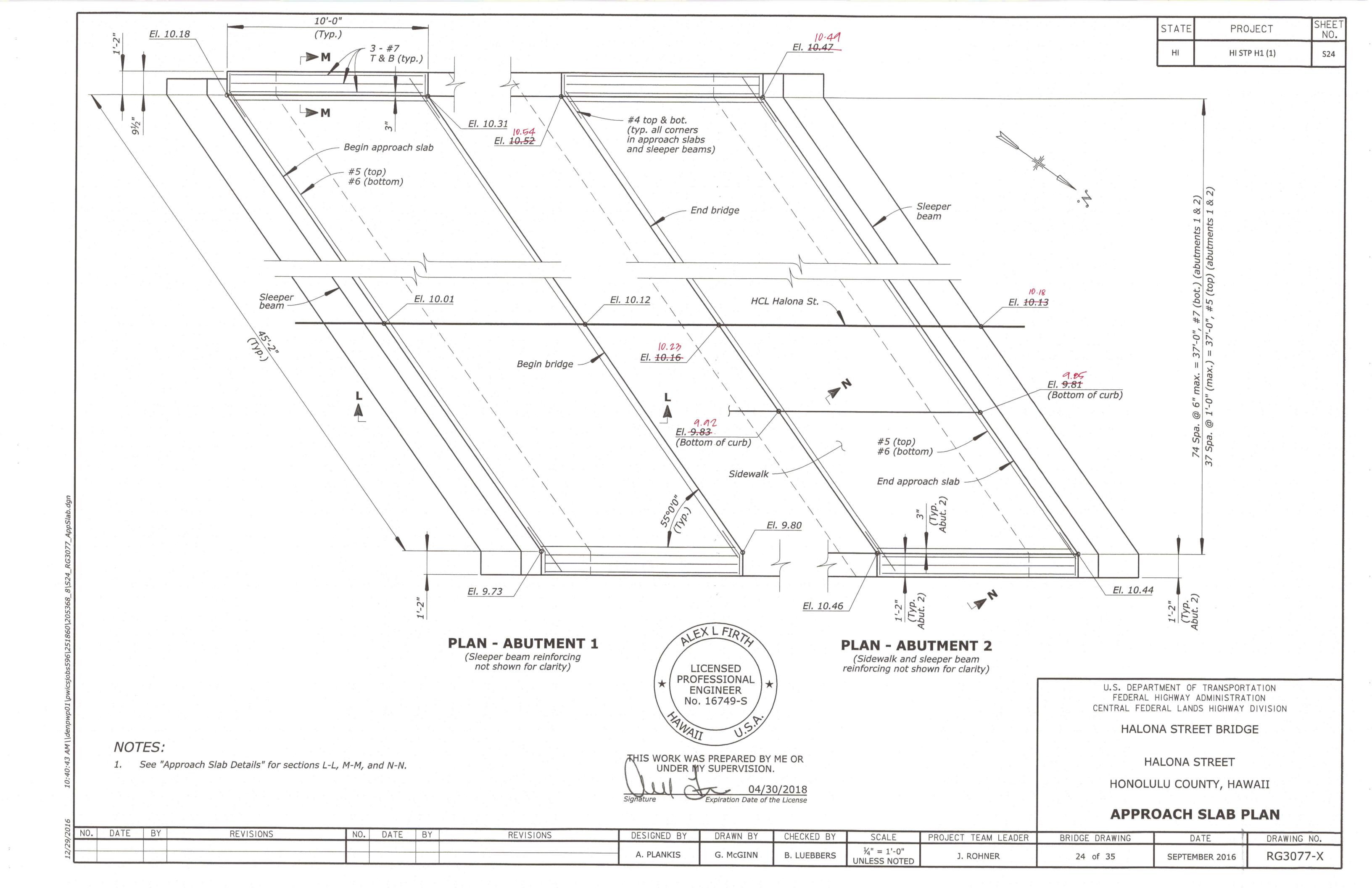
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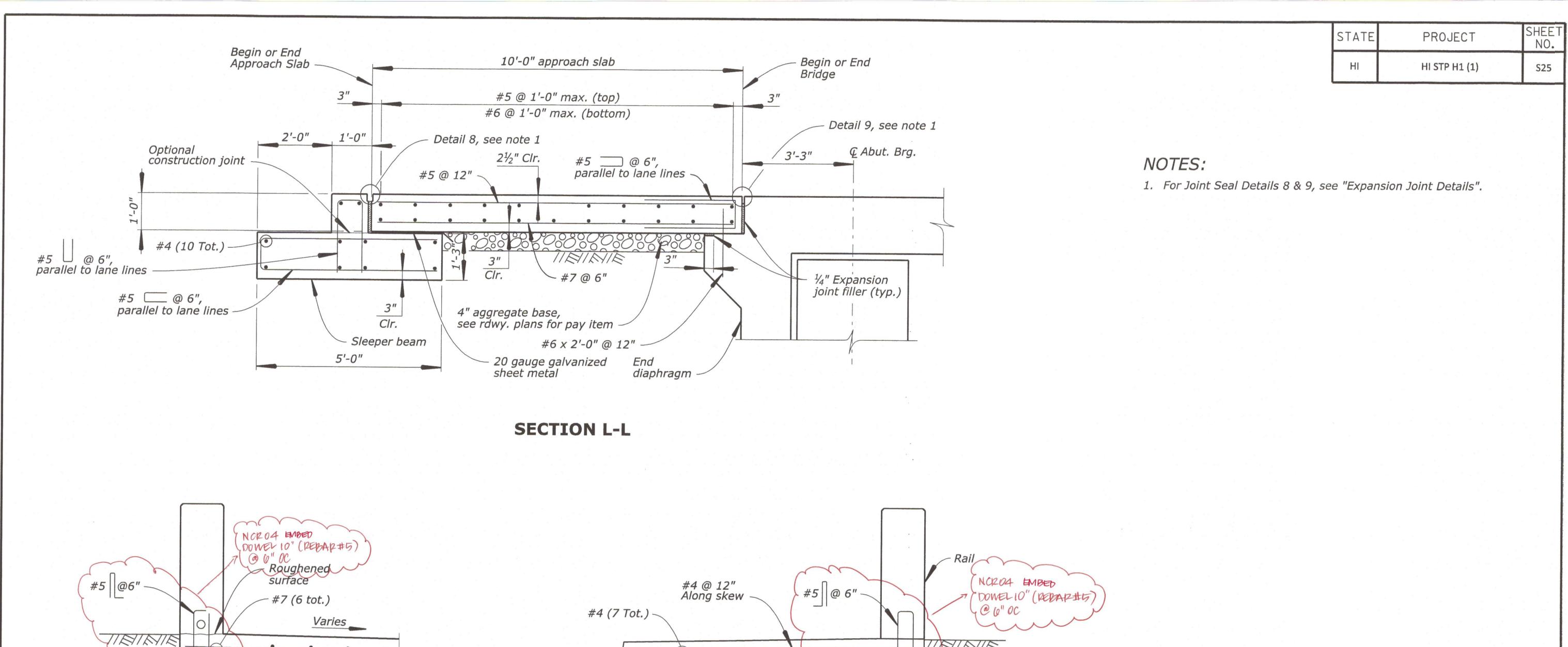
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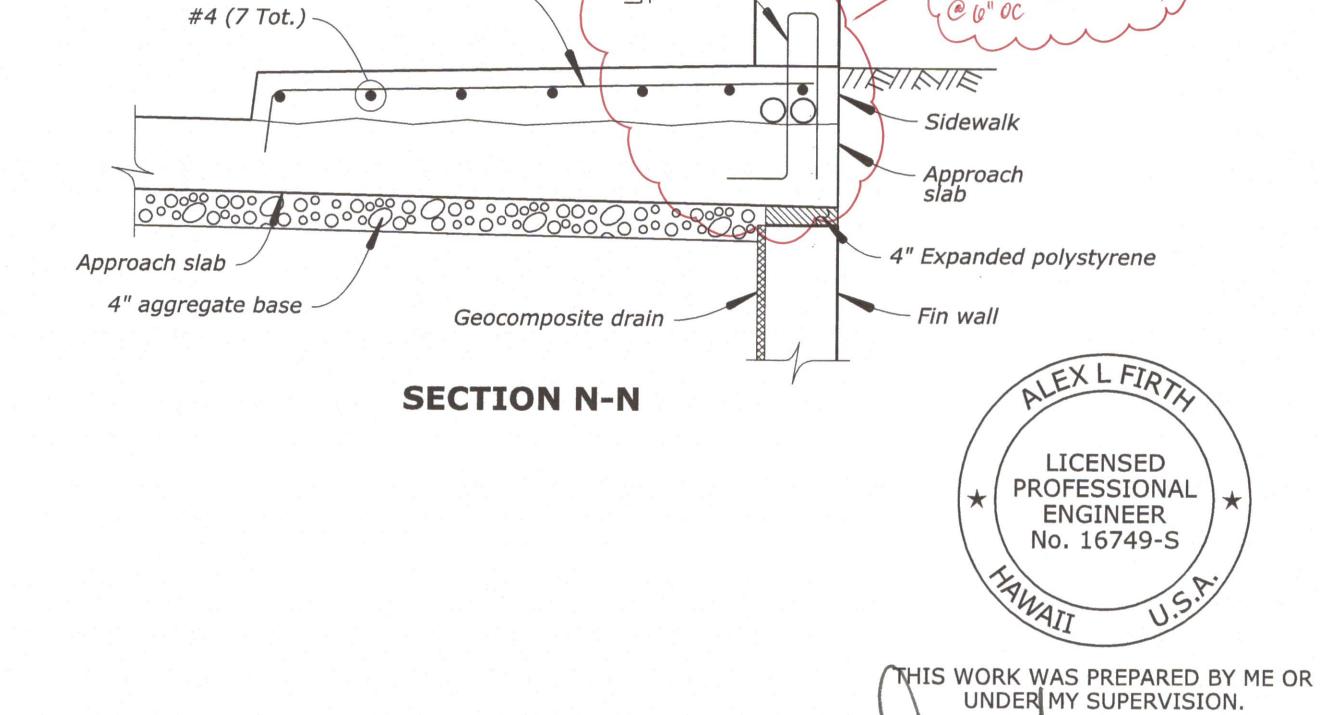
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## **END DIAPHRAGM DETAILS 3 OF 3**

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			A. PLANKIS	K. SCHNEIDER	B. LUEBBERS	$\frac{3}{8}$ " = 1'-0" UNLESS NOTED	J. ROHNER	23 of 35	SEPTEMBER 2016	RG3077-W







**SECTION M-M** 

Geocomposite drain

Approach slab

4" aggregate base

4" Expanded polystyrene

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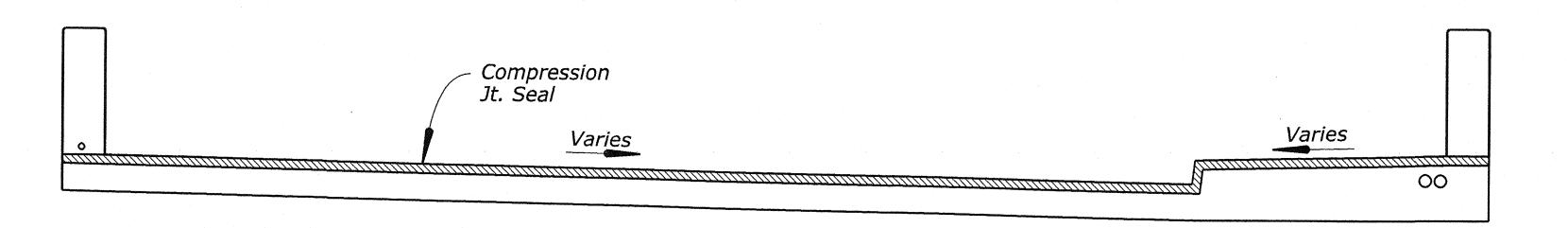
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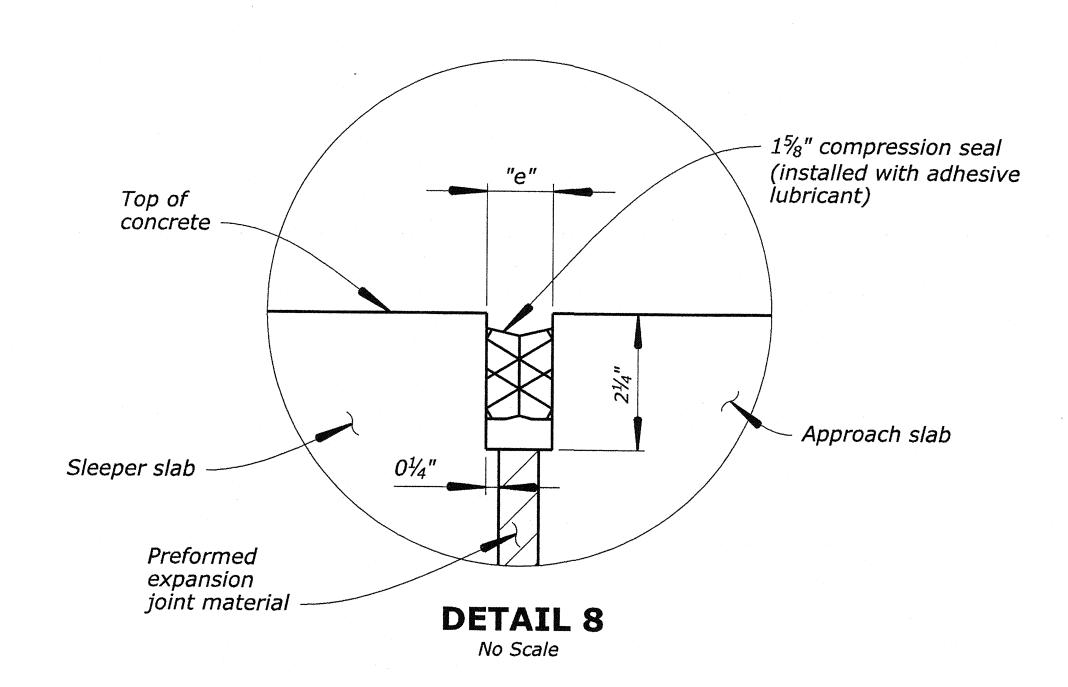
APPROACH SLAB DETAILS

29/20	10.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
12/													J. ROHNER	25 of 35	SEPTEMBER 2016	



### SECTION AT EXPANSION JOINT

(North joint shown, south joint similar but does not have sidewalk joint turns)



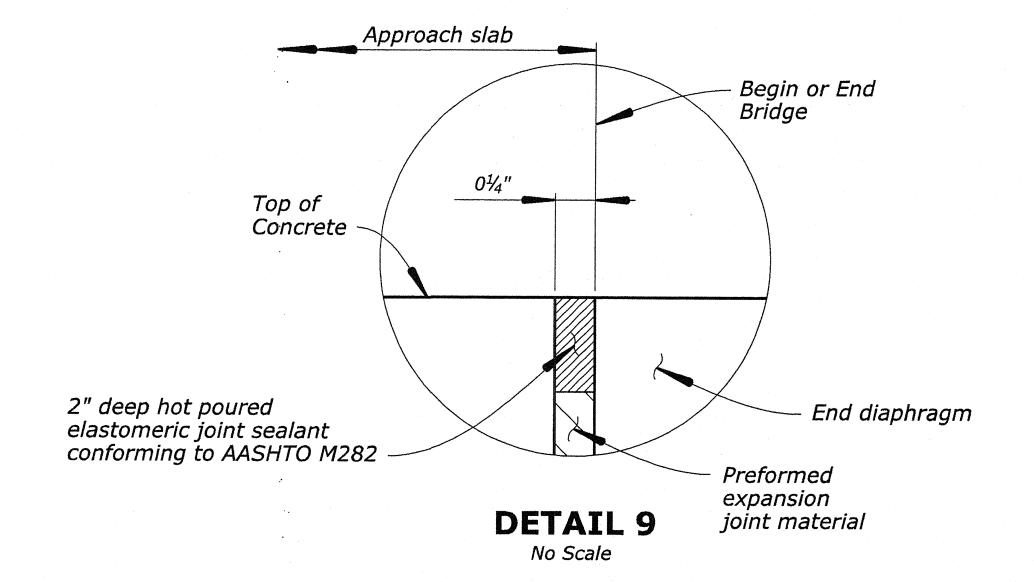
JOINT OP	FNING
Temperature (°f)	e (in.)
40	1.18
50 60	1.12
70	1.00
80 90	0.94
100	0.82

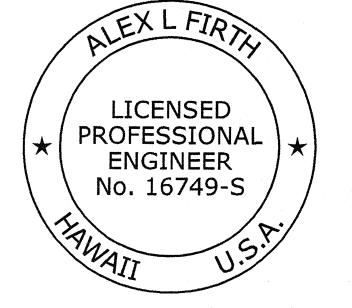
JOINT DAT	A
Joint Data	Abut. 1 & 2
Joint Type	15/8" Compression
Joint Size	15/8" x 17/8"
Movement Rating	0.60" min
"e" @ Mean Temp. of 70°	1.00"
Opening Adjustment per 10°F	0.06"

### COMPRESSION SEAL NOTES:

- Joint seals shall be one of the following or an approved equal:

   A. D.S. Brown Co. CV-1625
   B. Watson Bowman Acme Corp. WA-162
- 2. Joint seals other than those listed above may be submitted for approval provided they are similar to that shown and can demonstrate by test the specified joint design movements.
- 3. Seals shall conform to the requirements of AASHTO M297 for Preformed Polychloroprene Elastomeric Joint Seals.
- 4. Submit seal certification of performance, installation width and height information, detailed set of manufacturer's instructions for seal installation, and method of assuring proper joint width at time of installation to CO prior to end diaphragm and abutment top slab construction.
- 5. Joint upturns and downturns shall follow manufacturer's recommendations.





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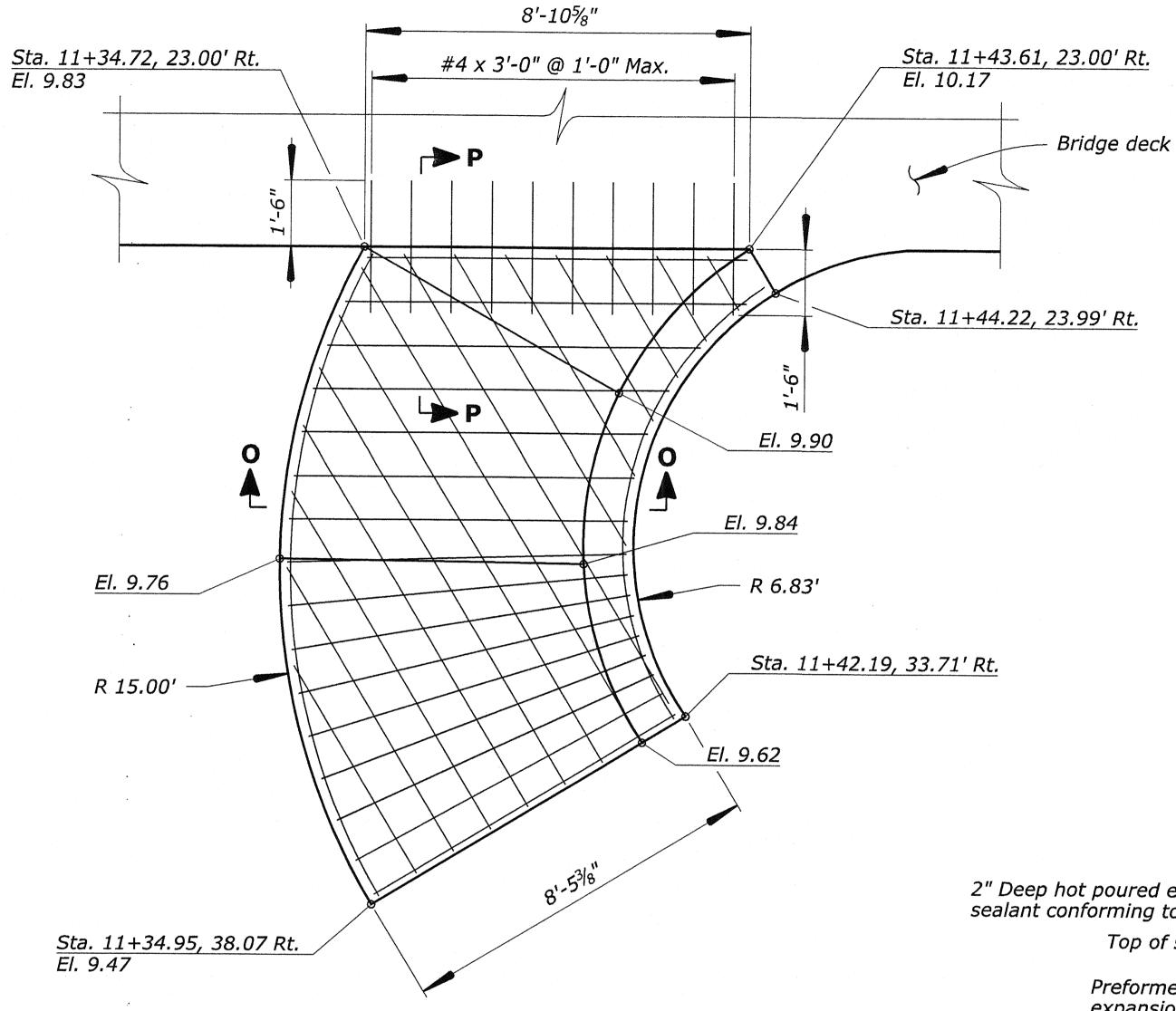
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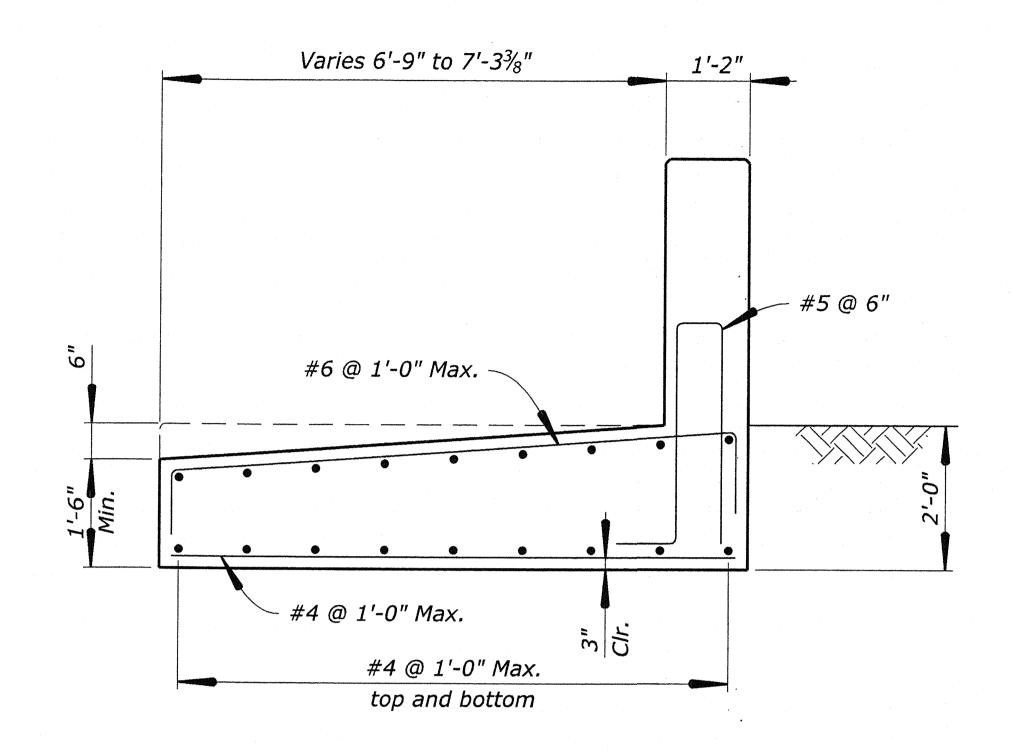
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### **EXPANSION JOINT DETAILS**

NO. DATE BY	REVISIONS	NO. DATE	BY REVI	SIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
12/2					A. PLANKIS	G. McGINN	B. LUEBBERS	NO SCALE	J. ROHNER	26 of 35	SEPTEMBER 2016	RG3077-Z

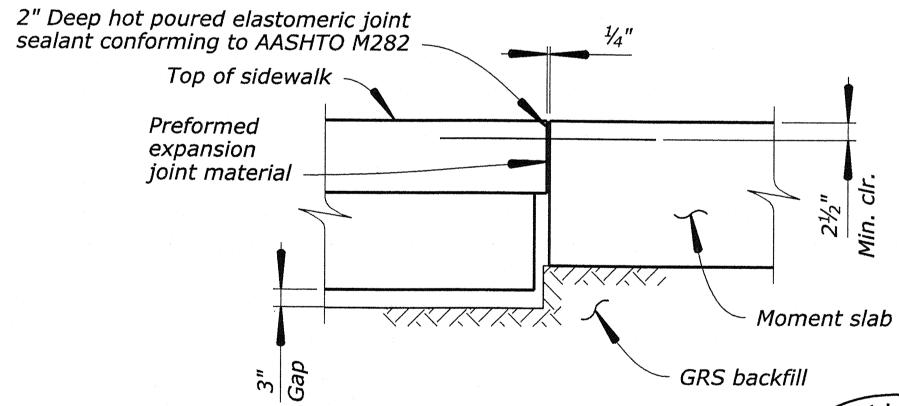


**PLAN**(Bottom mat reinforcing shown, top mat similar but most bars end at curb face)
Scale:  $\frac{1}{4}$ " = 1'-0"



## **SECTION 0-0**

Scale: 3/8" = 1'-0"



SECTION P-P Scale: 3/8" = 1'-0" LICENSED
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ENGINEER
No. 16749-S

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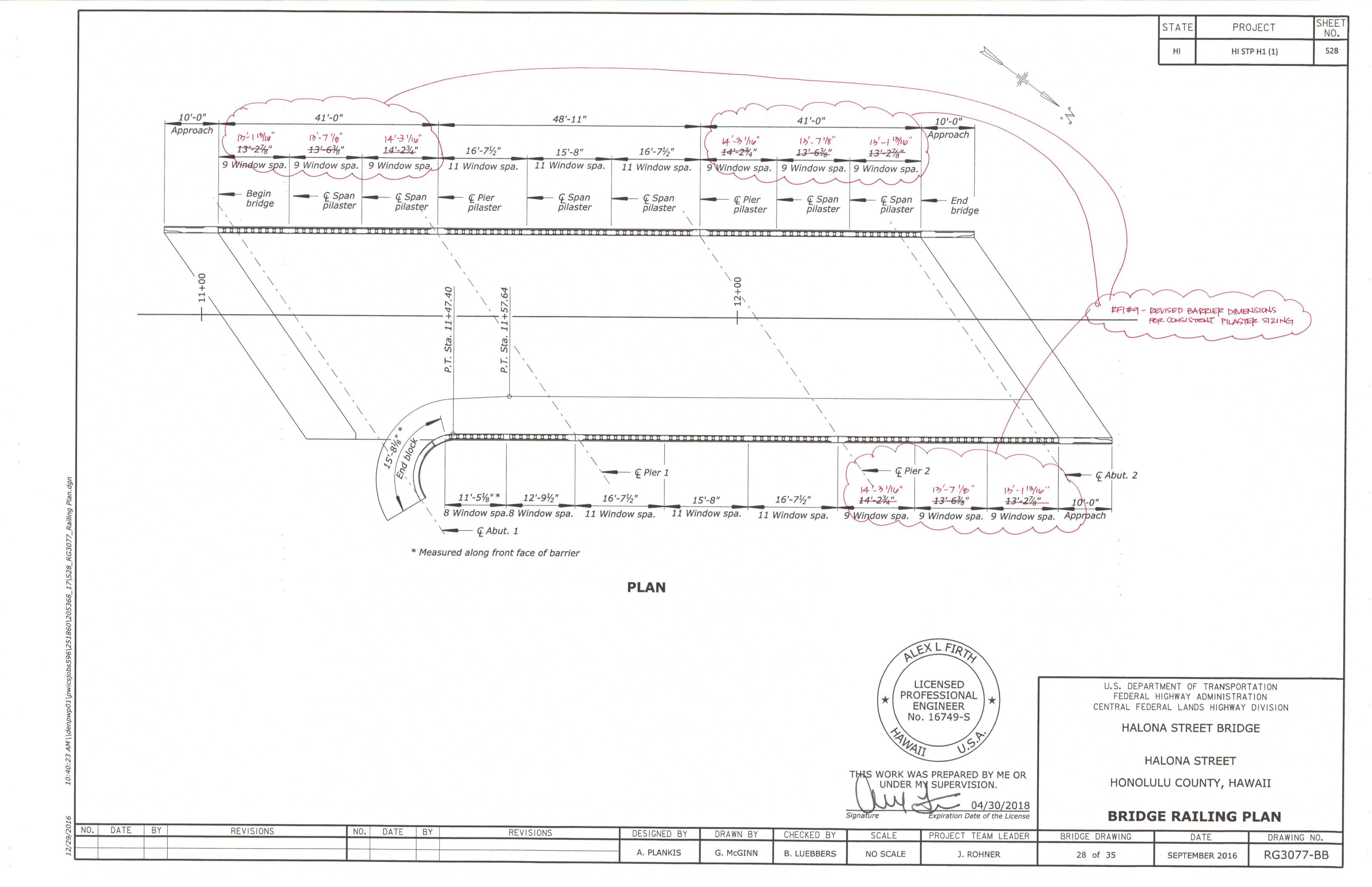
HALONA STREET BRIDGE

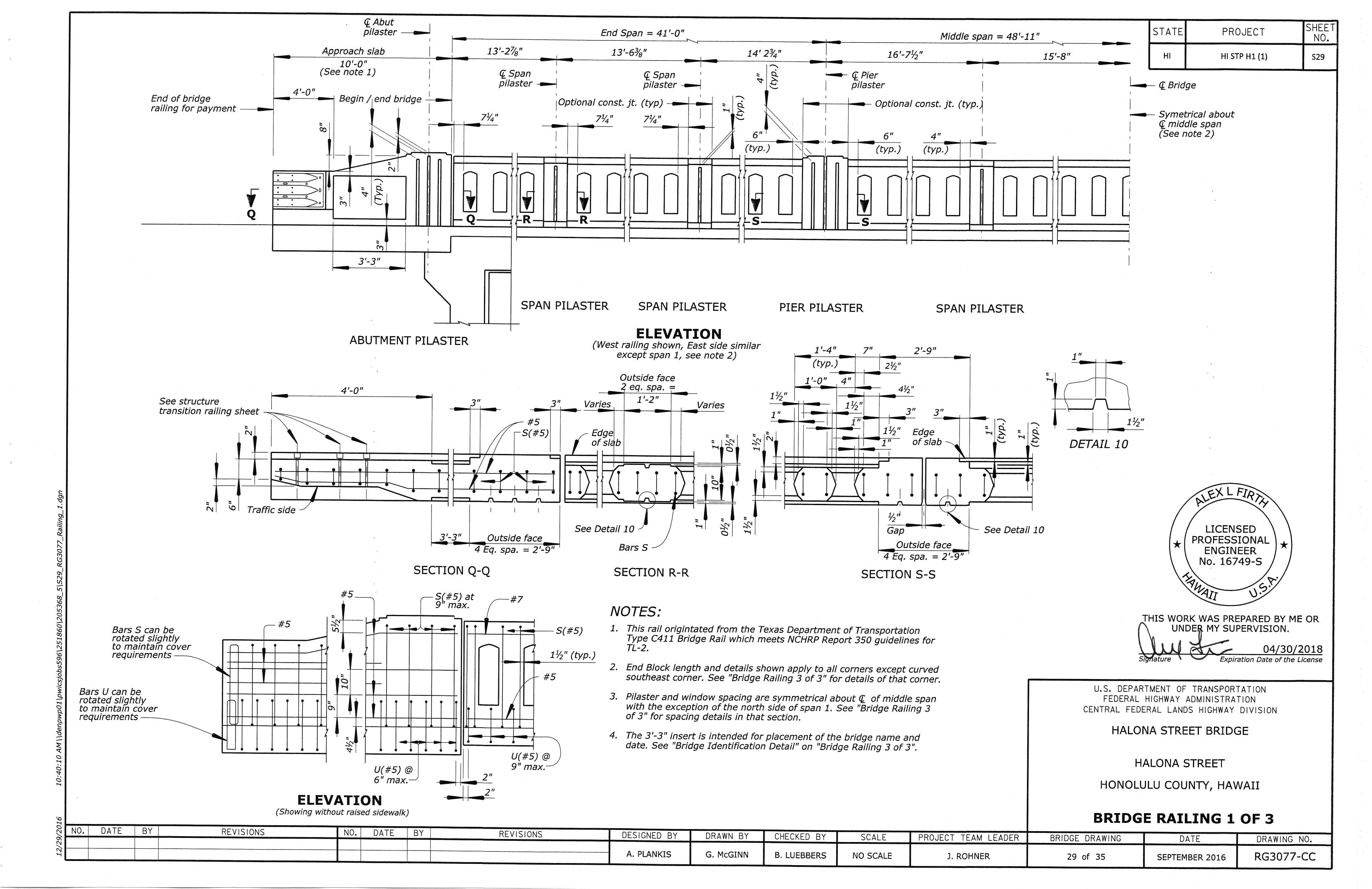
HALONA STREET

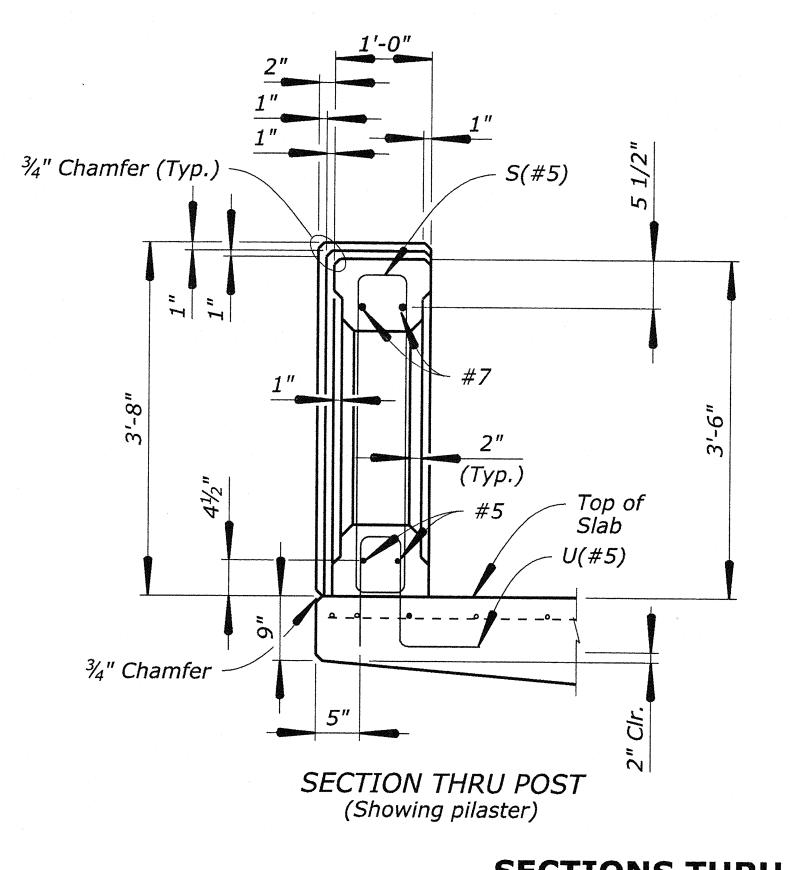
HONOLULU COUNTY, HAWAII

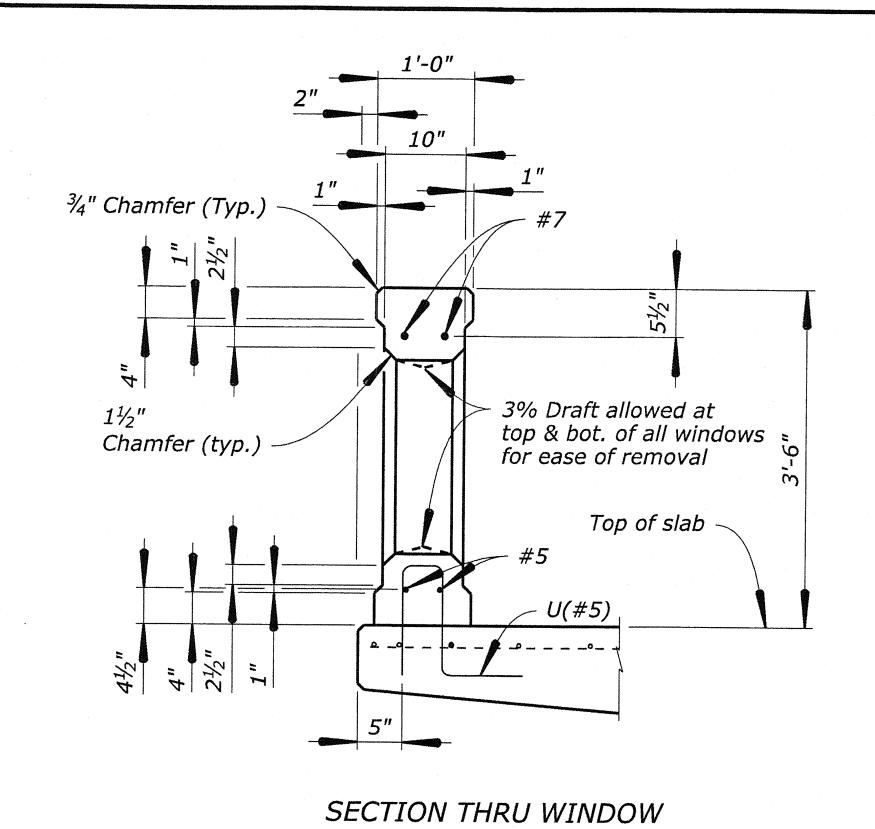
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17					-			A. PLANKIS	K. SCHNEIDER	B. LUEBBERS	$\frac{3}{8}$ " = 1'-0" UNLESS NOTED	J. ROHNER	27 of 35	SEPTEMBER 2016	RG3077-AA
											ONLESS NOTED				



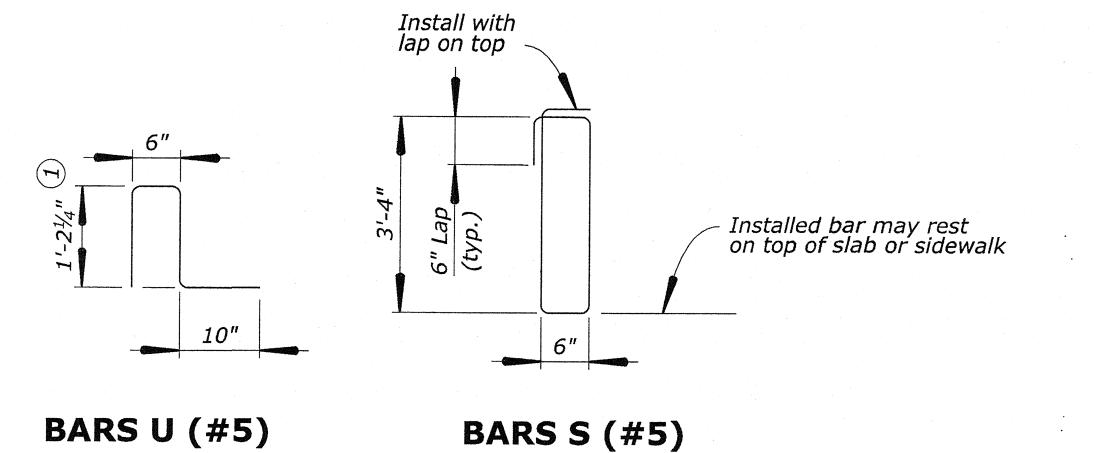






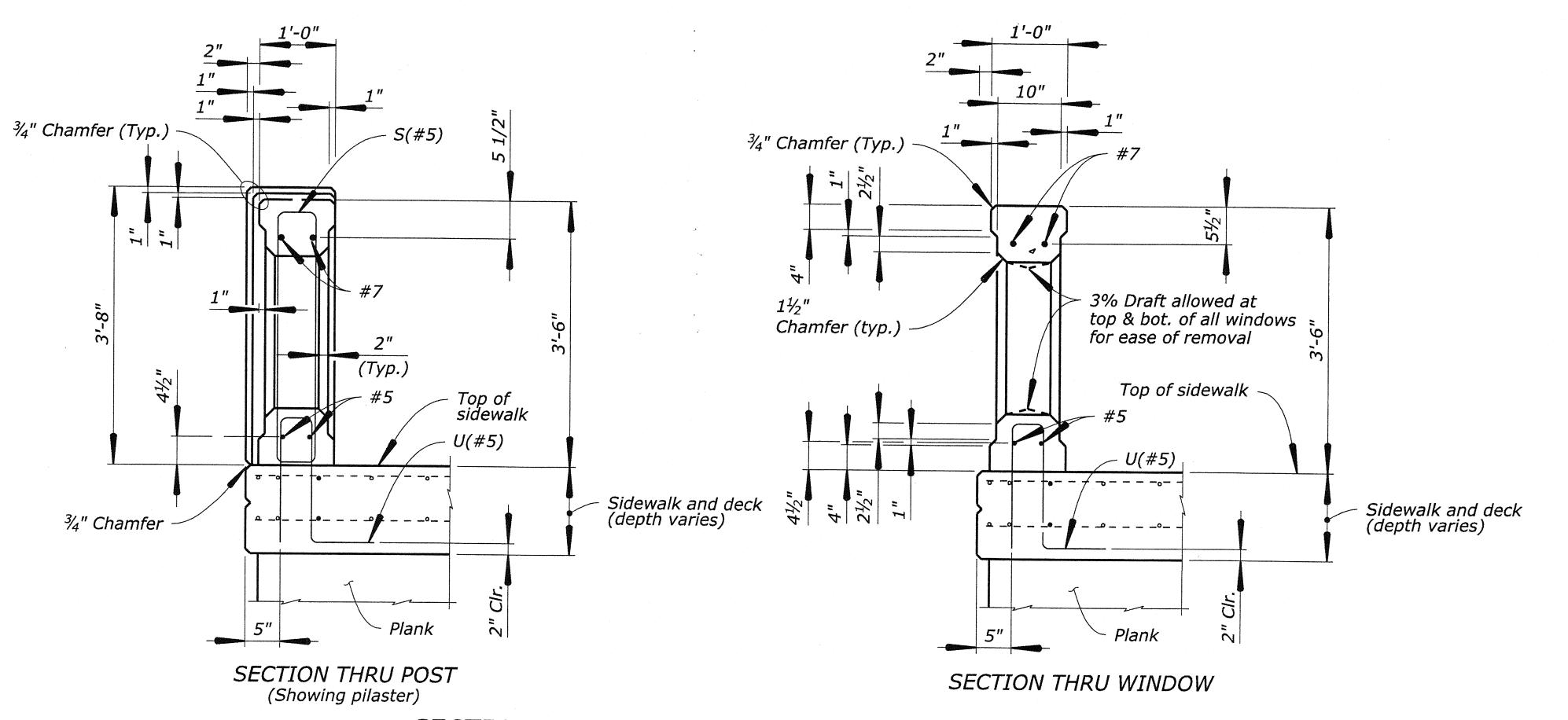
STATE PROJECT SHEET NO.

HI HISTP H1 (1) S30



1 Increase height by 6" at sidewlk.

## SECTIONS THRU RAIL WITHOUT SIDEWALK



SECTIONS THRU RAIL WITH SIDEWALK

076	NO DATE DV											DRIDGE RAILING 2 OF 3			
75/6	NU.	DATE	BY	REVISIONS	NO. DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
12/4								A. PLANKIS	G. McGINN	B. LUEBBERS	½" = 1'-0"	J. ROHNER	30 of 35	SEPTEMBER 2016	RG3077-DD



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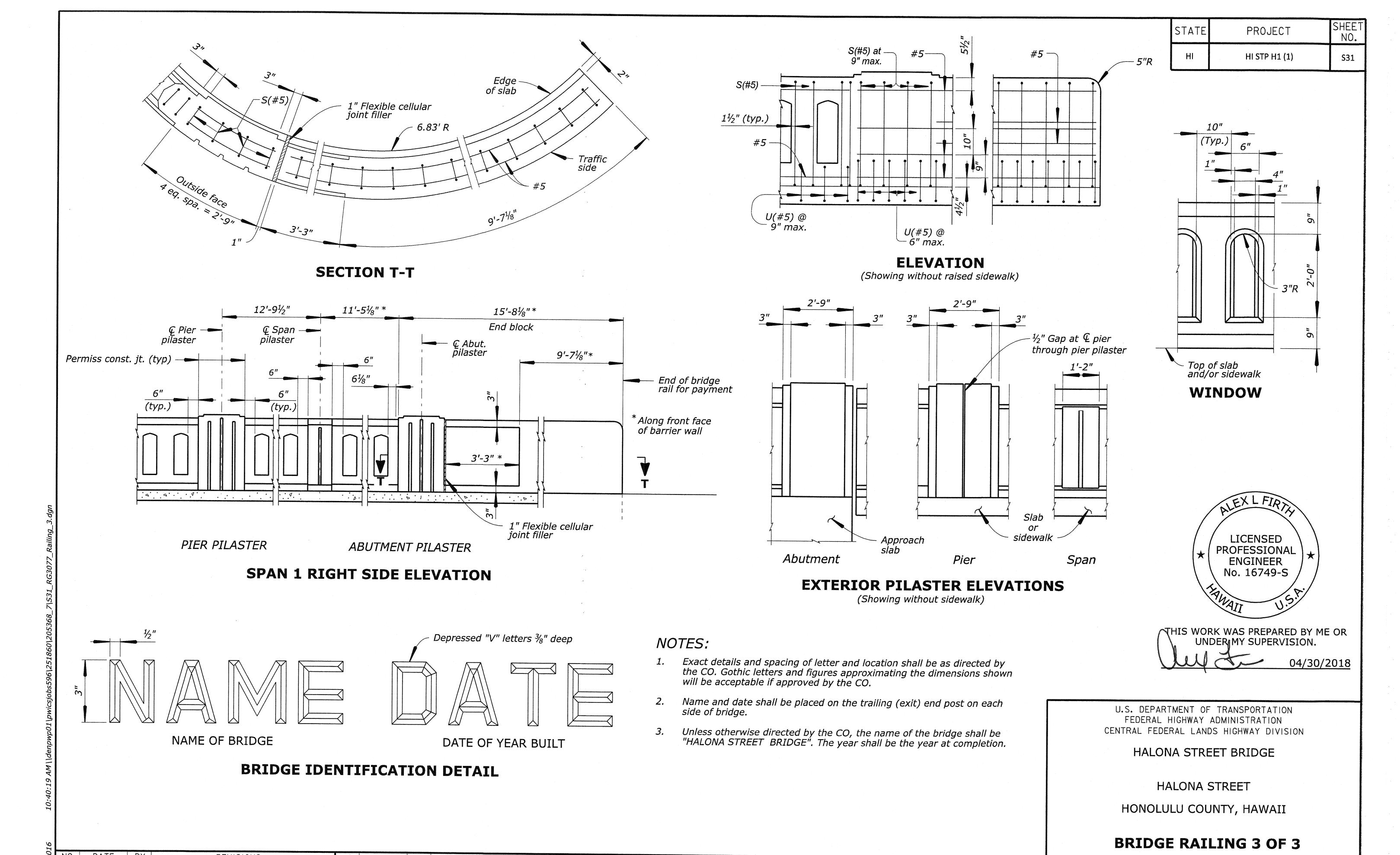
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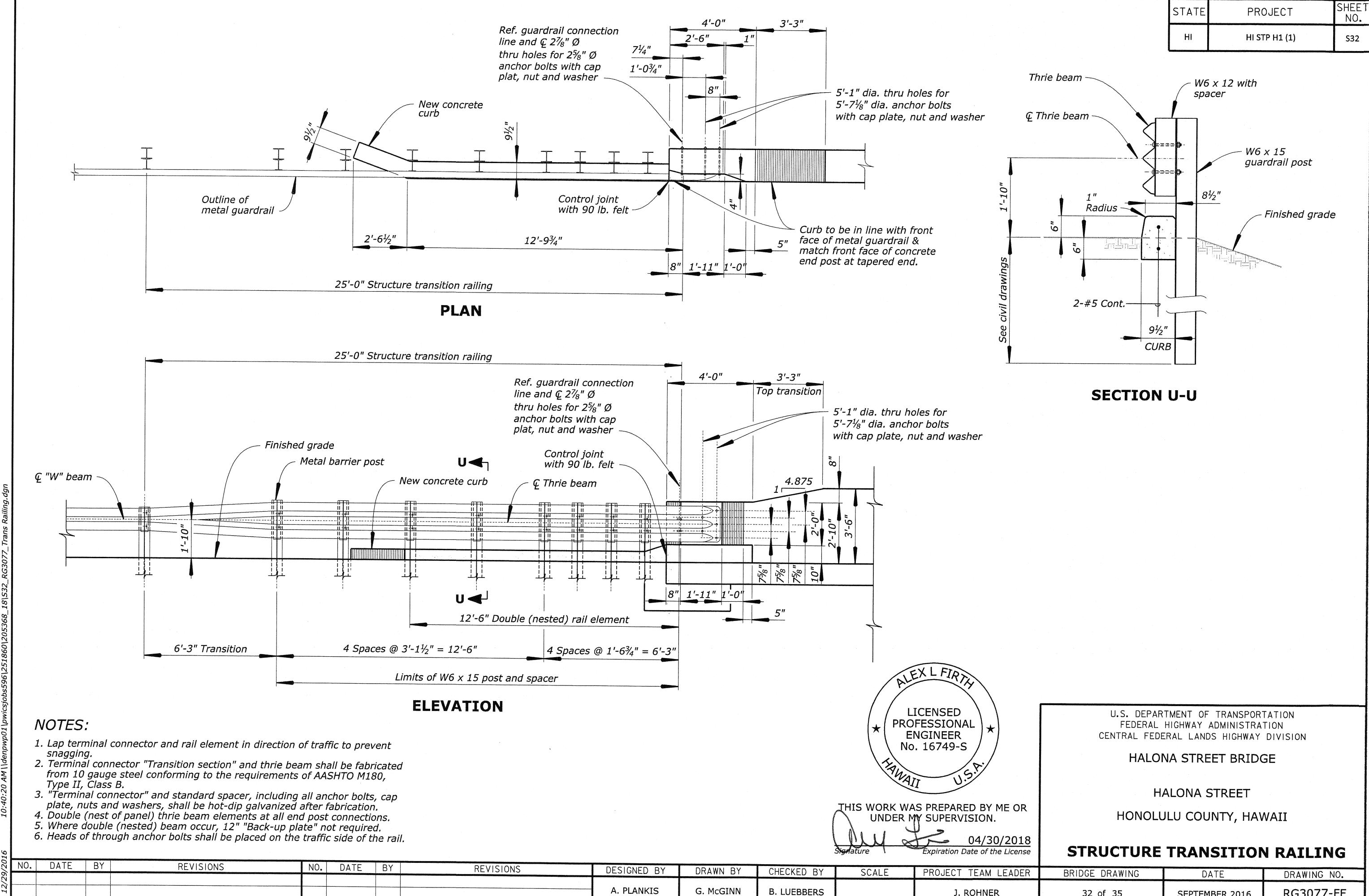
HALONA STREET BRIDGE

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HONOLULU COUNTY, HAWAII

**BRIDGE RAILING 2 OF 3** 



NO. DATE BY REVISIONS NO. DATE BY REVISIONS DESIGNED BY DRAWN BY CHECKED BY SCALE PROJECT TEAM LEADER BRIDGE DRAWING DATE DRAWING NO. A. PLANKIS G. McGINN B. LUEBBERS NO SCALE RG3077-EE J. ROHNER 31 of 35 SEPTEMBER 2016



A. PLANKIS

B. LUEBBERS

J. ROHNER

32 of 35

RG3077-FF SEPTEMBER 2016

