

General:

- A. Workmanship and materials shall conform to the building code of the City and County of Honolulu (amended UBC, 1994 Edition). However, where reference is made to performance conforming to other standards the more stringent shall apply.
- B. The contractor shall compare all the contract documents with each other and report in writing to the engineer all inconsistencies and omissions.
- C. The contractor shall take field measurements and verify field conditions and shall compare such field measurements and conditions with the drawings before commencing work. Report in writing to the engineer all inconsistencies and omissions.
- D. The contractor shall be responsible for coordinating the work of all trades.
- E. The contractor shall be responsible for methods of construction, workmanship and job safety. The contractor shall provide temporary shoring and bracing as required for stability of structural members and systems.
- F. The contractor shall be responsible for protection of the adjacent properties, structures, streets and utilities during the construction period.
- G. Details noted as typical on the structural drawings shall apply in all conditions unless specifically shown or noted.

Design criteria:

- A. Seismic \_\_\_\_\_ Zone 2a
- B. Basic wind speed and exposure \_\_\_\_\_ 80 mph, Exposure D
- C. Design live loads
  - a. Roof \_\_\_\_\_ 20 psf
  - b. Offices \_\_\_\_\_ 50 psf
  - c. Light storage \_\_\_\_\_ 125 psf
  - d. Heavy storage \_\_\_\_\_ 250 psf
  - e. Parking garage \_\_\_\_\_ HS20
- D. Allowable foundation bearing capacities
  - a. Dead load + live load \_\_\_\_\_ 3,000 psf
  - b. Dead load + live load + lateral load \_\_\_\_\_ 4,000 psf
- E. Retaining walls
  - a. Active lateral pressure \_\_\_\_\_ 40 pcf
  - b. Restrained lateral pressure \_\_\_\_\_ 55 pcf

Foundation:

- A. Foundation design is based on geotechnical investigation by Ernest k. Hirata & Associates, Inc. and report dated November 20, 1998.
- B. Contractor shall provide for de-watering of excavation from surface water, ground water or seepage.
- C. Contractor shall provide for design and installation of all cribbing, sheeting, and shoring necessary to preserve excavations and earth banks.
- D. Excavations for footings shall be observed by the geotechnical engineer prior to placement of concrete and reinforcing.
- E. Footings shall bear directly on the medium stiff to stiff silty clay. The bottom of footing excavations shall be thoroughly tamped and cleaned of loose material prior to placement of reinforcing steel and concrete.
- F. Footings located on or near the top of slopes shall be embedded such that a minimum horizontal distance of 5 feet is maintained between the bottom edge of the footing and the slope face.
- G. Building slabs-on-grade shall be underlain by at least 12 inches of imported granular fill. Prior to placement of the granular structural fill, the silty clay subgrade shall be scarified, moistened to slightly above the optimum moisture content, and compacted to a minimum 90 percent compaction as determined by ASTM D1557. The upper 4 inches shall consist of compacted #3 fine gravel (ASTM C33, size no. 67).
- H. The parking garage shall be underlain by at least 12 inches of imported granular fill. Prior to placement of the granular structural fill, the silty clay subgrade shall be scarified, moistened to slightly above the optimum moisture content, and compacted to a minimum 90 percent compaction as determined by ASTM D1557. The upper 6 inches shall consist of base course compacted to a minimum 95 percent compaction as determined by ASTM D1557.
- I. Imported granular structural fill shall be placed in lifts restricted to 8 inches in loose thickness and compacted to 95 percent compaction as determined by ASTM D1557.

Concrete:

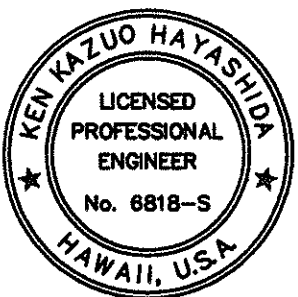
- A. Concrete construction shall conform to American Concrete Institute ACI 318R-89.
- B. Concrete shall be regular weight hard rock concrete and shall be Class "A", 3,000 psi
- C. Concrete delivery tickets shall record all free water in the mix: at batching by plant, for consistency by driver, and any additional request by contractor if permitted by the mix design.
- D. All inserts, anchor bolts, plates, and other items to be cast in the concrete shall be hot-dipped galvanized unless otherwise noted.
- E. Reinforcing bars, anchor bolts, inserts, and other items to be cast in the concrete shall be secured in position prior to placement of concrete.
- F. Conduits, pipes, and sleeves passing through a slab or footing and not conforming to typical details shall be located and submitted to the engineer for approval.
- G. Conduits, pipes, and sleeves embedded within a slab or wall (other than those merely passing through) shall be:
  - a. No larger in outside dimensions than one third the overall slab or wall thickness in which they are embedded.
  - b. Placed in the middle one third of slab or wall thickness
  - c. Spaced no closer than three diameters or widths on center.
- H. The contractor shall locate construction joints so as not to impair the strength of the structure and to minimize shrinkage stresses. Submit location of construction joints to the engineer for approval, unless otherwise noted.
- I. See architectural drawings for chamfers, edge radii, drips, reglets, finishes and other non-structural items not shown or specified on the structural drawings.
- J. Non-shrink grout shall be a premixed non-metallic formula, capable of developing a minimum compressive strength of 3,000 psi in 1 day and 5,000 psi in 28 days.

Reinforcing steel:

- A. Reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- B. Welded wire fabric shall conform to ASTM A185, galvanized.
- C. Clear concrete cover for reinforcing bars shall be as follows, unless otherwise noted:
  - a. Footings, grade beams, etc. cast against earth \_\_\_\_\_ 3"
  - b. Footings, grade beams, etc. formed and exposed to earth or weather \_\_\_\_\_ 2"
- D. Clear distance between the surface of a bar and any surface of a masonry unit shall be not less than 1/2 inch, unless otherwise noted.
- E. Reinforcing steel shall be spliced where indicated on plans. Provide lap splice length per typical details and schedule, unless otherwise noted.
- F. Mechanical splice connectors shall develop in tension 125 percent of the specified minimum yield strength of reinforcing bars.
- G. Bar bends and hooks shall be "standard hooks" in accordance with ACI 318.

FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HWY-0-03-98	2000	ADD. 38	60

ORIGINAL PLAN	SURVEY PLOTTED BY	DATE
NOTE BOOK	DRAWN BY	
	DESIGNED BY	
	QUANTITIES BY	
	CHECKED BY	



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S1.0

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION  
**GENERAL NOTES**  
  
**HAUULA BASEYARD IMPROVEMENTS**  
Project No. HWY-0-03-98  
SCALE: AS NOTED  
DATE: APRIL 2000

5-30-00	Revised General Notes
DATE	REVISION

Concrete masonry units (cmu):

- A. Concrete masonry units shall be type II, normal weight hollow load-bearing units conforming to ASTM C90 and have a minimum compressive strength of 1,900 psi.
- B. Mortar shall be type "m" conforming to ASTM C270 and have a minimum compressive strength of 2,500 psi at 28 days.
- C. Grout shall conform to ASTM C476 and have a minimum compressive strength of 2,500 psi at 28 days.
- D. All cells and bond courses with reinforcement and inserts shall be solid grouted. Cleanouts shall be provided for all grout pours over 5'-4" in height.
- E. When grouting is stopped for one hour or longer, horizontal construction joints shall be formed by stopping the grout pour 1 1/2 inches below the top of the uppermost unit.
- F. The contractor shall locate construction joints so as not to impair the strength of the structure and to minimize shrinkage stresses. Submit location of construction joints to the engineer for approval, unless otherwise noted. Maximum spacing between construction joints shall be 25 feet.
- G. Walls shall be constructed in conventional running bond, unless otherwise noted.
- H. See architectural drawings for laying pattern, height of units, surface texture, and joint type.
- I. Open-ended blocks may be substituted for standard concrete masonry units.

Metal roofing:

- A. Roofing shall be 22 gage galvanized ribbed steel, prefinished on the top.
- B. Roofing and all accessories shall be formed from steel sheets having minimum yield strength of 33 ksi and conforming to ASTM A653, SQ33, G90.
- C. Provide all closures as required to complete the work and make watertight. Open ends of all ribs shall be closed with neoprene or rubber filler strips.
- D. Metal roofing, including its attachments and accessories shall be capable of resisting a 32 psf wind uplift force.
- E. Submit structural calculations certified by a structural engineer licensed to practice in the State of Hawaii.

FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HWY-0-03-98	2000	39	60

ORIGINAL PLAN	SURVEY PLOTTED BY	DATE
NOTE BOOK	DRAWN BY	
No.	TRACED BY	
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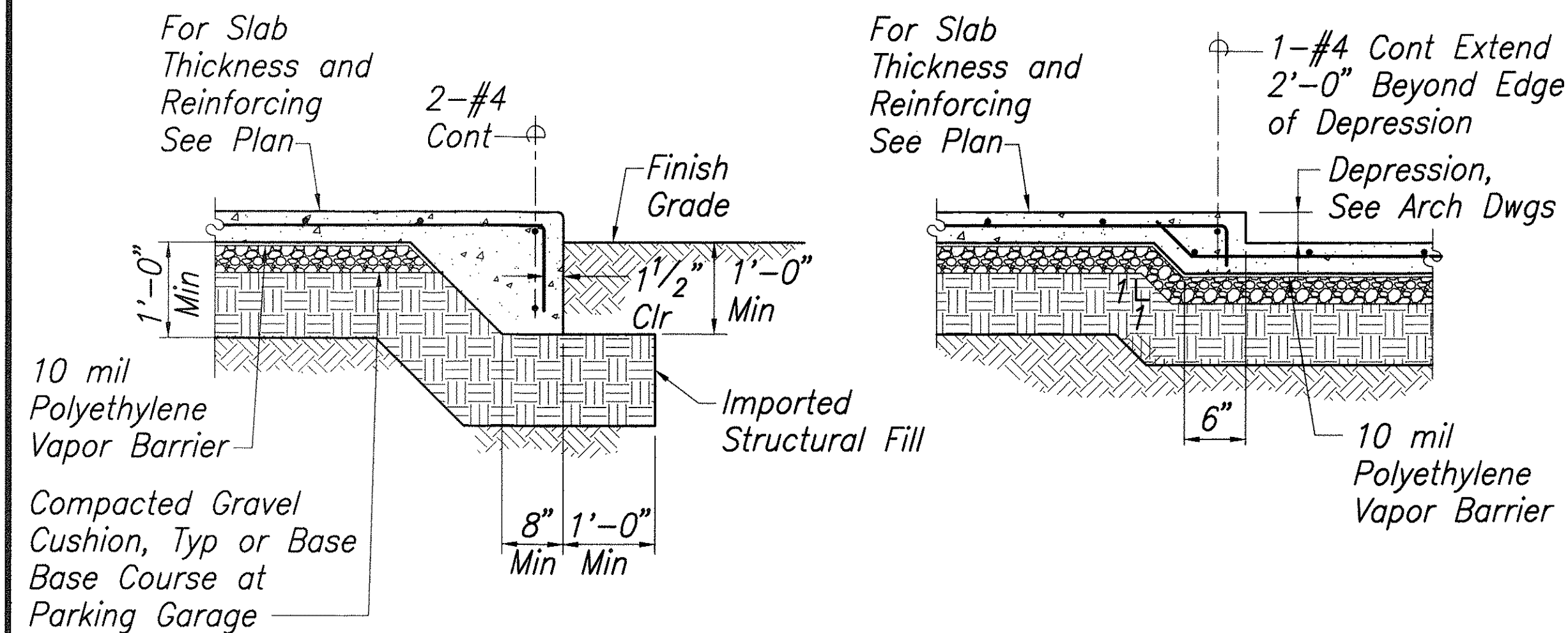


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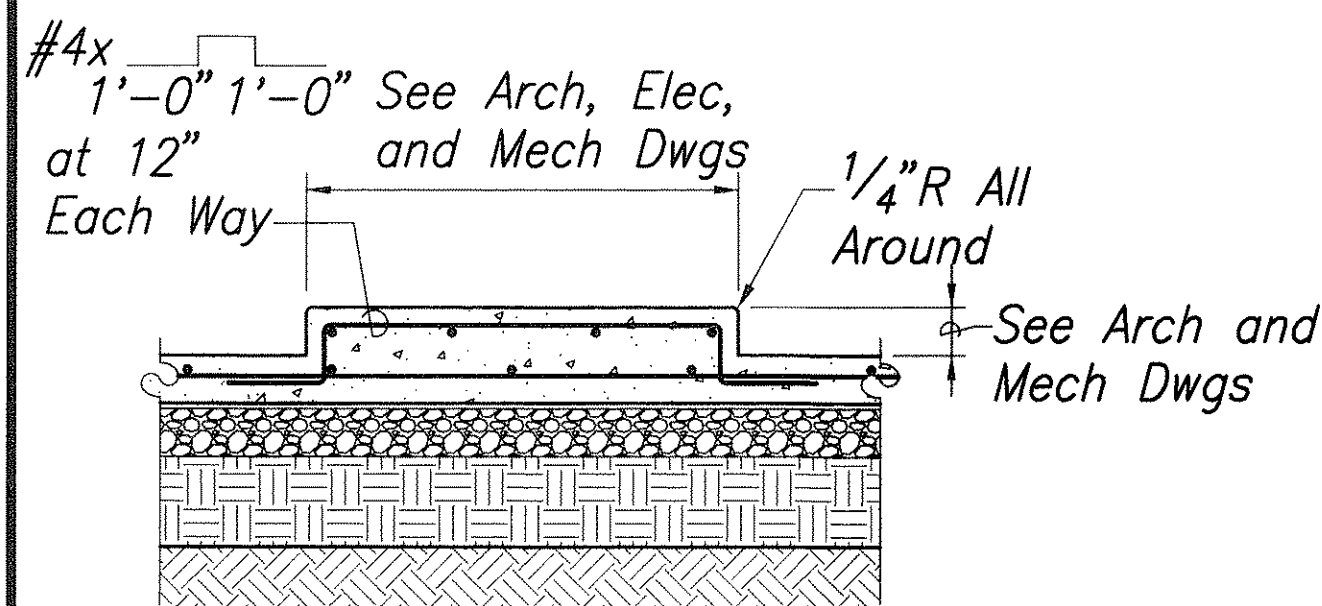
STATE OF HAWAII	
DEPARTMENT OF TRANSPORTATION	
HIGHWAYS DIVISION	
GENERAL NOTES	
HAUULA BASEYARD IMPROVEMENTS	
Project No. HWY-0-03-98	
SCALE: AS NOTED	DATE: APRIL 2000
SHEET No. S1.1 OF 60 SHEETS	



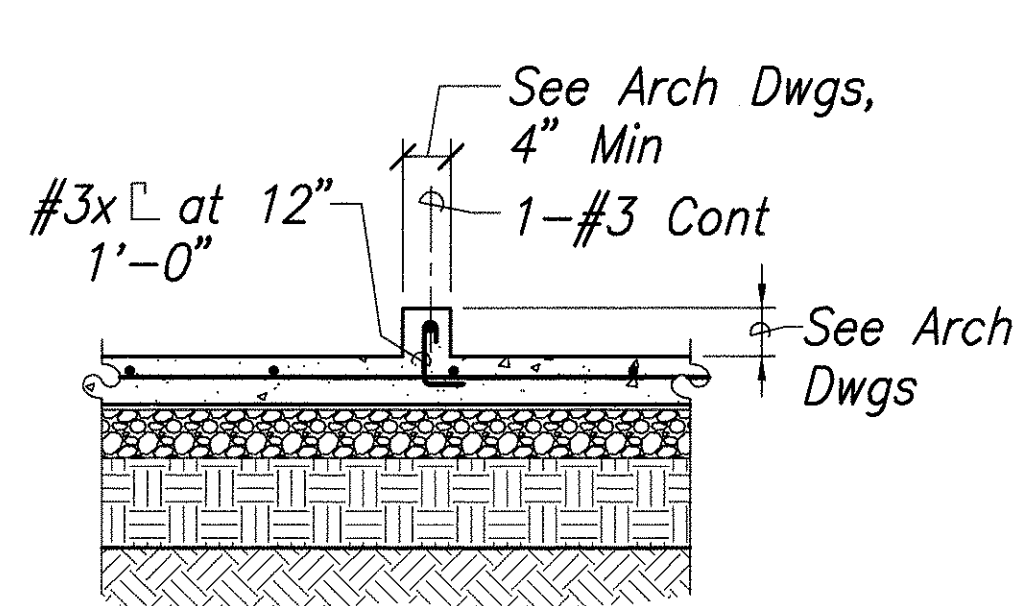


**At Slab Edge**

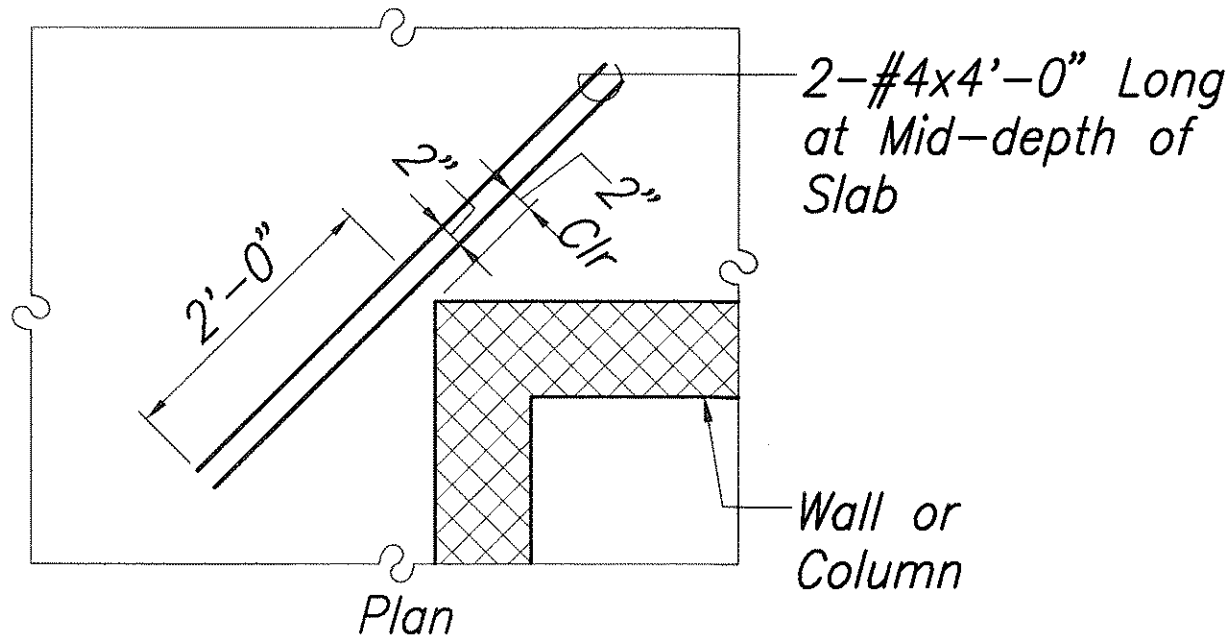
**At Depressed Slab**



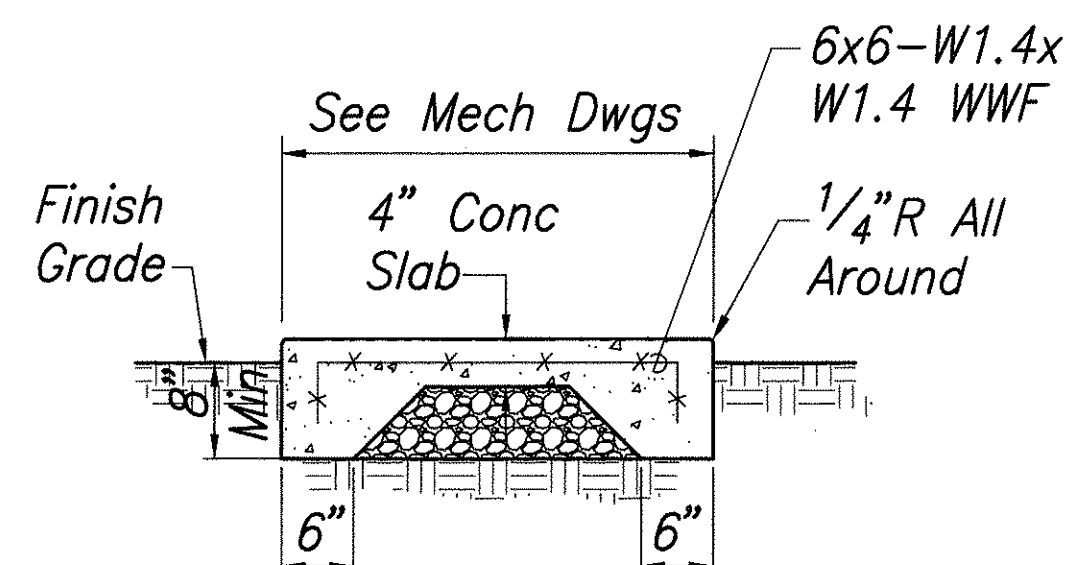
**At Interior Equipment Pad**



**At Curb**



**Reinforcing at Reentrant Corner**

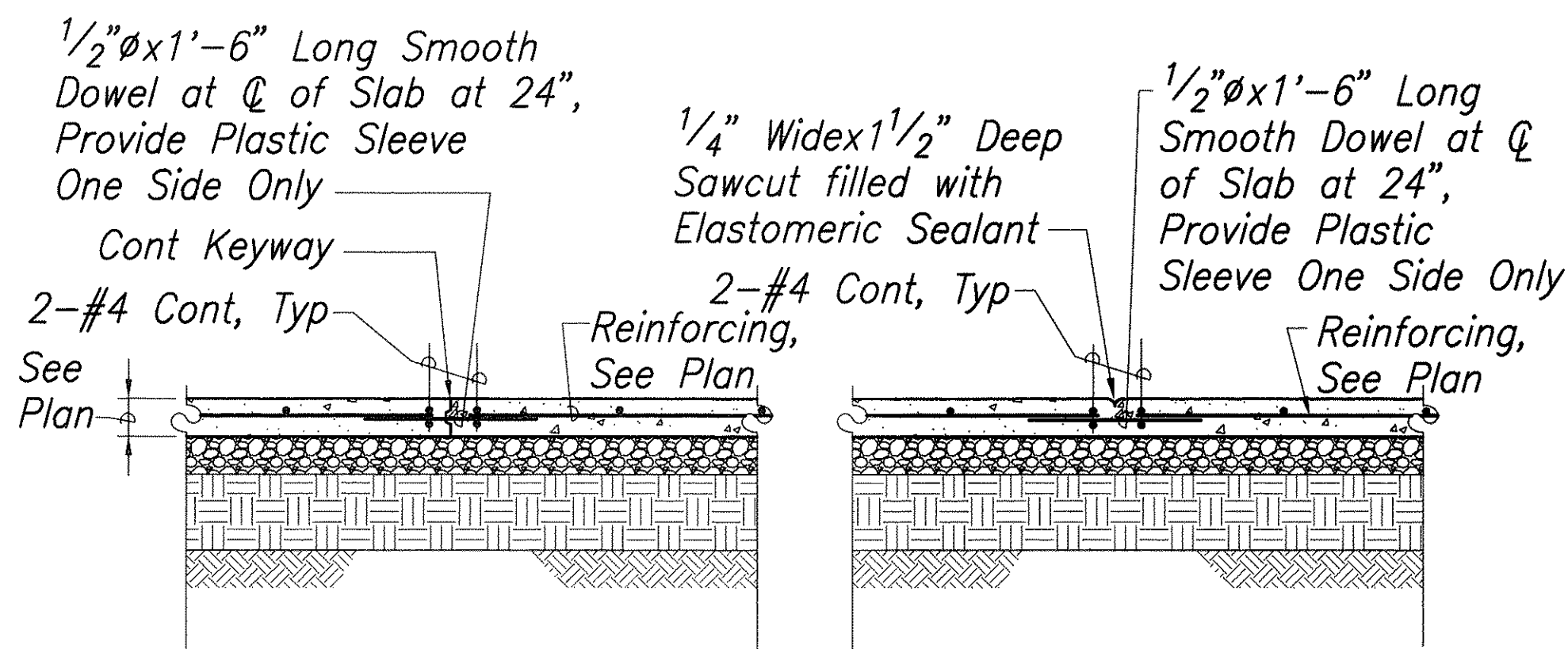


**At Exterior Equipment Pad**

**TYPICAL SLAB-ON-GRADE DETAILS**

Not To Scale

**A**  
S2 | S2



**Construction Joint, Cj-1**

**Control Joint, Cj-2**

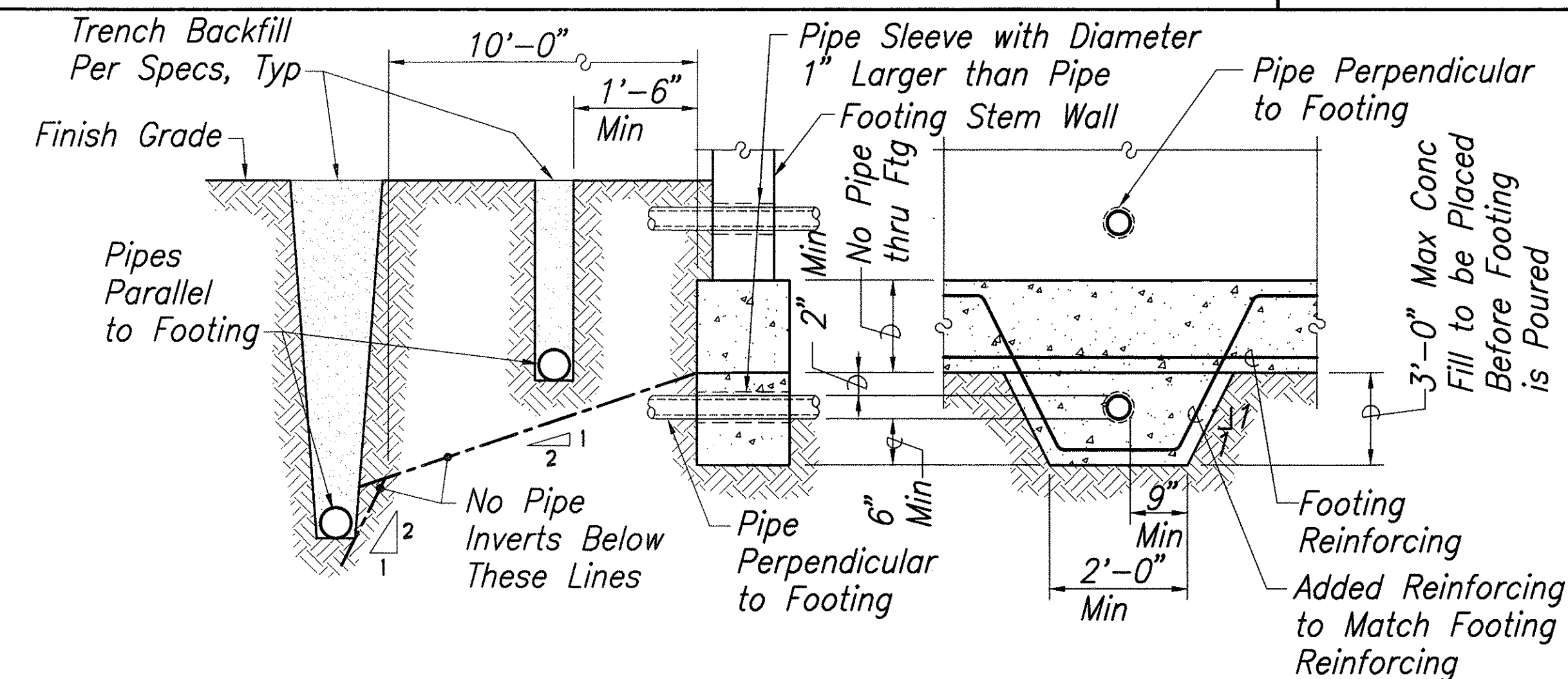
**Note:**

Saw Cutting Shall Occur as Soon as Concrete Surface is Firm Enough not be Torn by Cutting Blade and Before Shrinkage Cracking Occurs, But no Later Than 12 Hours After Concrete Has Been Poured.

**TYPICAL SLAB JOINT DETAILS**

Not To Scale

**B**  
S2 | S2



**Section**

**Elevation**

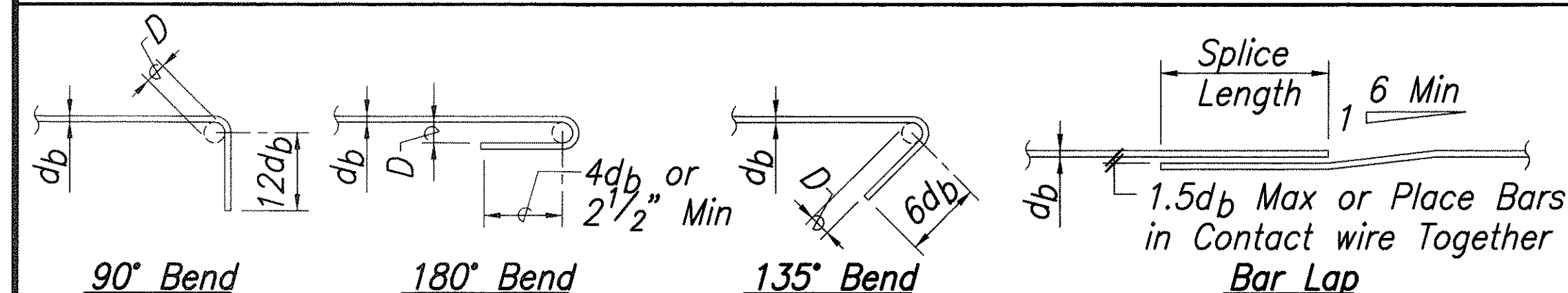
**TYPICAL PIPE AT FOOTING DETAIL**

Not To Scale

**D**  
S2 | S2

**Note:**

- For pipe perpendicular to footing more than 3'-0" below bottom of footing, trench may be backfilled with compacted fill. See Specifications.
- Depth of footing may be affected by location of pipes. General Contractor shall determine exact depth and location of pipes prior to excavation for footings.



**90° Bend:** D = 6db for #8 and Smaller  
D = 8db for #9 to #11

**Notes:**

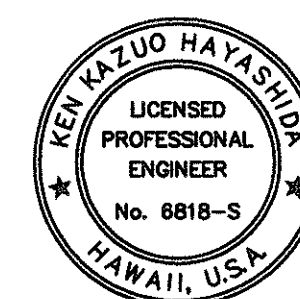
- Length are concrete with rebar spaced 6 bar diameters min oc. Increase bar length 25% for bars spaced less than 6 bar diameters.
- "Top Bars" are horizontal bars with 12" or more of concrete cast below.

**TYPICAL REBAR SPLICE AND EMBEDMENT LENGTH SCHEDULE**

Not To Scale

**E**  
S2 | S2

MINIMUM SPLICE and EMBEDMENT LENGTHS										
Concrete Strength = 3,000 psi						Concrete Strength = 2,500 psi				
Bar Size	Lap Splice		Embedment			Lap Splice		Embedment		
	Bottom Bar or Wall Bar	Top Bar	Straight		with Std Hook	Bottom Bar or Wall Bar	Top Bar	Straight		with Std Hook
			Bottom Bar or Wall Bar	Top Bar				Bottom Bar or Wall Bar	Top Bar	
#3	24"	28"	17"	22"	8"	24"	32"	18"	24"	9"
#4	24"	28"	17"	22"	8"	32"	42"	24"	32"	12"
#5	28"	36"	21"	27"	10"	39"	51"	30"	39"	15"
#6	32"	42"	25"	32"	12"	47"	62"	36"	47"	18"
#7	38"	50"	29"	38"	14"	55"	72"	42"	55"	21"
#8	44"	56"	33"	43"	16"	63"	82"	48"	63"	24"
#9	48"	64"	37"	48"	18"	72"	94"	55"	71"	28"
#10	58"	76"	45"	58"	20"	80"	104"	61"	80"	31"
#11	72"	93"	55"	71"	22"	89"	115"	68"	88"	34"



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S2

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DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

**TYPICAL DETAILS**

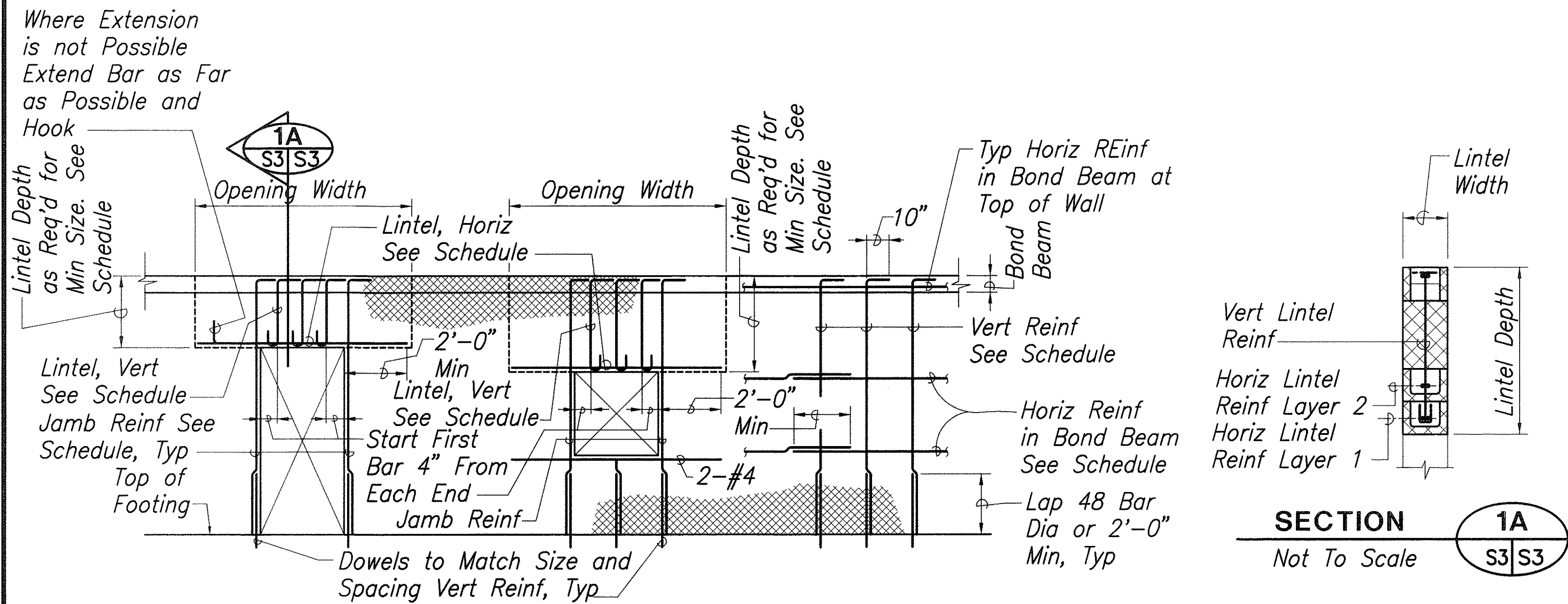
**HAUULA BASEYARD  
IMPROVEMENTS**

Project No. HWY-0-03-98

SCALE: AS NOTED

DATE: APRIL 2000

SHEET No. S2 OF 60 SHEETS



OPENING SCHEDULE						
Opening Width	Lintel Width	Lintel Depth	Reinforcing			Remarks
			Jamb	Lintel Horiz Layer 1	Lintel Horiz Layer 2	
W < 4'-0"	8"	1'-4"	2-#5	2-#4	-	#4 at 8"
4'-1" < W < 8'-0"	8"	2'-8"	2-#5	2-#5	-	#4 at 8"

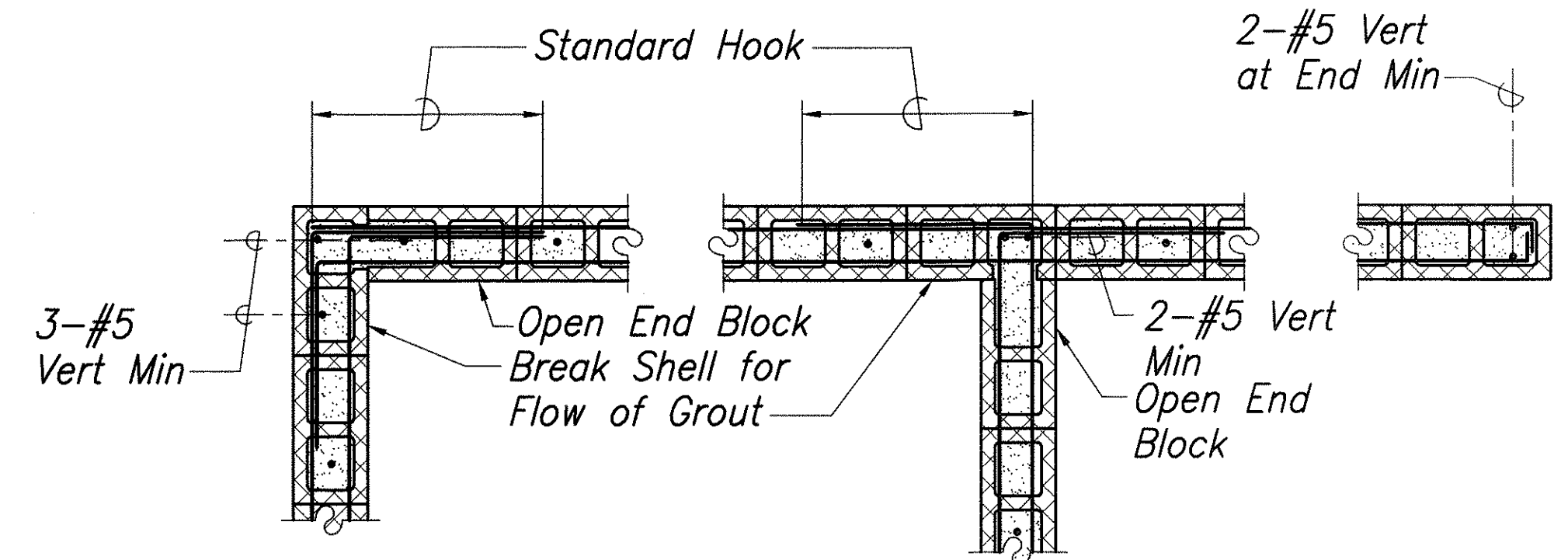
CMU WALL REINFORCING SCHEDULE				
Mark	Wall Thickness	Bar Size and Spacing		
		Horiz	Vert	Remarks
Cmu-1	8"	2-#4 at 48"	#4 at 24"	
Cmu-2	8"	2-#4 at 48"	#4 at 8"	
Cmu-3	6"	2-#4 at 48"	#4 at 24"	

TYPICAL CMU WALL ELEVATION DETAIL

Not To Scale

A  
S3 S3

Note:  
All walls shall be solid grouted.



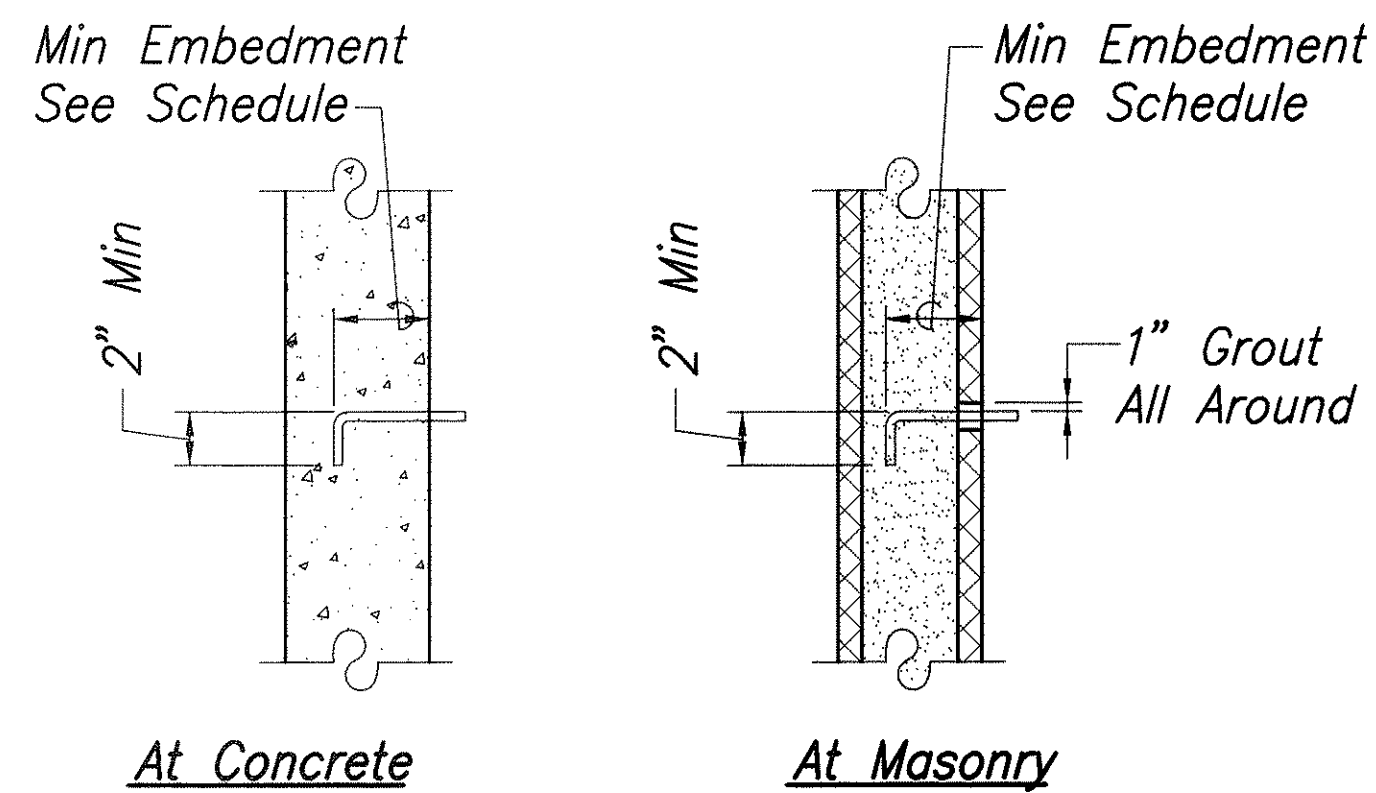
Note:

Provide horizontal joint reinforcement at 16"

TYPICAL CMU WALL REINFORCING

Not To Scale

B  
S3 S3

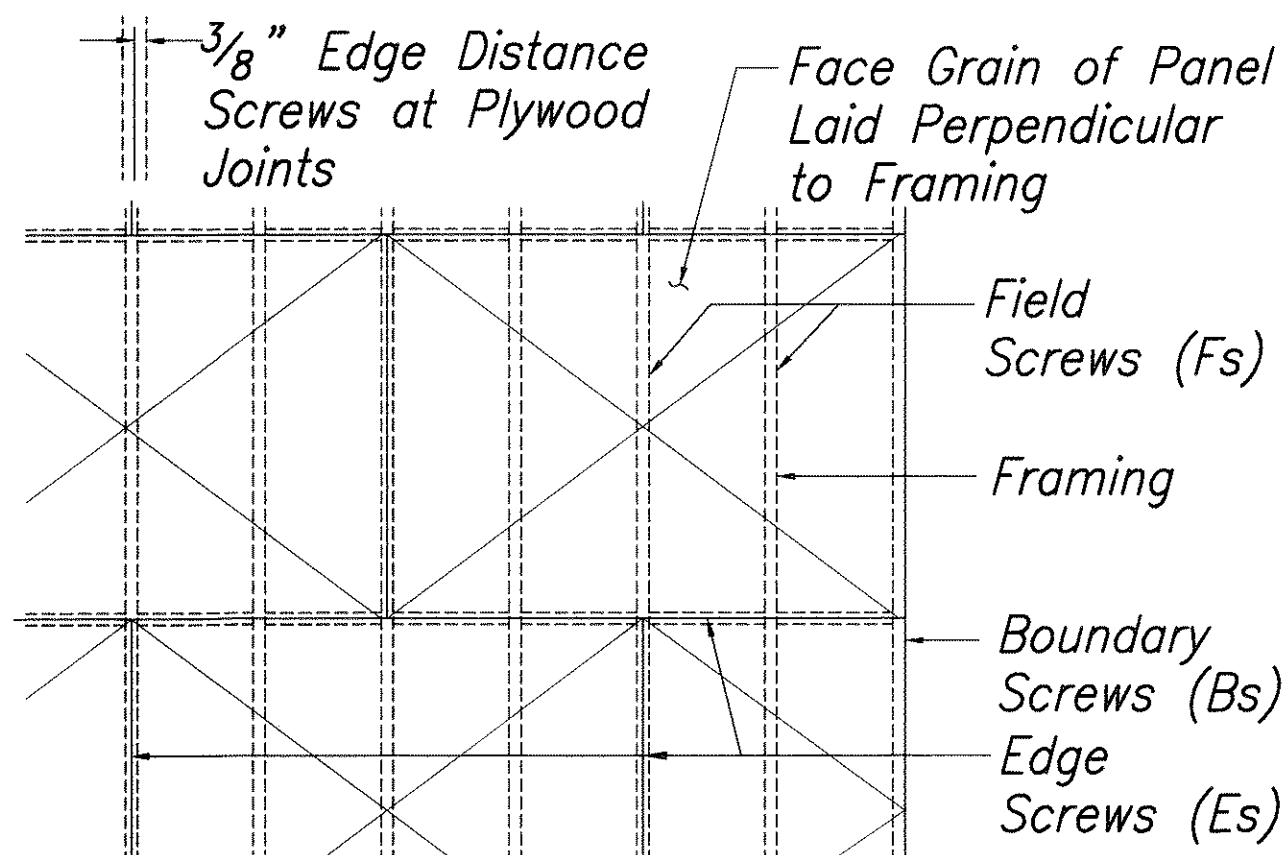


Bolt Diameter	Minimum Embedment
1/2"	4"
5/8"	5"
3/4"	6"
7/8"	7"
1"	7"

TYPICAL ANCHOR BOLT DETAIL

Not To Scale

D  
S3 S3

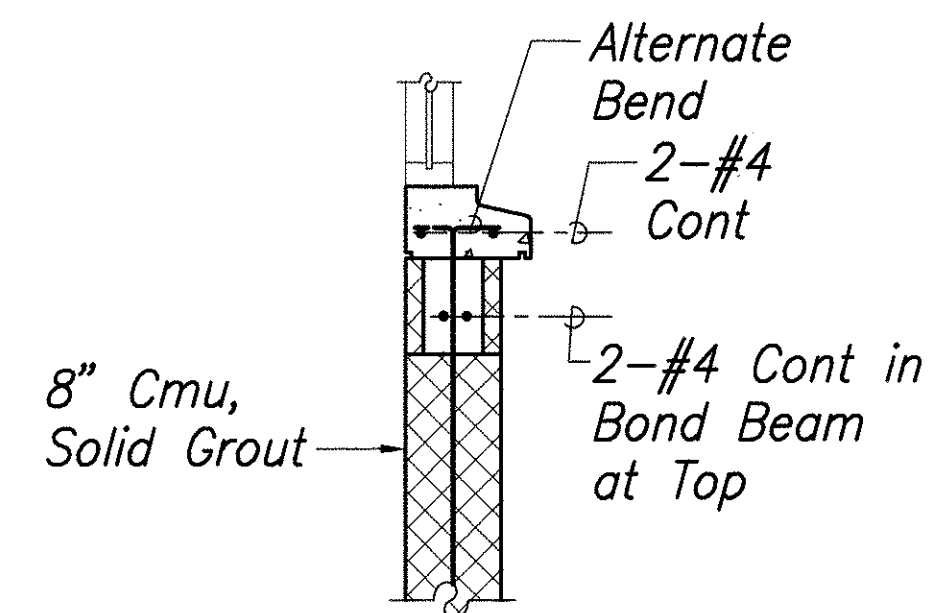


ROOF SHEATHING				
Type	Boundary Screws	Edge Screws	Field Screws	Remarks
Roof	#10 at 6"	#10 at 6"	#10 at 12"	

PLYWOOD ROOF SHEATHING

Not To Scale

E  
S3 S3



Note:

See Architectural Drawings for Size, Reveal, Drip, Etc of Sill

TYPICAL CAST IN PLACE CONCRETE SILL DETAIL

Not To Scale

F  
S3 S3



S3

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION  
TYPICAL DETAILS

HAUULA BASEYARD  
IMPROVEMENTS  
Project No. HWY-0-03-98

SCALE: AS NOTED

DATE: APRIL 2000

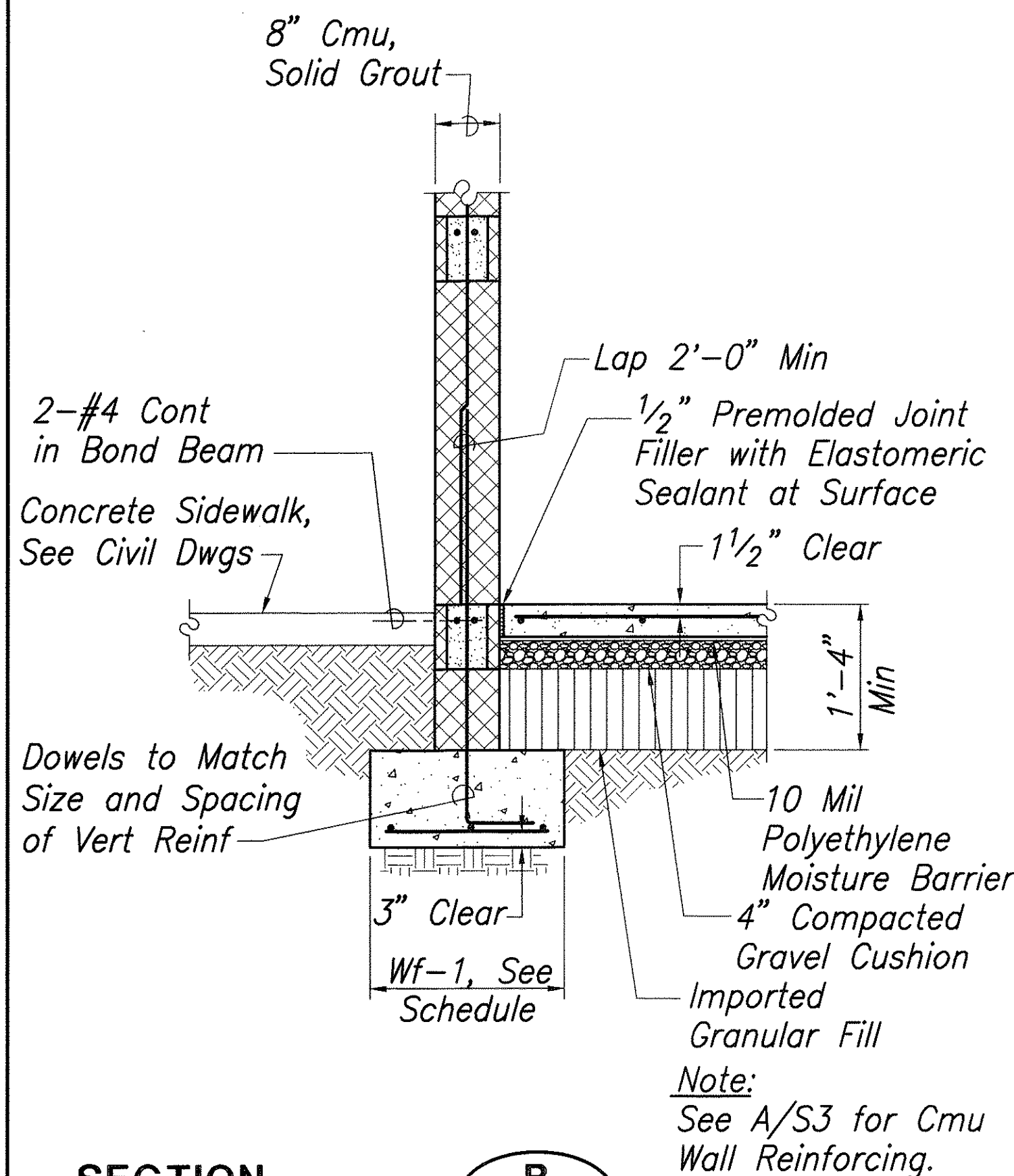
SHEET No. S3 OF 60 SHEETS



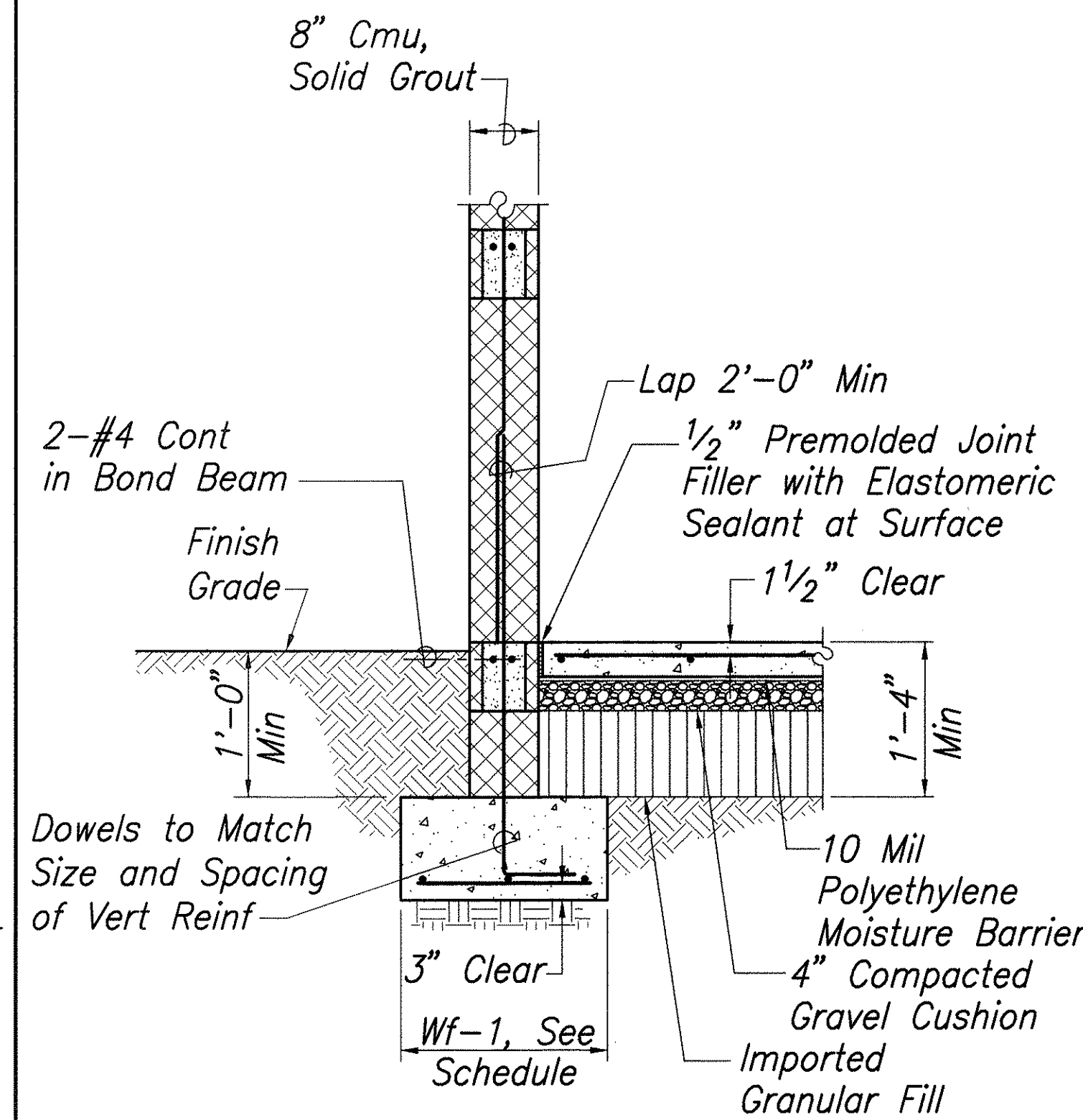
FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HWY-0-03-98	2000	42	60

FOOTING SCHEDULE		
Mark	Size	Reinforcement
Wf-1	2'-0"x1'-0" Thick	3-#4 Bottom Cont with #4 at 12"
Wf-2	3'-0"x6'-0" x1'-0" Thick	4-#4 Bottom Long Way 8-#4 Bottom Short Way
Wf-3	4'-6" Cont x1'-0" Thick	6-#4 Top and Bottom Cont with #4 Top and Bottom at 12"
Wf-4	2'-3" Cont x1'-0" Thick	3-#4 Top and Bottom Cont with #4 Top and Bottom at 12"
F-1	2'-0"x2'-8" x1'-0" Thick	3-#4 Bottom Long Way 4-#4 Bottom Short Way
RW-1	3'-8" Cont x1'-0" Thick	5-#4 Top Cont with #5 Top at 12"

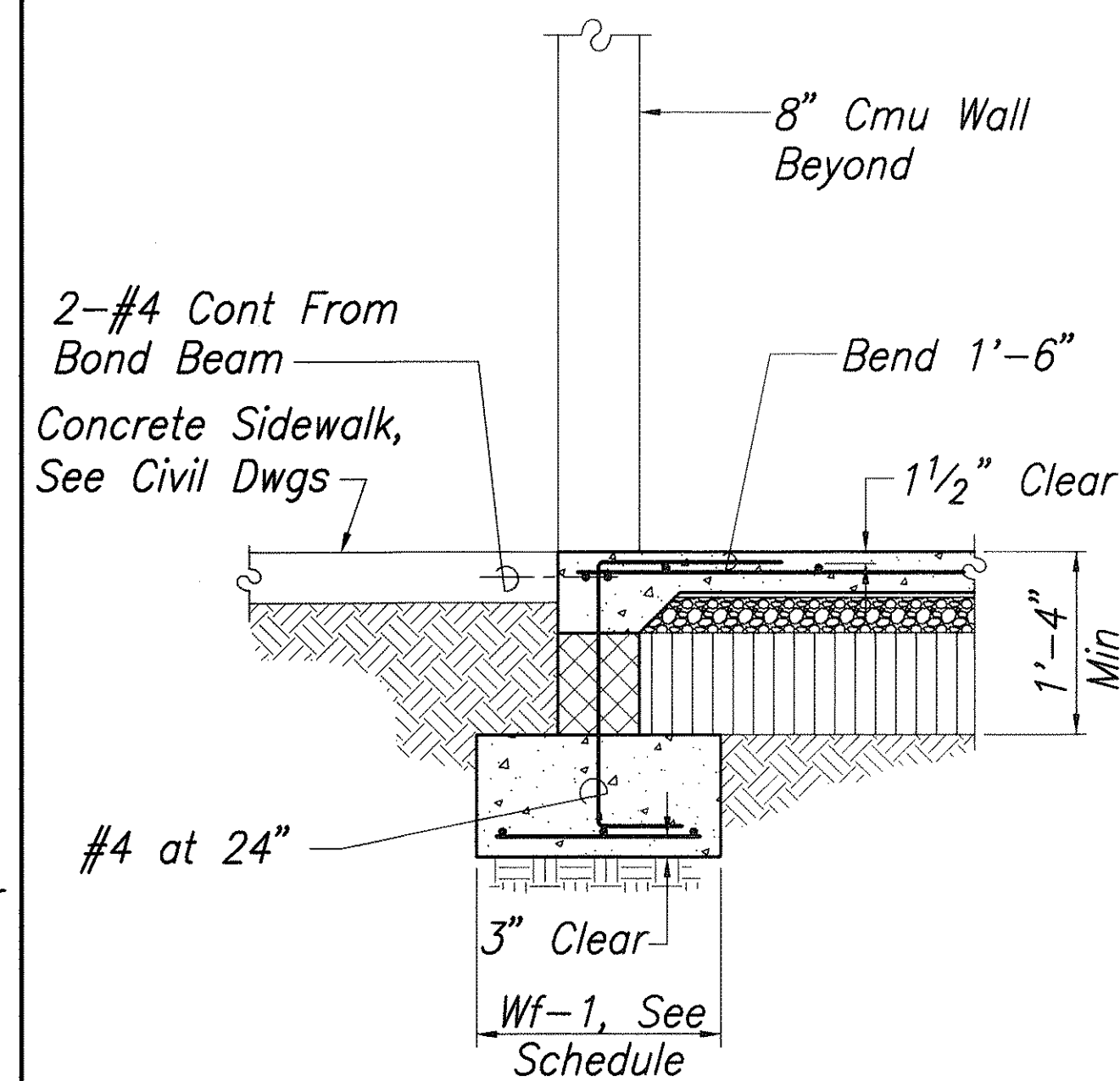
FOOTING SCHEDULE A  
No Scale



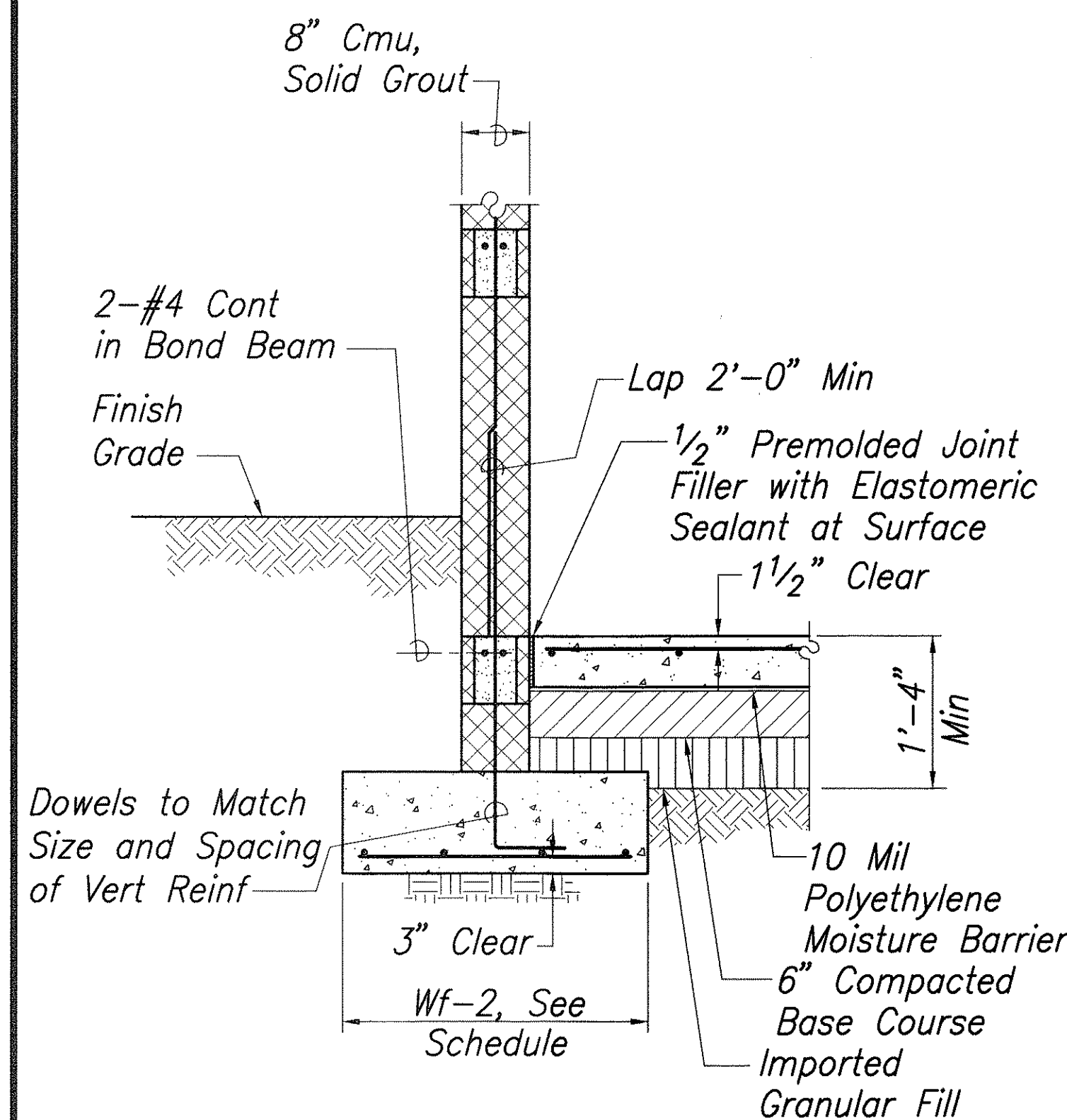
SECTION B  
Scale: 3/4" = 1'-0"



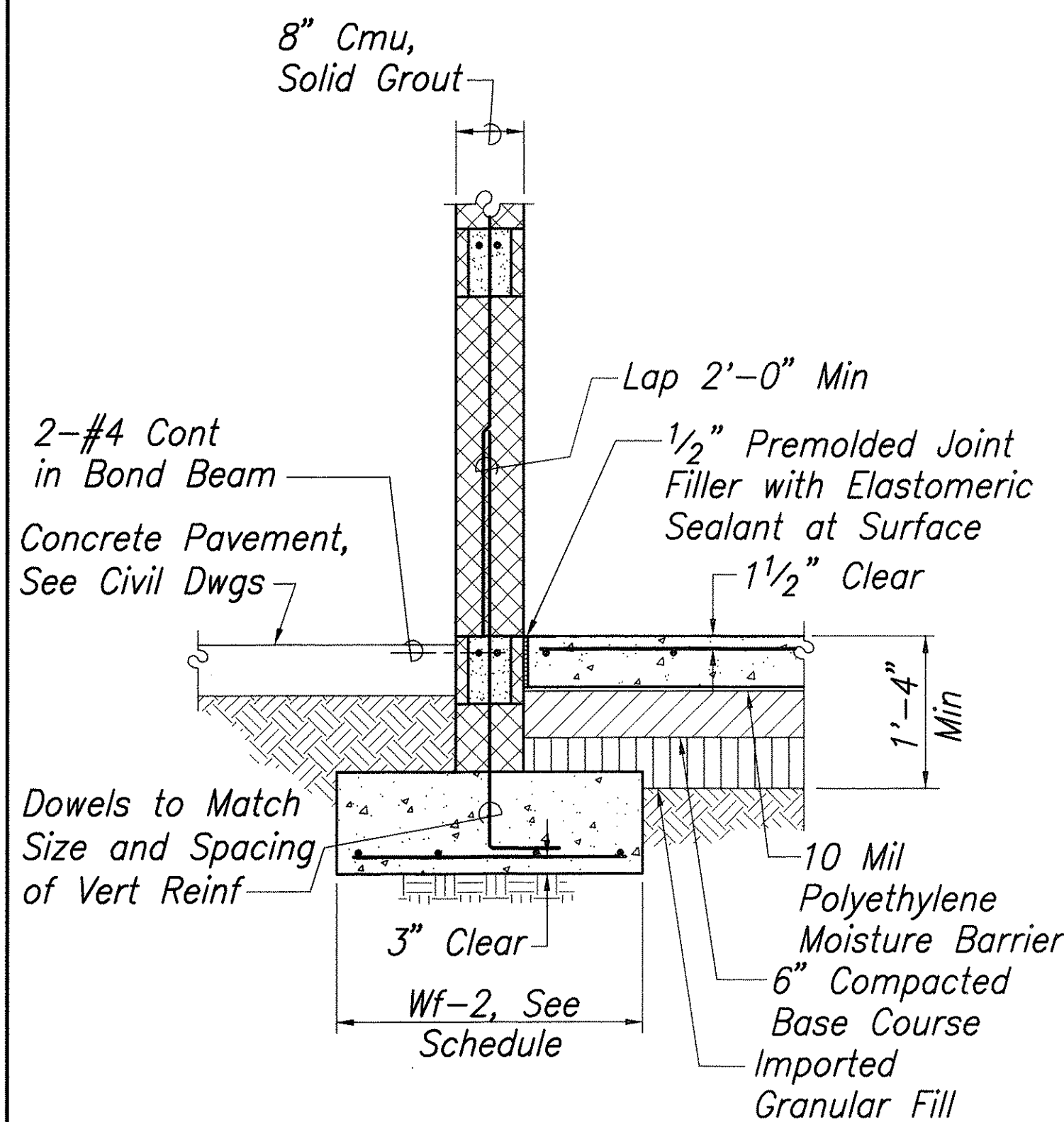
SECTION C  
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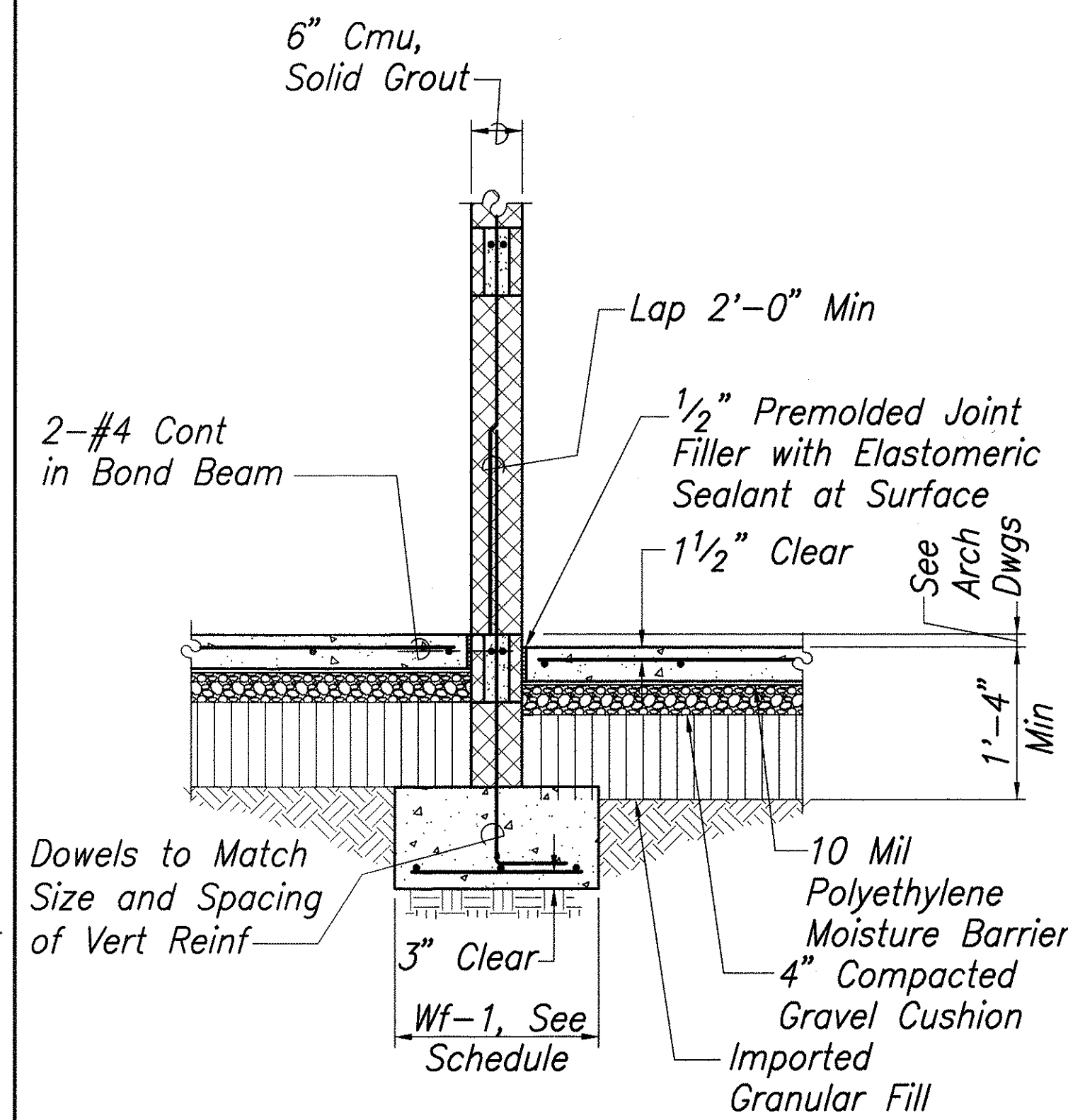
SECTION D  
Scale: 3/4" = 1'-0"



SECTION E  
Scale: 3/4" = 1'-0"



SECTION F  
Scale: 3/4" = 1'-0"



SECTION G  
Scale: 3/4" = 1'-0"

ORIGINAL PLAN	DATE
NOTED BY	DATE
CHECKED BY	DATE
QUANTITIES BY	DATE
TRACED BY	DATE
DRAWN BY	DATE
SURVEY PLOTTED BY	DATE

Ken Hazuo Hayashida  
Professional Engineer  
No. 6818-S  
HAWAII, USA

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S4

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

**FOUNDATION SECTIONS  
AND DETAILS**

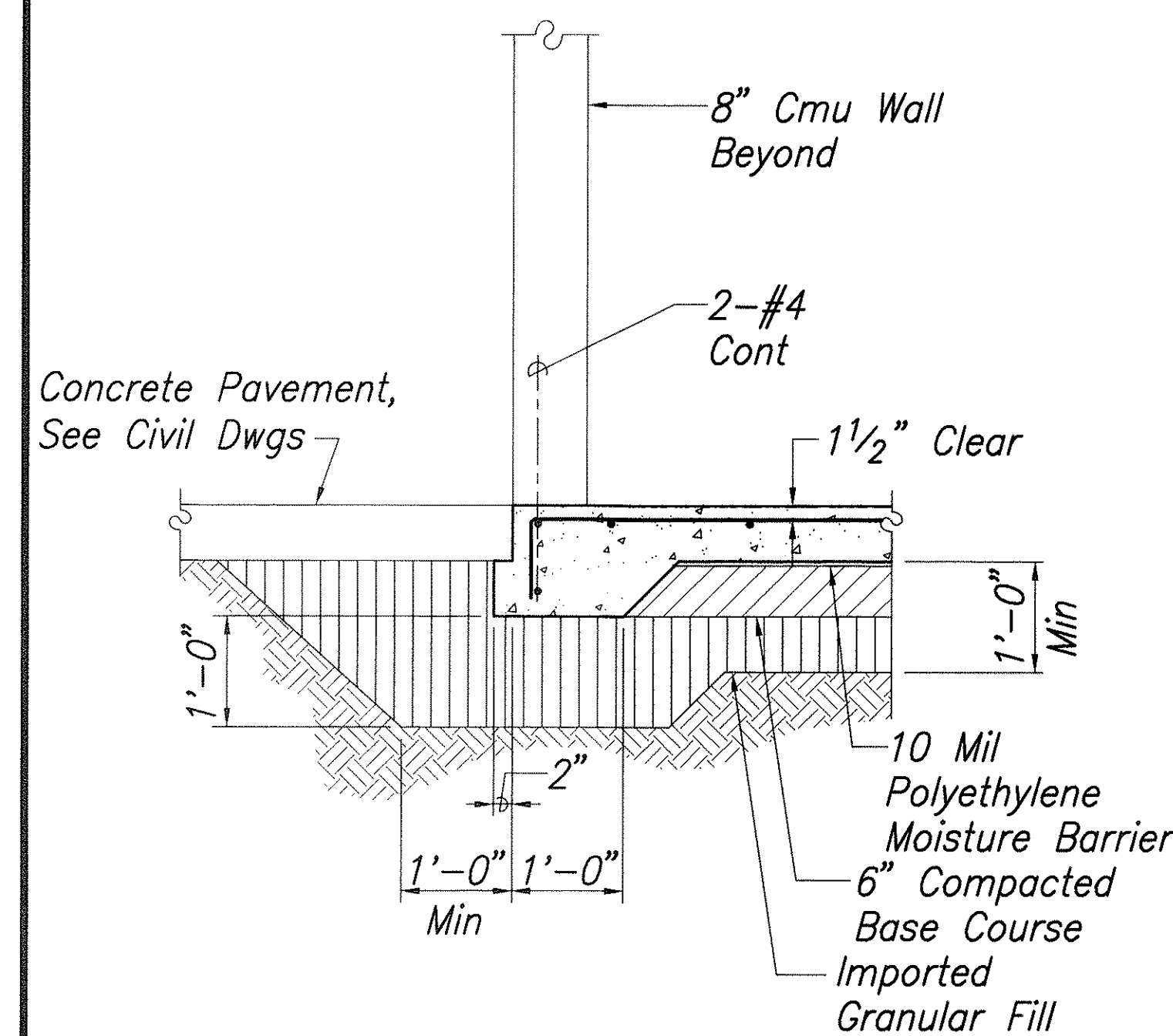
**HAUULA BASEYARD  
IMPROVEMENTS**

Project No. HWY-0-03-98

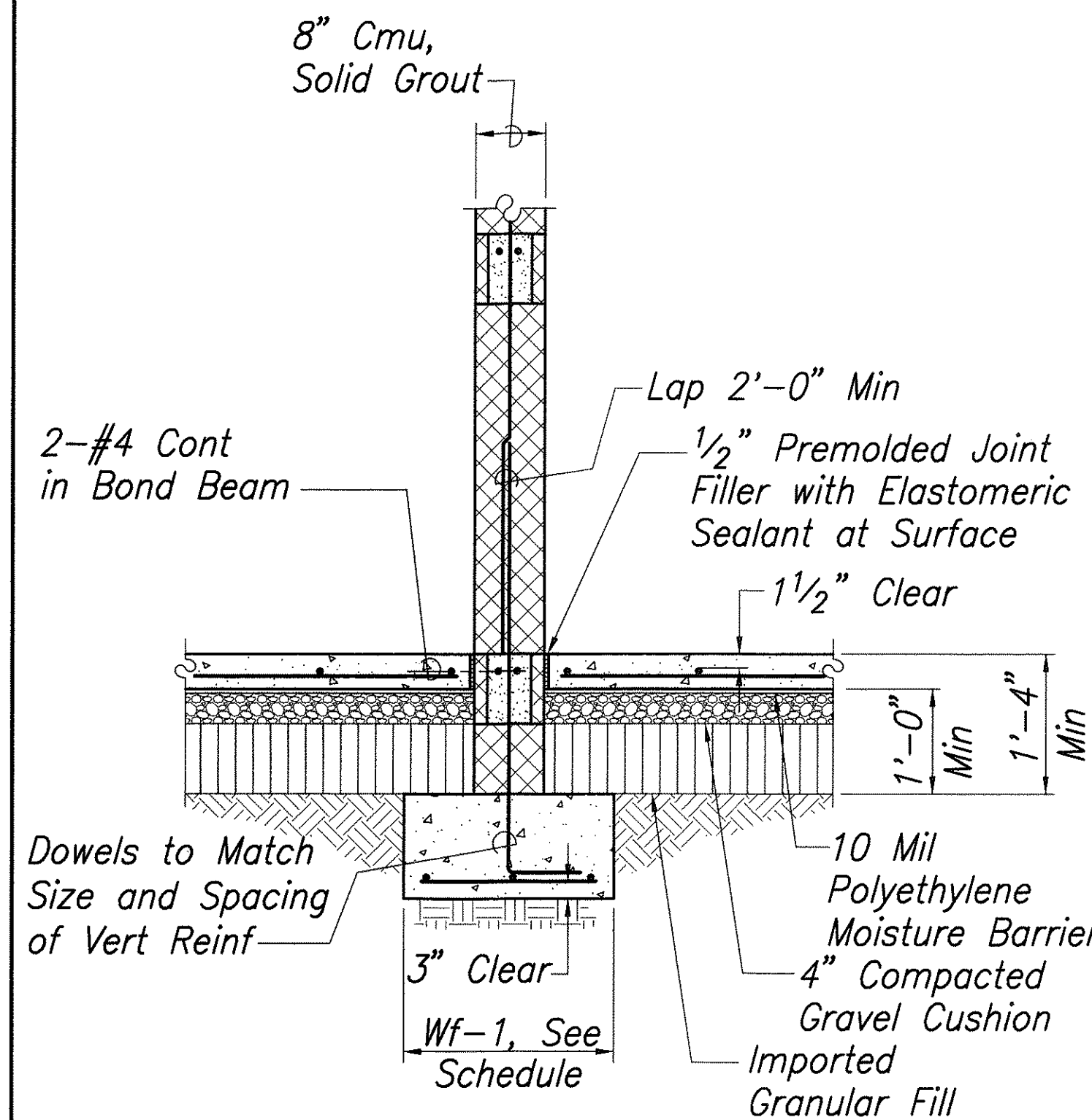
SCALE: AS NOTED      DATE: APRIL 2000

SHEET No. S4 OF 60 SHEETS

FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HWY-0-03-98	2000	43	60

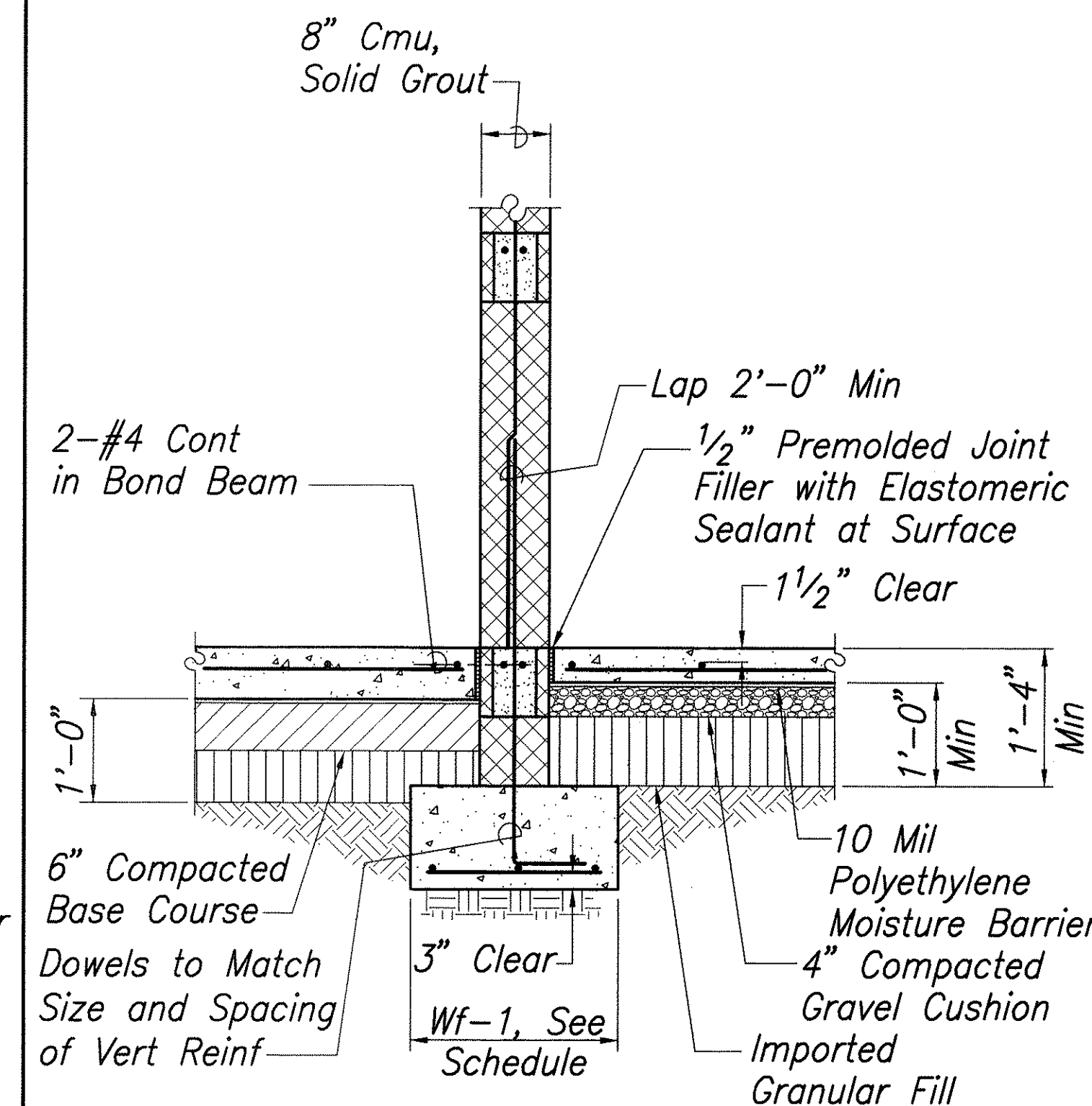


**SECTION A**  
Scale: 3/4" = 1'-0"  
S7/S5

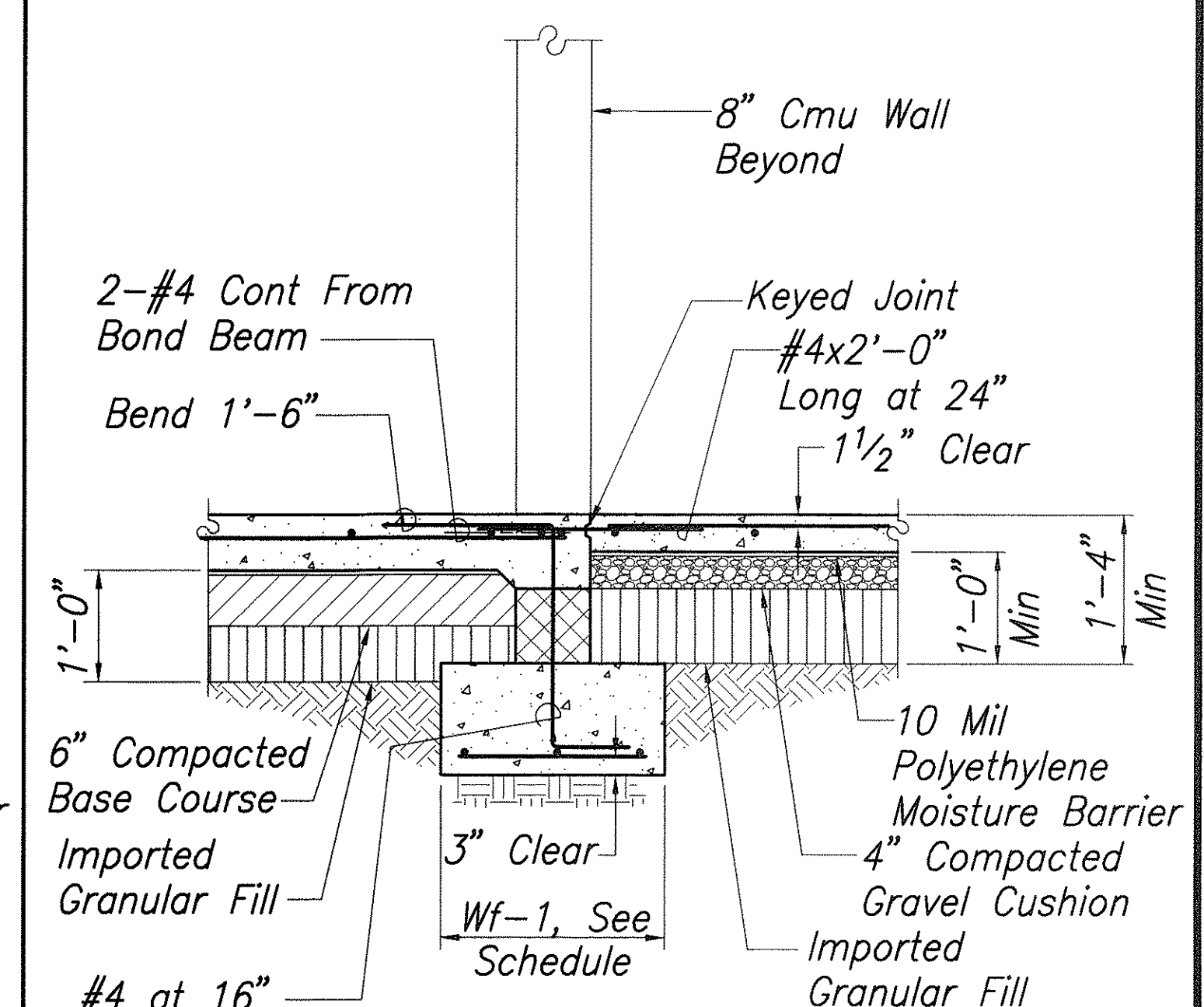


**SECTION B**  
Scale: 3/4" = 1'-0"  
S7/S5

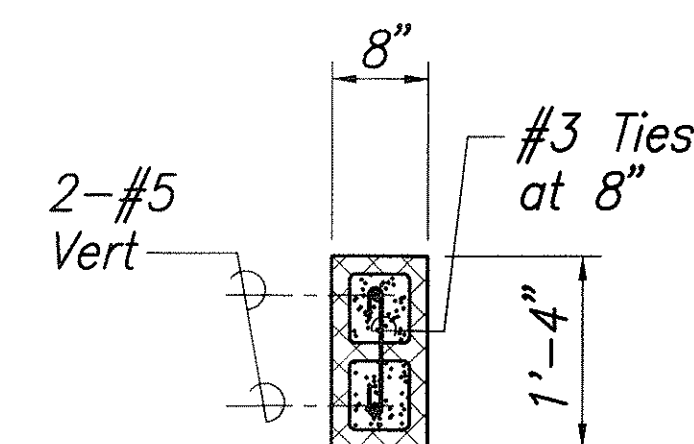
Note:  
See A/S3 for  
Cmu Wall  
Reinforcing



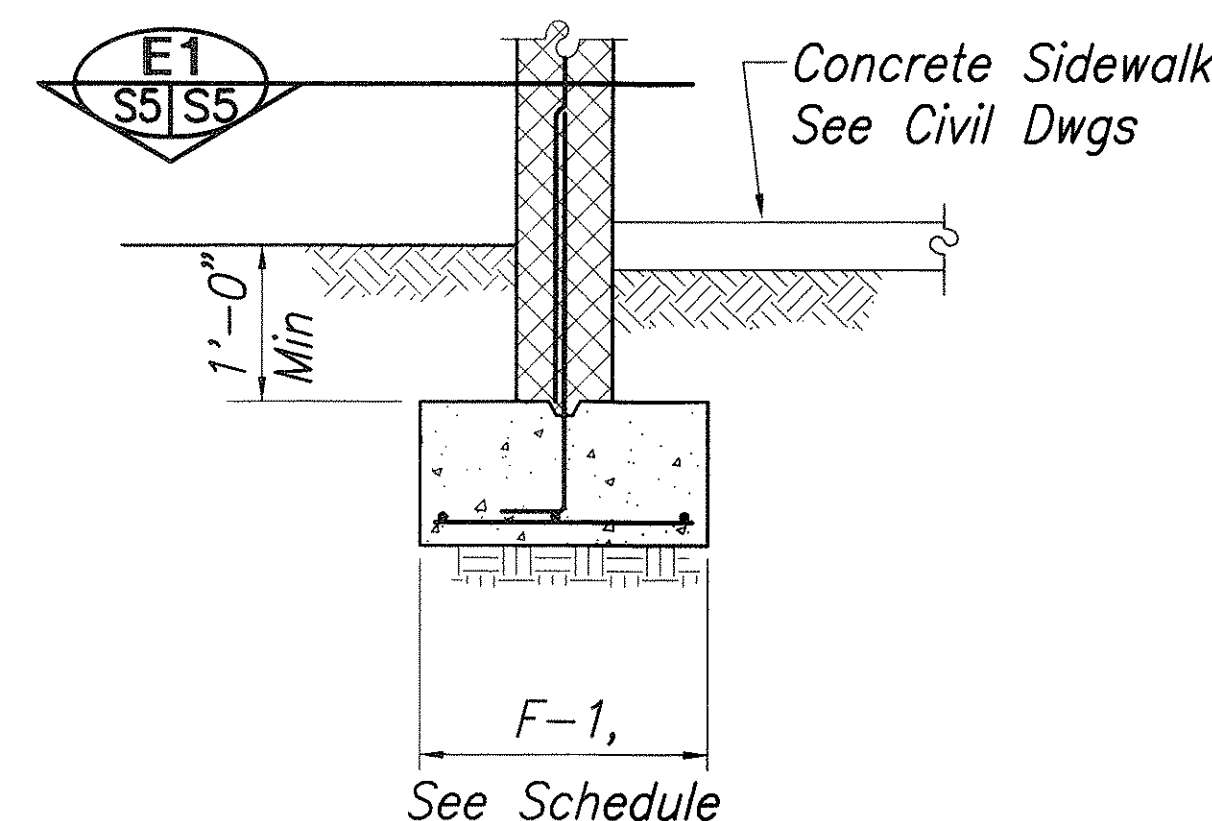
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S7/S5



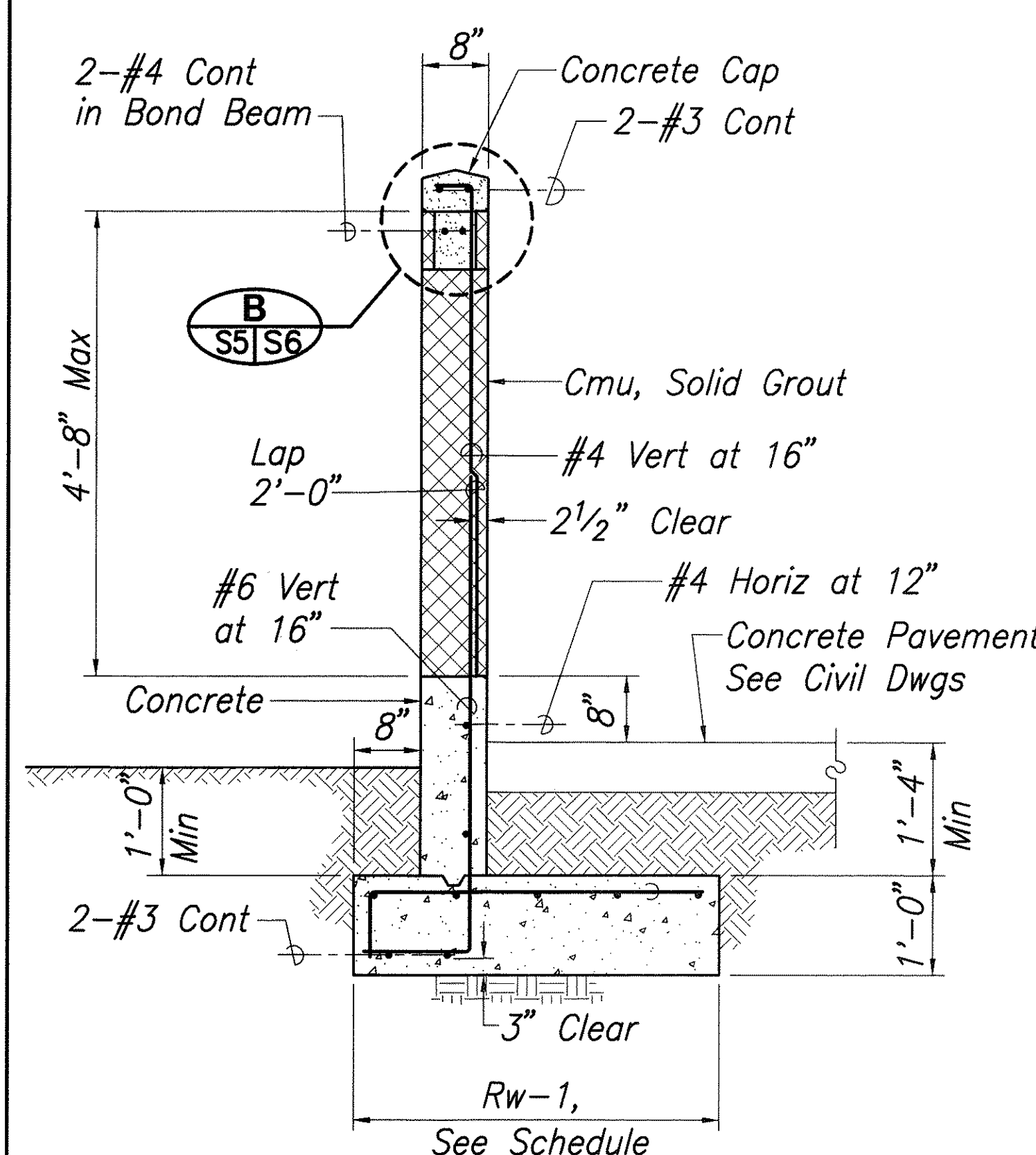
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S7/S5



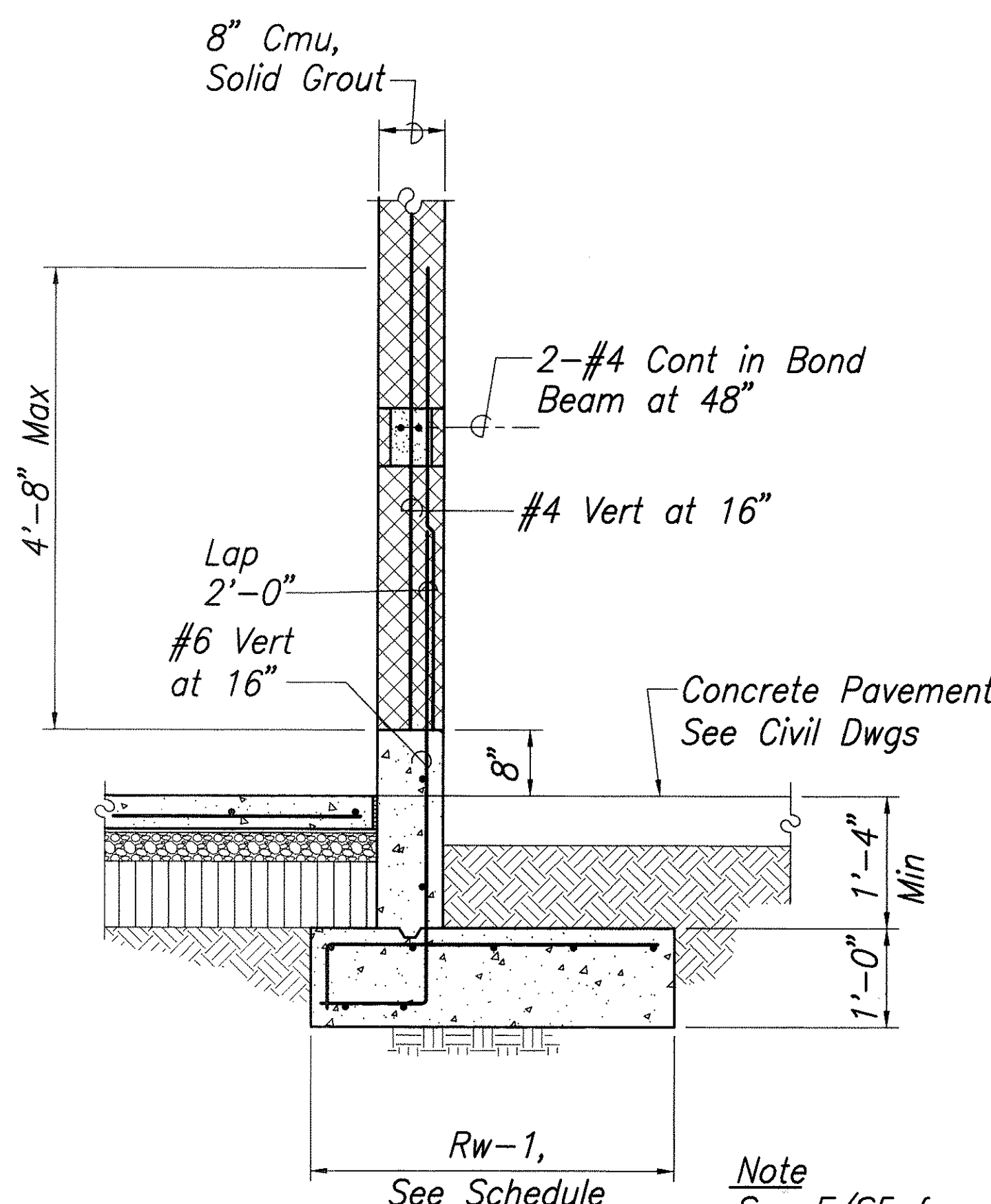
**SECTION E1**  
S5/S5



**DETAIL E**  
Scale: 3/4" = 1'-0"  
S7/S5



**SECTION F**  
Scale: 3/4" = 1'-0"  
S7/S5



**SECTION G**  
Scale: 3/4" = 1'-0"  
S7/S5

Note  
See F/S5 for  
Balance of  
Information



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S5

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION  
**FOUNDATION SECTIONS  
AND DETAILS**  
HAUULA BASEYARD  
IMPROVEMENTS  
Project No. HWY-0-03-98  
SCALE: AS NOTED DATE: APRIL 2000  
SHEET No. S5 OF 60 SHEETS



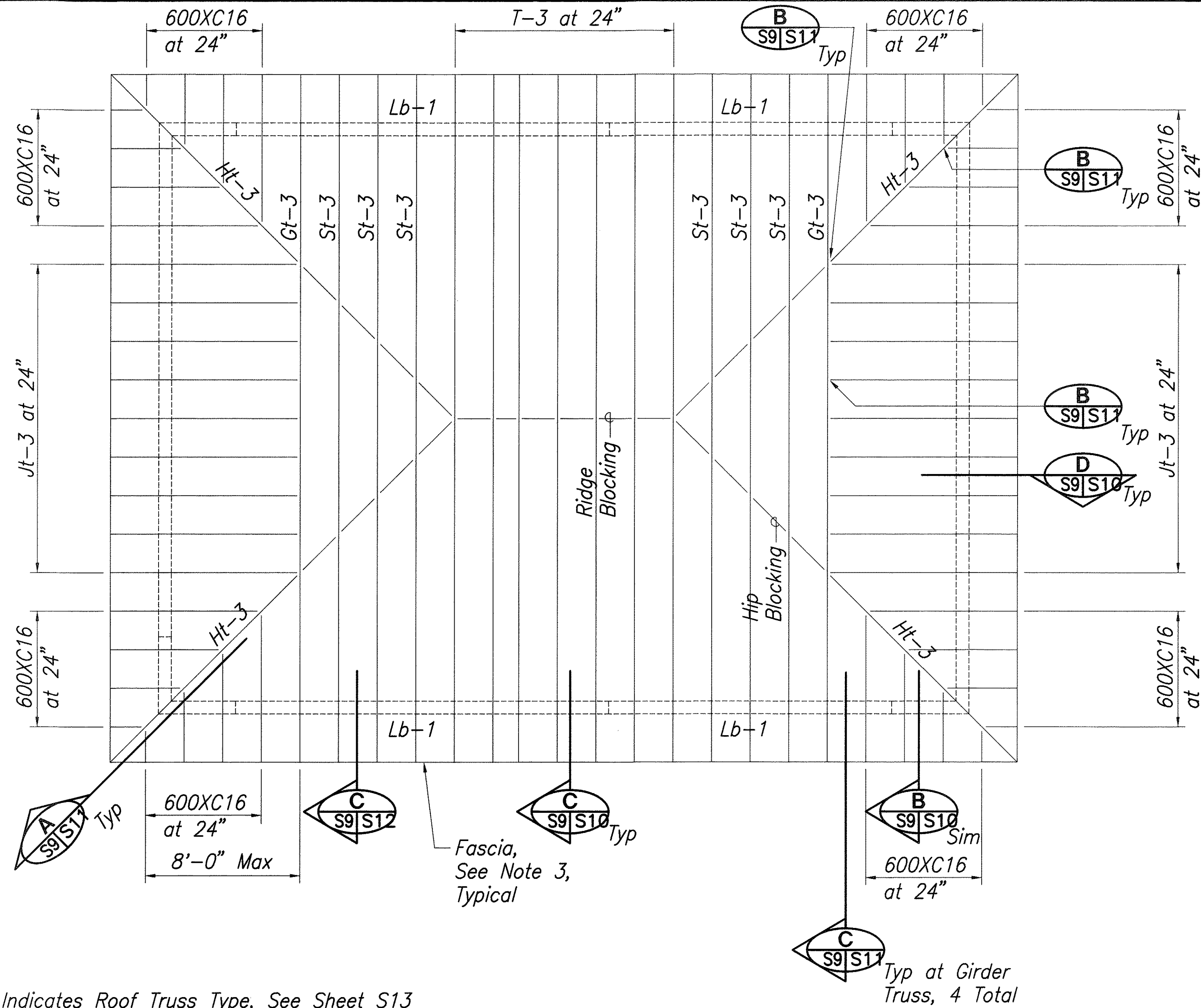








FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HWY-0-03-98	2000	47	60



Legend

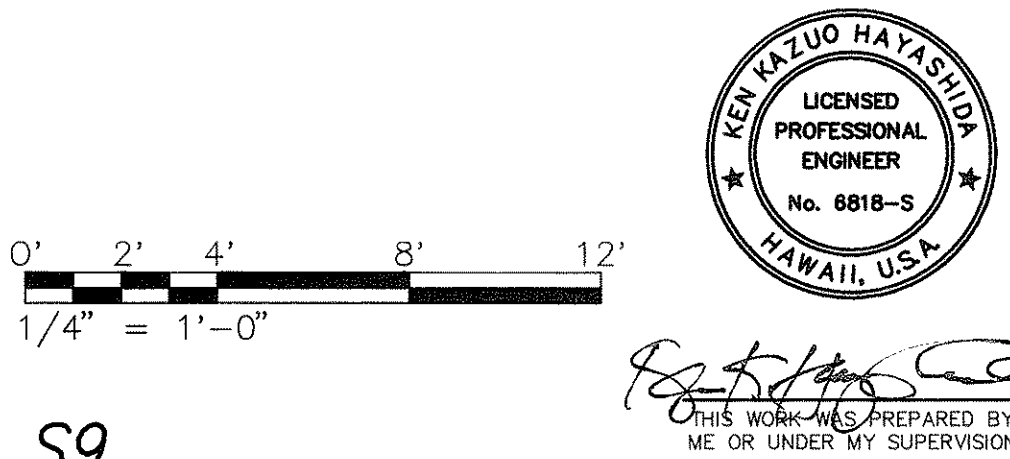
- T-1 Indicates Roof Truss Type, See Sheet S13  
St-1 Indicates Roof Step Down Truss Type, See Sheet S13  
Gt-1 Indicates Roof Girder Truss Type, See Sheet S13  
Ht-1 Indicates Roof Hip Truss Type, See Sheet S13  
Jt-1 Indicates Roof Jack Truss Type, See Sheet S13  
Indicates 8" cmu wall below  
Lb-1 Indicates Cmu Lintel Type

Notes

1. Roof sheathing shall be  $\frac{3}{4}$ " plywood thruout, See E/S3
2. See Sheet S8 for Low Roof Framing Plan.
3. Fascia Channel Shall be Continuous with no Splices  
Allowed, within 10'-0" from Corners.

HIGH ROOF FRAMING PLAN

Scale:  $\frac{1}{4}" = 1'-0"$

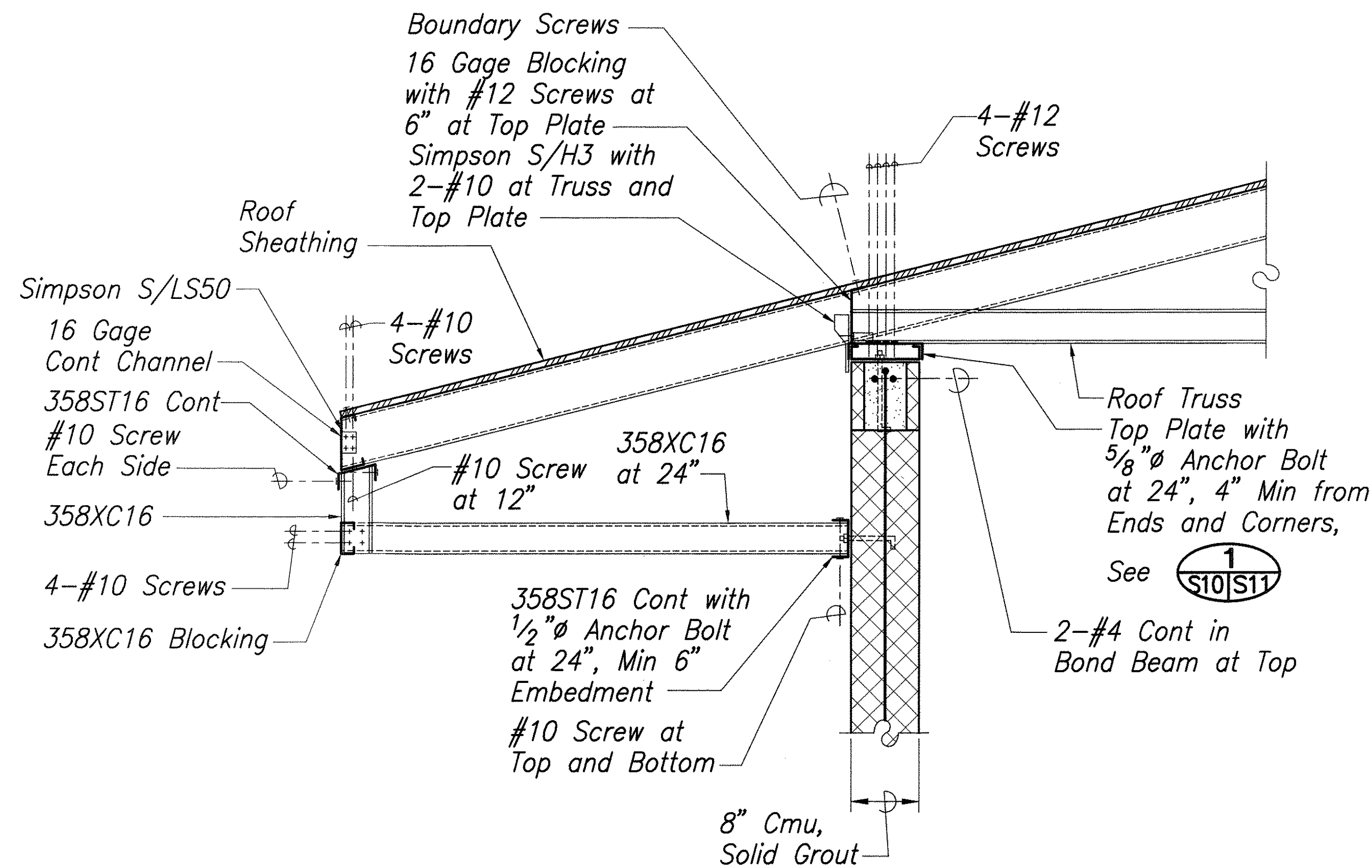


S9

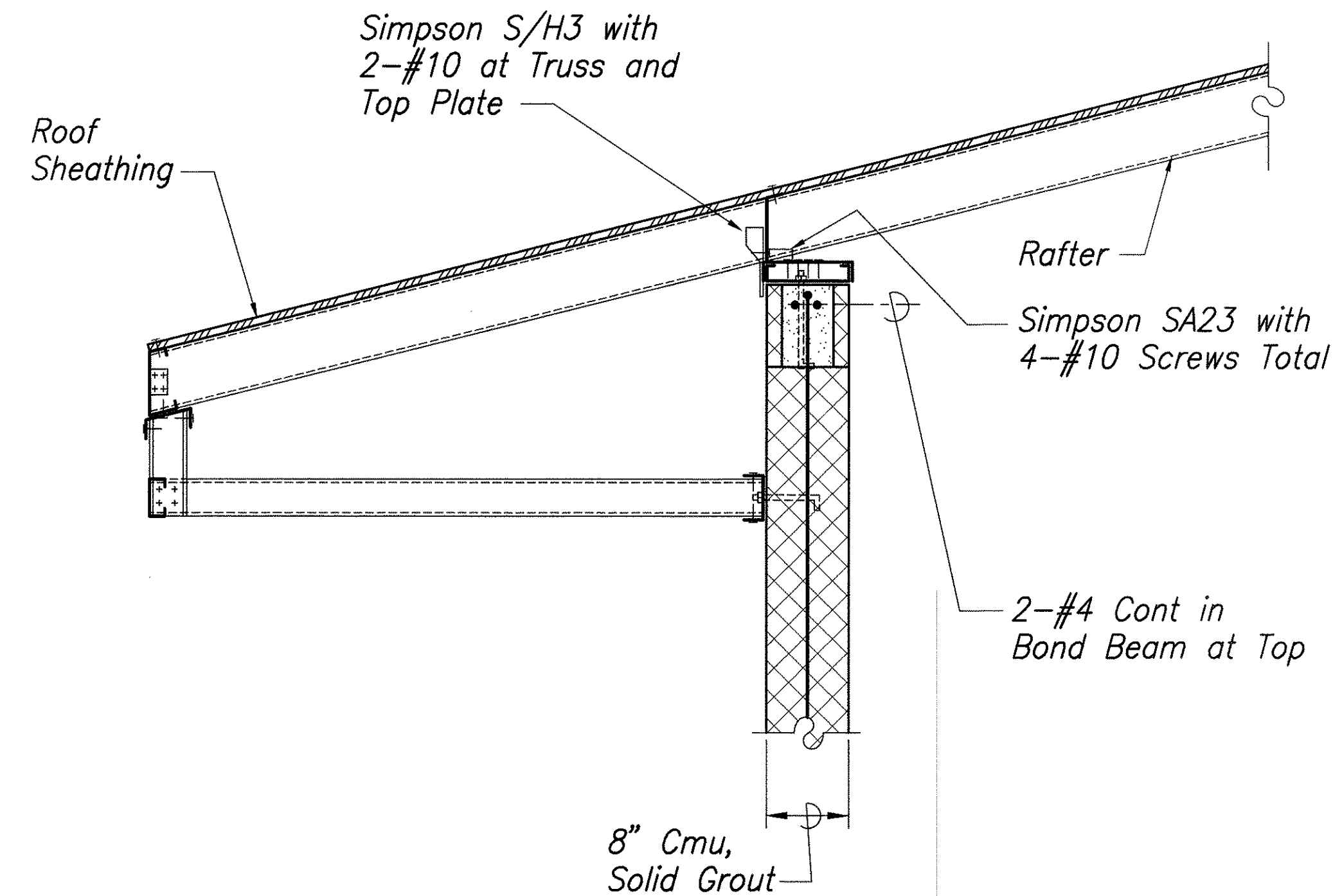
STATE OF HAWAII	
DEPARTMENT OF TRANSPORTATION	
HIGHWAYS DIVISION	
<u>HIGH ROOF FRAMING PLAN</u>	
HAUULA BASEYARD	
IMPROVEMENTS	
Project No. HWY-0-03-98	
SCALE: AS NOTED	DATE: APRIL 2000
SHEET No. S9 OF 60 SHEETS	



FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HWY-0-03-98	2000	48	60

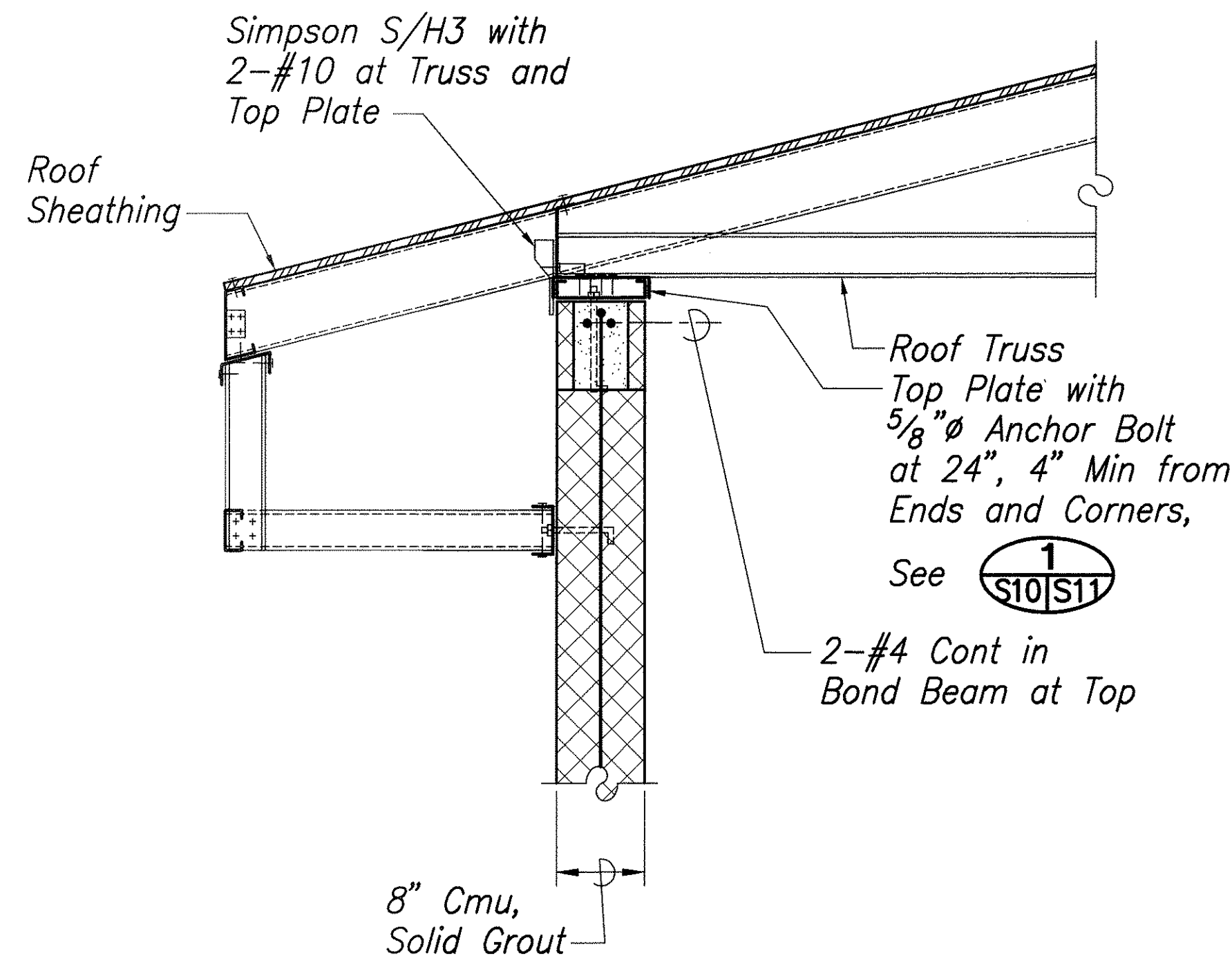


SECTION **A**  
Scale: 1" = 1'-0"



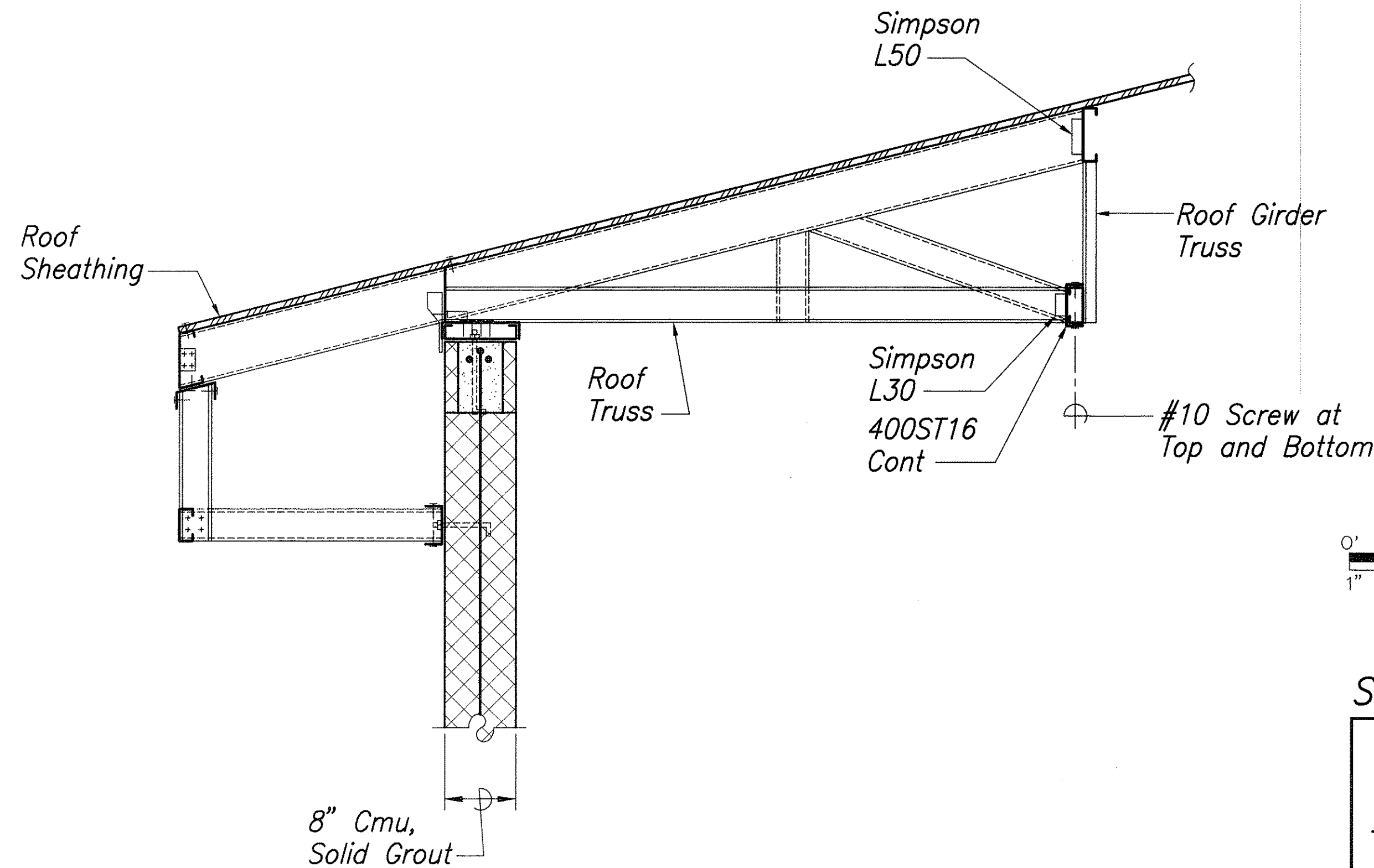
SECTION **B**  
Scale: 1" = 1'-0"

Note:  
See A/S10 for  
Balance of Detail



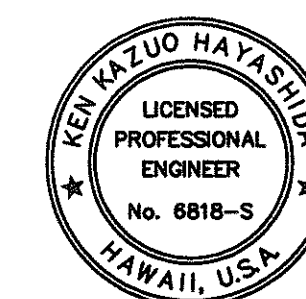
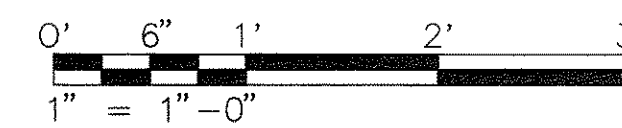
SECTION **C**  
Scale: 1" = 1'-0"

Note:  
See A/S10 for  
Balance of Detail



SECTION **D**  
Scale: 1" = 1'-0"

Note:  
See A/S10 for  
Balance of Detail



S10

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

ROOF SECTIONS AND DETAILS

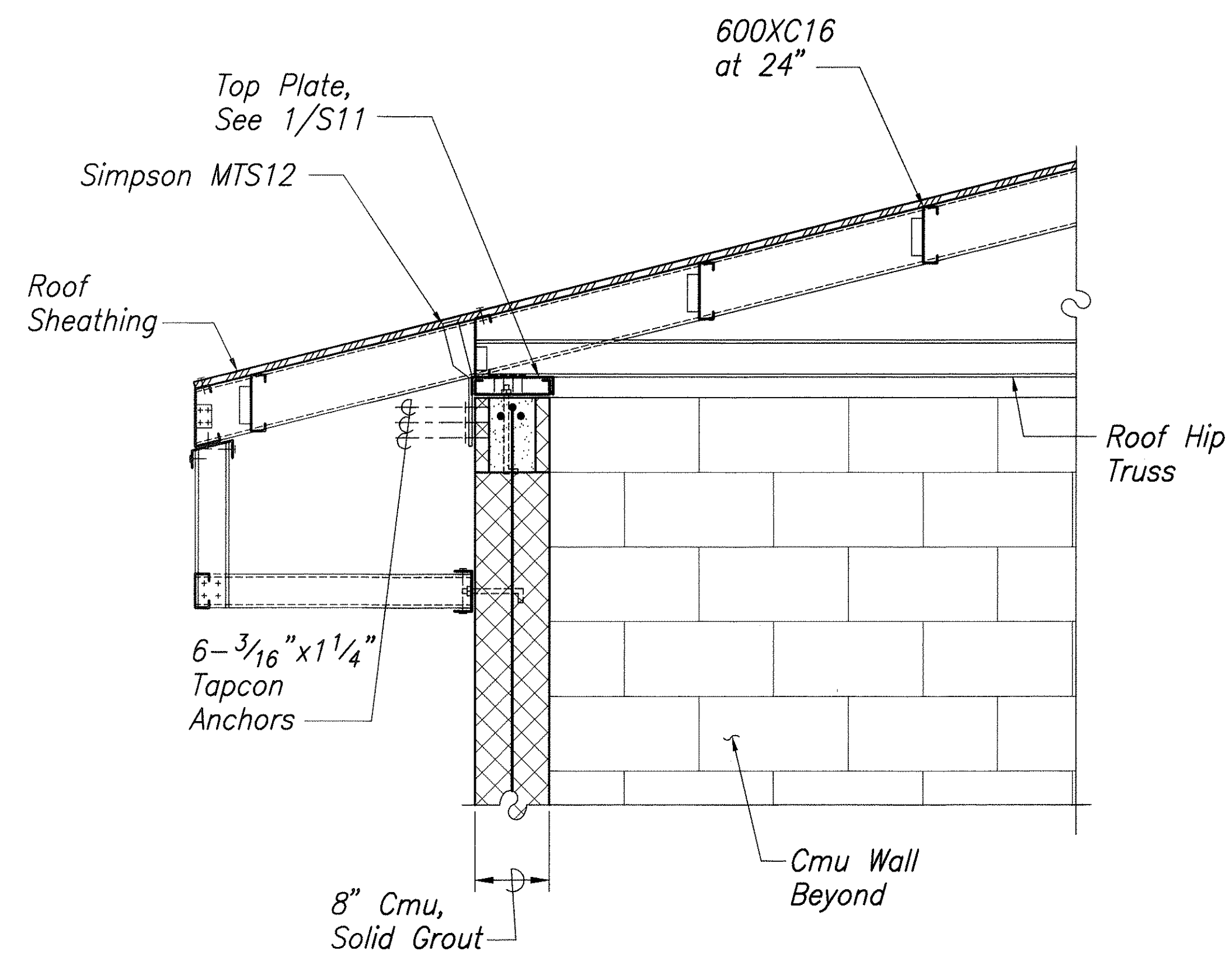
HAUULA BASEYARD  
IMPROVEMENTS  
Project No. HWY-0-03-98

SCALE: AS NOTED DATE: APRIL 2000

SHEET No. S10 OF 60 SHEETS

SURVEY PLOTTED BY	DATE
DRAWN BY	
CHECKED BY	
NOTED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
No.	

	FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
	HAWAII	HAW.	HWY-0-03-98	2000	49	60



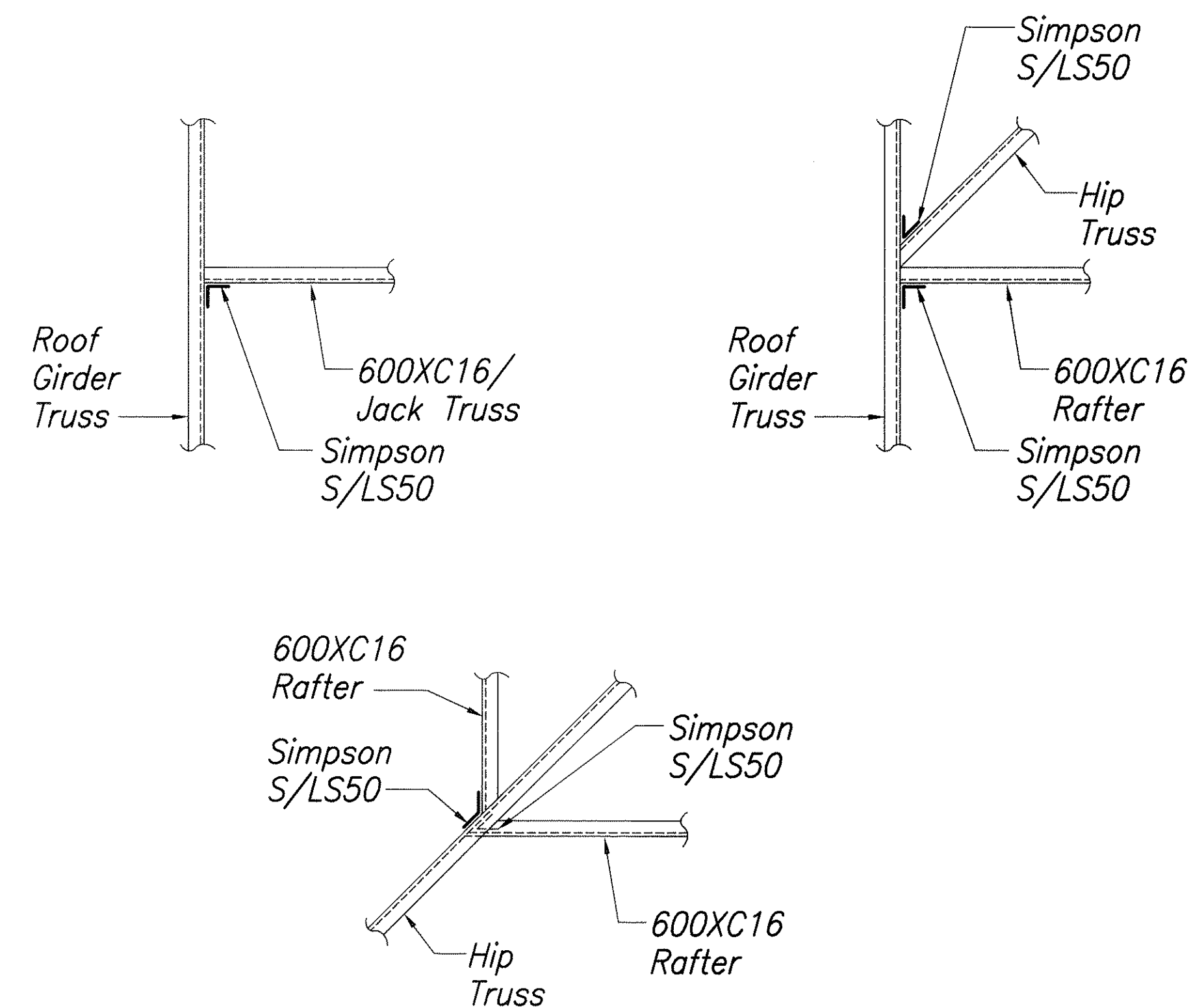
**SECTION**

*Scale: 1" = 1'-0"*

**A**

S8, S9 | S11

Note:  
See A/S10 for  
Balance of Detail

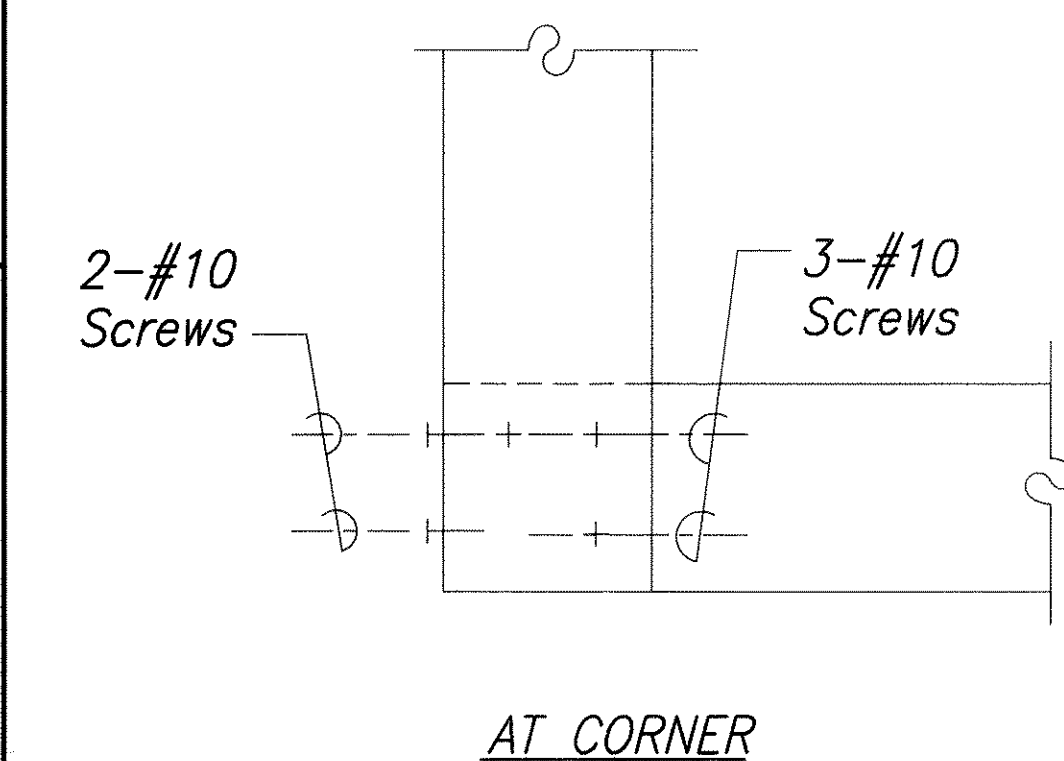
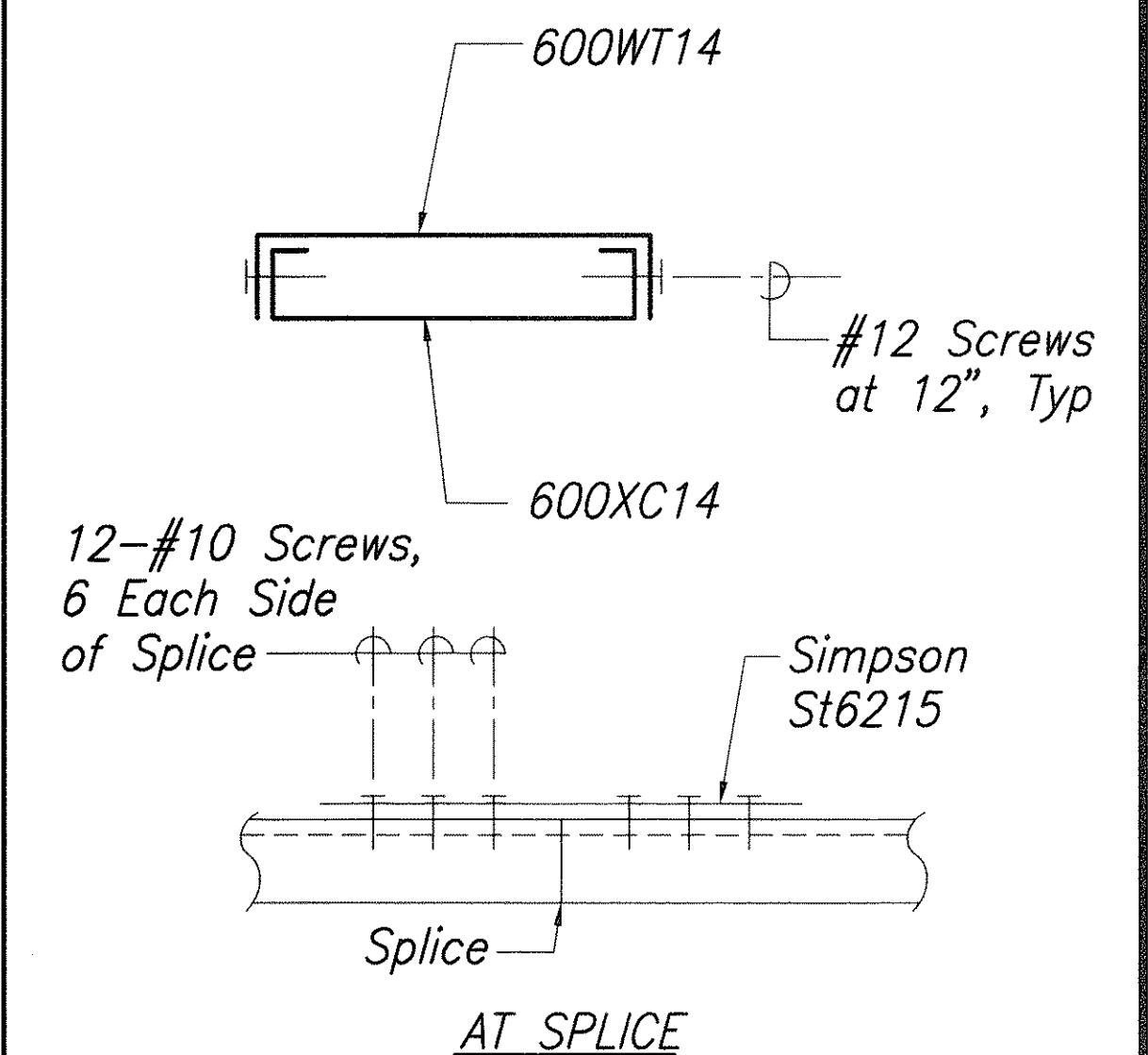


**TYPICAL RAFTER AND HIP CONNECTIONS**

Scale: 1" = 1'-0"

S8, S9, S11

Scale: 1" = 1'-0"



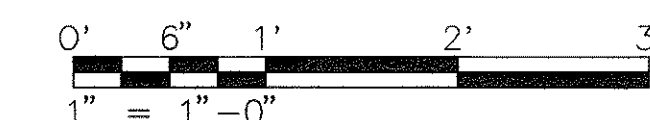
**TYPICAL TOP  
PLATE DETAIL**

*Scale: 3" = 1'-0"*

S10, S11, S12 | S11

1

Scale: 3" = 1'-0" S10, S11, S12 | S11



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S11

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

*ROOF SECTIONS AND DETAILS*

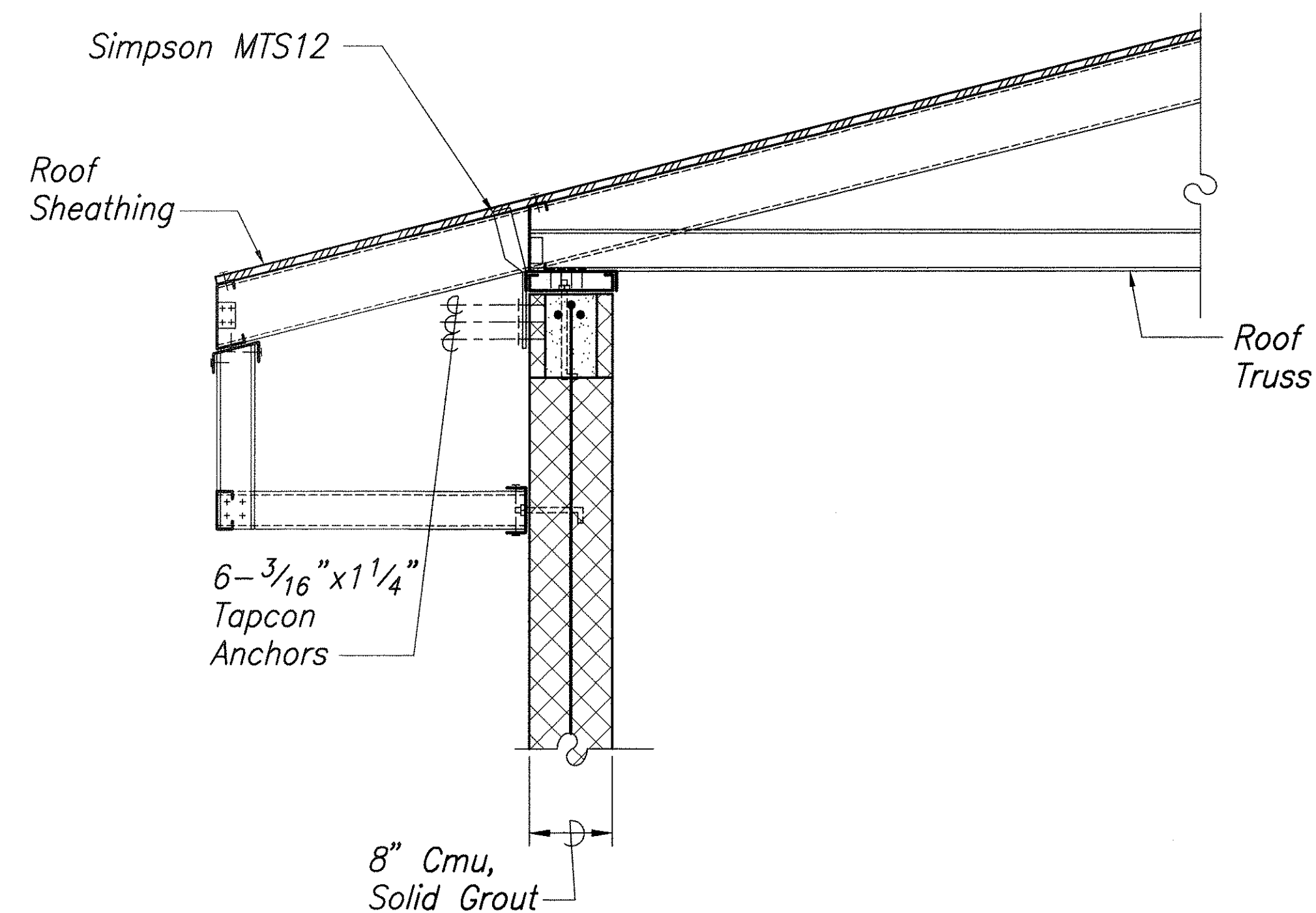
•

•

HAUULA BASEYARD  
IMPROVEMENTS  
*Project No. HWY-0-03-98*


SCALE: AS NOTED DATE: APRIL 2000

SHEET No. S11 OF 60 SHEETS

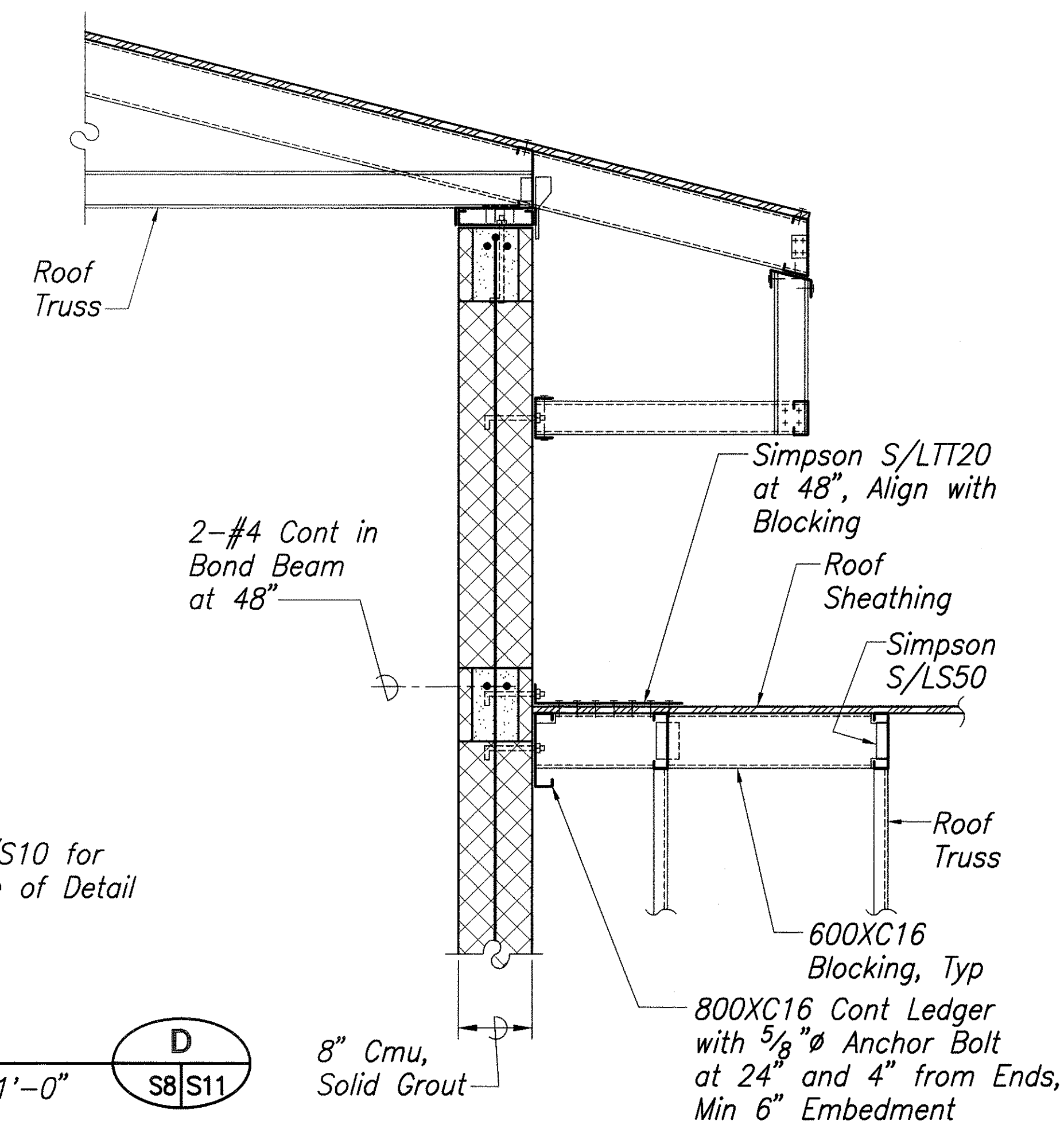


**SECTION**

Scale: 1" = 1'-0"

S9 — 

Note:  
See A/S10 for  
Balance of Detail



**SECTION**

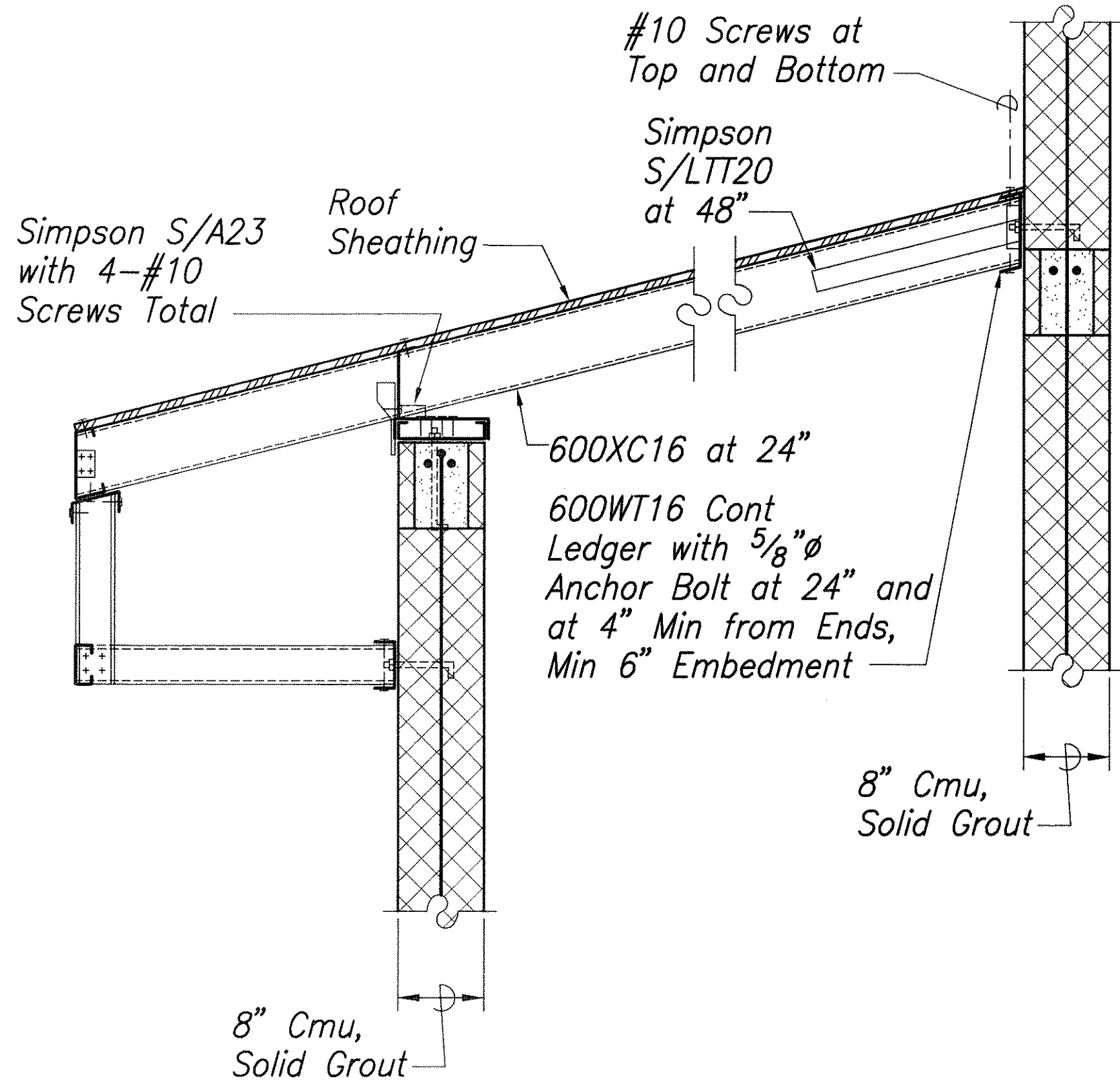
Scale: 1" = 1'-0"

Note:  
See A/S10 for  
Balance of Detail

ORIGINAL PLAN	SURVEY PLOTTED BY _____ DATE _____
NOTE BOOK	DRAWN BY _____
	TRACED BY _____
	DESIGNED BY _____
	QUANTITIES BY _____
No. _____	CHECKED BY _____



FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HWY-0-03-98	2000	50	60



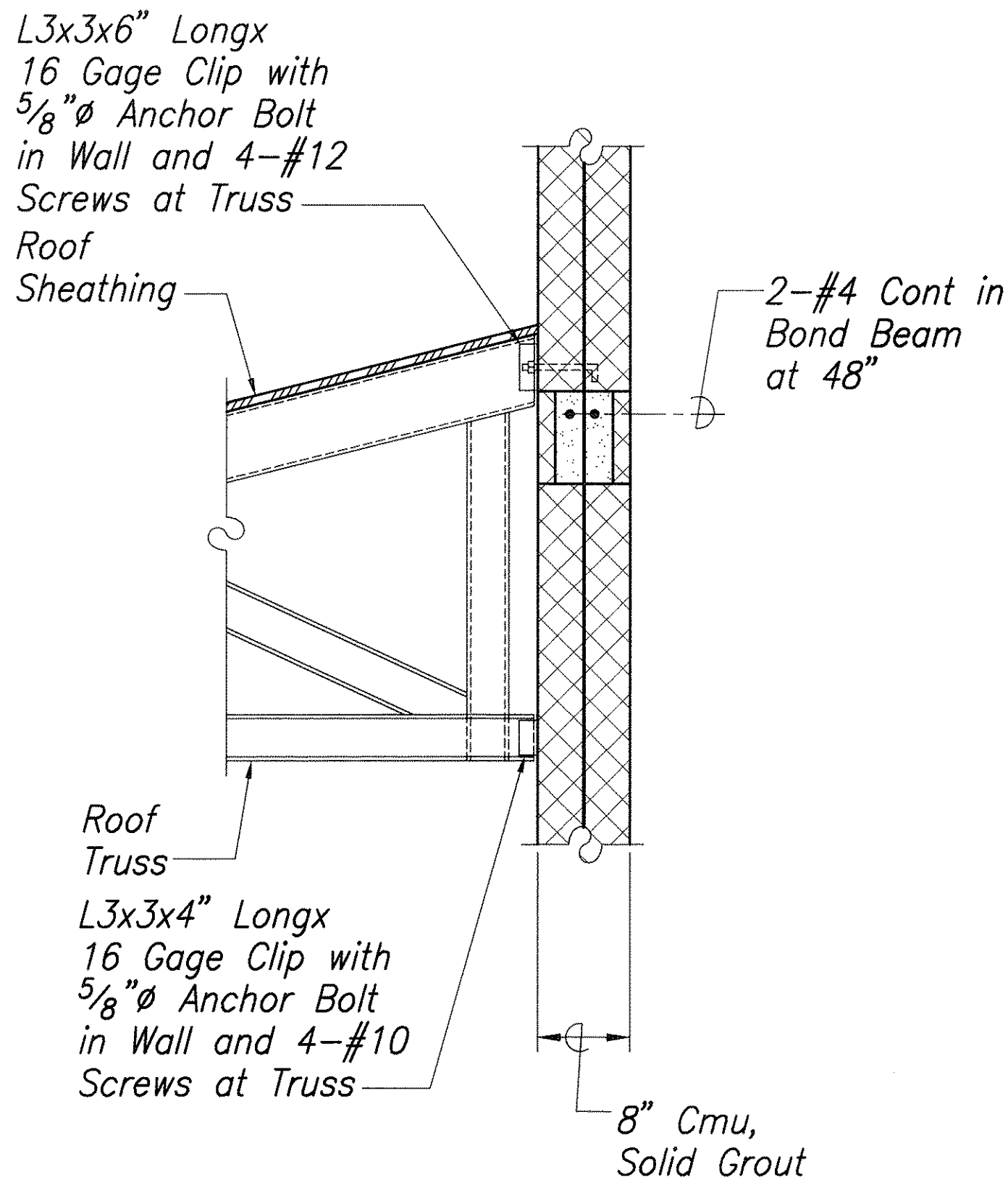
SECTION

Scale: 1" = 1'-0"

A  
S8/S12

Note:

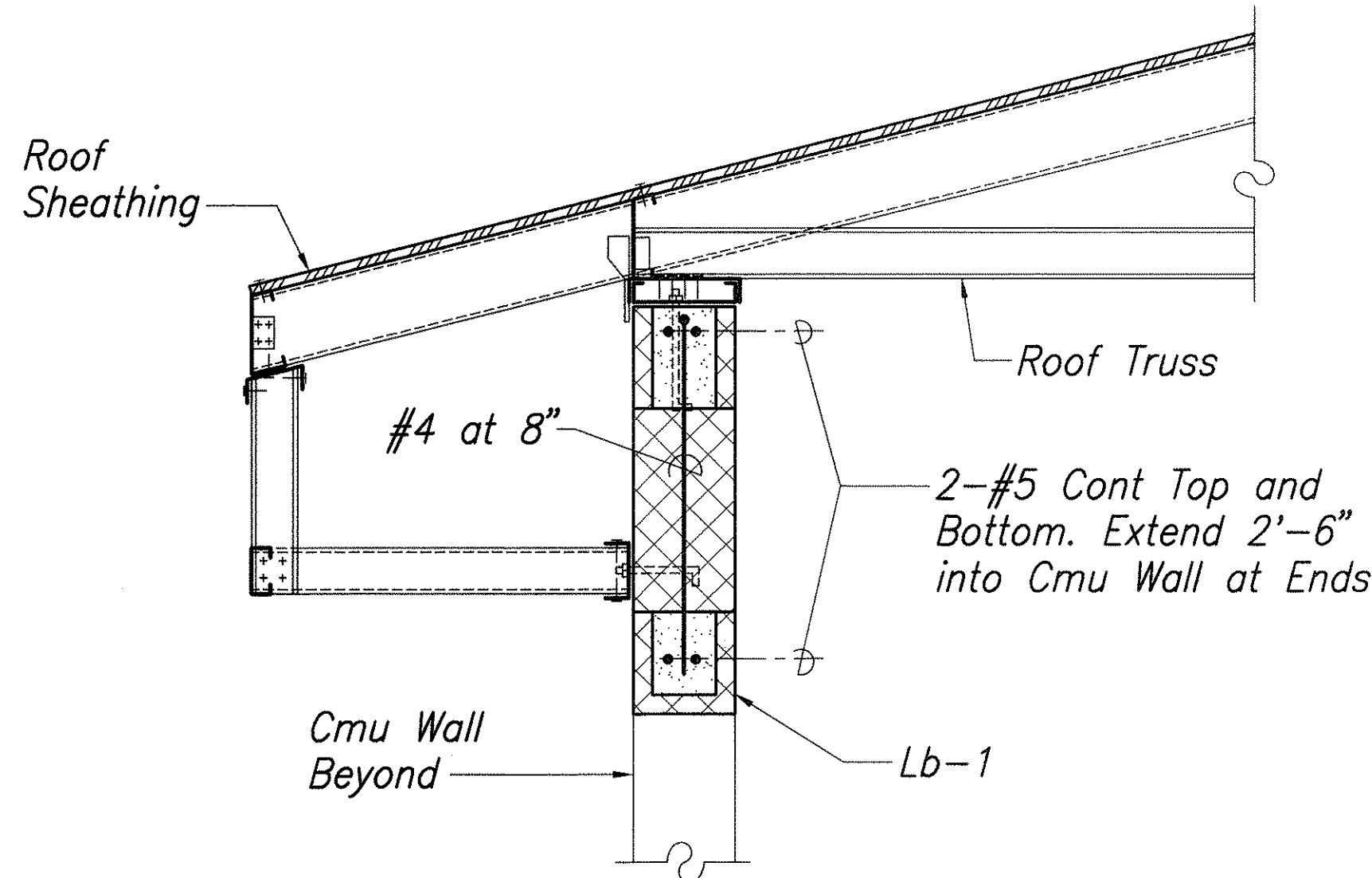
See A/S10 for  
Balance of Detail



SECTION

Scale: 1" = 1'-0"

B  
S8/S12



SECTION

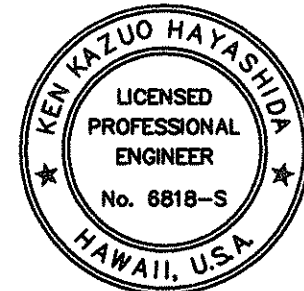
Scale: 1" = 1'-0"

C  
S9/S12

Note:

See A/S10 for  
Balance of Detail

SURVEY PLOTTED BY	DATE
DRAWN BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
No.	



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ME OR UNDER MY SUPERVISION

S12

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

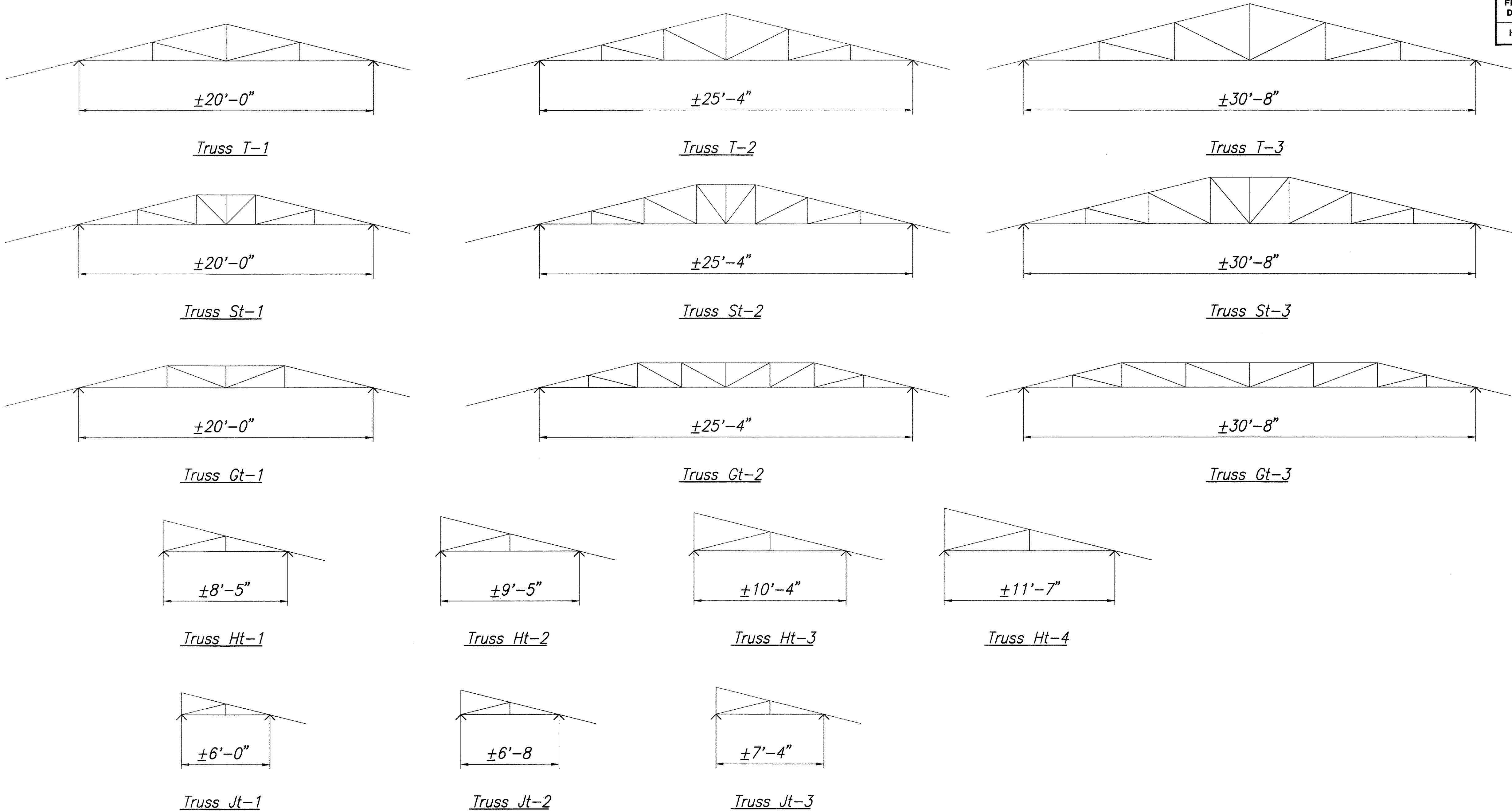
ROOF SECTIONS AND DETAILS

HAUULA BASEYARD  
IMPROVEMENTS  
Project No. HWY-0-03-98

SCALE: AS NOTED  
SHEET No. S12 OF 60 SHEETS

DATE: APRIL 2000

FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	HWY-0-03-98	2000	51	60



Pre-engineered Cold-Formed Steel Roof Trusses:

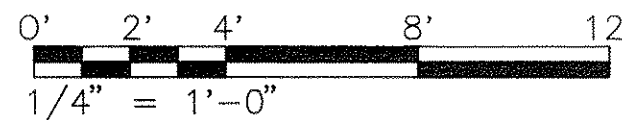
- All work shall comply with the "Specification For The Design Of Cold-Formed Steel Structural Members" of the American Iron and Steel Institute, Latest Edition.
- Cold-formed steel shall conform to the Metal Stud Manufacturers Association. All members 16, 14, and 12 gauge shall meet the minimum requirements of ASTM A653 Sq Grade 50. All members 20 and 18 gauge shall meet the minimum requirements of ASTM A653 Sq Grade 33. All members shall have a minimum G90 galvanized coating.
- The fabricator shall have a minimum of 3 years successful experience in the fabrication of prefabricated steel trusses, similar to the trusses required for this project. The fabricator shall have sufficient production capacity to produce, transport and deliver the required trusses without causing delay in the work.
- Structural calculations and shop drawings shall be submitted to the engineer for review and approval, prior to fabrication. Shop drawings shall include layout of trusses, truss configuration, member sizes, connection details, fabrication details and bracing requirements. Structural calculations shall be stamped by a structural engineer licensed to practice in the State of Hawaii.

- Design shall be based on the UBC or the following minimum criteria:
  - Roof dead load (not including self weight of trusses) 10 psf
  - Roof live load top chord 20 psf
  - Bottom chord live load 10 psf
  - Wind loads inward 8 psf
  - Wind loads outward 22 psf
  - Wind loads on overhang outward 64 psf
  - Top chord and bottom chord live load need not act concurrently.

- Wind loads along collector trusses are reversible. Loads along top chord shall be transferred to bottom chord through truss member connections.
- Member designations are per metal stud manufacturer's association. Truss members shall be minimum 16 gauge with minimum sizes as follows:
  - Top chord 600XC
  - Bottom chord 400XC
  - Web members 400XC

- Connection plates shall be the same gauge as thickest member being connected.

Not Concurrent



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S13

STATE OF HAWAII	
DEPARTMENT OF TRANSPORTATION	
HIGHWAYS DIVISION	
<b>TRUSS ELEVATIONS</b>	
HAUULA BASEYARD	
IMPROVEMENTS	
Project No. HWY-0-03-98	
SCALE: AS NOTED	DATE: APRIL 2000
SHEET No. S13 OF 60 SHEETS	