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Н	HI STP SR 30(1)	S1

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<u> </u>	539	TYPICAL DETAILS	RG3104-X	S65	STA. 482+12 - BOX CULVERT SECTION
	S40	TYPICAL DETAILS			LONGITUDINAL SECTION
	S41	BRIDGE CONSTRUCTION SEQUENCE	RG3104-Y	S66	WINGWALL ELEVATION AND REINFORC'G
	1		RG3104-Z	S67	WINGWALL SECTION AND SCHEDULE
	S42	LUAKOI STREAM BOX CULVERT	RG3104-AA	S68	WINGWALL SECTIONS
,		STA. 422+02 HONOAPIILANI HIGHWAY	RG3104-BB	S69	WINGWALL SECTIONS
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	S43	STA. 422+02 - BOX CULVERT INLET ELEV.	RG3104-DD	S71	STA. 434+30 - BOX CULVERT WINGWALL
	S44	STA. 422+02 - BOX CULVERT INCLT ELEV.	10000		A&B ELEVATION AND REINFORCING
	S44 S45	STA. 422+02 - BOX CULVERT BOTTOM	RG3104-EE	S72	STA. 434+30 - BOX CULVERT WINGWALL
_		SLAB REINFORCING PLAN			A&B ELEVATION AND REINFORCING
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	S48	STA. 422+02 - BOX CULVERT EXTERIOR SECTION	RG3104-HH	S75	STA. 434+30 - BOX CULVERT WINGWALL A&B SECTION
	549	STA. 422+02 - BOX CULVERT SECTION	RG3104-II	S76	RETAIN'G WALL SECTION AT STA. 482+12
1		LONGITUDINAL SECTION	RG3104-JJ	S77	RETAIN'G WALL SECTION AT STA. 482+12
	S50	LAUNIU POKO STREAM BOX CULVERT STA. 434+30 HONOAPIILANI HIGHWAY MAUI COUNTY HAWAII	RG3104-KK	S78	STA. 422+02 - BOX CULVERT BACKFILL PLAN
	S51	MAUI COUNTY, HAWAII STA. 434+30 - BOX CULVERT INLET	RG3104-LL	S79	STA. 434+30 - BOX CULVERT BACKFILL PLAN
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ļ	S54	STA. 434+30 - BOX CULVERT TOP SLAB REINFORCING PLAN	RG3104-QQ	S84 S85	BACKFILL ELEVATION AT WINGWALL C&D
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ļ	S55	STA. 434+30 - BOX CULVERT INTERIOR SECTION	RG3104-SS	S86	
	S56	STA. 434+30 - BOX CULVERT	RG3104-TT	S87	STA. 434+30 - WINGWALL A&B BACKFILI ELEVATION
	S57	EXTERIOR SECTION AT INLET STA. 434+30 - BOX CULVERT SECTION	RG3104-UU	588	STA. 434+30 - WINGWALL A&B BACKFILI SECTION
J	1	LONGITUDINAL SECTION	RG3104-VV	S89	BACKFILL DETAILS
1	S58	LAUNIUPOKO STREAM BOX CULVERT STA. 482+12 HONOAPIILANI HIGHWAY	RG3104-WW	S90	HEADWALL SECTION BACKELL TRANSITION SECTION
****	1	MAUI COUNTY, HAWAII	RG3104-XX	S91	BACKFILL TRANSITION SECTION PETAINING WALL BACKFILL SECTION
	S59	STA. 482+12 - BOX CULVERT INLET ELEV.	RG3104-YY	S92	RETAINING WALL BACKFILL SECTION AT STA. 482+12
	S60	STA, 482+12 - BOX CULVERT OUTLET ELEV.	RG3104-ZZ	S93	CONCRETE BARRIER ELEVATION
	S61	STA. 482+12 - BOX CULVERT BOTTOM	RG3104-AAA	594	CHAIN LINK FENCE DETAILS
1	1 '	SLAB REINFORCING PLAN	RG3104-BBB	595	CONCRETE BARRIER AND STRUCTURE
+	S62	STA. 482+12 - BOX CULVERT TOP SLAB REINFORCING PLAN			TRANSITION RAILING
	S63	STA. 482+12 - BOX CULVERT	RG3104-CCC	S96	CONCRETE BARRIER TRANSITION RAILING
1	1	EXTERIOR SECTION	RG3104-DDD	S97	TRANSITION SECTIONS
-+	S64	STA. 482+12 - BOX CULVERT	RG3104-EEE	S98	CONCRETE BARRIER TRANSITION
J	, ,	END SECTION	1,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		PAILING NOTES AND DETAILS

RAILING NOTES AND DETAILS

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RG3104-HHH	S101	BOX CULVERT TYPICAL DETAILS						
RG3104-III	S102	BOX CULVERT TYPICAL DETAILS						
RG3104-JJJ	S103	HIGHWAY LIGHT STANDARD FOUNDATION						



THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

4-30-18

EXPIRATION DATE OF THE LICENSE

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

> LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

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								E. MATSUMOTO	C. TANABE	D. FUJIWARA	NO SCALE	D. FUJIWARA	1 of 103	JUNE 1, 2017	RG3103-A

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DESCRIPTION

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BORING LOGS-1

ABUTMENT PLAN

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STRUCTURAL GENERAL NOTES

STRUCTURAL GENERAL NOTES

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WINGWALL (MAKAI) ELEVATIONS

ABUTMENT WALL SECTION

ABUTMENT WALL SECTION

ABUTMENT WALL SECTION

DECK REINFORCING PLAN

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POST TENSIONING NOTES AND

ANCHORAGE SYSTEM SECTIONS AND

LAHAINA BYPASS ROAD - BRIDGE RAIL'G

& STRUCTURE TRANSITION RAILING

LAHAINA BYPASS ROAD - BRIDGE

PUNAKEA STREET - END POST &

STRUCTURE TRANSITION RAILING

PUNAKEA STREET - END POST SECTIONS

PUNAKEA STREET - CONCRETE BARRIER

END POST & STRUCTURE TRANSITION

STRUCTURE TRANSITION RAILING

STRUCTURE TRANSITION RAILING NOTES

WINGWALL SECTION WINGWALL SECTION

WINGWALL SECTION TYPICAL BRIDGE SECTION

CAMBER DIAGRAM

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RAILING SECTIONS

& END POST SECTIONS

RAILING

DETAILS

AND DETAILS

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TYPICAL DETAILS

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RG3104-K

RG3104-L

RG3104-M

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RG3104-0

RG3104-P

RG3104-R

RG3104-S

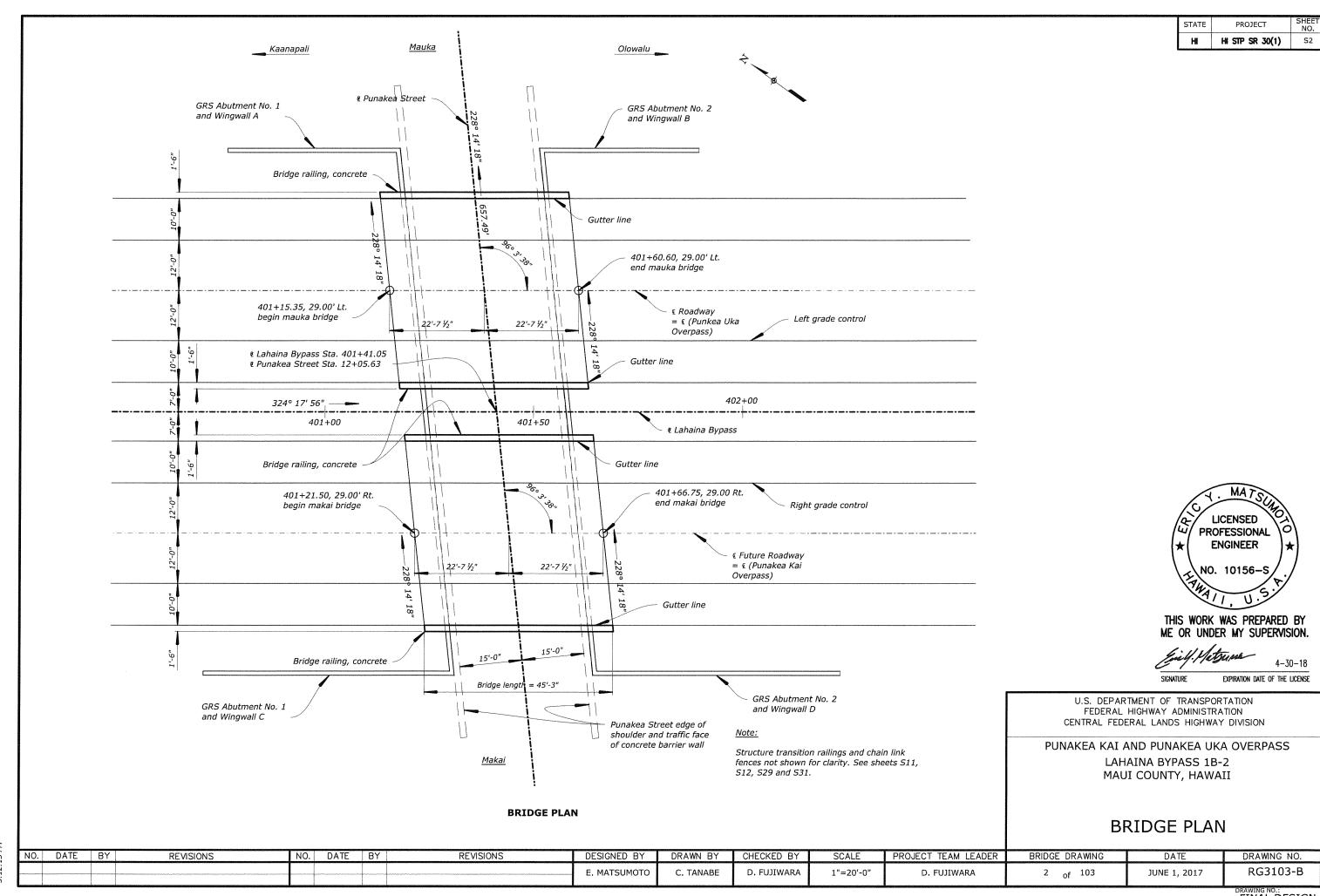
RG3104-T

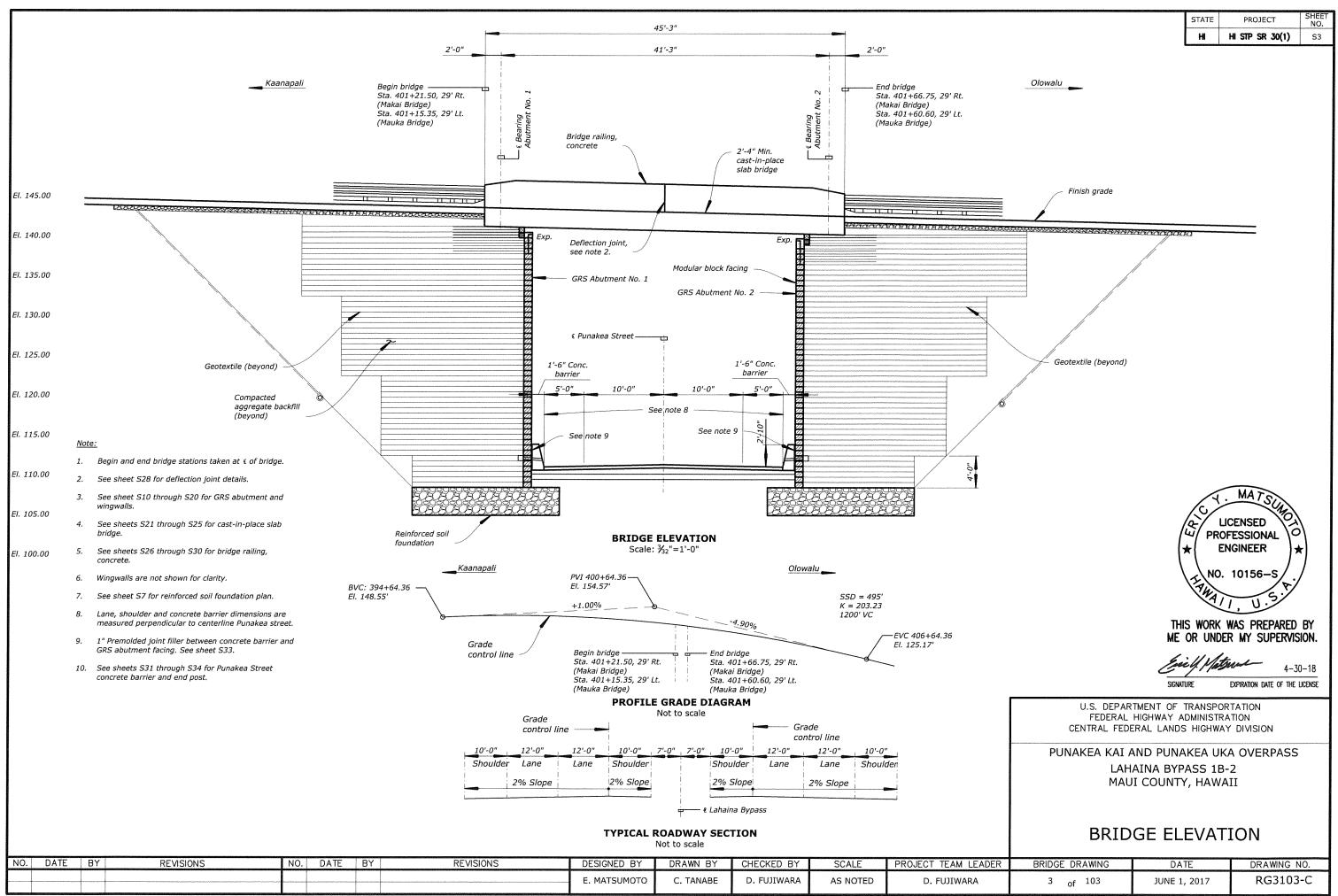
RG3104-U

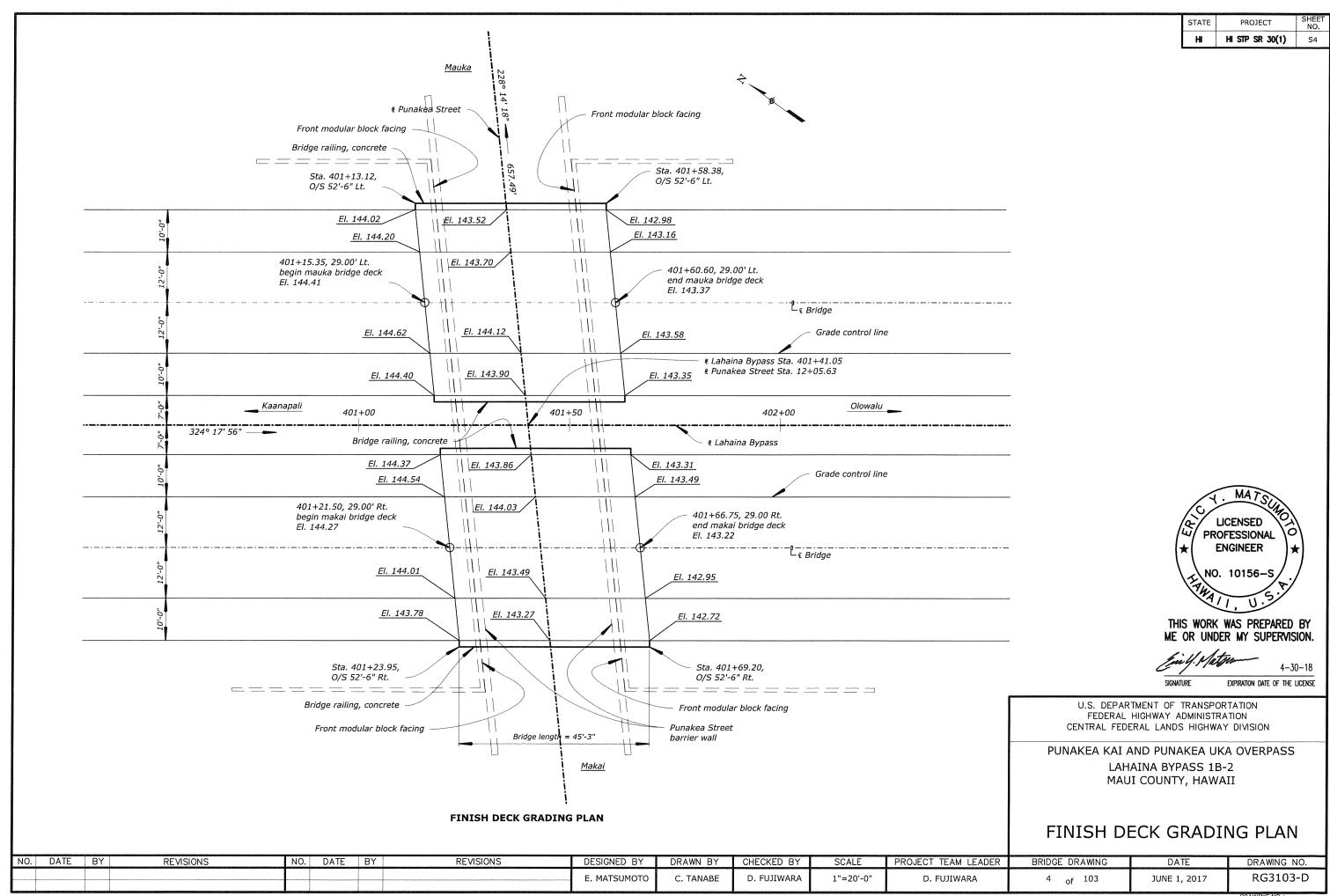
RG3104-V

RG3104-W

END SECTION







STRUCTURAL GENERAL NOTES

5. MATERIALS: (Continued)

H. Structural steel:

- (1) All structural steel shapes and plates shall conform to ASTM A 36 hot-dip galvanized, unless otherwise noted.
- (2) All anchor bolts, threaded rods, and other hardware, including nuts and washers, which connect steel to concrete shall be high-strength bolts conforming to ASTM F1554. Grade 105, unless otherwise noted.
- (3) All structural steel shapes and plates shall be ASTM A123 Hot-Dipped galvanized after fabrication. Bolts, anchor bolts, nuts, and washers shall be ASTM A153 Hot-Dip galvanized.
- (4) All welds shall be in conformity with the structural welding code AWS D1.1 or AWS D1.5. Electrodes shall be E70.
- I. Expanded polystyrene shall be Type XI EPS per ASTM C578 with density of 0.70 lb/cu ft. and a maximum compressive resistance of 5 psi at 10% deformation.
- J. Premolded joint filler shall be ASTM D1751 with a minimum compressive strength of 100 psi at 50% deformation.

6. REINFORCEMENT:

- A. The covering measured from the surface of the concrete to the face of any reinforcing bars shall be as follows, except as otherwise shown:
- (1) Top of deck slab = $2\frac{3}{4}$ " Bottom of deck = $1\frac{1}{2}$ "
- (2) Bridge railings, concrete barriers and end posts = 2"

(3) Box culvert

(a) Top slab top reinforcing = 2" (b) Top slab bottom reinforcing = 2"

(c) Walls = 2"

(d) Bottom slab top reinforcing $= 2^n$ (e) Bottom slab bottom reinforcing $= 3^n$

- (4) Concrete cast against and permanently exposed to earth = 3"
- (5) All others unless otherwise noted = 2"
- B. Reinforcing bars shall be detailed in accordance with the latest edition of the American Concrete Institute (ACI) Detailing Manual unless otherwise noted.
- C. Unless otherwise noted, reinforcing splices shall be staggered. Minimum distance between staggered lap splice shall be equal to the length required for the lap splice. Number of bars spliced at sections normal to axis of member shall not exceed 33 percent of the total main reinforcing steel in the member.
- D. Minimum lap splice length for steel reinforcing shall be 40 bar diameters or 2'-0", whichever is greater, unless otherwise noted. Increase lap length by multiplying the minimum lap splice length by 1.3 for bars having more than 12" concrete below bars.
- E. Minimum clear spacing between parallel bars in a horizontal or vertical layer shall be 1 1/2 times the maximum size of the coarse aggregate or 1 1/2", whichever is greater. Bundled bars shall be in close contact for the length of the bundles.
- F. All dimensions relating to reinforcing bars are to centers of bars unless otherwise noted
- G. Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of the intersections is less than 12 inches in each direction, in which case alternate intersections shall be tied.
- H. Dissimilar metals shall be separated at contact points using teflon tape.
- I. Strands for prestressing shall be Grade 270, seven wire, low relaxation type conforming to ASTM A416.
- Glass Fiber Reinforced Polymer (GFRP) reinforcing bars shall have a minimum elastic modulus of 8,800,000 psi and shall have a guaranteed minimum tensile strength of 141 ksi for #4 bar, 163 ksi for #5 bar, and 150 ksi for #6 bar.
- K. Tie wire used for GFRP reinforcing shall be alloy 302 or 304 stainless steel or

- L. All work for GFRP bars including materials and bends shall follow manufacturer's recommendations.
- M. GFRP bars may be cut in the field with a masonry or diamond blade.
- N. Smooth GFRP dowels shall be Hughes Brothers Aslan 600 GFRP dowels or approved
- O. Minimum lap length for GFRP reinforcing shall be 42 bar diameter or 2'-6", whichever is greater, unless otherwise noted.

7. CONSTRUCTION NOTES:

- A. See FP-14 and Special Contract Requirements.
- B. The contractor shall verify all dimensions and site conditions and shall report any discrepancies in writing to the Contracting Officer (CO) before commencing work or ordering materials.
- C. The contractor shall be solely responsible for the protection of adjacent properties, utilities and existing and new structures from damage due to construction.
- D. The contractor shall verify the location of all utility lines and notify the respective owners before commencing with excavation, and any temporary piling or sheeting.
- E. All vertical dimensions are measured plumb, unless otherwise noted.
- F. Construction joints may be relocated or additional ones added subject to the approval of the CO.
- G. All exposed concrete edges shall be chamfered 3/4" x 3/4", unless otherwise noted.
- H. Class 2 rubbed surface finish shall be applied to the surfaces of all new concrete railings, concrete barriers and walls to at least 12 inches below finished ground.
- I. Location of drilled holes shown in plans are approximate. Prior to placing holes in concrete, the contractor shall locate all reinforcing steel and adjust the location of the holes to clear all reinforcing bars. Final hole locations are subject to the approval of the CO.
- Construction joints between separate pours shall not be feather edged. All joints shall be made square and vertical for the entire thickness of the new pour prior to pouring, except as directed by the CO.
- K. Construction joints shall be roughened to $\frac{1}{4}$ " amplitude.



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SIGNATURE EXPIRATION DATE OF THE LICENSE

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

STATE

Н

PROJECT

HI STP SR 30(1)

NO.

S5

LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

STRUCTURAL GENERAL NOTES

NO. DATE BY REVISIONS NO. DATE BY REVISIONS DESIGNED BY DRAWN BY CHECKED BY SCALE PROJECT TEAM LEADER BRIDGE DRAWING DATE DRAWING NO. E. MATSUMOTO C. TANABE D. FUJIWARA NO SCALE D. FUJIWARA 5 of 103 JUNE 1, 2017 RG3103-E	8															
E. MATSUMOTO C. TANABE D. FUJIWARA NO SCALE D. FUJIWARA 5 of 103 JUNE 1, 2017 RG3103-E	NO	. DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
									E. MATSUMOTO	C. TANABE	D. FUJIWARA	NO SCALE	D. FUJIWARA	5 _{of} 103	JUNE 1, 2017	RG3103-E

1. GENERAL SPECIFICATIONS:

2. DESIGN SPECIFICATIONS:

3. GENERAL:

4. LOADS:

C. Seismir Loads:

A. Standard Specifications for Construction of Roads and Bridges on Federal Highway

Projects, FP-14, and the Special Contract Requirements (SCR) prepared for this

A. American Association of State Highway and Transportation Officials (AASHTO) 2014

revisions and the Hawaii Department of Transportation (HDOT) document dated

In accordance with AASHTO LFRD Bridge Design Specifications, 7th edition 2014.

spectrum acceleration, S_{DS} = 0.6321g. Long period acceleration at 1.0 seconds

D. Future Utility Load: An allowance of 150 plf on each side of bridge for utility loads

Classes of

Concrete

B. A shrinkage reducing admixture (SRA), (See SCR 711.03(b)) shall be added to the

C. A migrating corrosion inhibitor amine carboxylate water-based admixture (See SCR

711.03(a)) shall be added to all concrete mix for item numbers under note 5.(A).

The minimum dosage requirement shall be 1 $\frac{1}{2}$ pints per cubic yard of concrete.

glass macrofiber. The fiber shall be 1-1/2" long minimum and shall have an aspect

D. Concrete for item (1) under note 5.A. shall contain 13 lbs./cyd of alkali resistant

F. Reinforcing steel shall be American Society of Testing and Materials (ASTM) A615,

Grade 60, deformed bars unless otherwise noted. Where welded connections are

water-based, epoxy-modified portland cement bonding agent. Bond strength shall

required, reinforcing steel shall be ASTM A706 grade 60 deformed bars.

G. Bonding agent shall be a three-component, preproportioned, anti-corrosion,

requirement shall be one gallon per cubic yard of concrete.

E. The use of any calcium chloride in any concrete is prohibited.

exceed 2,400 psi in accordance with ASTM C882.

concrete mix for item numbers (1) and (2) under note 5.(A). The minimum dosage

 $(S_1=0.1534g)$ modified by the Site Coefficient (Fv = 1.65) to give the long period spectrum acceleration, $S_{D1}=0.2526g$. Site Class = C. Seismic Zone = 1.

Peak Ground Acceleration (PGA = 0.2332g), modified by the Site Coefficient (F_{PGA} = 1.17)

Maximum

Water

Cement

(W/C)

0.40

0.40

0.45

Compressive

Strength

f'c (28 Days)

6000 psi

5000 psi

4500 psi

Maximum

Cement

Content

(lbs/cyd)

670

670

625

to give a spectrum acceleration, $A_S=0.274g$. Short period acceleration at 0.2 seconds ($S_S=0.5326g$) modified by the site coefficient ($F_a=1.19$) to give the short period

August 8, 2014 with subject title "Design Criteria for Bridges and Structures".

A. Structural general notes apply to the structural plans unless otherwise noted.

A. Dead Loads: Concrete self weight assumed to be 160 pcf.

B. Live Load: AASHTO HL-93 Loading.

has been provided for in the design.

F. Wind: Base Design Wind Velocity = 105 MPH

G. Future wearing surface: 25 psf curb to curb

A. All concrete strengths shall be as noted below:

Bridge railings, concrete

barriers, end posts, box

culvert, cutoff wall and

All others, except as

otherwise noted

E. Railing Test Level: TL-4

Item No. Structural Parts

wingwall

Concrete deck

5. MATERIALS:

(1)

(2)

LRFD Bridge Design Specifications, 7th Edition, including the 2015 and 2016 interim

STRUCTURAL GENERAL NOTES (CONTINUED)

8. FOUNDATION NOTES: For Punakea Uka Overpass and Punakea Kai Overpass

- A. Foundation design entitled "Geotechnical Engineering Exploration is based on the report prepared by Geolabs, Inc., Lahaina Bypass, Phase 1B-2, Lahaina, Maui, Hawaii" dated May 23. 2017
- B. Unless otherwise indicated, refer to FP-14 and SCRs for foundation preparation, compaction requirements, and other requirements.
- C. Soil Design Parameters

(1) Soil bearing pressures

Extreme Event Limit State = 24,000 psf

Strength Limit State = 10,000 psf

Service Limit State = 8,000 psf

(2) Coefficient of Friction
Extreme Event Limit State = 0.45
Strength Limit State = 0.36

(3) Passive earth pressure
(a) Extreme event limit state = 300 pcf
(b) Strength limit state = 150 pcf

(4) Static lateral earth pressure Backfill condition: Level backfill above water

(a) Active = 30 pcf(b) At-rest = 48 pcf

(5) Dynamic lateral earth pressure (a) Lateral movement of 1 in. 5.1 H ² lbs/ft

Geosynthetic reinforced soil
(a) Reinforced fill friction angle 38 degrees
(b) Reinforced fill unit weight 135 lb/ft

9. FOUNDATION NOTES: For Culverts

- A. Foundation design entitled "Geotechnical Engineering Exploration is based on the report prepared by Geolabs, Inc., Lahaina Bypass, Phase 1B-2, Lahaina, Maui, Hawaii" dated May 23, 2017.
- B. Unless otherwise indicated, refer to FP-14 and SCRs for foundation preparation, compaction requirements, and other requirements.
- C. Soil Design Parameters
 - (1) Box Culvert

(a) Lateral earth pressures
Active:
Level backfill above water
Level backfill below water = 77 pcf

At rest: Level backfill above water = 48 pcf

Level backfill below water = 87 pcf

b) Soil bearing pressures
Extreme event limit state = 12,000 psf
Strength limit state = 5,000 psf

Service limit state = 4,000 psf
(2) New end post and wingwall foundations

(a) Soil bearing pressures

Extreme limit state = 12,000 psf

Strength limit state = 5,000 psf

Service limit state = 4,000 psf

(b) Coefficient of friction
Extreme limit state = 0.45
Strength limit state = 0.36

(c) Passive Earth pressure = On site clayey silt Extreme limit state = 300 pcf Strength limit state = 150 pcf

(d) Active pressure

Level backfill above water = 30 pcf

Level backfill below water = 77 pcf

Note: Lateral earth pressure does not account for the reduction due to the Geosynthetic reinforcement.

(e) Dynamic lateral Earth forces - Level backfill, unrestrained 1" Lateral movement 5.1H² lbs/ft

10. <u>GEOSYNTHETIC REINFORCED SOIL</u>: For Punakea Uka Overpass and Punakea Kai Overpas:

1. Geosynthetic Reinforcement Placement:

Pull the geosynthetic taut to remove any wrinkles and lay flat prior to placing and compacting the backfill material. Splices shall be staggered with minimum 24 inch overlap and splices are not allowed in the bearing reinforcement zone. No equipment is allowed directly on the geosynthetic. Place a minimum 6-inch layer of granular fill prior to operating only rubber tired equipment over the geosynthetic at speeds less than 5 miles per hour with no sudden braking or sharp turning. The geosynthetic reinforcement shall extend between layers of modular block to provide a frictional connection.

Reinforced Soil Foundation (RSF) Construction:

The RSF shall be encapsulated in geotextile reinforcement on all sides with minimum overlaps of 5.0 feet. Wrapped corners shall be tight without exposed soil. Compact backfill material in lifts less than 6-inches in compacted height. Grade and level the top of the RSF prior to final encapsulation, as this layer will serve as the leveling pad for the modular blocks of the GRS abutment.

3. GRS Wall Face Alignment:

Check for level alignment of the modular block row at least every other layer of the GRS abutment. Correct any alignment deviations greater than 0.25 inches. Check vertical GRS wall for plumbness at least every other layer and correct any deviations greater than 0.25 inches.

4. Integrated Approach Placement:

Following the placement of the superstructure, geotextile reinforcement layers are placed along the back of the superstructure, built-in maximum lift heights of 6-inches (maximum vertical reinforcement spacing 12-inches). The top of the final wrap shall be approximately 2-inches below the top of the superstructure to allow at least 2-inches of aggregate base cover over the geosynthetic to protect it from hot mix asphalt.

MODULAR BLOCK FACING ELEMENT

 $Compressive\ strength = 4,000\ psi\ minimum$

Water absorption limit = 5%

Block dimensions = See sheet S8.1

Note: modular blocks shall be hollow core, split face and solid corner block conforming to ASTM C90 with the modification that the blocks have a minimum compressive strength of 4,000 psi and a maximum water absorption limit of 5%.

REINFORCED BACKFILL GRADATION

Reinforced Backfill Gradation = Use backfill materials in accordance with SCR 704.03 (c).

GEOSYNTHETIC REINFORCEMENT PROPERTIES

Required ultimate tensile strength = 4,800 lb/ft in accordance with ASTM D 4595 for bioaxial geotextile.

Minimum tensile strength of geotextile at 2% strain = 960 lb/ft.

11. LOAD RATING:

A. New governing load rating results as follows:

	akea Uka Overpass akea Kai Overpass	Rating Factor	Distribution Factor	Load Effect	Controlling Member
	HL-93 Inventory	1.93	0.081	Pos. M	Slab
Design	HL-93 Operating	2.50	0.081	Pos. M	Slab

For Sta	. 422+02	Rating Factor	Distribution Factor	Load Effect	Controlling Member
Danier	HL-93 Inventory	1.41	0.08	Neg. M	Slab
Design	HL-93 Operating	1.83	0.08	Neg. M	Slab

For Sta	. 434+30	Rating Factor	Distribution Factor	Load Effect	Controlling Member
D(HL-93 Inventory	1.41	0.08	Neg. M	Slab
Design	HL-93 Operating	1.82	0.08	Neg. M	Slab

For Sta. 482+12		Rating Factor	Distribution Factor	Load Effect	Controlling Member	
0	HL-93 Inventory	1.32	0.07	Neg. M	Slab	
Design	HL-93 Operating	1.72	0.07	Neg. M	Slab	

STATE	PROJECT	SHEET NO.
Н	HI STP SR 30(1)	S6

Р	UNAKEA UKA OVERPASS AND PUNAKEA KA	I OVERPASS I	ESTIMATE	
Bid Item Number	Item Description	Quantities	Unit	Notes
20435-2000	Granular backfill material	8,800	CY	-
20801-0000	Structure excavation	3,516	CY	-
20803-0000	Structure backfill	50	CY	-
55204-1000	Structural concrete, class state DOT (class D)	19	CY	-
55204-2000	Structural concrete, class state DOT (class E)	390	CY	-
55303-0000	Prestressing system	LPSM	LPSM	-
55401-1000	Reinforcing steel	93,556	LB	-
55601-0500	Bridge railing, concrete	182	LNFT	(1) (4)
57401-0000	GRS-IBS Geosynthetic reinforcement	44,778	SQYD	-
57403-0000	GRS-IBS Concrete masonry unit	1142	SQYD	-
58401-1000	Glass fiber reinforced polymer reinforcing	980	LBS	-
61707-0000	Structure transition railing	100	LNFT	(3)
61801-0000	Concrete barrier	314	LNFT	(1) (2) (4) (6)

	CULVERT ESTIMATE							
Bid Item Number	Item Description	Quantities	Unit	Notes				
20720-0400	Reinforcement, Geosynthetic, Type 4	4330	SQYD	-				
20801-0000	Structure excavation	3540	CUYD	-				
20803-0001	Structure backfill	3130	CUYD	-				
55204-1000	Structural concrete, class state DOT (class D)	1593	CUYD	(2) (5) (6)				
55401-1000	Reinforcing steel	380,000	LB	-				
55401-2000	Reinforcing steel, epoxy coated	1736	LB	-				
55601-0500	Bridge railing, concrete	20	LNFT	(1) (4)				
58401-0000	Glass fiber reinforced polymer reinforcing	4180	LB	-				
61707-0000	Structure transition railing	50	LNFT	(3)				

ESTIMATE NOTES:

- (1) Includes costs of structural concrete and rebar.
- (2) Includes costs of furnishing and installing weepholes.
- (3) Includes costs of furnishing and installing posts, blocks, thrie and W-beam rail elements, anchor plates, and installation hardware.
- (4) End post barrier, including curbs, shall be paid for in item no. 55601-0500.
- (5) Includes costs of box culverts, wingwalls, cutoff walls, headwalls, and retaining walls.
- (6) Includes cost of smooth GFRP dowels.



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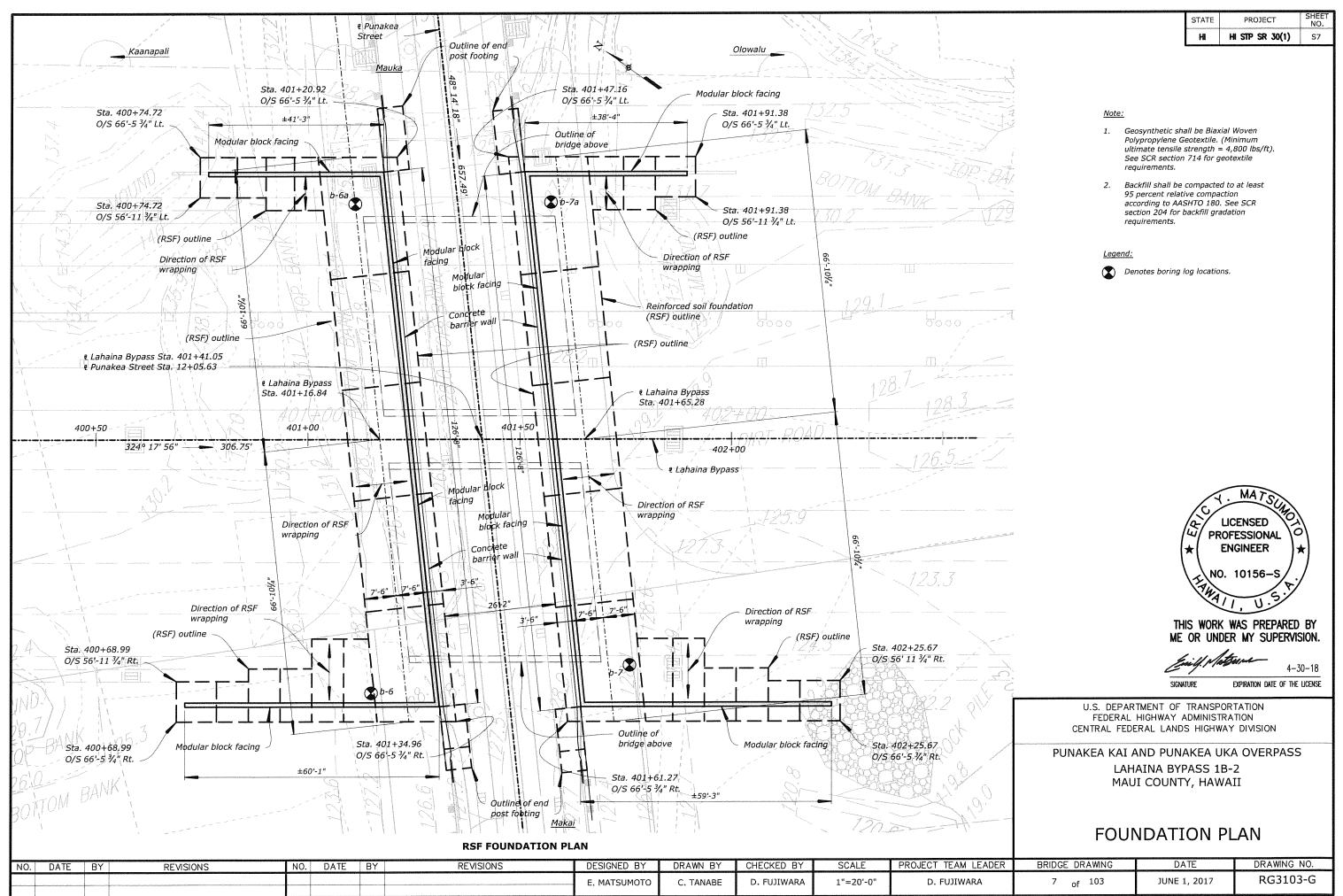
SIGNATURE EXPIRATION DATE OF THE LICENSE

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

STRUCTURAL GENERAL NOTES

DRAWING NO. BRIDGE DRAWING DATE DESIGNED BY DRAWN BY CHECKED BY SCALE PROJECT TEAM LEADER NO. DATE BY REVISIONS NO. DATE BY REVISIONS RG3103-F D. FUJIWARA NO SCALE D. FUJIWARA 6 of 103 JUNE 1, 2017 E. MATSUMOTO C. TANABE





GEOLABS, INC.

Soil Log Legend

UNIFIED SOIL CLASSIFICATION SYSTEM (USCS)

	MAJOR DIVISION	IS		cs	TYPICAL DESCRIPTIONS
	GRAVELS	CLEAN GRAVELS	000	GW	WELL-GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES
COARSE-	GRAVELS	LESS THAN 5% FINES	0.0	GP	POORLY-GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES
GRAINED SOILS	MORE THAN 50% OF COARSE	GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES
	FRACTION RETAINED ON NO. 4 SIEVE	MORE THAN 12% FINES		GW WELL-GRADED GRAVELS, MIXTURES, LITTLE OR NO GP POORLY-GRADED GRAVELS, MIXTURES, LITTLE OR NO GM SILTY GRAVELS, GRAVEL- MIXTURES GC CLAYEY GRAVELS, GRAVEL- MIXTURES SW WELL-GRADED SANDS, GR LITTLE OR NO FINES SP POORLY-GRADED SANDS, GR LITTLE OR NO FINES SM SILTY SANDS, SAND-SILT I SC CLAYEY SANDS, SAND-SILT I CLAYEY SANDS, SAND-SILT I ML ROCK FLOUR SILTY OR CI OR CLAYEY SILTY OR CI OR CLAYEY SILTY OR CI CLAYS OF LOW PLASTICITY GRANIC SILTS AND ORG. CLAYS OF LOW PLASTICITY GRANIC SILTS AND ORG. CLAYS OF LOW PLASTICITY CH INORGANIC SILTS AND ORG. CLAYS OF LOW PLASTICITY HORGANIC SILTS AND ORG. CLAYS OF LOW PLASTICITY OH ORGANIC SILTS OF HIGH OH ORGANIC CLAYS OF HIGH ORGANIC CLAYS OF HIGH ORGANIC CLAYS OF GRANIC SILTY OR CANDELLY OH ORGANIC CLAYS OF HIGH OH ORGANIC CLAYS OF HIGH ORGANIC CLAYS OF MEDIL PLASTICITY, ORGANIC SILTY ORGANIC SILTY OR CLAYS OF HIGH ORGANIC CLAYS OF MEDIL PLASTICITY, ORGANIC SILTY ORGANIC SILTY OR CLAYS OF MEDIL PLASTICITY, ORGANIC SILTY ORG	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES
	0.1100	CLEAN SANDS	• 0	sw	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
MORE THAN 50% OF MATERIAL	SANDS	LESS THAN 5% FINES		SP	POORLY-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
RETAINED ON NO. 200 SIEVE	50% OR MORE OF COARSE FRACTION PASSING	SANDS WITH FINES		SM	SILTY SANDS, SAND-SILT MIXTURES
200 SIEVE	THROUGH NO. 4 SIEVE	MORE THAN 12% FINES		sc	CLAYEY SANDS, SAND-CLAY MIXTURES
	011.70			ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
FINE- GRAINED SOILS	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50		CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
COLLO				OŁ	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
				мн	INORGANIC SILT, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
50% OR MORE OF MATERIAL PASSING THROUGH NO, 200 SIEVE	SILTS AND CLAYS	LIQUID LIMIT 50 OR MORE		СН	INORGANIC CLAYS OF HIGH PLASTICITY
SIEVE	30113			он	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
HI	GHLY ORGANIC SO	DILS	7 77 77 77 77	PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

1. A geotechnical engineering technical memorandum entitled

3. The information presented in the logs of borings depict the

4. The penetration resistance shown on the logs of borings

type used. The blow counts may need to be factored to obtain the Standard Penetration Test (SPT) blow counts.

"Lahaina Bypass Phase 1B - 2, Geotechnical Recommendations -GRS-IBS at Punakea Loop" dated May 23, 2017 has been

subsurface conditions encountered at that specified location and

conditions from those depicted in the logs of borings may occur

indicate the number of blows required for the specific sampler

at the time of the field exploration only. Variations of subsoil

LEGEND

(2-INCH) O.D. STANDARD PENETRATION TEST (3-INCH) O.D. MODIFIED CALIFORNIA SAMPLE

SHELBY TUBE SAMPLE GRAB SAMPLE CORE SAMPLE

WATER LEVEL OBSERVED IN BORING AT TIME OF DRILLING

WATER LEVEL OBSERVED IN BORING AFTER DRILLING WATER LEVEL OBSERVED IN BORING OVERNIGHT

GEOTECHNICAL NOTES:

prepared by Geolabs, Inc.

2. For boring locations, see Sheets X thru X.

between and beyond the borings.

LL LIQUID LIMIT (NP=NON-PLASTIC)

PLASTICITY INDEX (NP=NON-PLASTIC)

TORVANE SHEAR (tsf)

UC UNCONFINED COMPRESSION
OR UNIAXIAL COMPRESSIVE STRENGTH

TXUU UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION (ksf)

Plate A-0.1

WOH: Weight of Hammer WOR: Weight of Driff Rods

SPT: Standard Penetration Test Split-Spoon Sample

MOISTURE CONTENT DEFINITIONS

Wet: Visible free water, usually soil is below water table

ABBREVIATIONS

Dry: Absence of molsture, dry to the touch

Molst: Damp but no visible water

MCS: Modified California Sampler PP: Pocket Penetrometer

*Soil descriptions are based on ASTM D2488-09a, Visual-Manual Procedure, with the above modifications by Geolabs, Inc. to the Unified Soil Classification System (USCS).

GRAIN SIZE DEFINITION							
Description	Sieve Number and / or Size						
Boulders	> 12 Inches (305-mm)						
Cobbles	3 to 12 Inches (75-mm to 305-mm)						
Gravel	3-inch to #4 (75-mm to 4.75-mm)						
Coarse Gravel	3-Inch to 3/4-Inch (75-mm to 19-mm)						
Fine Gravel	3/4-inch to #4 (19-mm to 4.75-mm)						
Sand	#4 to #200 (4.75-mm to 0.075-mm)						
Coarse Sand	#4 to #10 (4.75-mm to 2-mm)						
Medium Sand	#10 to #40 (2-mm to 0,425-mm)						
Fine Sand	#40 to #200 (0.425-mm to 0.075-mm)						

A-0.2

Geotechnical Engineering

GEOLABS, INC.

Soil Classification Log Key (with deviations from ASTM D2488)

GEOLABS, INC. C	CLASSIFICATION*
GRANULAR SOIL (- #200 <50%)	COHESIVE SOIL (-#200 ≥50%)
PRIMARY constituents are composed of the largest percent of the soil mass. Primary constituents are capitalized and bold (i.e., GRAVEL, SAND)	PRIMARY constituents are based on plasticity. Primary constituents are capitalized and bold (i.e., CLAY, SILT)
SECONDARY constituents are composed of a percentage less than the primary constituent. If the soil mass consists of 12 percent or more fines content, a cohesive constituent is used (SiLTY or CLAYEY); otherwise, a granular constituent is used (GRAVELLY or SANDY) provided that the secondary constituent consists of 20 percent or more of the soil mass. Secondary constituents are capitalized and bold (i.e., SANDY GRAVEL, CLAYEY SAND) and precede the primary constituent.	 SECONDARY constituents are composed of a percentage less than the primary constituent, but more than 20 percent of the soil mass. Secondary constituents are capitalized and bold (i.e., SANDY CLAY, SILTY CLAY, CLAYEY SILT and precede the primary constituent.
accessory descriptions compose of the following: with some: >12% with a filter: 5 - 12% with traces of: <5% accessory descriptions are lower cased and follow the Primary and Secondary Constituents (i.e., SILTY GRAVEL with a little sand)	accessory descriptions compose of the following: with some: >12% with a little: 5 - 12% with traces of: <5% accessory descriptions are lower cased and follow the Primary and Secondary Constituents (i.e., SILTY CLAY with some sand)

EXAMPLE: Soll Containing 60% Gravel, 25% Sand, 15% Fines. Described as: SILTY GRAVEL with some sand

DEL ATIVE	DENSITY /	CONSISTENCY

	Granular Soil	s		Coh	esive Soils	
N-Value (E	Blows/Foot)	Relative		(Blows/Foot) PP Reading		Consistency
SPT	MCS	Density	SPT	MCS	(tsf)	Consistency
0 - 4	0 - 7	Very Loose	0 - 2	0-4		Very Soft
4 - 10	7 - 18	Loose	2 - 4	4 - 7	< 0.5	Soft
10 - 30	18 - 55	Medium Dense	4 - 8	7 - 15	0.5 - 1.0	Medlum Stiff
30 - 50	55 - 91	Dense	8 - 15	15 - 27	1.0 - 2.0	Stiff
> 50	> 91	Very Dense	15 - 30	27 - 55	2.0 - 4.0	Very Sttff
			> 30	> 55	> 4.0	Hard

GRAIN	SIZE	DEFINITION	

	Description	Sleve Number and / or Size
Г	Boulders	> 12 Inches (305-mm)
Г	Cobbles	3 to 12 Inches (75-mm to 305-mm)
	Gravel	3-inch to #4 (75-mm to 4.75-mm)
Г	Coarse Gravel	3-Inch to 3/4-Inch (75-mm to 19-mm)
	Fine Gravel	3/4-inch to #4 (19-mm to 4.75-mm)
	Sand	#4 to #200 (4.75-mm to 0.075-mm)
	Coarse Sand	#4 to #10 (4.75-mm to 2-mm)
	Medium Sand	#10 to #40 (2-mm to 0.425-mm)
_	Fine Sand	#40 to #200 (0.425-mm to 0.075-mm)

Geotechnical Engineering

GEOLABS, INC.

Rock Log Legend

ROCK DESCRIPTIONS

屬	BASALT		FINGER CORAL
8	BOULDERS	井	LIMESTONE
	BRECCIA		SANDSTONE
××	CLINKER	X X X X X X X X X X X X X X X X X X X	SILTSTONE
188	COBBLES		TUFF
0 0	CORAL		VOID/CAVITY

ROCK DESCRIPTION SYSTEM

ROCK FRACTURE CHARACTERISTICS

Greater than 24 inches apart

Slightly Fractured: 12 to 24 inches apart 6 to 12 Inches apart Moderately Fractured: Closely Fractured: Severely Fractured: Less than 3 inches apart

DEGREE OF WEATHERING

The following terms describe the chemical weathering of a rock.

Rock shows no sign of discoloration or loss of strength.

Slightly Weathered:

Highly Weathered: Most minerals decomposed with some corestones present in residual soll mass. Can be broken by hand.

Saprolite, Mineral residue completely decomposed to soil but fabric and structure preserved Extremely Weathered:

HARDNESS

Very Hard:

Medium Hard:

Very Soft:

The following terms describe the resistance of a rock to indentation or scratching:

Specimen breaks with some difficulty after several hammer blows Example; Veskular, vugular, coarse-grained rock

Specimen can be broked by one hammer blow. Cannot be scraped by knife. SPT may penetrate by ~25 blows per inch with bounce. knample: Porous trock such as clinker, cinder, and coral reef

Can be indented by one hammer blow. Can be scraped or peeled by knife. SPT can penetrate by -100 blows per foot. Example: Weathered rock, chalk-like coral reef

Crumbles under hammer blow. Can be peeled and carved by knife. Can be indented by finger

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

A-0.3

PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

BORING LOG LEGENDS AND NOTES

DRAWING NO. REVISIONS DESIGNED BY DRAWN BY CHECKED BY SCALE PROJECT TEAM LEADER BRIDGE DRAWING DATE NO. DATE BY REVISIONS NO. DATE BY RG3103-H J. SEIDMAN H. CHUN NO SCALE G. SEKI 8 of 103 JUNE 1, 2017 J. SEIDMAN

and the same of	STATE	PROJECT	SHEET NO.
	Н	HI STP SR 30(1)	S9

		Geo			3S, IN Engine				L	AHAINA BYPASS PHASE 1B - 2 LAHAINA, MAUI, HAWAII 6
Other Tests	Moisture Content (%)	Dry Unit Weight (pcf)	Core Recovery (%)	RQD (%)	Resistance (blows/foot)	Pocket Pen. (tsf)	Depth (feet)	Sample Graphic	nscs	Approximate Ground Surface Elevation (feet MSL): 127.24 *
5 Direct Shear Sieve - #200 = 29.9% UC= 5820 psi Sieve - #200 = 9.7%	9 5 5 5	93	38 70 75	RG	高麗 10/6" +50/2" 38 50/1" 70 50/4"		5 10 15 20 25		SW-SM	Description Brown SILTY SAND with some gravel, very stiff to hard, dry (alluvium) Brownish gray GRAVELLY SAND with a little silt, very dense, dry (alluvium) grades with cobbles and boulders
					50/2"		30	11.0		Boring terminated at 31.7 feet Latitude: 20.855145° N Longitude: 156.657514° W
Date Sta Date Cor Logged I	nplet 3y:	ed:	Augus D. Gre	st 31, emmi	2016 2016 nger		30.			Water Level: Not Encountered Drill Rig: CME-75DG1 (Energy Transfer Ratio = 80.3%) Drilling Method: 4" Solid Stem Auger & PQ Coring
Total De Work Or			31.7 f 7378-							Drilling Method: 4" Solid Stem Auger & PQ Coring Driving Energy: 140 lb. wt., 30 in. drop

					BS, IN		,		L	AHAINA BYPASS PHASE 1B - 2 LAHAINA, MAUI, HAWAII 7	
Other Tests	isture ntent (%)	Dry Unit Weight (pcf)	e sovery (%)	RQD (%)	Penetration Resistance (blows/foot)	Pocket Pen. Gr(tsf)	Depth (feet)	Sample Graphic	sosn	Approximate Ground Surface Elevation (feet MSL): 127.71 *	
5	₽Ō	We	Ro	RO	돌음골	Poor (tsf	De	Sign	ns	Description	
Direct Shear Sieve - #200 = 45.6%	13	69 98	18		48 24/6" +50/4" 65/6"		5	X	SM	Brown SILTY SAND with a little gravel and cobbles, medium dense to very dense, dry (alluvium)	
UC= 5970 psi	4		95 14		50/3"		15	000 pg - g - g - g - g - g - g - g - g - g		Brownish gray GRAVELLY COBBLES with some boulders and sandy silt seams, very dense, dry (alluvium)	0
			41 52		50/2"		20	03 08 08 0 00 00 00 00 00 00 00 00 00 00 0			******
							30	200		Boring terminated at 31.1 feet]
					50/1"		35-			Latitude: 20.855019° N Longitude: 156.657394° W	
Date Sta	rted:		Augus	st 31.	2016		. 55			Water Level: ¥ Not Encountered	\exists
Date Cor					1, 2016						
Logged I			D. Gr							Drill Rig: CME-75DG1 (Energy Transfer Ratio ≈ 8	30.3%)
Total De			31.1 f							Drilling Method: 4" Solid Stem Auger & PQ Coring	
Work Or	der:		7378-	00						Driving Energy: 140 lb. wt., 30 in. drop	

		GEOLABS, INC. LA Geotechnical Engineering									AHAINA BYPASS PHASE 1B - 2 LAHAINA, MAUI, HAWAII 6A	
Other Tests	Moisture Content (%)	Dry Unit Weight (pcf)	Core Recovery (%)	RQD (%)	Penetration Resistance (blows/foot)	Pocket Pen. (tsf)	Depth (feet)	Sample	abuic	scs	Approximate Ground Surface Elevation (feet MSL): 129.57 *	
Direct Shear	<u>აც</u> შ	<u>ŏ š</u> 87	လူ	RC	18 18	3.0 (ts	<u> </u>	X		SOSN SM ML	Description Reddish brown SILTY SAND with some gravel, medium dense, moist (fill) Brown SANDY SILT with some cobbles and	<u> </u> f]
UC= 13480 psi			97		15/0" Ref.		5				boulders (basaltic), hard, moist (alluvium)	
	21		15		15/0" Ref.		10					-
UC=			51		26/6" +35/1"		15 20					
4720 psi Consol.	31		49		50/5"		25					1
(01, 1381);	9		47		15/0" Ref. 52/6"		30				Boring terminated at 31.1 feet	- 145
0.00 GPJ 05:04.A88.0					+30/1"		35		1		Latitude: 20.8553387543424° N Longitude: 156.657246702818° W	2.3%)
Date Sta	mplet	ed:	Septe	mber	· 28, 201 · 28, 201						Water Level: ♀ Not Encountered	
Logged I Total De Work Or	pth:		B. Aiu 31.1 f 7378-	eet							Drill Rig: CME-75DG1 (Energy Transfer Ratio = 8i Drilling Method: 4" Solid Stem Auger & HQ Coring Driving Energy: 140 lb. wt., 30 in. drop	3%)

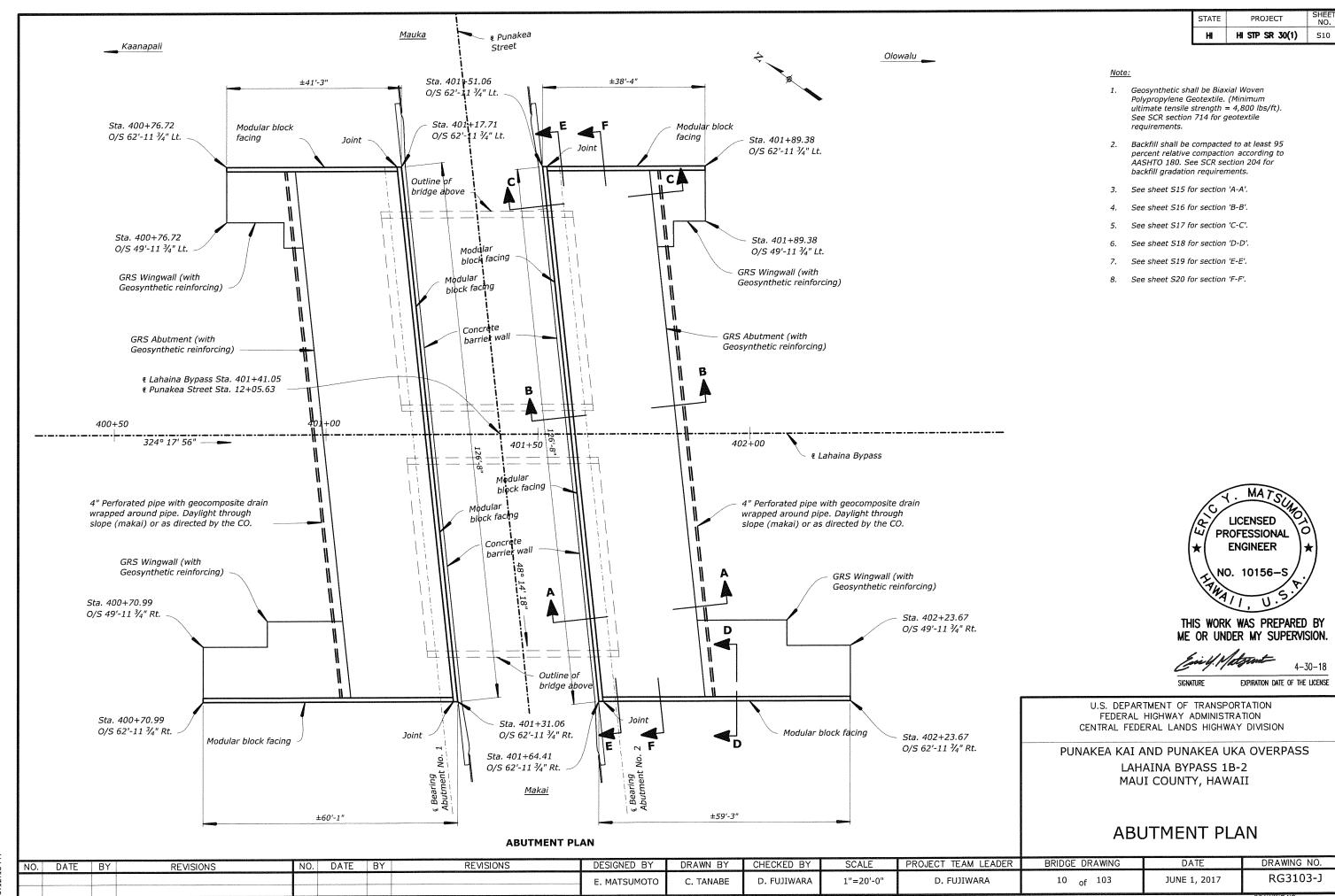
					3S, IN Engine				L	LAHAINA BYPASS PHASE 1B - 2 LAHAINA, MAUI, HAWAII 7A						
Other Tests	loisture ontent (%)	Dry Unit Weight (pcf)	ore ecovery (%)	RQD (%)	Penetration Resistance (blows/foot)	V Pocket Pen. G (tsf)	Depth (feet)	Sample Graphic	nscs		oximate Ground ation (feet MSL): Description					
O LL=49 PI=8	<u>≥0</u> 15 26	<u>o ≶</u> 117	37	α.	66 55/5"	<u>a t)</u> >4.5	5		ML ML	Reddish brown s medium stiff, mo Brown SANDY s boulders (basali	SANDY SILT wi pist (fill) SILT with some	cobbles a	nd			
	38		7		72		10	H								
UC= 460 psi			25		15/0" Ref.		15									
			53		15/0" Ref.		20									
			67		15/0" Ref.		25									
					15/0" Ref.		30-			Boring terminate Latitude: 20.855 Longitude: 156.6	2360837836° N	W				
Date Sta					28, 201 28, 201		35-			Water Level: ¥	Not Encountered					
Logged I Total De Work Or	By: pth:		B. Aiu 30 fee 7378-	et .						Drill Rig: Drilling Method: Driving Energy:	CME-75DG1 4" Casing & HQ Co 140 lb, wt., 30 in, o	oring	sfer Ratio = 80.3%			

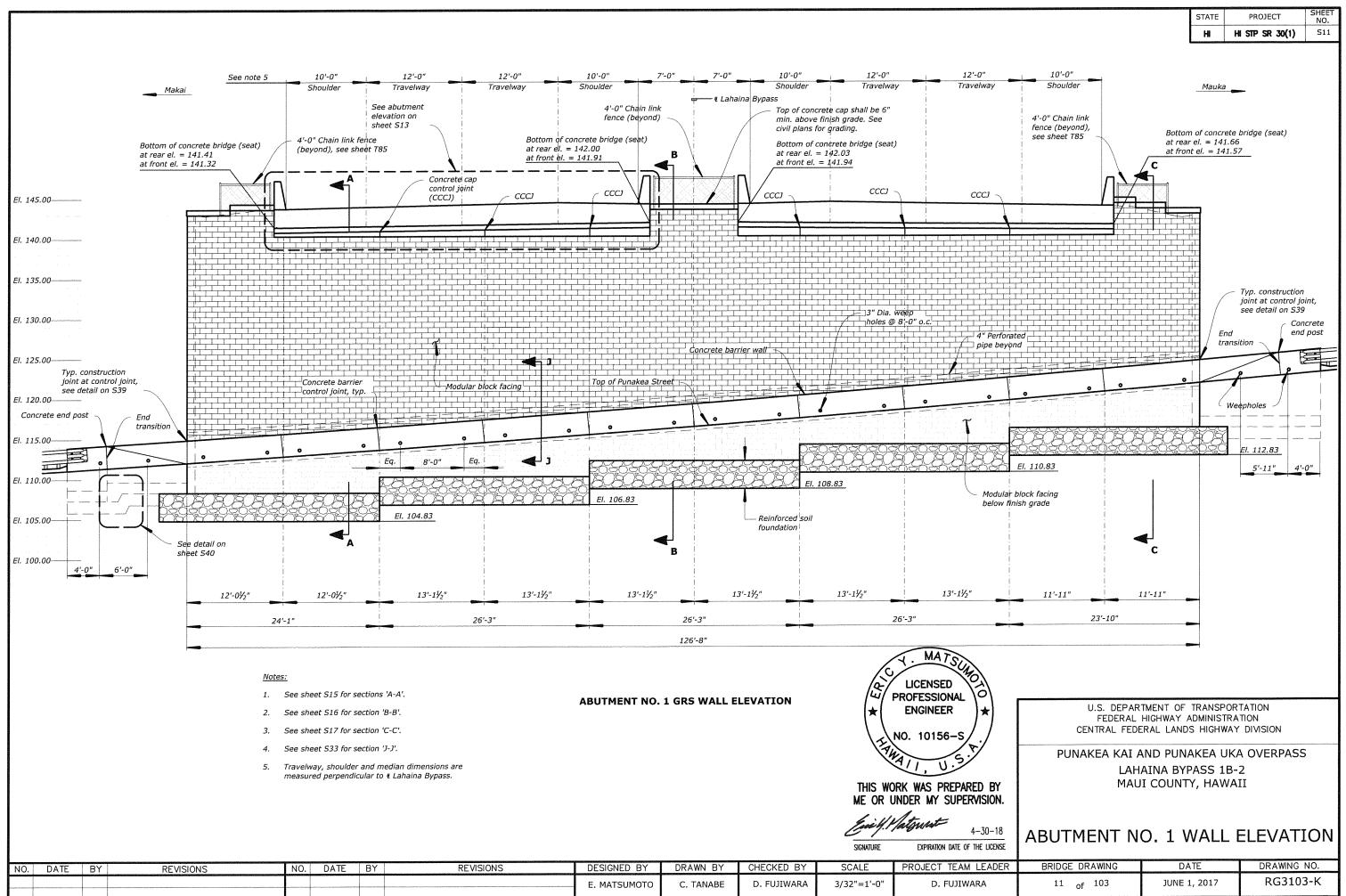
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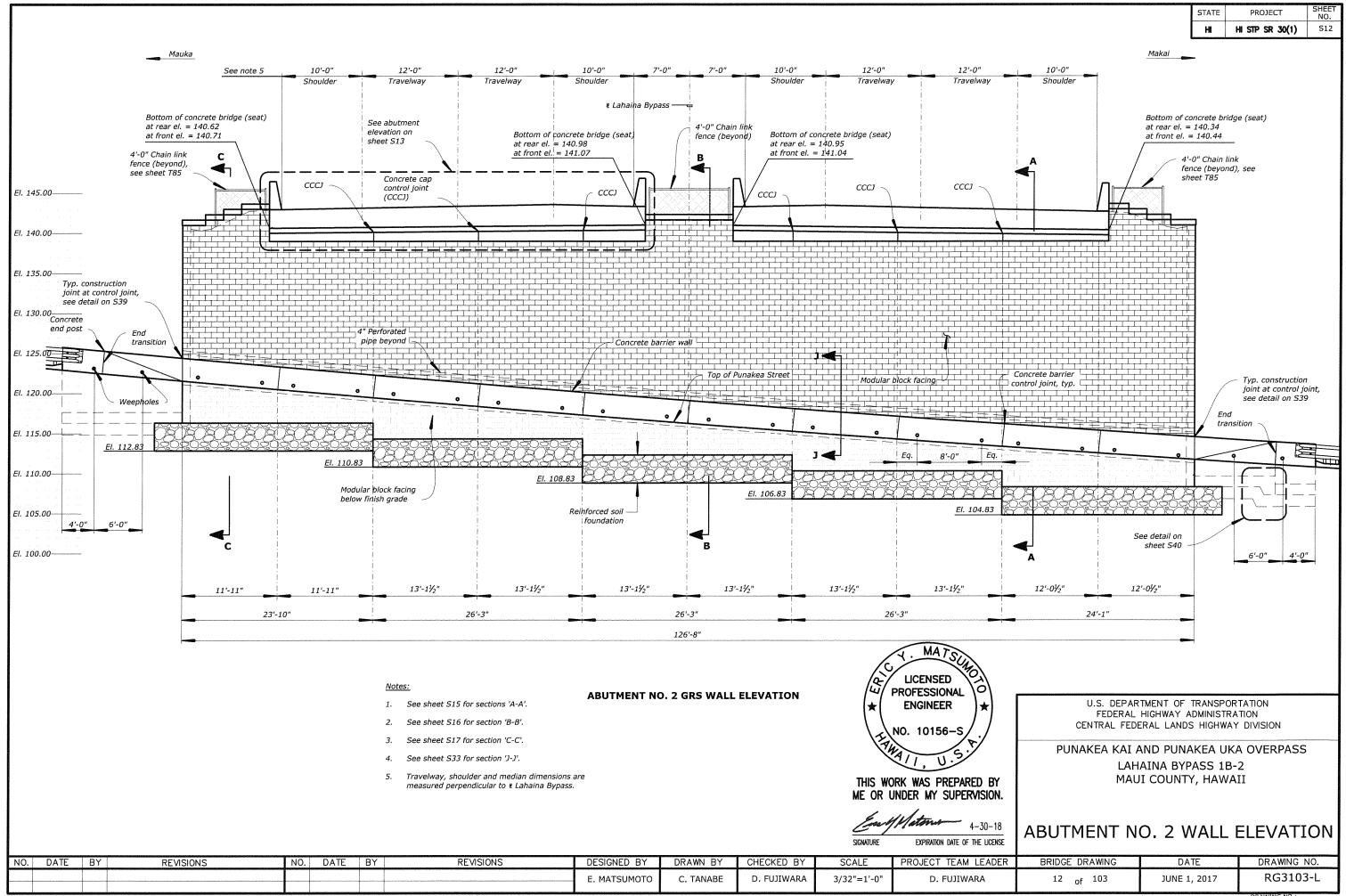
PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

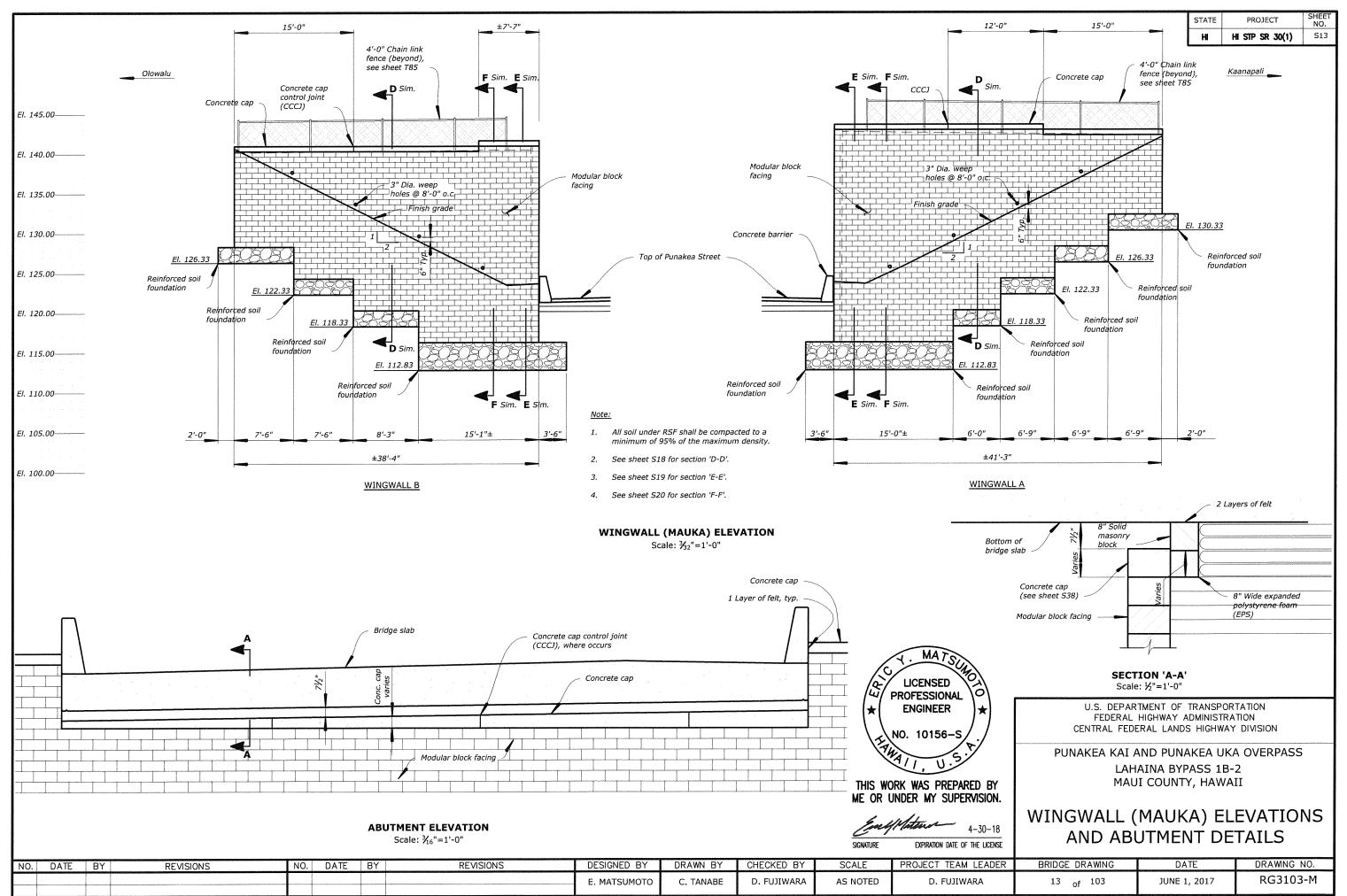
BORING LOG-1

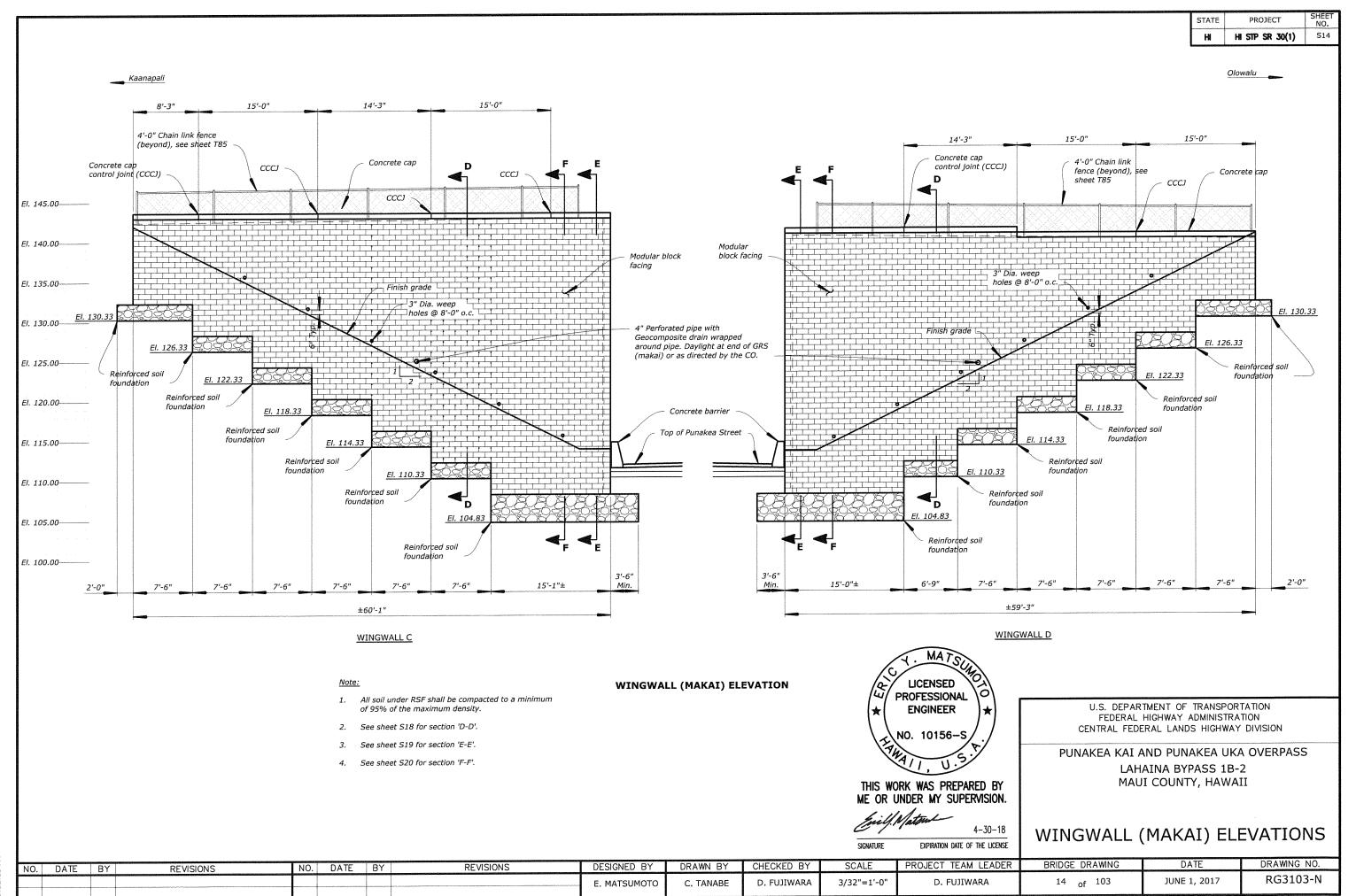
N	IO. DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
								J. SEIDMAN	J. SEIDMAN	H. CHUN	NO SCALE	G. SEKI	9 of 103	JUNE 1, 2017	RG3103-I

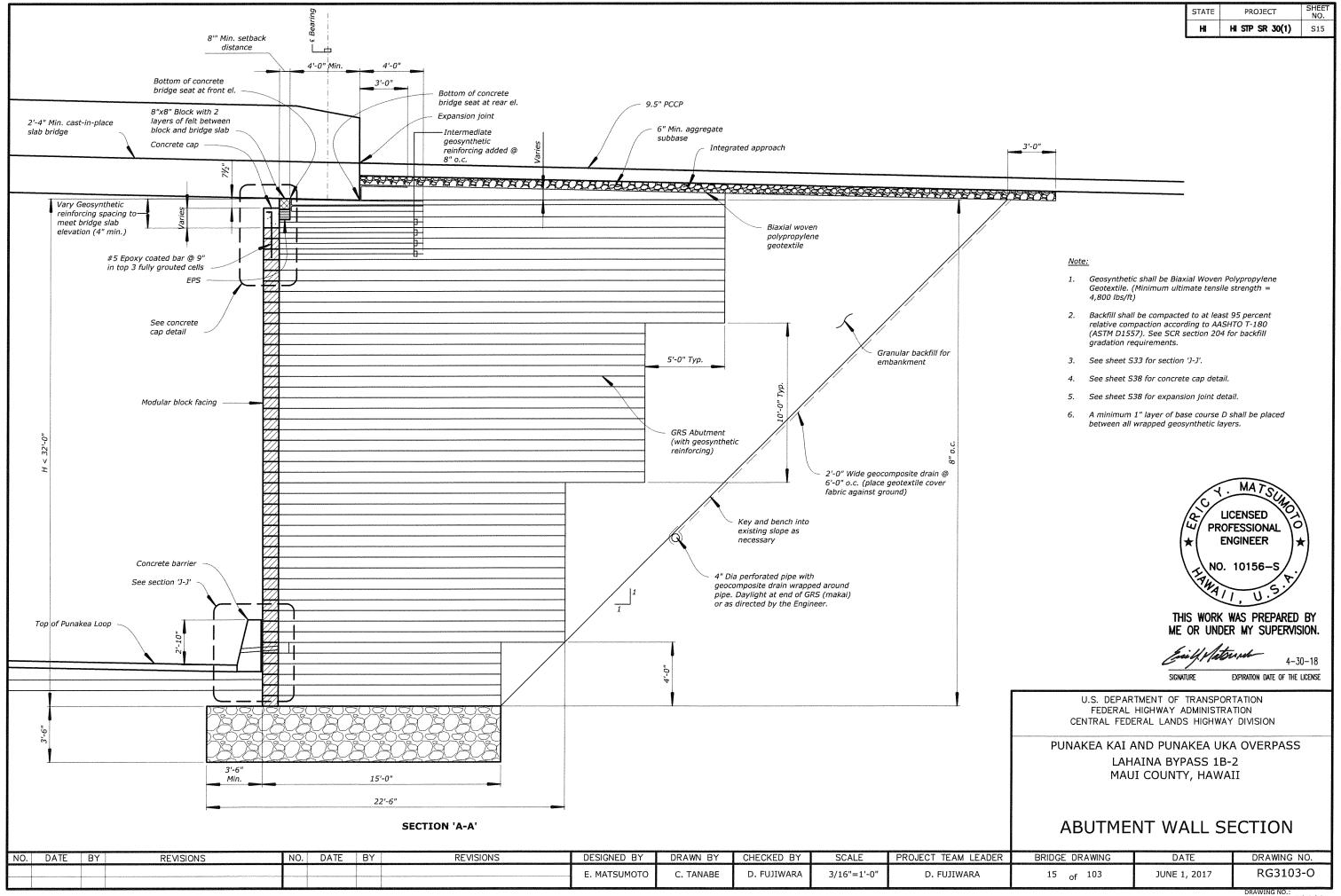


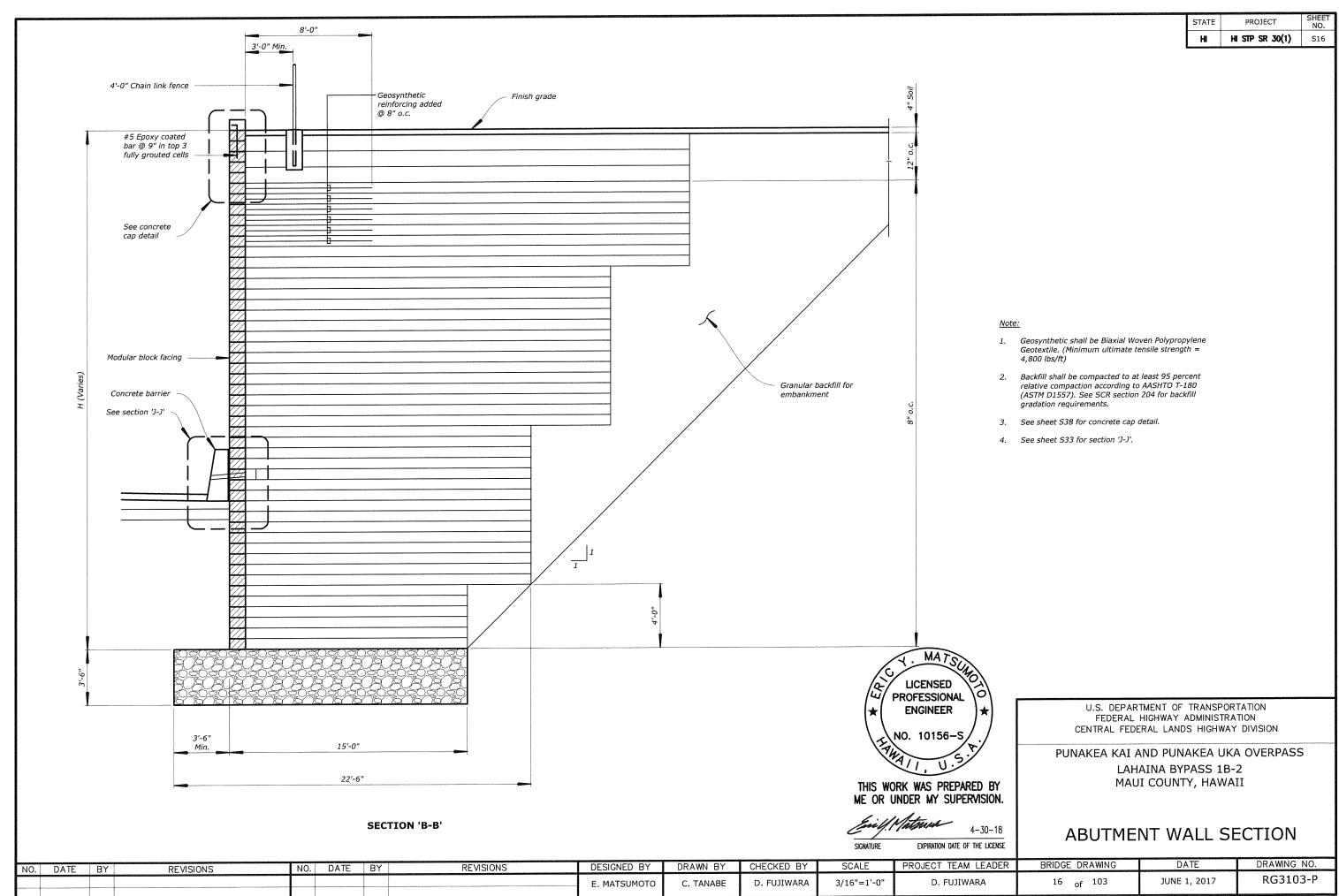












3'-0" Min. 4'-0" Chain link fence Unpaved/Grassed reinforcing added @ 8" o.c. 4" Aggregate subbase slope, 1V:2H max. #5 Epoxy coated bar @ 9" in top 3 fully grouted cells See concrete cap detail Modular block facing Concrete barrier See section 'J-J' 3'-6" 15'-0" SECTION 'C-C'

8'-0"

Note:

- Geosynthetic shall be Biaxial Woven Polypropylene Geotextile. (Minimum ultimate tensile strength = 4,800 lbs/ft)
- Backfill shall be compacted to at least 95 percent relative compaction according to AASHTO T-180 (ASTM D1557). See SCR section 204 for backfill gradation requirements.
- 3. See sheet S38 for concrete cap detail.
- 4. See sheet S33 for section 'J-J'.



THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

SIGNATURE EXPIRATION DAT

EXPIRATION DATE OF THE LICENSE

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

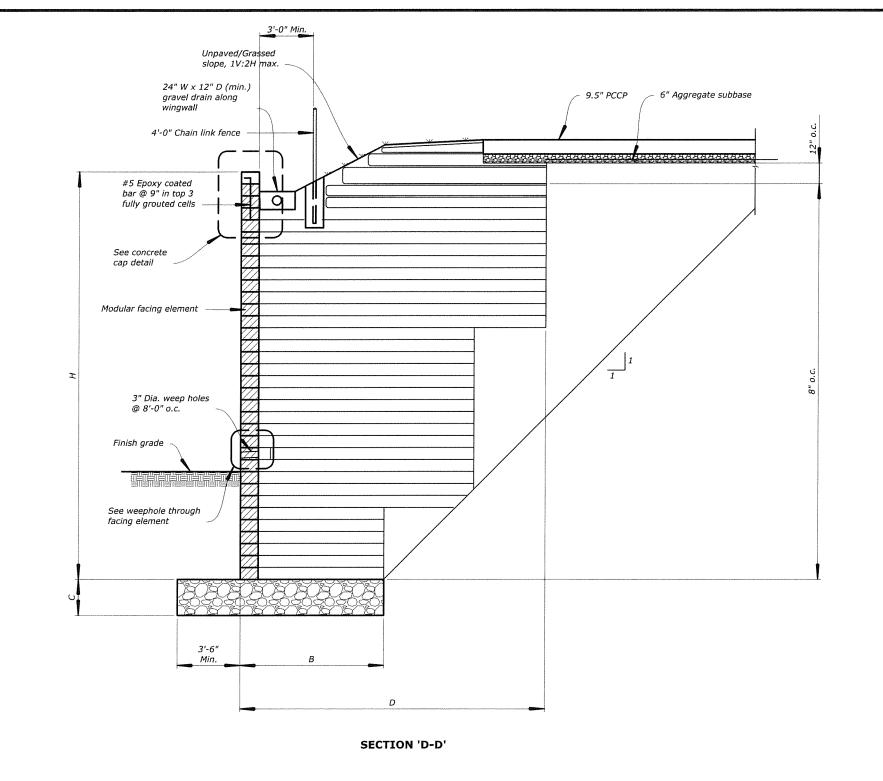
PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

ABUTMENT WALL SECTION

J.	NO. DATE E	IY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
3:12:.						-		E. MATSUMOTO	C. TANABE	D. FUJIWARA	3/16"=1'-0"	D. FUJIWARA	¹⁷ of ¹⁰³	JUNE 1, 2017	RG3103-Q

 STATE
 PROJECT
 SHEET NO.

 Hi
 Hi STP SR 30(1)
 S18



Н	В	С	D
≤ 36′-0″	15'-0"	3'-6"	22'-6"
≤ 24'-0"	8'-0"	2'-0"	17'-0"
≤ 16'-0"	5'-0"	2'-0"	12'-0"

Note:

- 1. Geosynthetic shall be Biaxial Woven Polypropylene Geotextile. (Minimum ultimate tensile strength = 4,800 lbs/ft)
- Backfill shall be compacted to at least 95 percent relative compaction according to AASHTO T-180 (ASTM D1557). See SCR section 204 for backfill gradation requirements.
- 3. See sheet S38 for concrete cap detail.
- 4. See sheet S40 for weephole through facing element.

LICENSED PROFESSIONAL ENGINEER

NO. 10156-S

NO. 10156-S

THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

Lisif Matsund 4-30-18

SIGNATURE

EXPIRATION DATE OF THE LICENSE

MAUI COUNTY, HAWAII

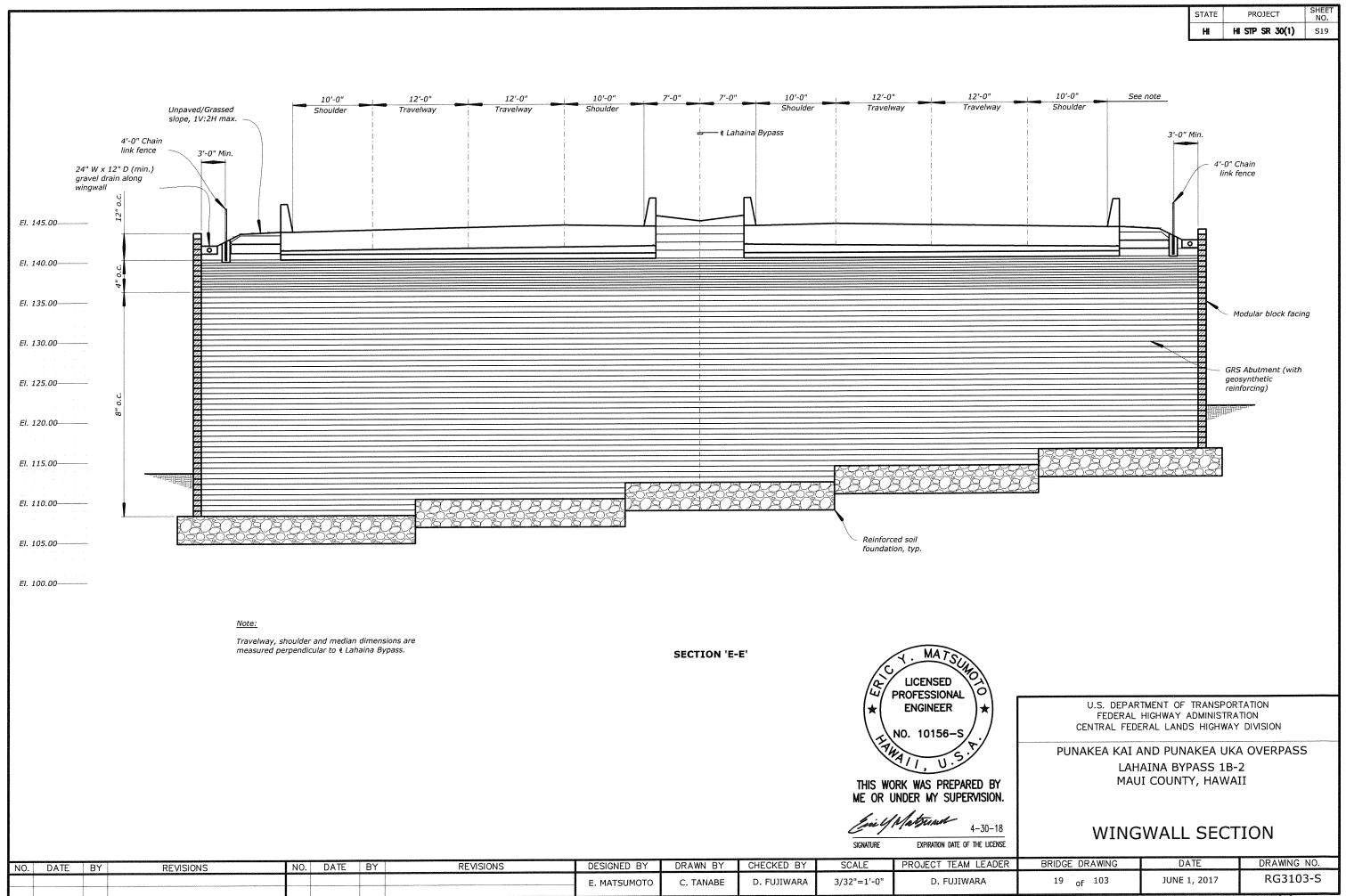
WINGWALL SECTION

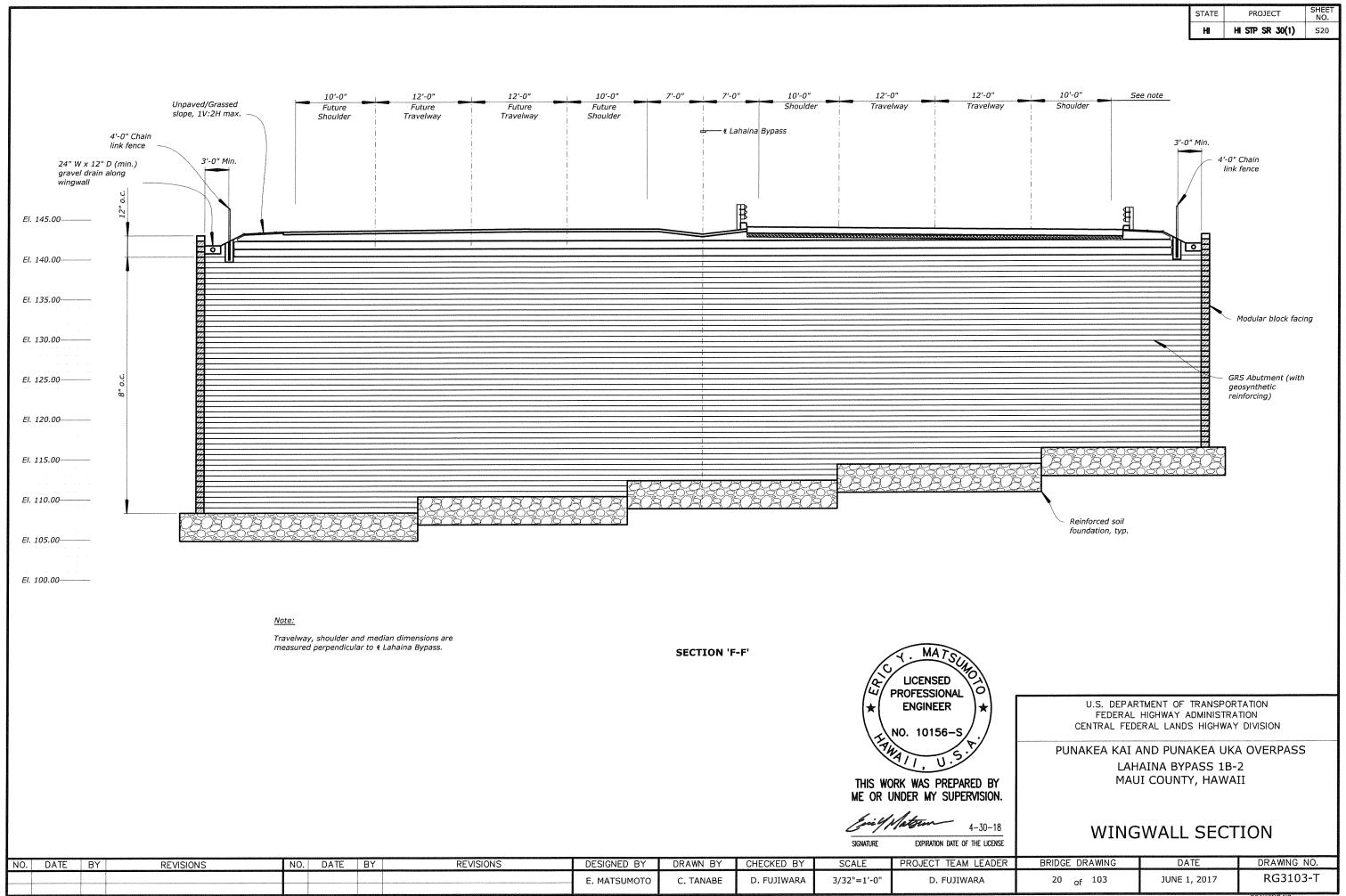
U.S. DEPARTMENT OF TRANSPORTATION

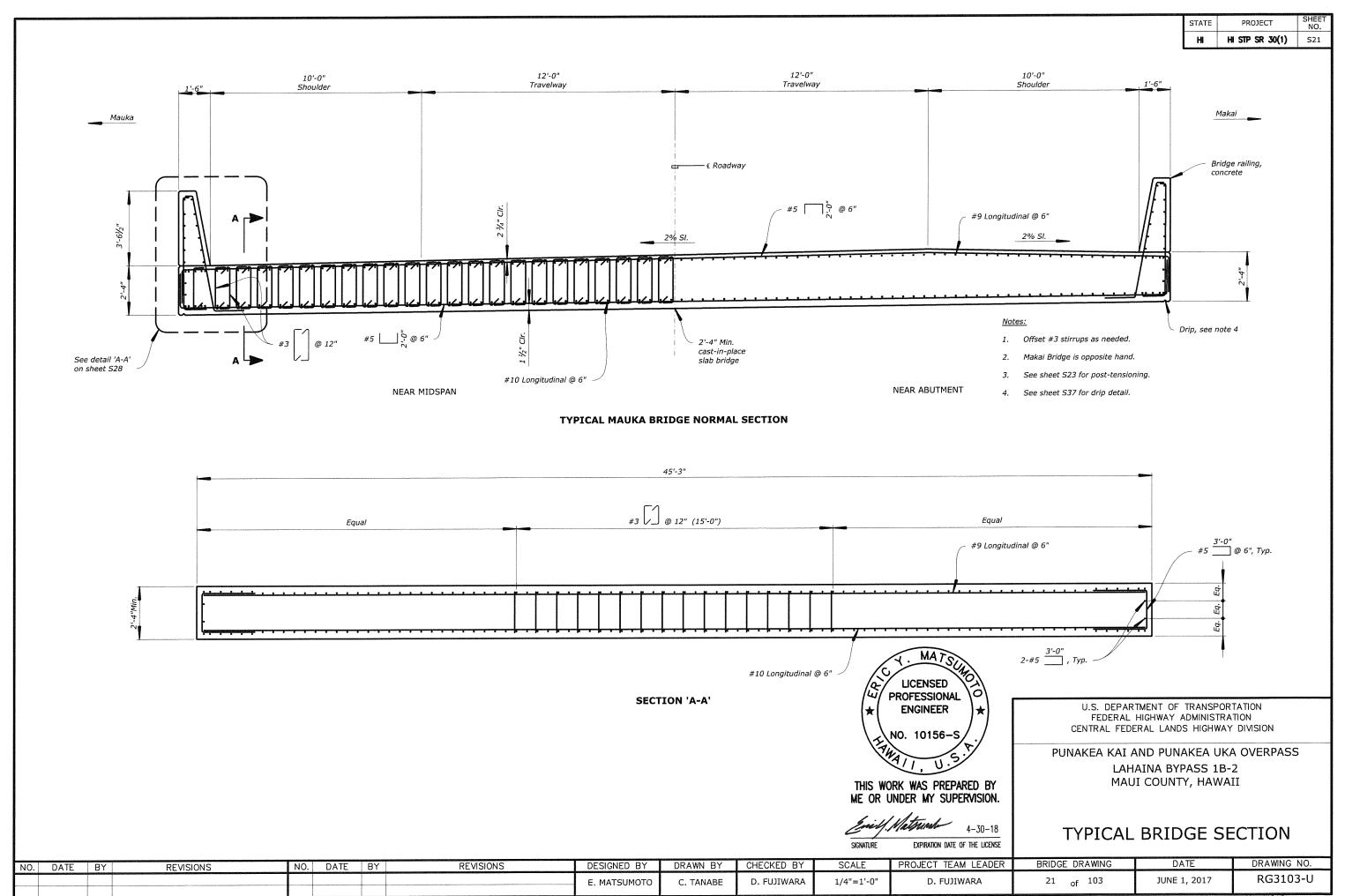
FEDERAL HIGHWAY ADMINISTRATION
CENTRAL FEDERAL LANDS HIGHWAY DIVISION

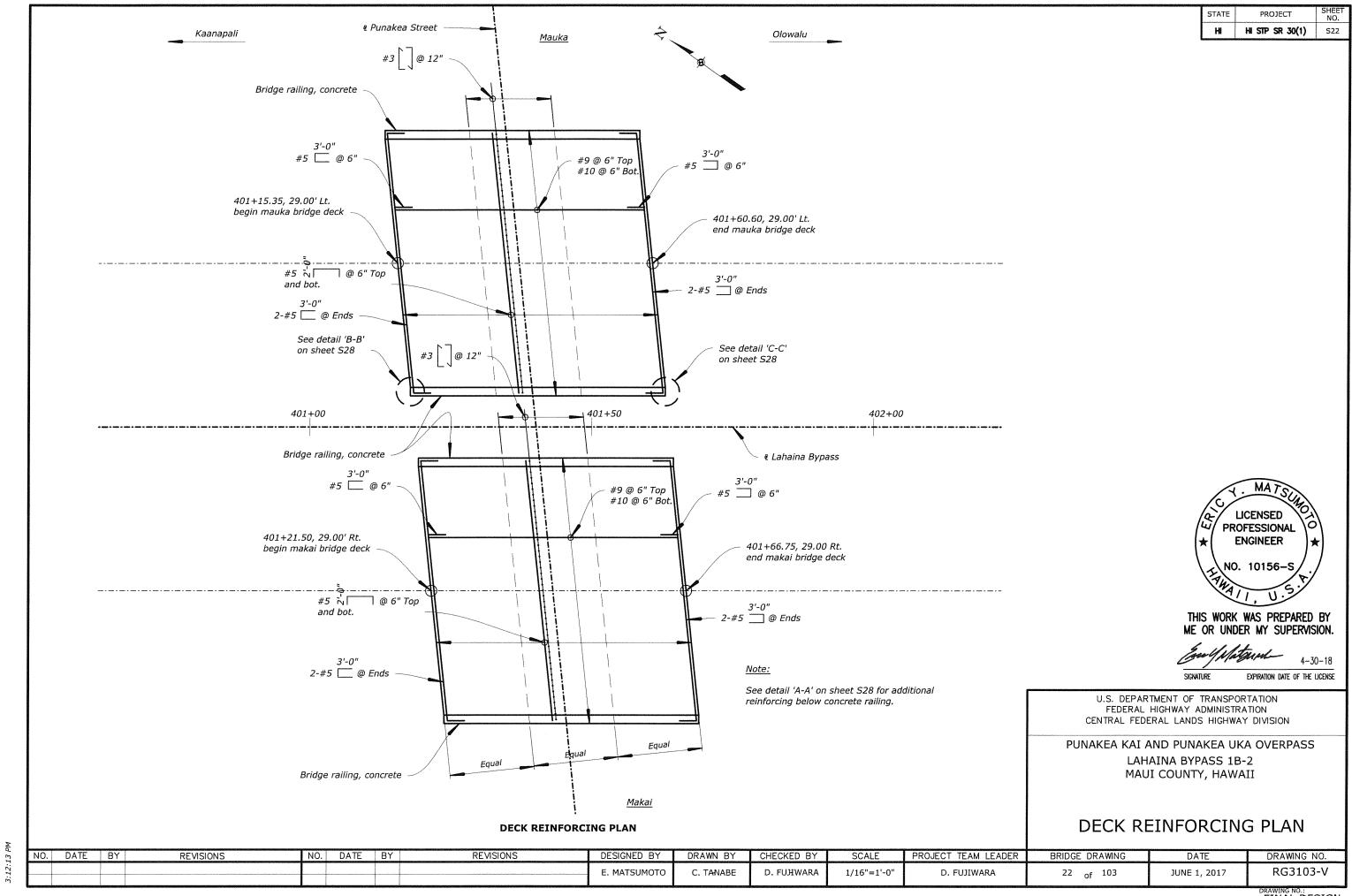
PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2

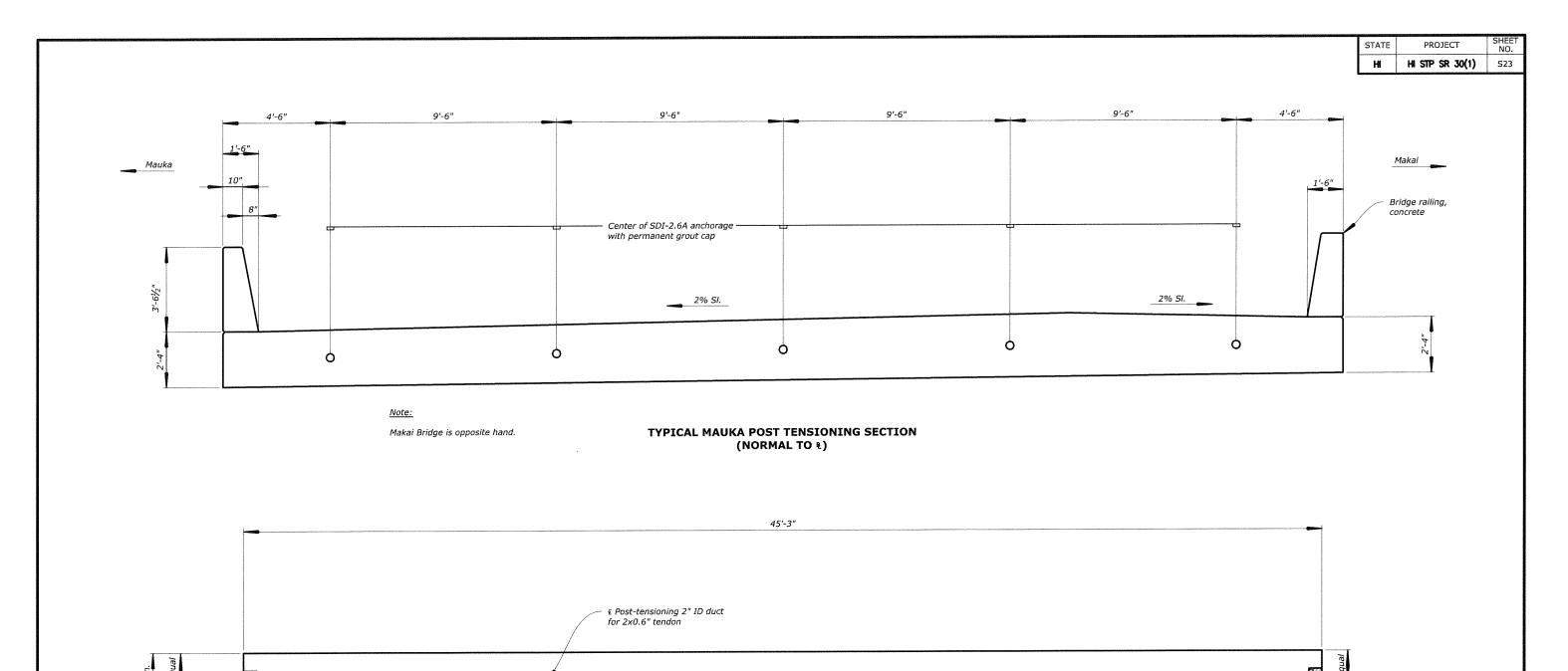
E	NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
									E. MATSUMOTO	C. TANABE	D. FUJIWARA	3/16"=1'-0"	D. FUJIWARA	18 _{of} 103	JUNE 1, 2017	RG3013-R











Note:

See sheet S25 for anchorage details.

POST TENSIONING PROFILE



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EXPIRATION DATE OF THE LICENSE

4-30-18

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

POST-TENSIONING SECTION AND PROFILE

DRAWING NO. BRIDGE DRAWING DATE NO. DATE BY REVISIONS DESIGNED BY DRAWN BY CHECKED BY SCALE PROJECT TEAM LEADER NO. DATE BY REVISIONS RG3103-W C. TANABE JUNE 1, 2017 E. MATSUMOTO D. FUHWARA 1/4"=1'-0" D. FUJIWARA 23 _{of} 103

POST-TENSIONING NOTES

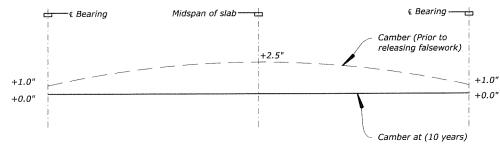
- Post-tensioning strands shall be ASTM A416 Grade 270 0.6" diameter uncoated 7 wire low relaxation steel strands.
- 2. Post-tensioning ducts shall be Spiro-Type Semi-Rigid Galvanized.
- After Post-tensioning, tendons shall be pressure grouted with Masterflow 1205 cable grout, Sika Grout 300 PT, or approved equal. Ducts shall have grouting vents at anchorage and at each high point along the tendon profile.
- 4. Ducts shall be secured to prevent misalignment or leakage during concrete pour.
- Prevent ducts, at all times, from getting plugged or damaged. Ducts shall be checked to show that ducts are clear and contain no obstructions prior to installing prestressing steel and stressing the member.
- The post-tensioning design assumptions are as follows:

Curve Friction Coefficient 0.18
Wobble Friction Coefficient 0.0002/ft.
Anchor Set 3/4"

- Stress one strand in each tendon to 30 kips at 24 hours from Kaanapali end. A minimum Compressive Strength of 2,000 psi shall be attained in the slab before the application of post-tensioning.
- 8. Stress all strands to 44 kips a minimum of 3 days after casting the concrete slab and after the concrete slab has attained a minimum compressive strength of 3,000 psi from Kaanapali end.
- Post-tensioning shall comply with section 10 of AASHTO LRFD Bridge Construction Specifications including the latest interim revisions unless otherwise noted.
- 10. Prior to grouting and within two days after installing strands, a corrosion inhibitor amine carboxylate powder (Cortec MCI-309) shall be blown into the ducts.
- Purpose of post-tensioning is to relieve bond stresses and avoid tensile shrinkage stresses.
- 12. The force in Prestressing Steel shall not be less than shown in note 8 above.

The force in the Prestressing Steel shall be considered as the smaller of the two values as determined by the measured elongation and the gage pressure. If the difference in stress as obtained by the measured elongation and the measured gage pressure exceeds 5 percent of the required prestressing force, the stressing process shall be terminated and shall not resume until the Contractor submits data indicating the cause of such difference and makes corrections, approved by the CO, to rectify such difference.

- 13. See Specifications Section 553 for additional requirements.
- 14. See sheet S41 for construction sequence.



CAMBER DIAGRAM



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NATURE EXPIRATION DATE OF THE LICENSE

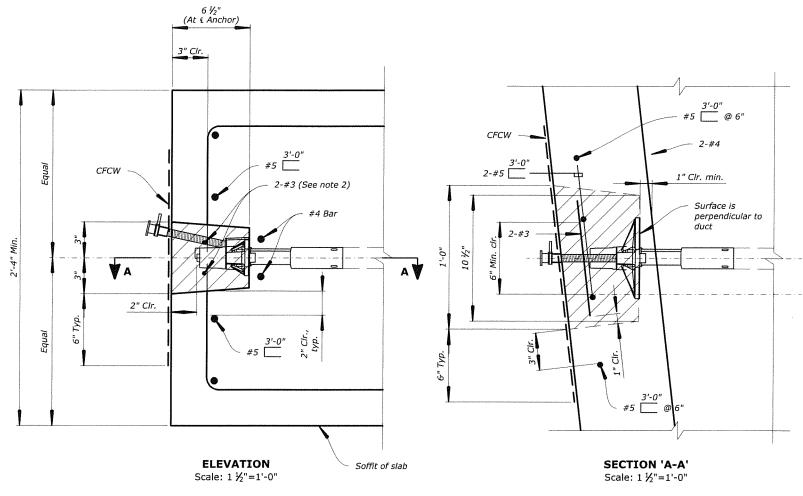
U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION
CENTRAL FEDERAL LANDS HIGHWAY DIVISION

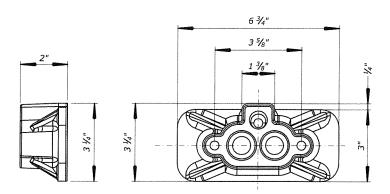
PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

POST-TENSIONING NOTES AND CAMBER DIAGRAM

NO. DATE BY REVISIONS	NO. DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
				E. MATSUMOTO	C. TANABE	D. FUJIWARA	NO SCALE	D. FUJIWARA	24 _{of} 103	JUNE 1, 2017	RG3103-X







2.6A ANCHORAGE Scale: 3"=1'-0"

<u>Legend</u>



Indicates pour back with Duratop gel or equivalent

Notes:

- 1. Remove grout tube prior to pour back.
- 2. Place 2-#3
- 3. Coat pocket and anchorage with Duralprep ac or equivalent.
- 4. Pour back pocket with Duraptop gel or equivalent.
- 5. Apply CFCW.



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100 4-30-18

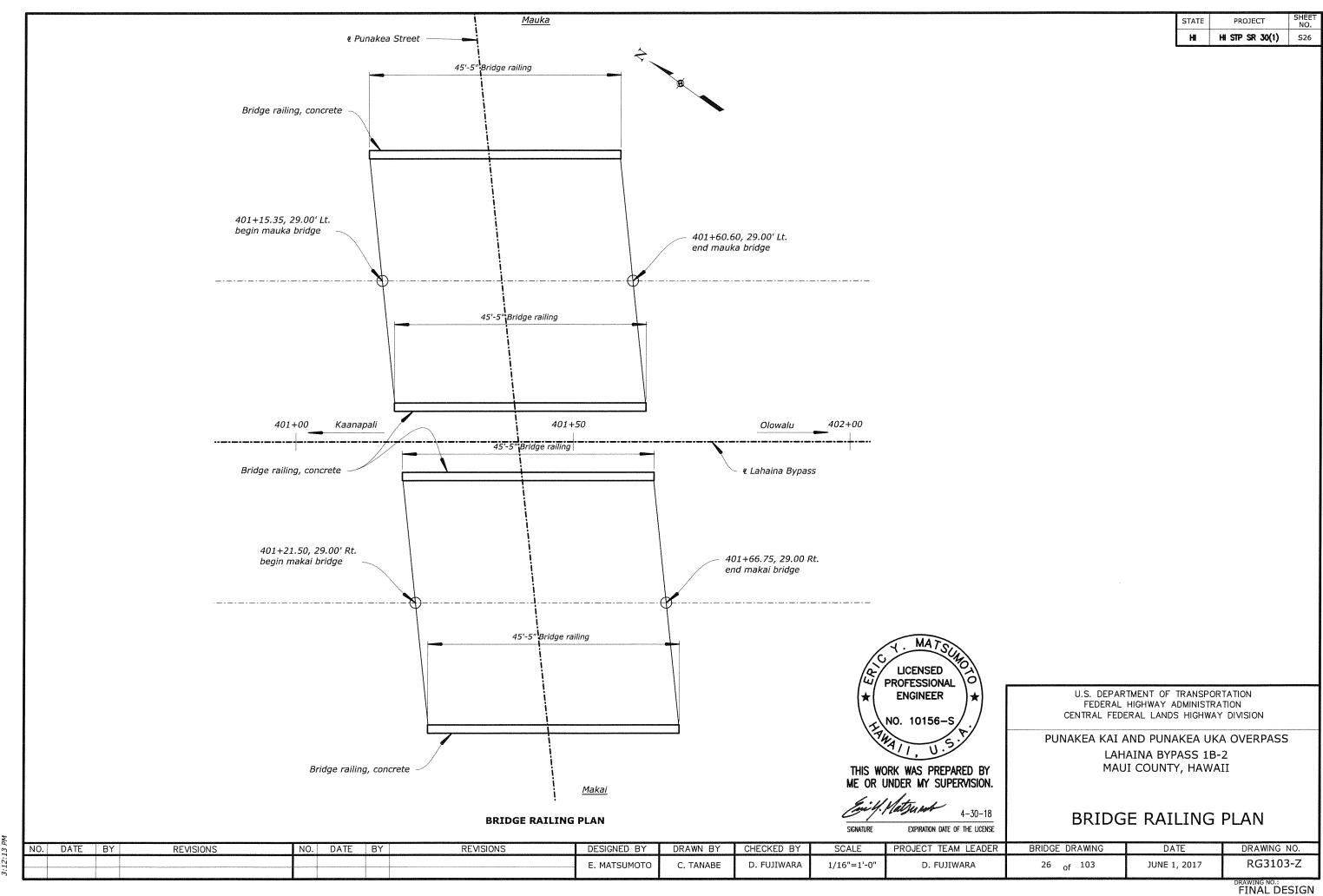
EXPIRATION DATE OF THE LICENSE

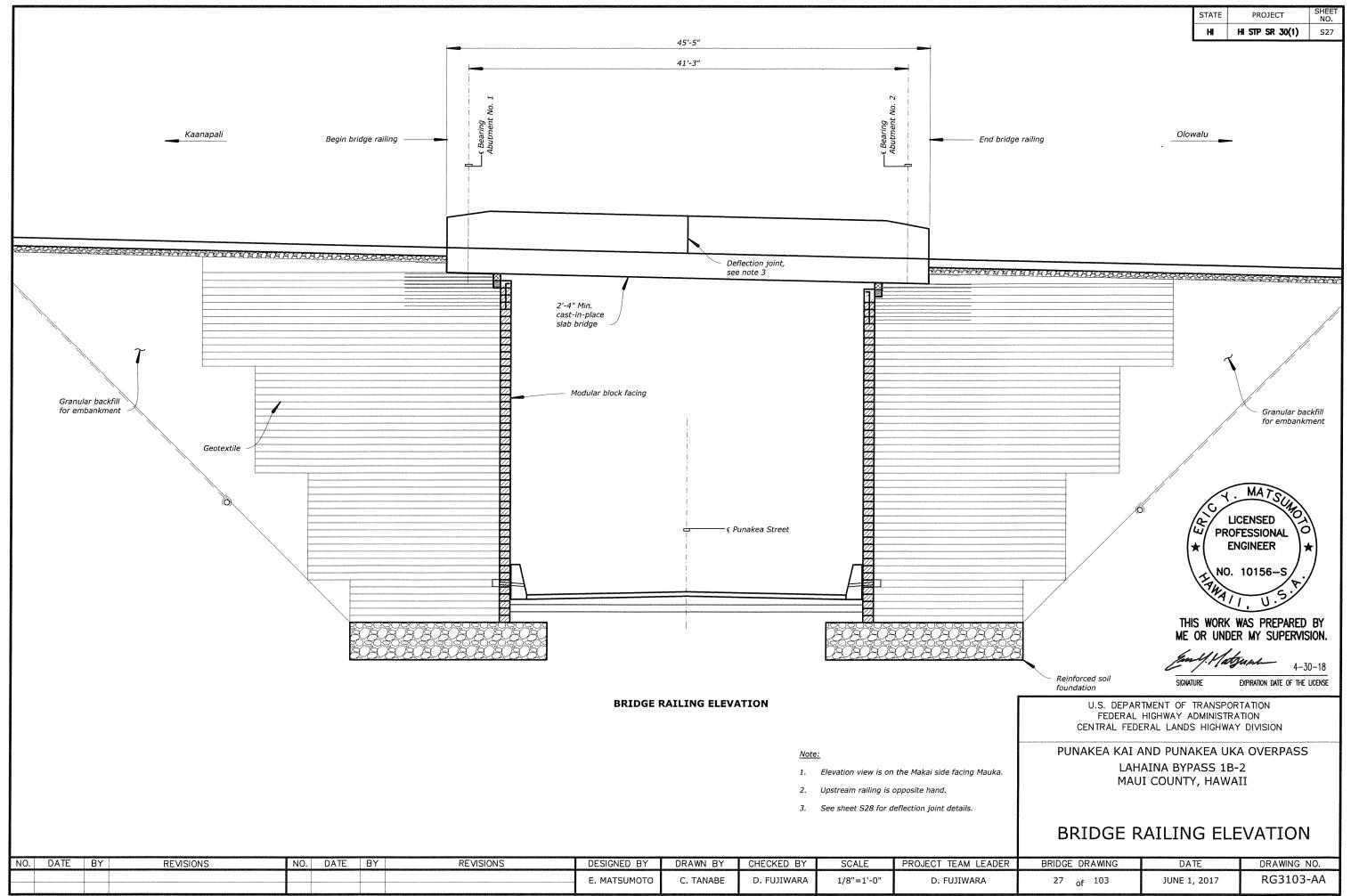
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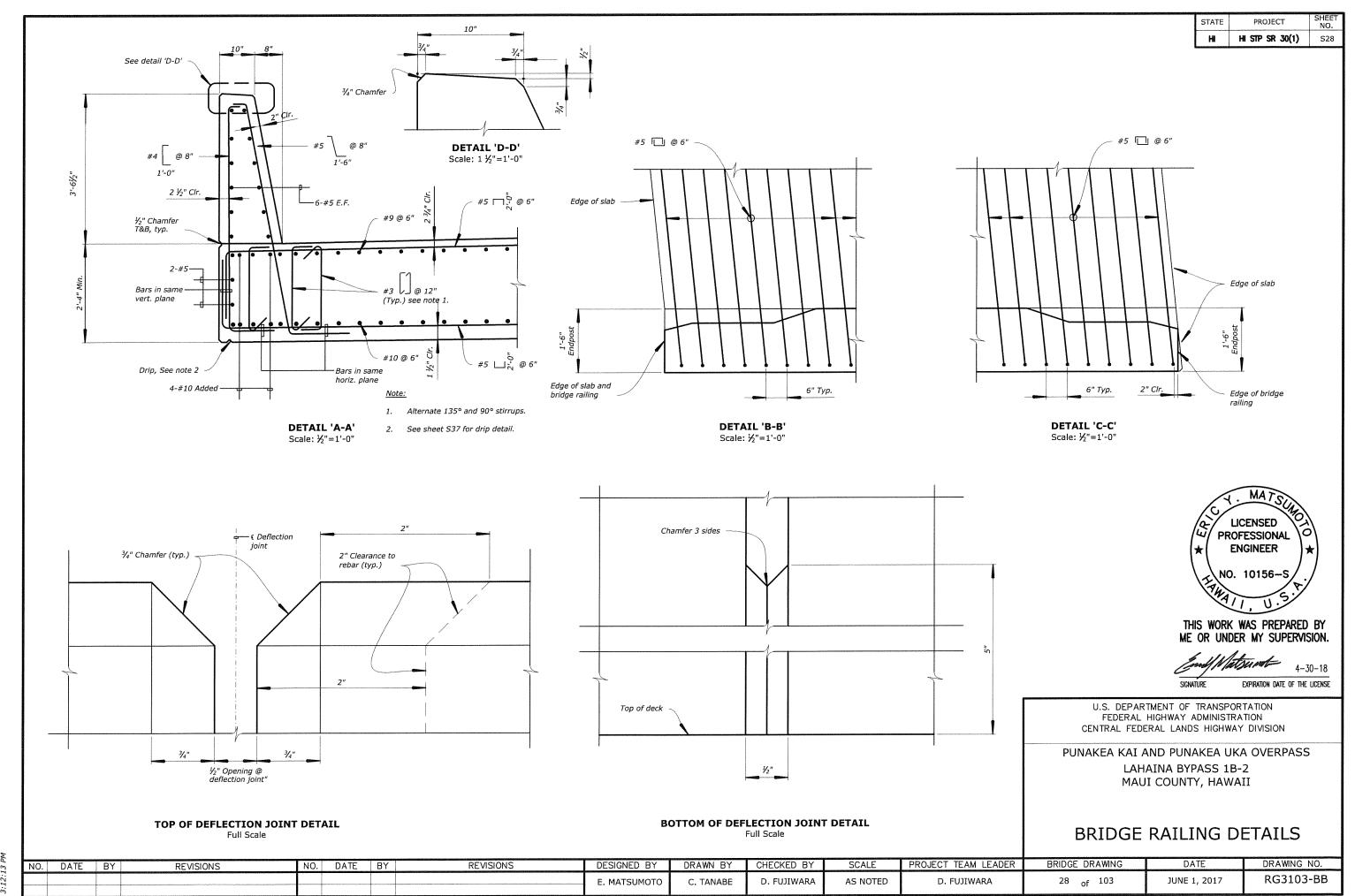
PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

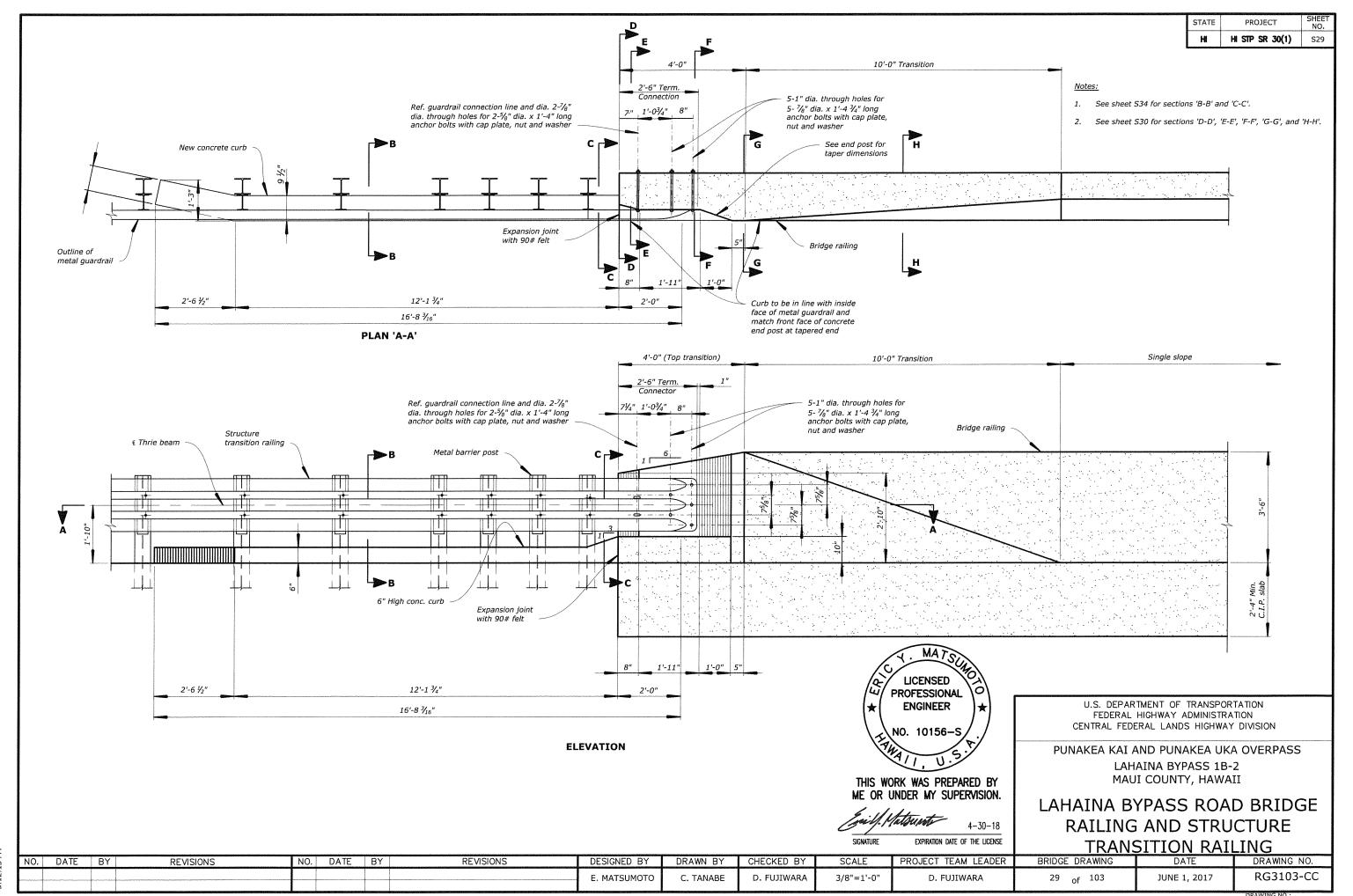
ANCHORAGE SYSTEM SECTIONS AND DETAILS

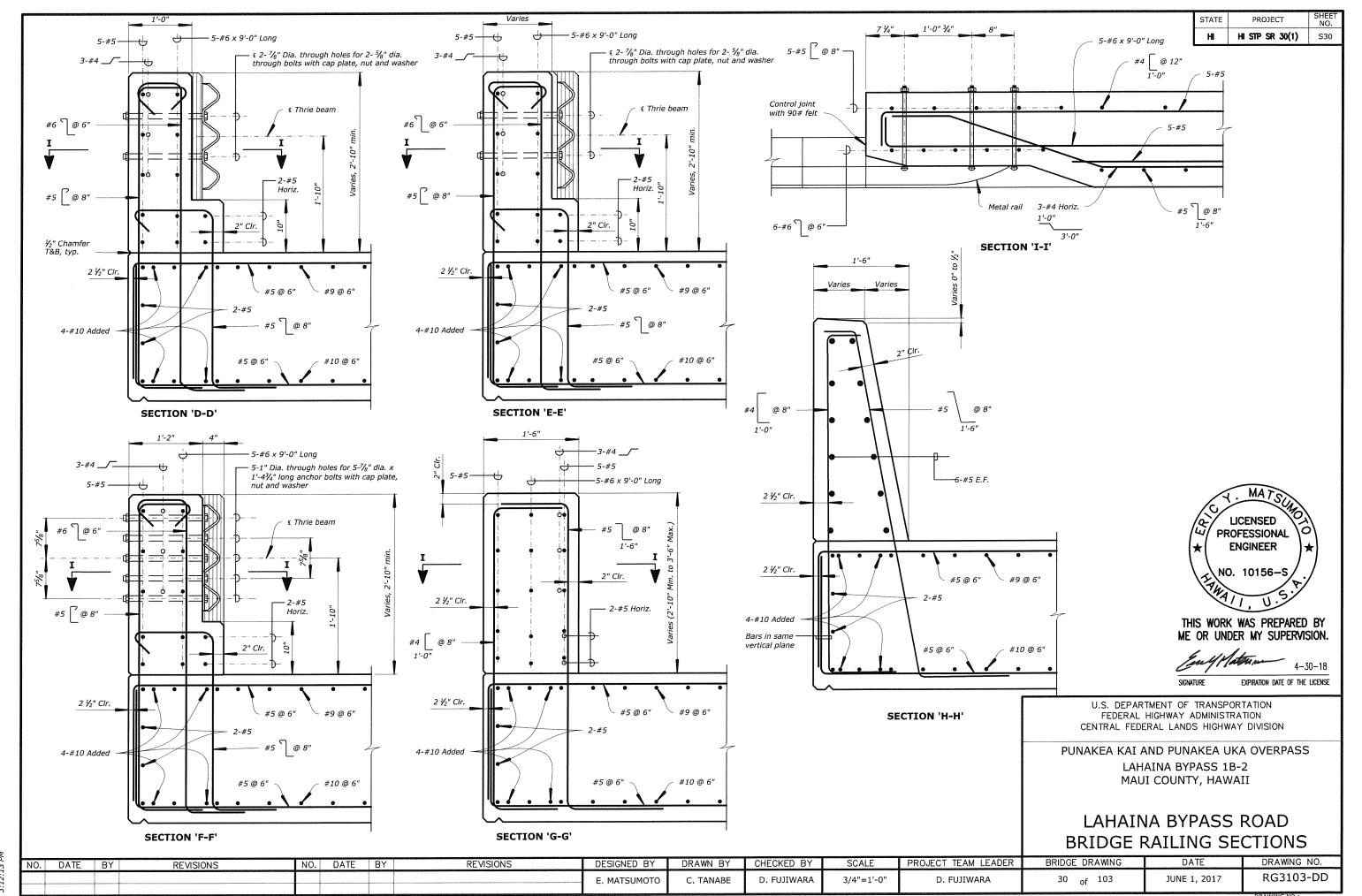
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								E. MATSUMOTO	C. TANABE	D. FUJIWARA	AS NOTED	D. FUJIWARA	25 _{of} 103	JUNE 1, 2017	RG3103-Y

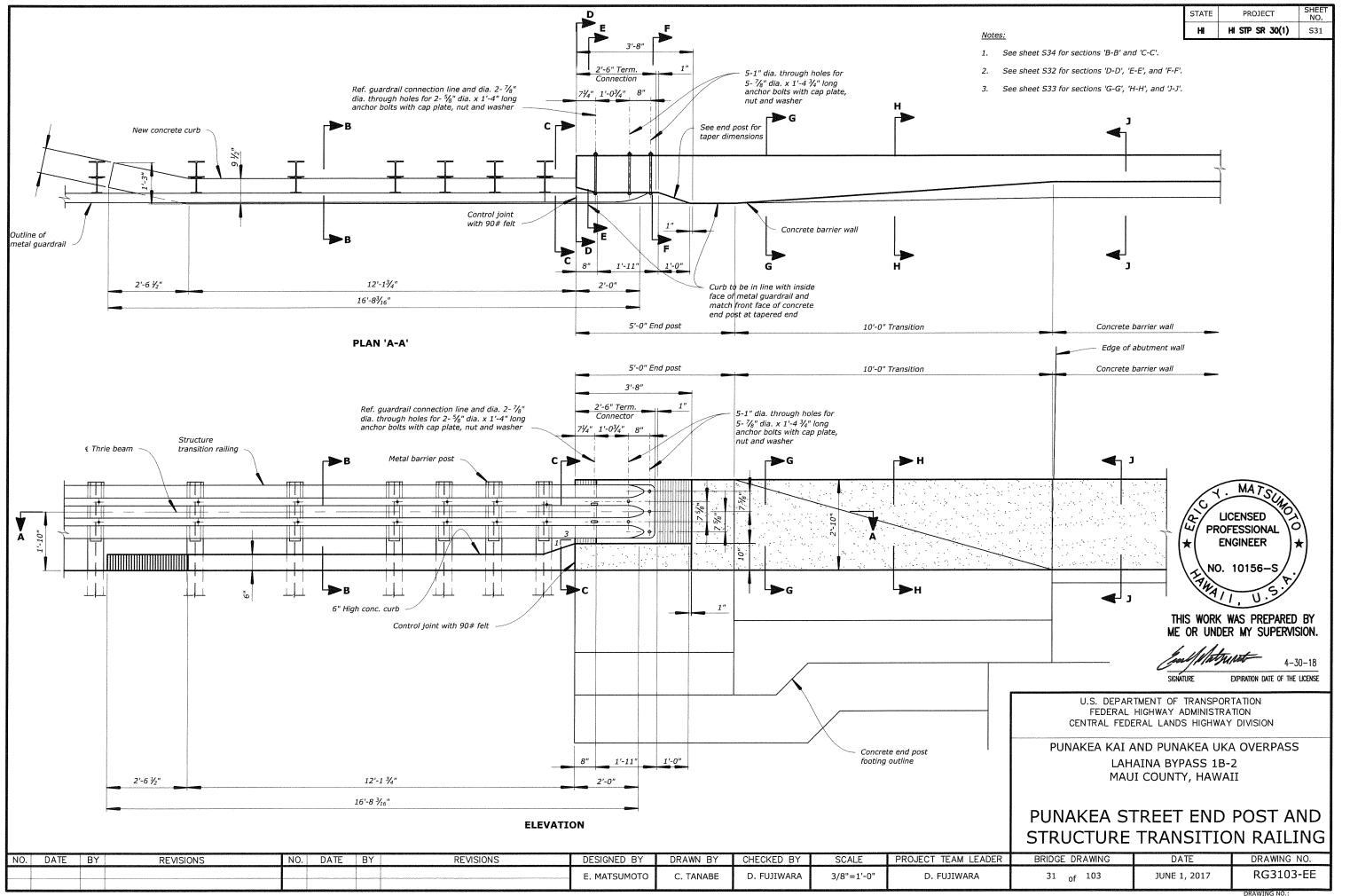


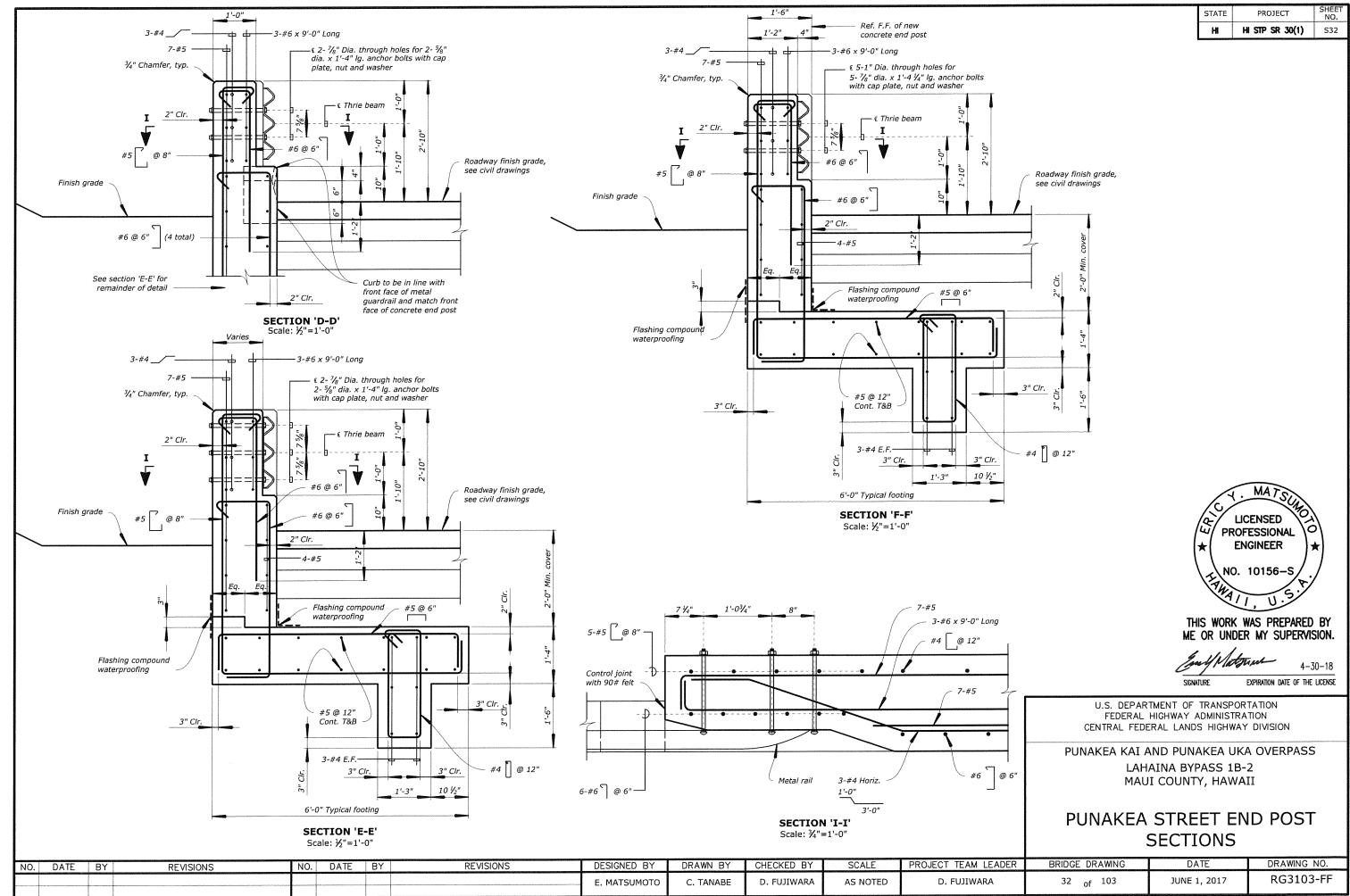


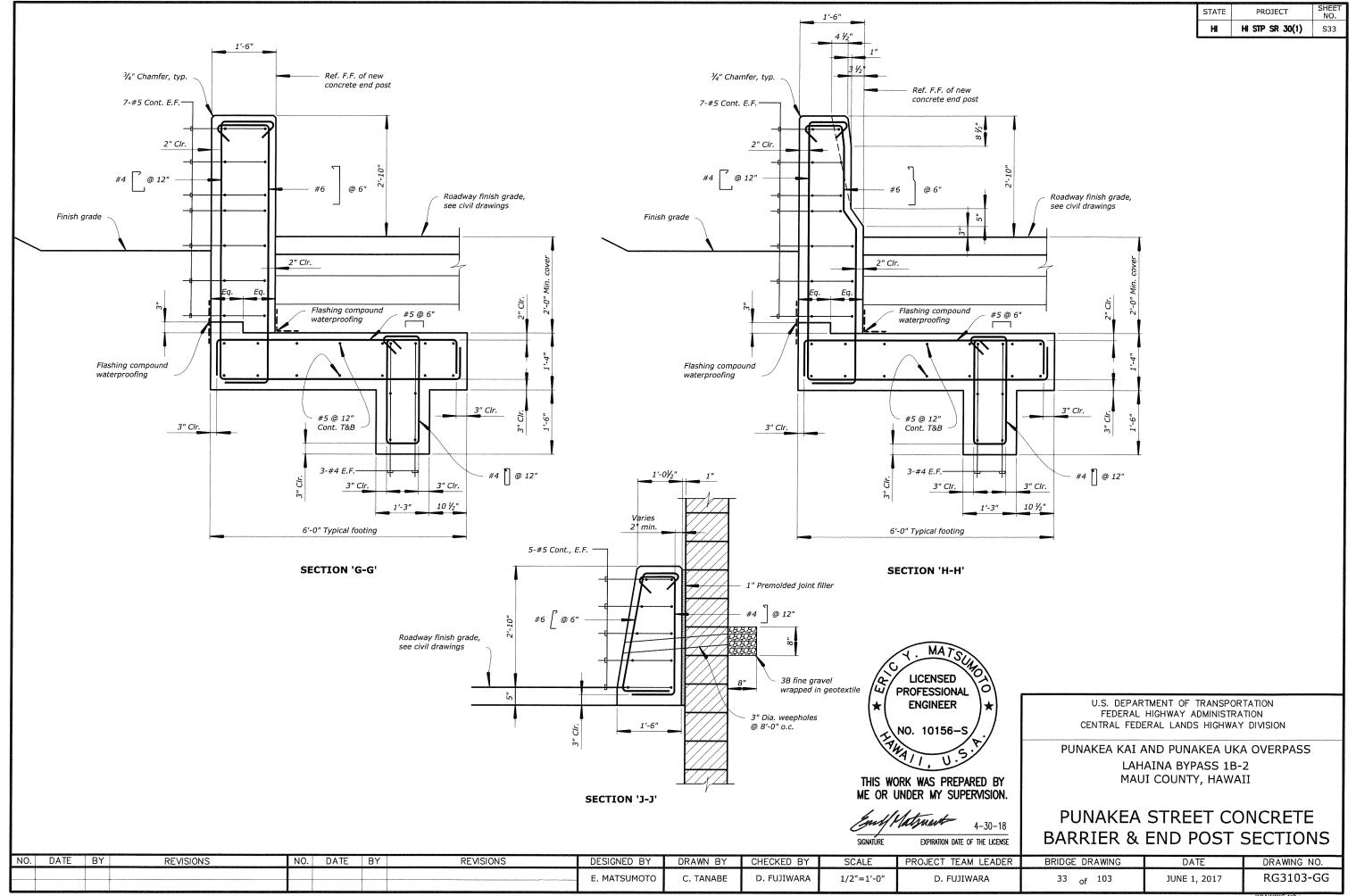


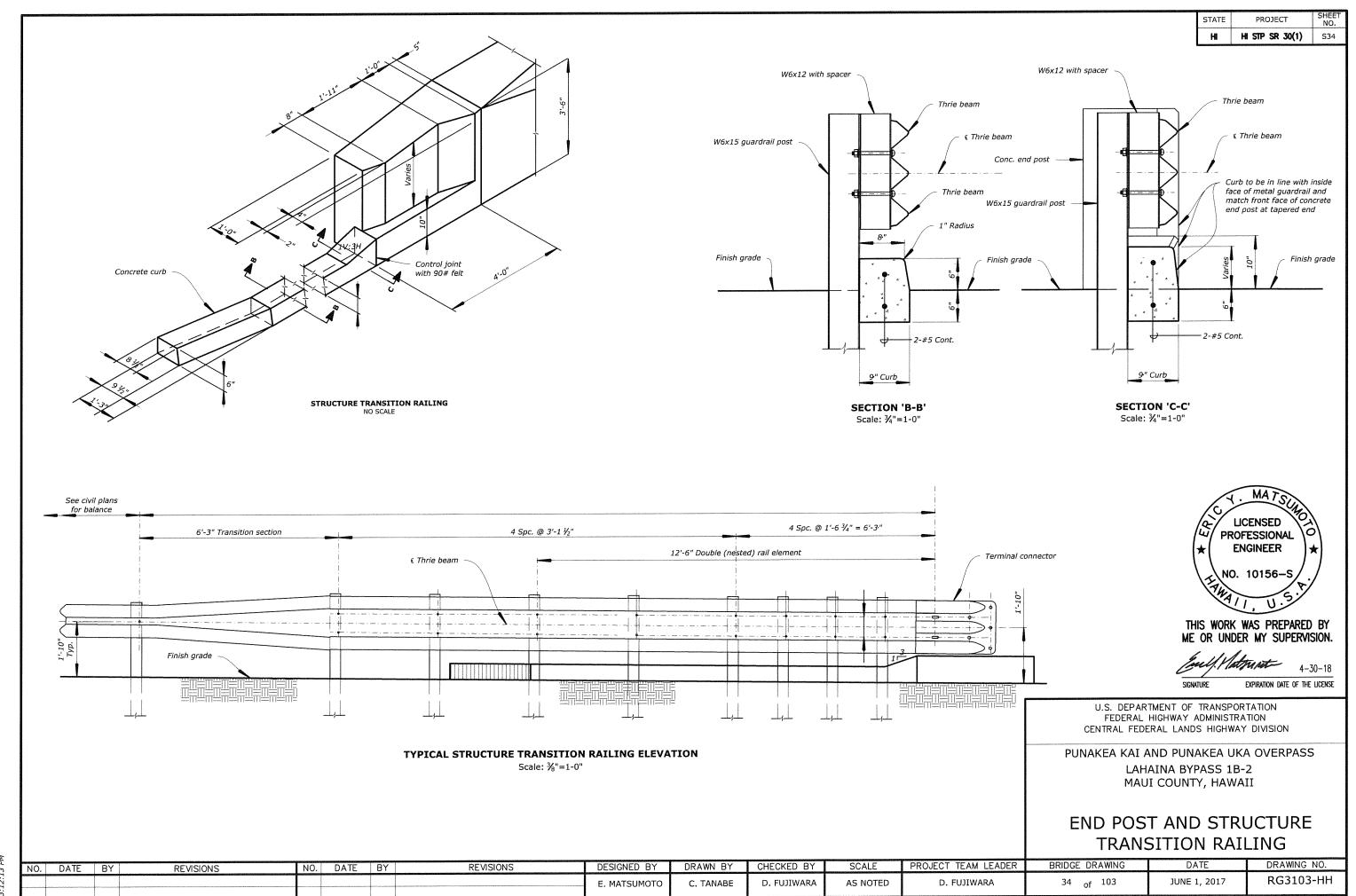


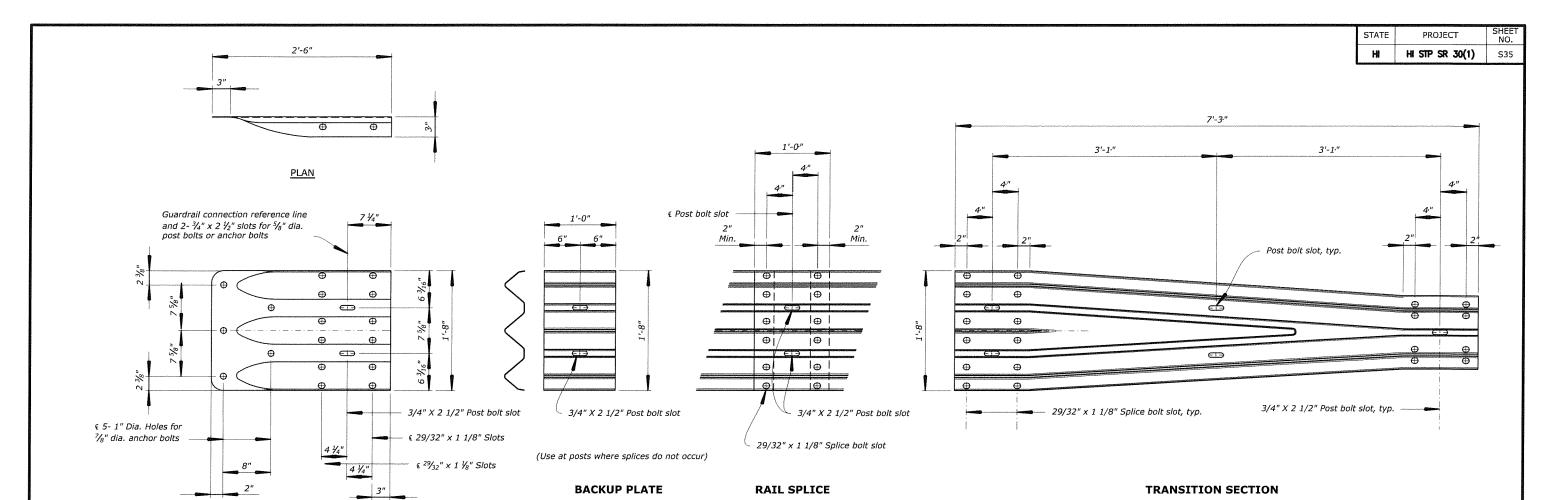












ELEVATION

2'-6"

TERMINAL CONNECTOR

The work percent to connect a

NOTES:

- The work necessary to connect guardrail to concrete end post shall include all labor, materials, tools, equipment and incidentals necessary to complete the work and will not be paid for separately.
- 2. Lap terminal connector and rail element in direction of traffic to prevent snagging.
- 3. Splice bolt (FBB01), rail bolt (FBB02), and recessed nut shall conform to AASHTO M180 and be hot-dip galvanized in accordance with AASHTO M232, Class C.
- 4. Post hex bolt and nut shall conform to Section 617.
- "Terminal Connector" and standard spacer, including all anchor bolts, nuts and washers, shall be hot-dip galvanized after fabrication.
- "Terminal Connector", "Transition Section" and thrie beam shall be fabricated from 10 gauge steel conforming to the requirements of AASHTO M180, Type II, Class B. All other rail shall conform to the requirements of AASHTO M180, Type II, Class A.
- 7. First 25'-0" of guardrail adjoining "Terminal Connector" shall be galvanized steel and supports spaced as shown on the detail drawings. This section of rail shall be placed on tangent to end post or parallel to roadway, unless conditions at site renders it impossible to do so. Flare point to be determined in field.
- 8. Double (nest 1st panel) thrie beam elements at all end post connections, except on highways with one-way traffic pattern, use single thrie beam elements at end post on trailing end only.
- 9. Where double (nested) beam occur, 12" "Back-Up Plate" not required.
- 10. Heads of through anchor bolts shall be placed on the traffic side of the rail.
- 11. W-beam structural steel shall conform to ASTM A992. Structural steel plates and bars shall conform to ASTM A36.
- 12. No bolts are to be cut after manufacturing and galvanizing.



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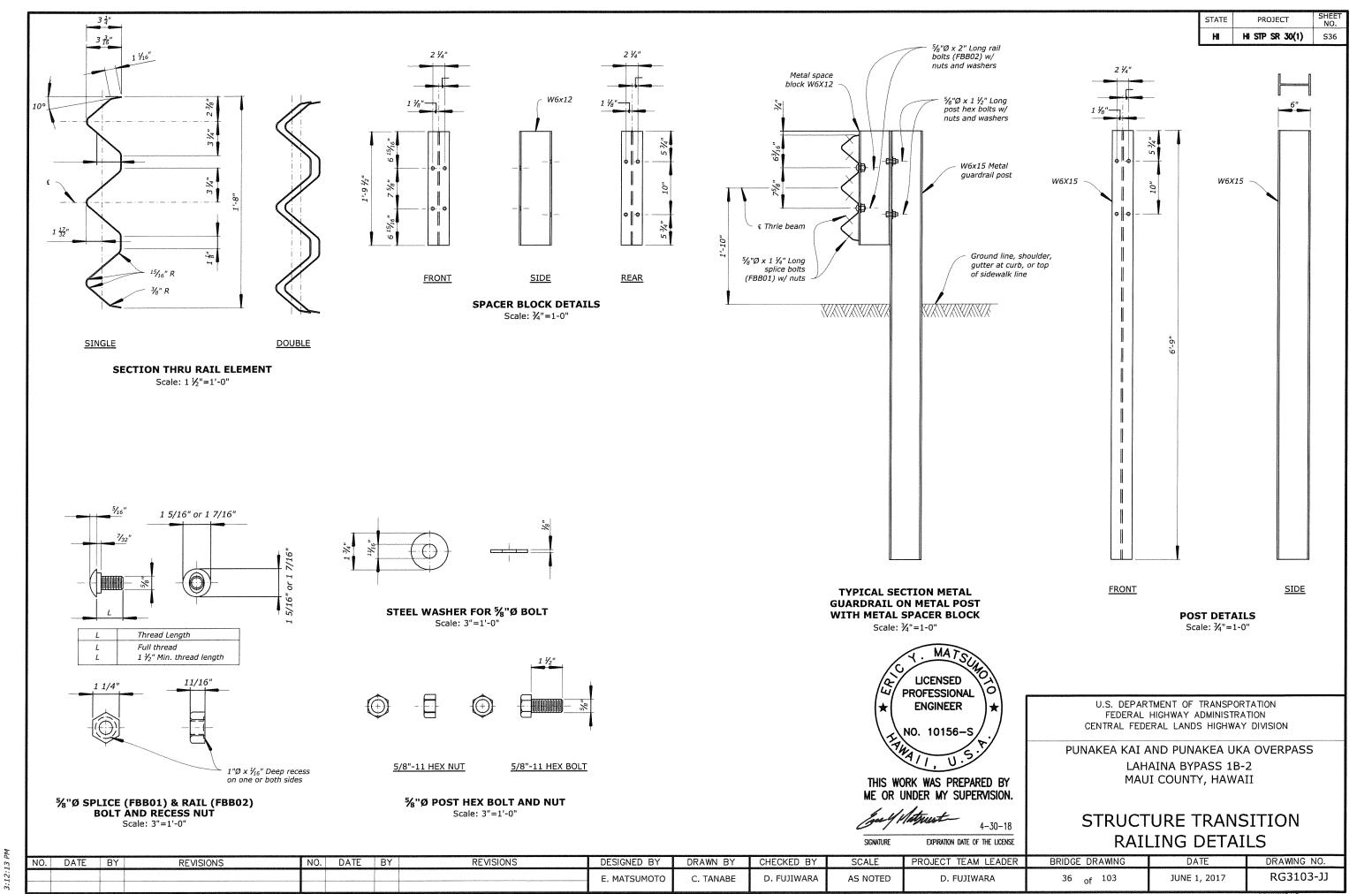
SIGNATURE EXPIRATION DATE OF THE LICENSE

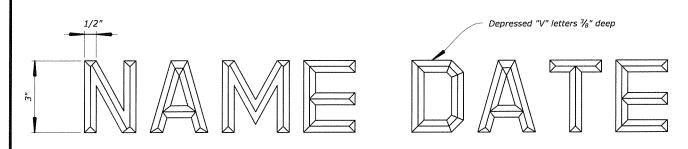
U.S. DEPARTMENT OF TRANSPORTATION
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CENTRAL FEDERAL LANDS HIGHWAY DIVISION

PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

STRUCTURE TRANSITION RAILING NOTES AND DETAILS

E. MATSUMOTO C. TANABE D. FUJIWARA 3/4"=1'-0" D. FUJIWARA 35 of 103 JUNE 1, 2017 RG3103-II	NC	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
						arehan alian arang			E. MATSUMOTO	C. TANABE	D. FUJIWARA	3/4"=1'-0"	D. FUJIWARA	35 _{of} 103	JUNE 1, 2017	RG3103-II





NAME OF BRIDGE

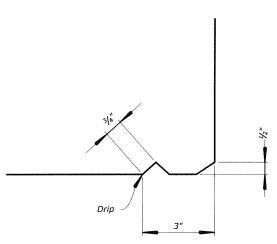
DATE OF YEAR BUILT

Notes:

- Exact details and spacing of letter and location shall be as directed by the CO. Gothic letters and figures approximating the dimensions shown will be acceptable if approved by the CO.
- 2. Name and date shall be placed on the trailing (exit) end post on each side of bridge.
- Unless otherwise directed by the CO, the names of the bridges shall be "Punakea Uka Overpass" for mauka bridge and "Punakea Kai Overpass" for makai bridge. The year shall be the year at completion.

BRIDGE IDENTIFICATION DETAIL

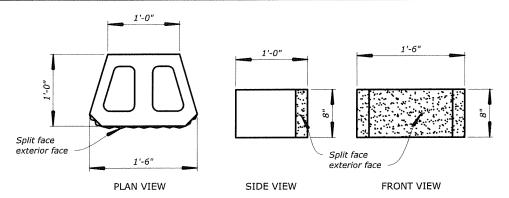
Scale: 3"=1'-0"



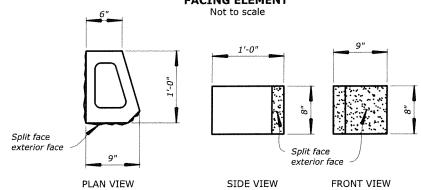
Note

End continuous drip under concrete deck 6" from front face of abutment.

DRIP DETAIL
Scale: 3"=1'-0"

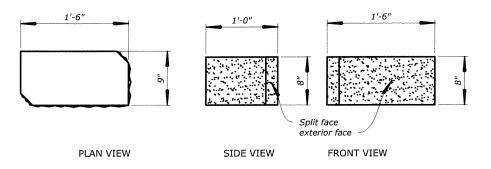


TYPICAL GRS FULL MODULAR BLOCK FACING ELEMENT



TYPICAL GRS HALF MODULAR BLOCK FACING ELEMENT

Not to scale



TYPICAL GRS CORNER MODULAR BLOCK FACING ELEMENT

Not to scale



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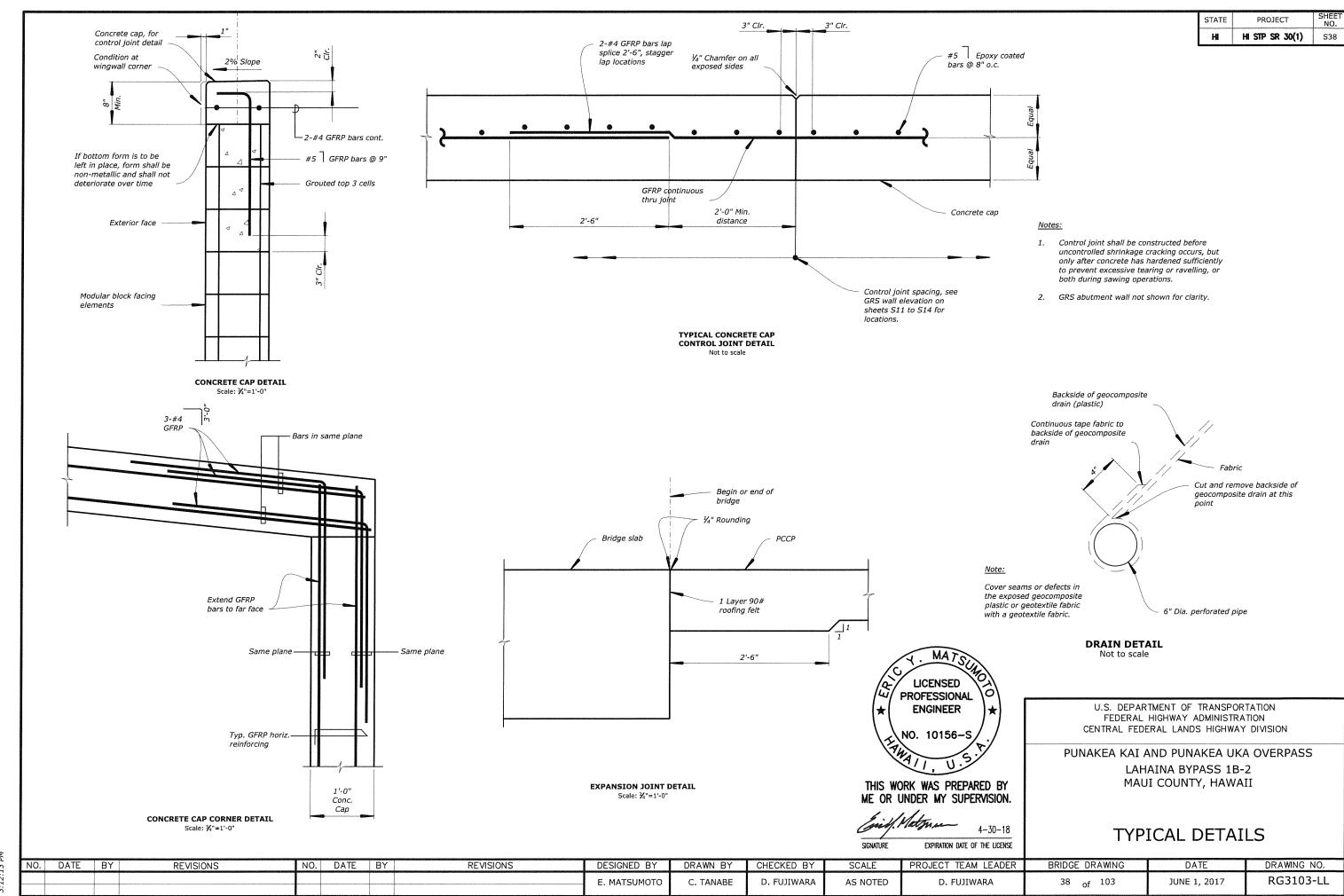
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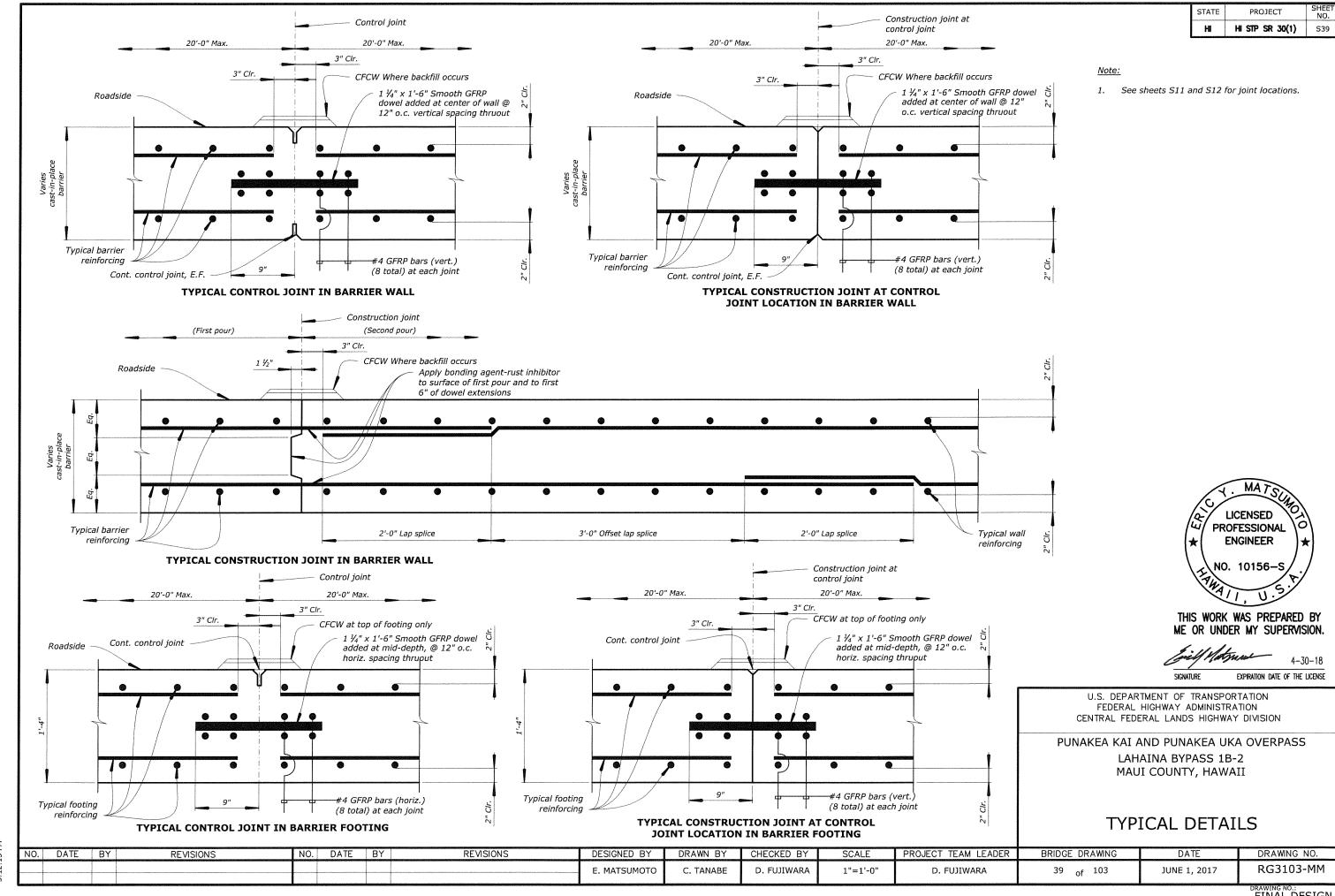
U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

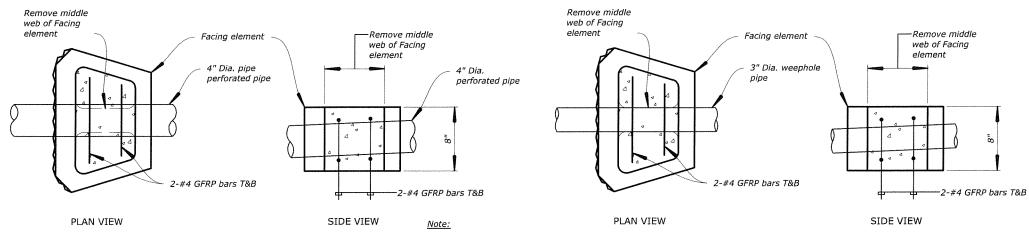
PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

TYPICAL DETAILS

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						***************************************			E. MATSUMOTO	C. TANABE	D. FUJIWARA	AS NOTED	D. FUJIWARA	37 _{of} 103	JUNE 1, 2017	RG3103-KK







PERFORATED PIPE THROUGH MODULAR BLOCK **FACING ELEMENT**

Scale: 1"=1'-0"

1. All GFRP bars shall have a minimum concrete cover of 3/4".

2. Fill facing element with perforated pipe with grout.

PERFORATED PIPE THROUGH FACING ELEMENT

LICENSED

PROFESSIONAL

ENGINEER

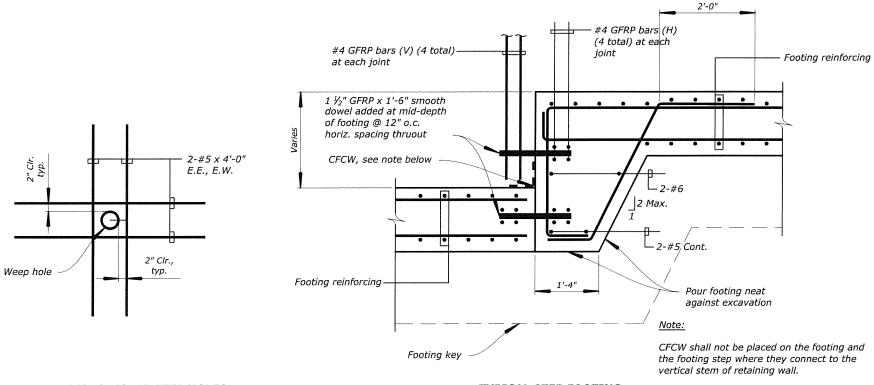
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Scale: 1"=1'-0"



ADDED REINFORCING AT WEEP HOLES

No Scale

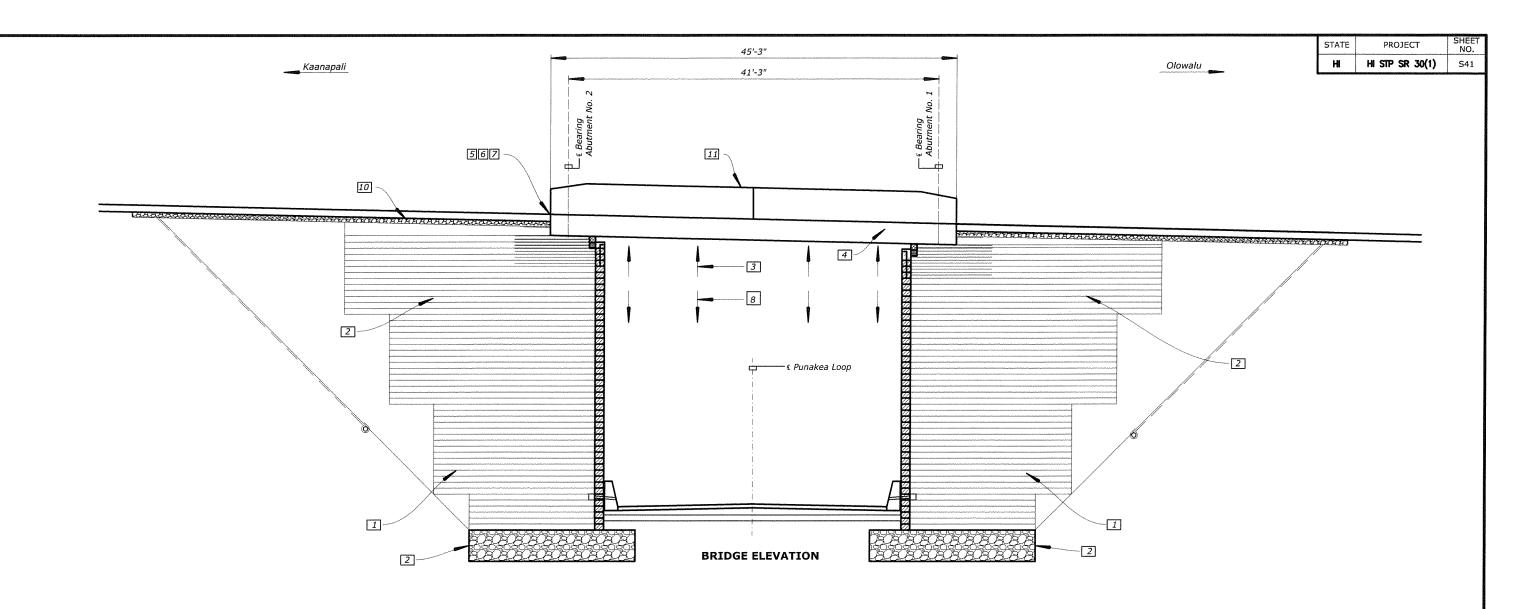
TYPICAL STEP FOOTING No Scale

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PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

TYPICAL DETAILS

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,	NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
3.12.1									E. MATSUMOTO	C. TANABE	D. FUJIWARA	AS NOTED	D. FUJIWARA	40 _{of} 103	JUNE 1, 2017	RG3103-NN



CONSTRUCTION SEQUENCE:

- Excavate for RSF (Reinforced Soil Foundation).
- 2 Construct RSF and GRS system.
- 3 Erect falsework and forms.
- 4
- Post-tension one strand to 30k in each tendon at fc=2,000 psi and 24 hours after casting slab in stage $\boxed{4}$.
- Post-tension remainder of slab tendons to 44k after 3 days and f'c = 3,000 psi.
- 7 Grout tendons and fill pockets.
- Release falsework and forms after 14 days from stage 5
- 9 Waterproof joints.
- 10 Backfill.
- Construct bridge railing.
- Grind and groove deck slab.
- Bridge may be opened for traffic after the concrete slab and bridge railing have attained a compressive strength of 6,000 psi and 5,000 psi, respectively and at least 10 days after the newest pour.

CONSTRUCTION SEQUENCE NOTES:

- 1. Order of construction sequence shall not be changed.
- 2. Each sequence stage shall be completely finished before proceeding to the next stage unless otherwise noted. The CO will be the sole judge of whether the sequence stage is complete, and may direct the Contractor to stop work on a sequence stage to complete work on the preceeding sequence
- Contractor shall submit overweight vehicular details for approval prior to their use.



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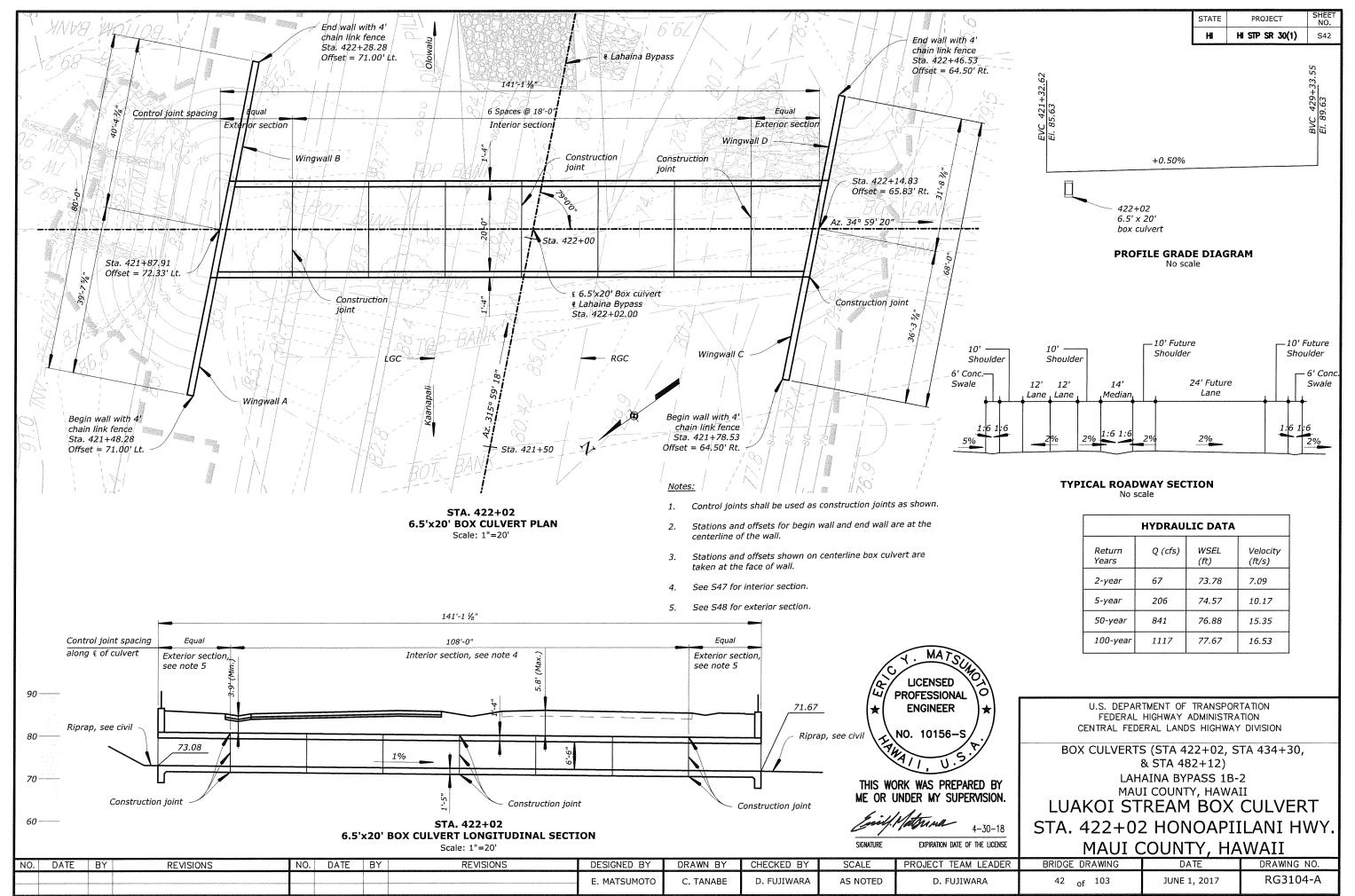
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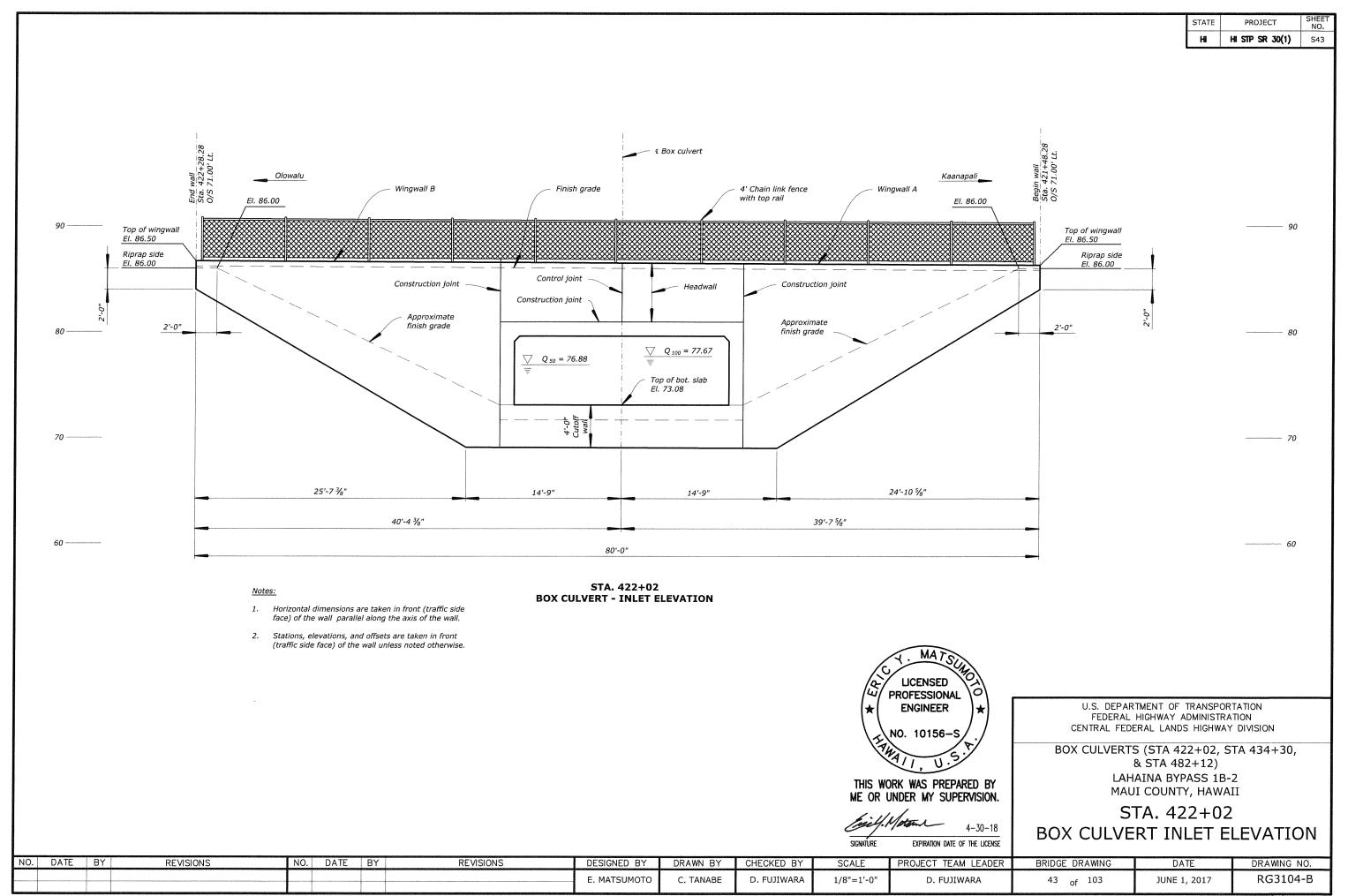
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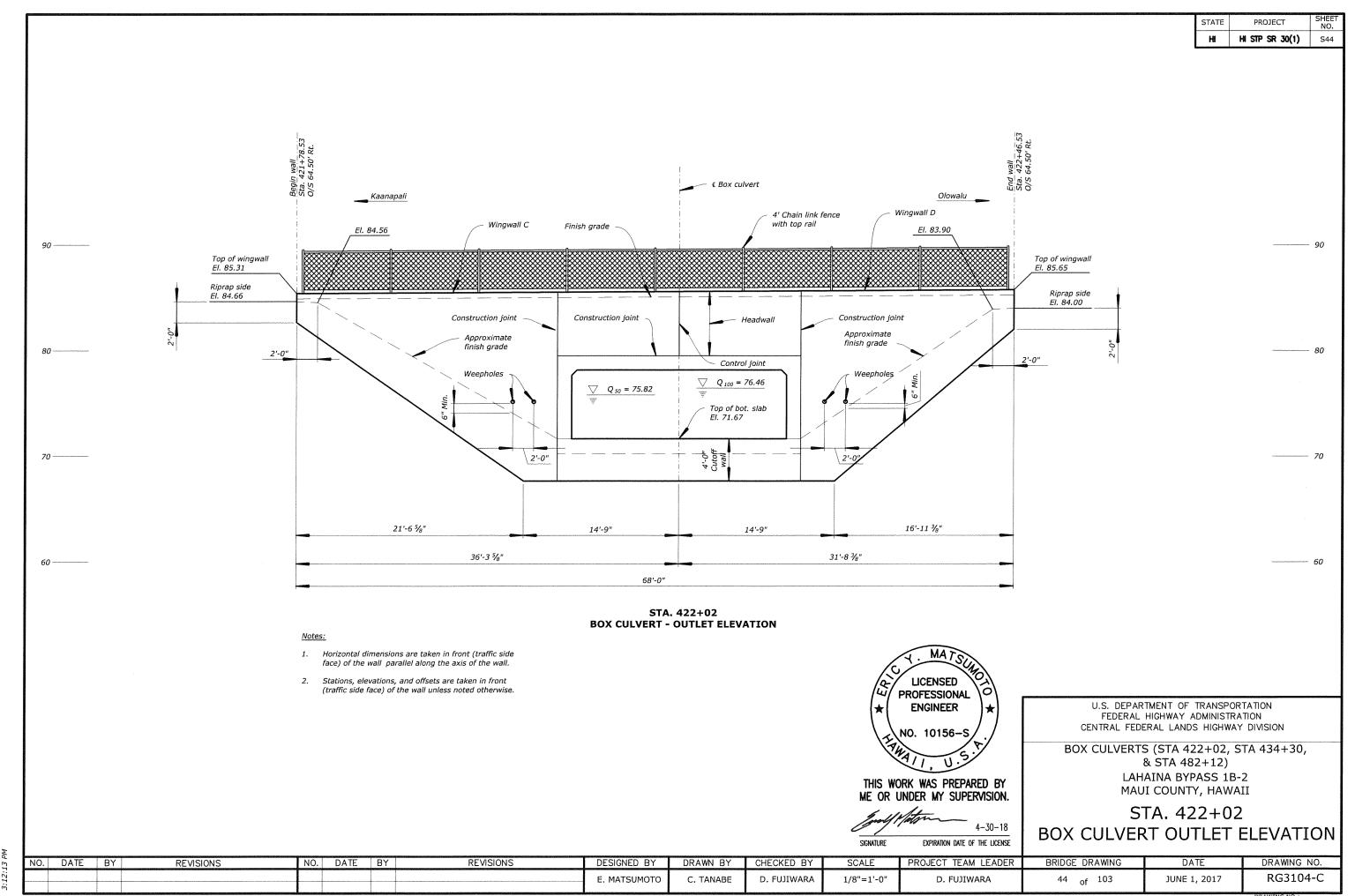
PUNAKEA KAI AND PUNAKEA UKA OVERPASS LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

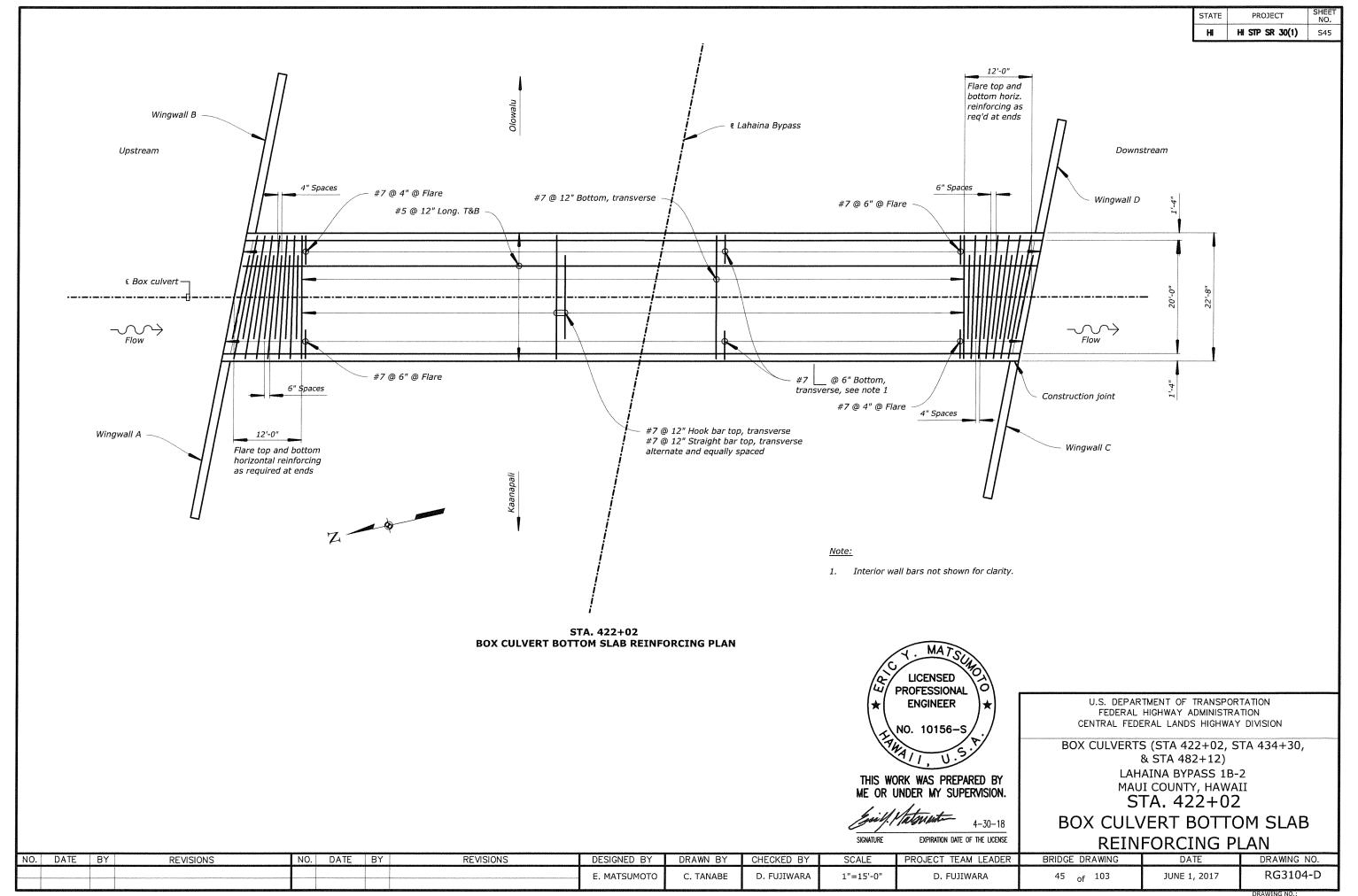
BRIDGE CONSTRUCTION SEQUENCE

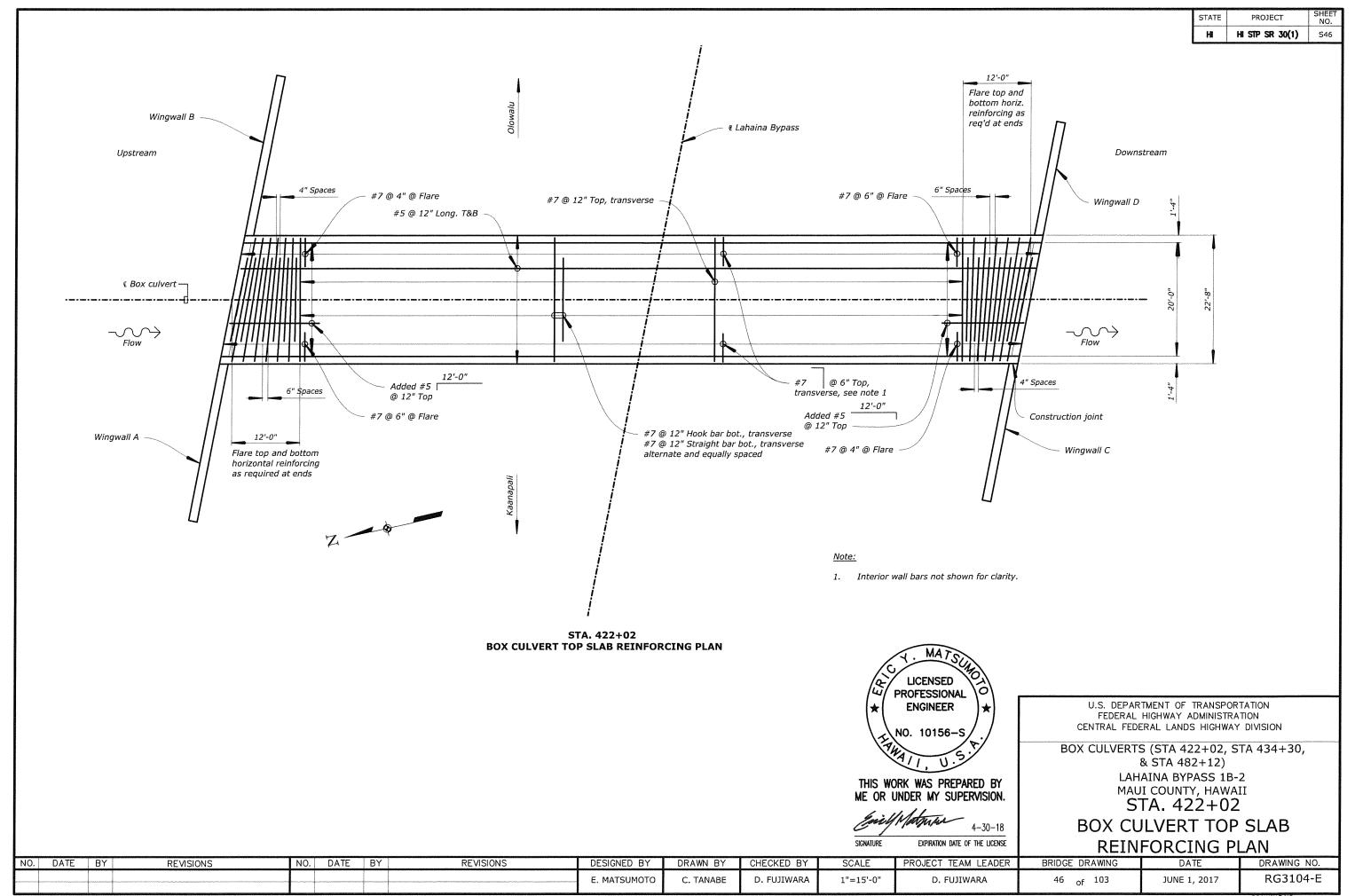
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	NO. DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
: f								E. MATSUMOTO	C. TANABE	D. FUJIWARA	3/32"=1'-0"	D. FUJIWARA	41 _{of} 103	JUNE 1, 2017	RG3103-00

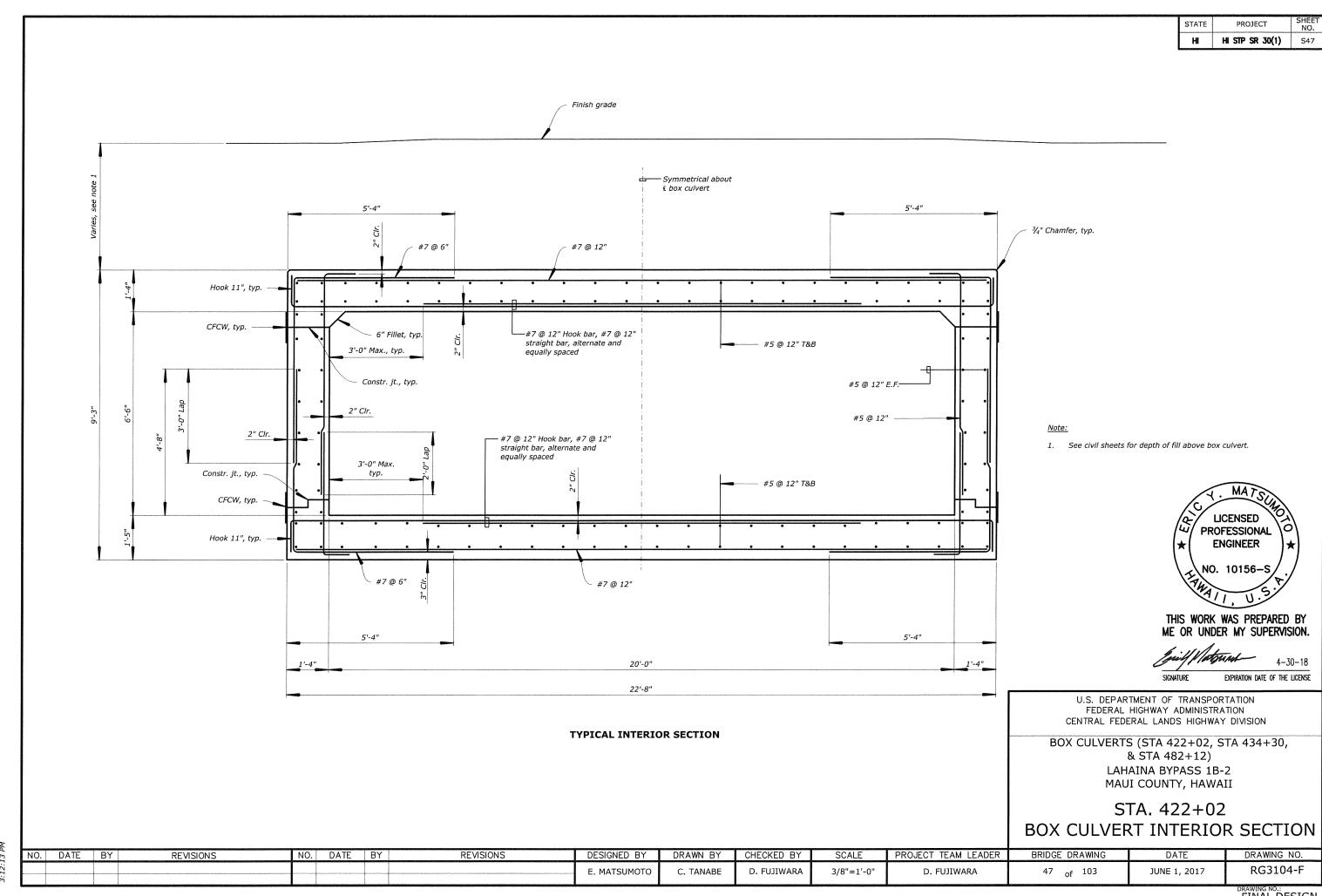


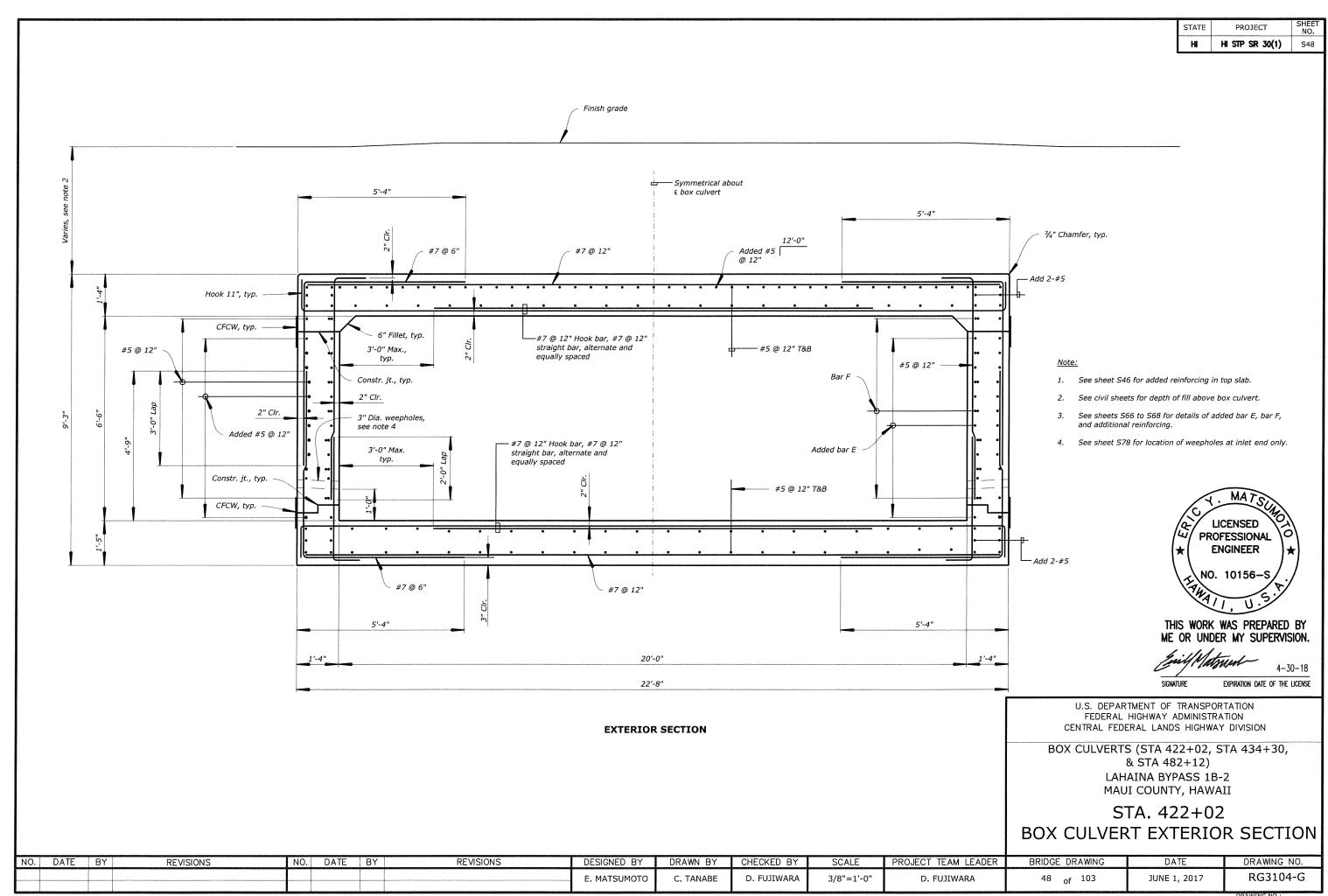


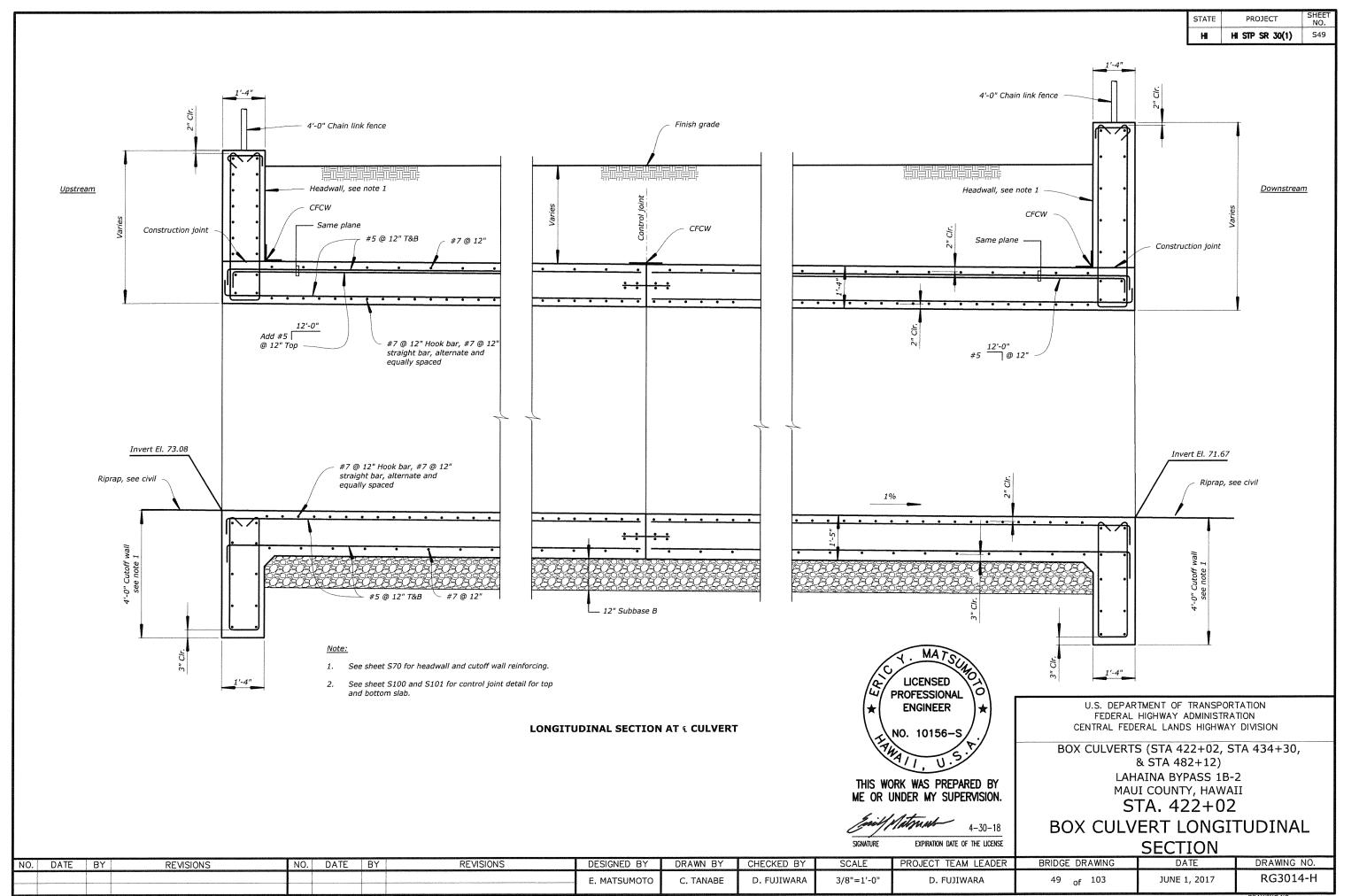


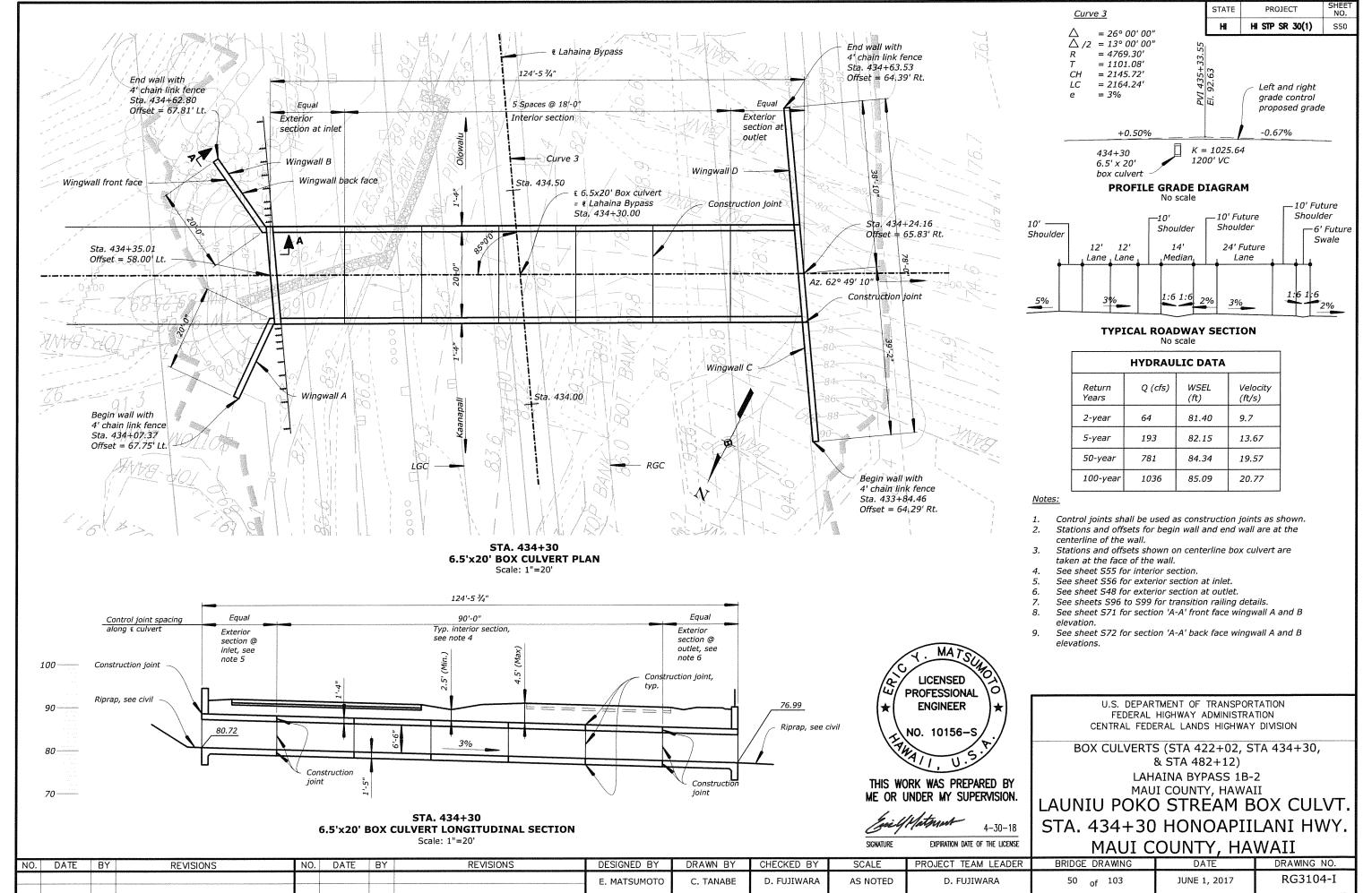


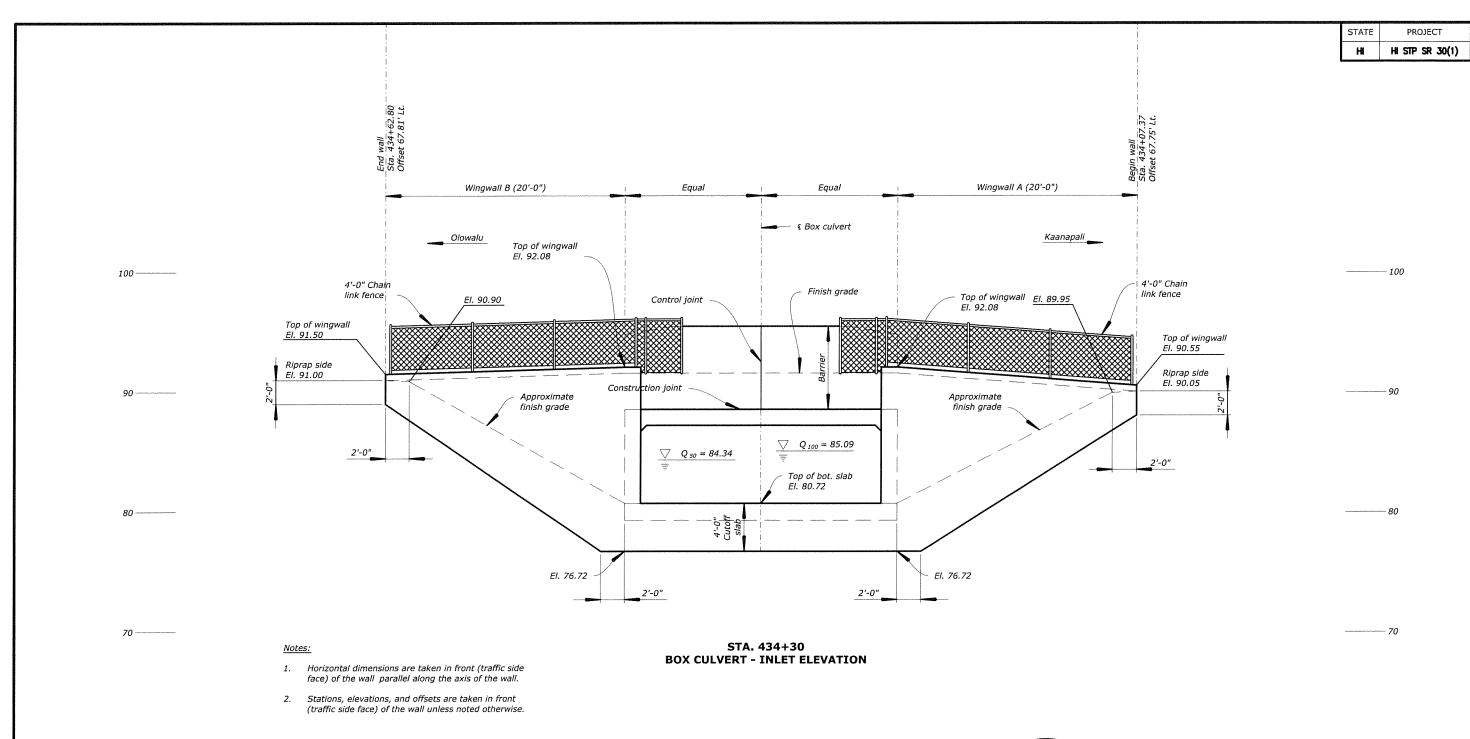














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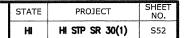
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

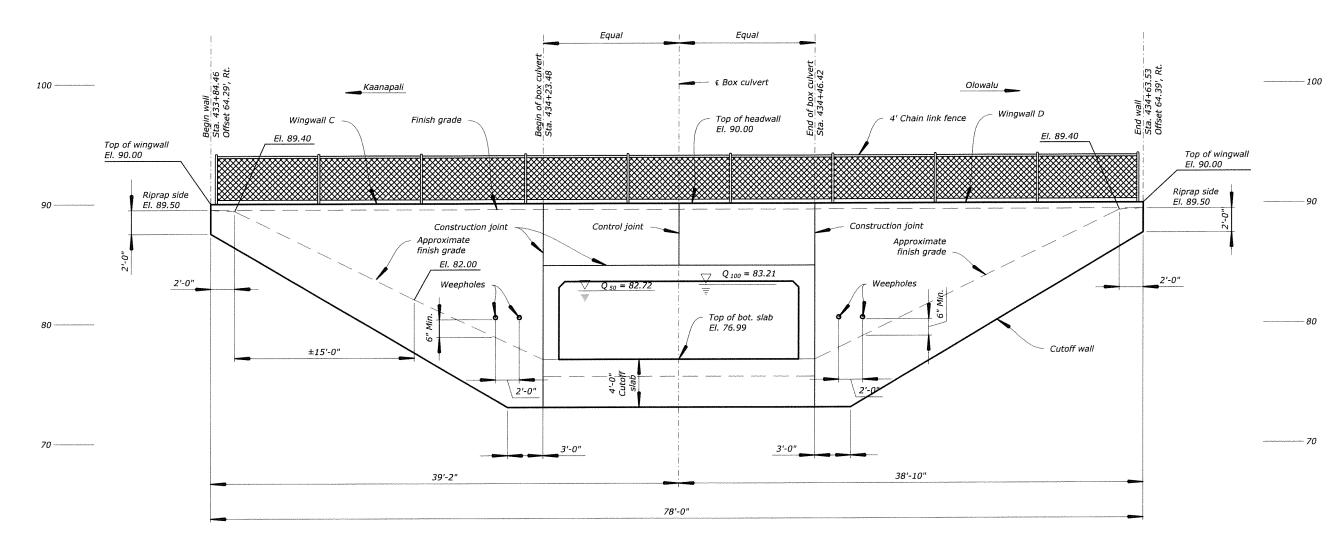
STA. 434+30 BOX CULVERT INLET ELEVATION

BRIDGE DRAWING BY REVISIONS NO. DATE BY REVISIONS DESIGNED BY DRAWN BY CHECKED BY SCALE PROJECT TEAM LEADER DRAWING NO. DATE RG3104-J D. FUJIWARA E. MATSUMOTO C. TANABE D. FUJIWARA 1/8"=1'-0" 51 of 103 JUNE 1, 2017

SHEET NO.

S51





- Horizontal dimensions are taken in front (traffic side face) of the wall parallel along the axis of the wall.
- 2. Stations, elevations, and offsets are taken in front (traffic side face) of the wall unless noted otherwise.

STA. 434+30 **BOX CULVERT - OUTLET ELEVATION**



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4-30-18

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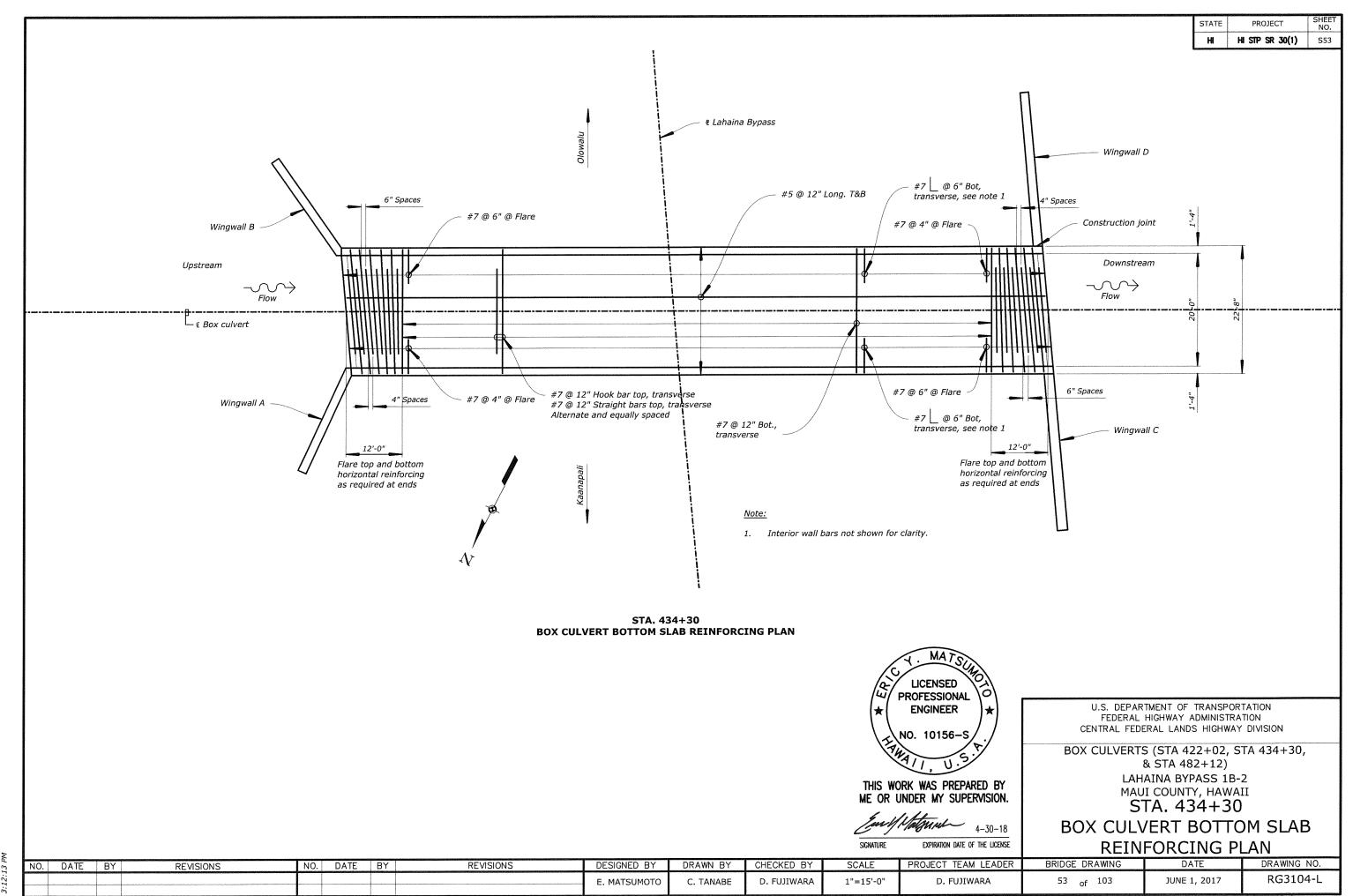
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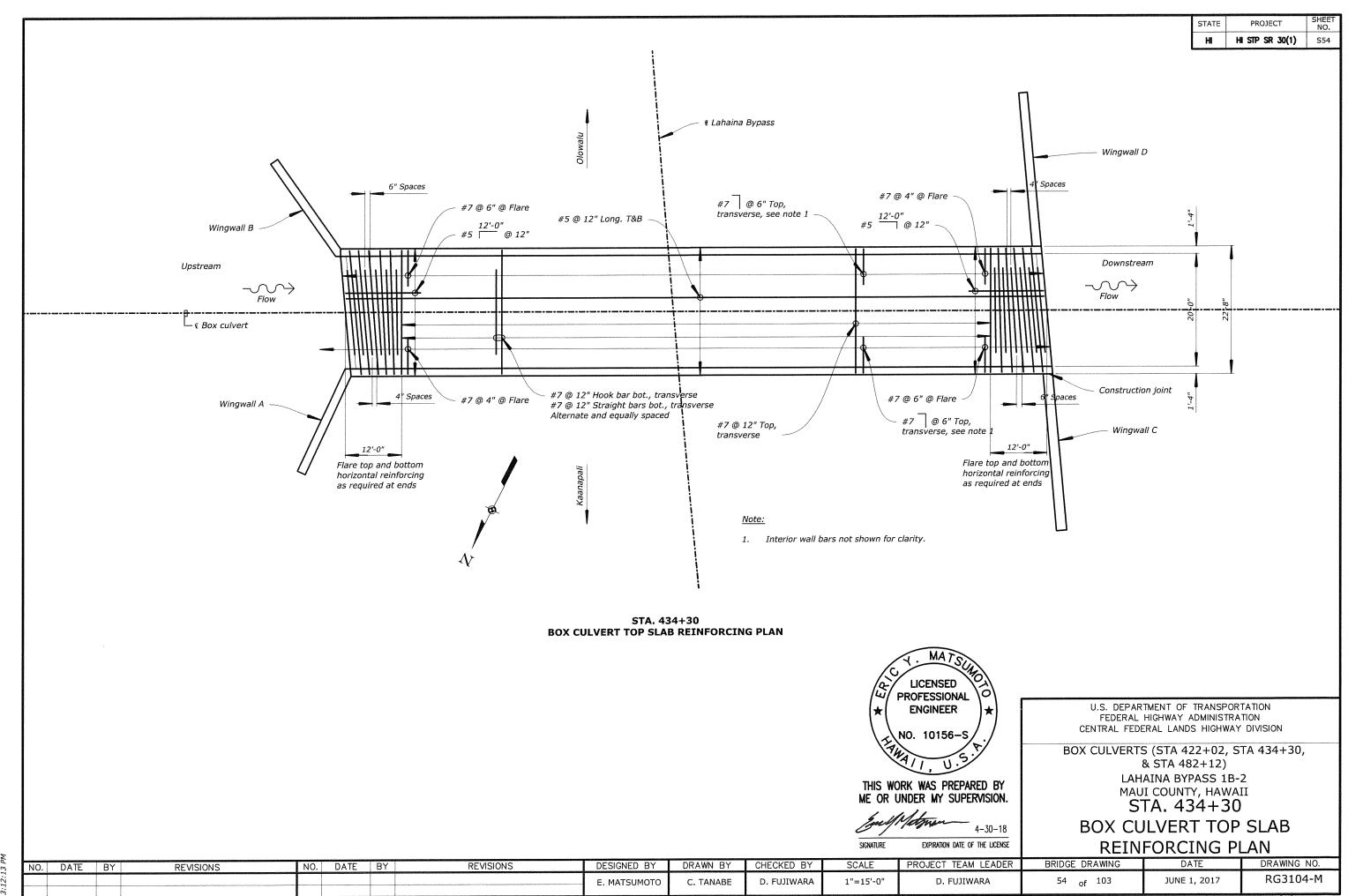
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

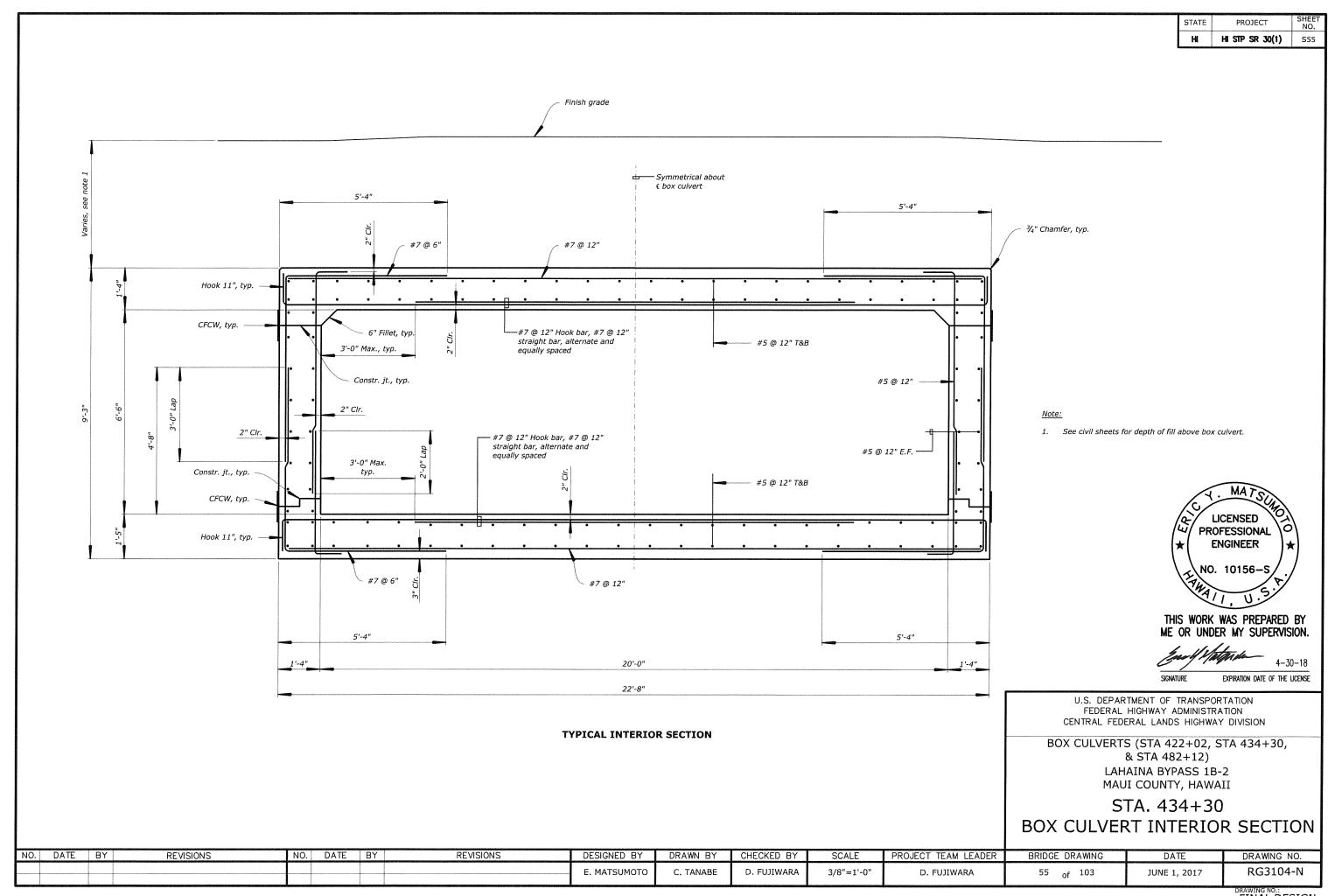
STA. 434+30

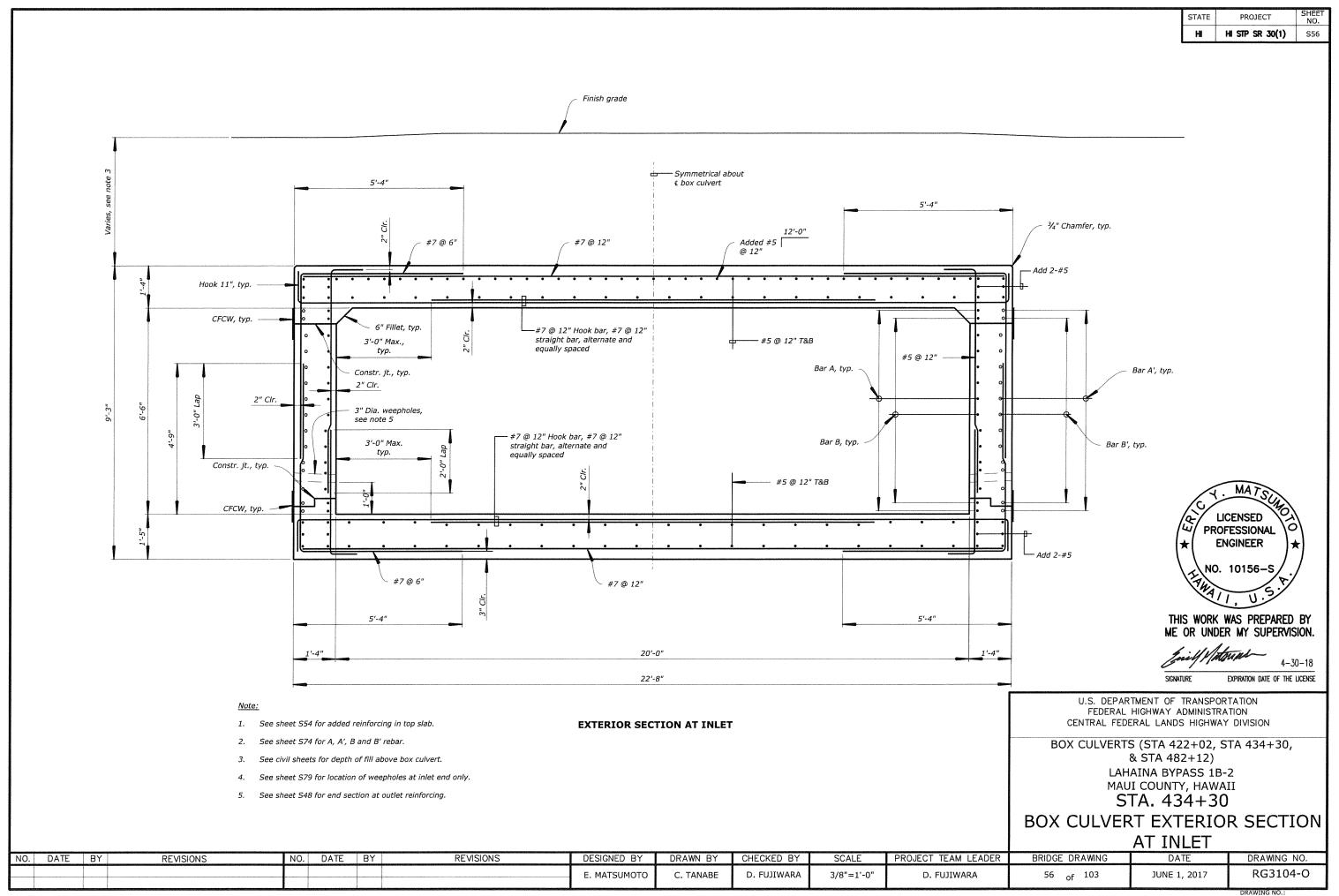
BOX CULVERT OUTLET ELEVATION

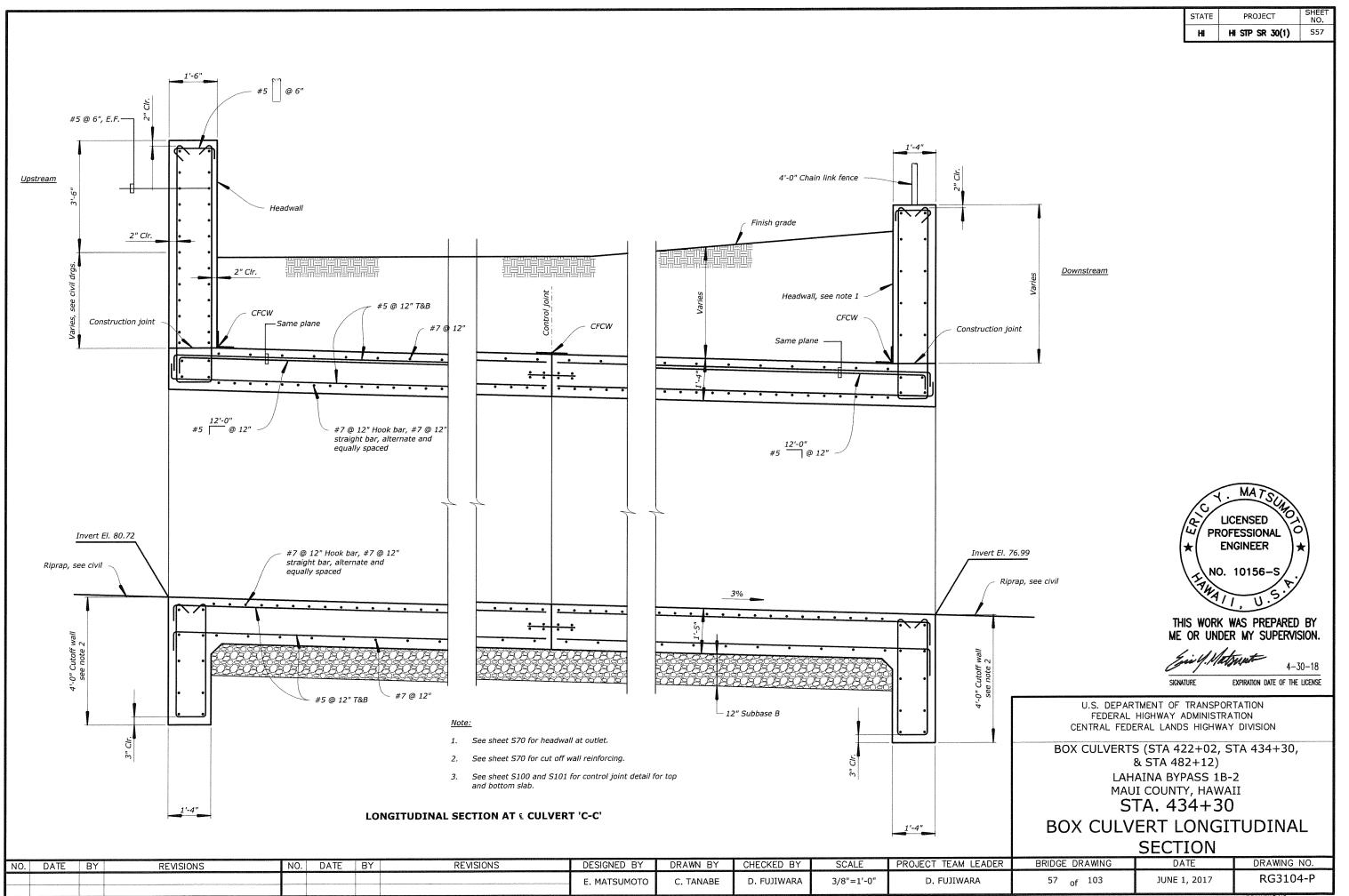
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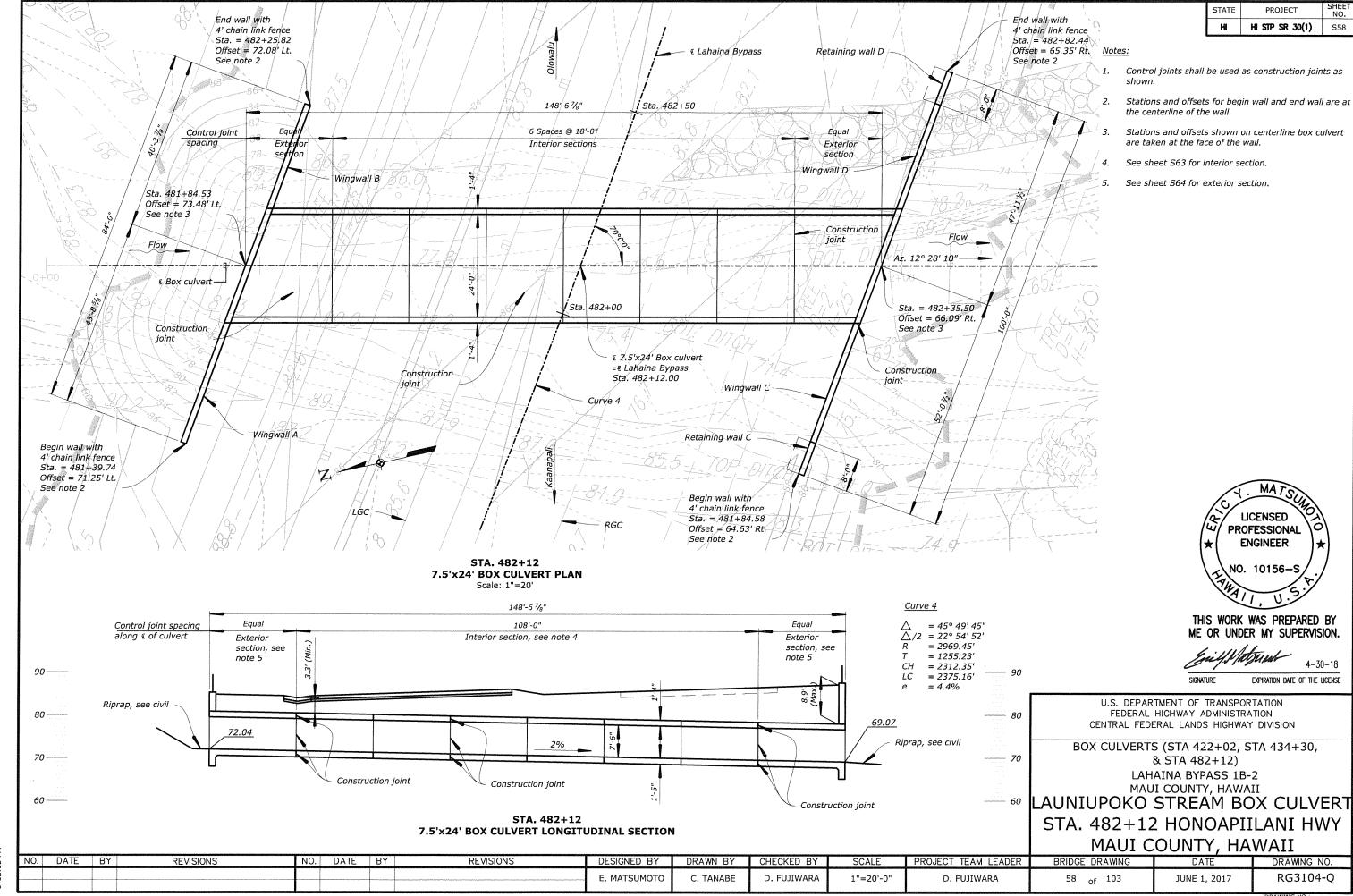


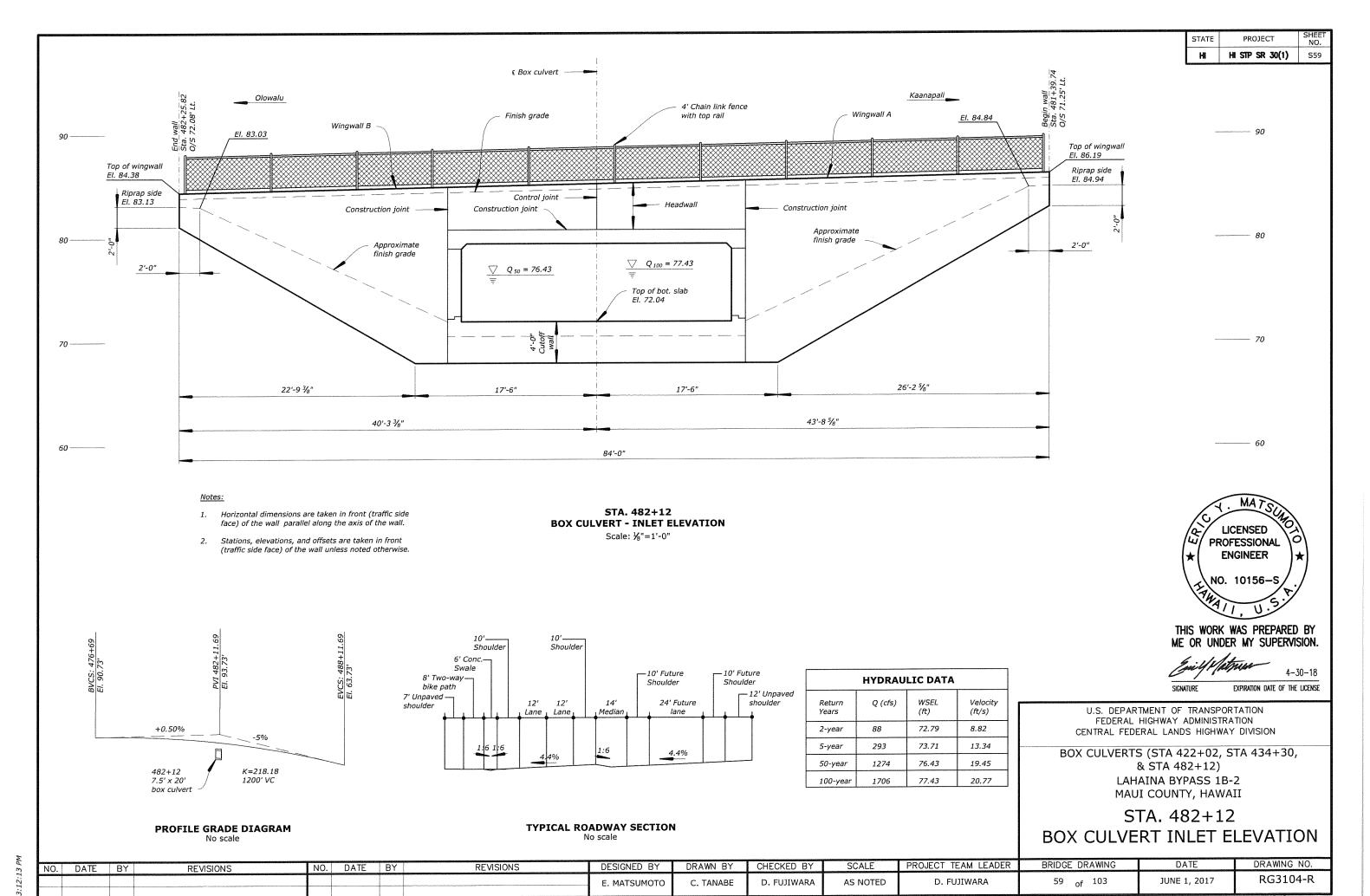


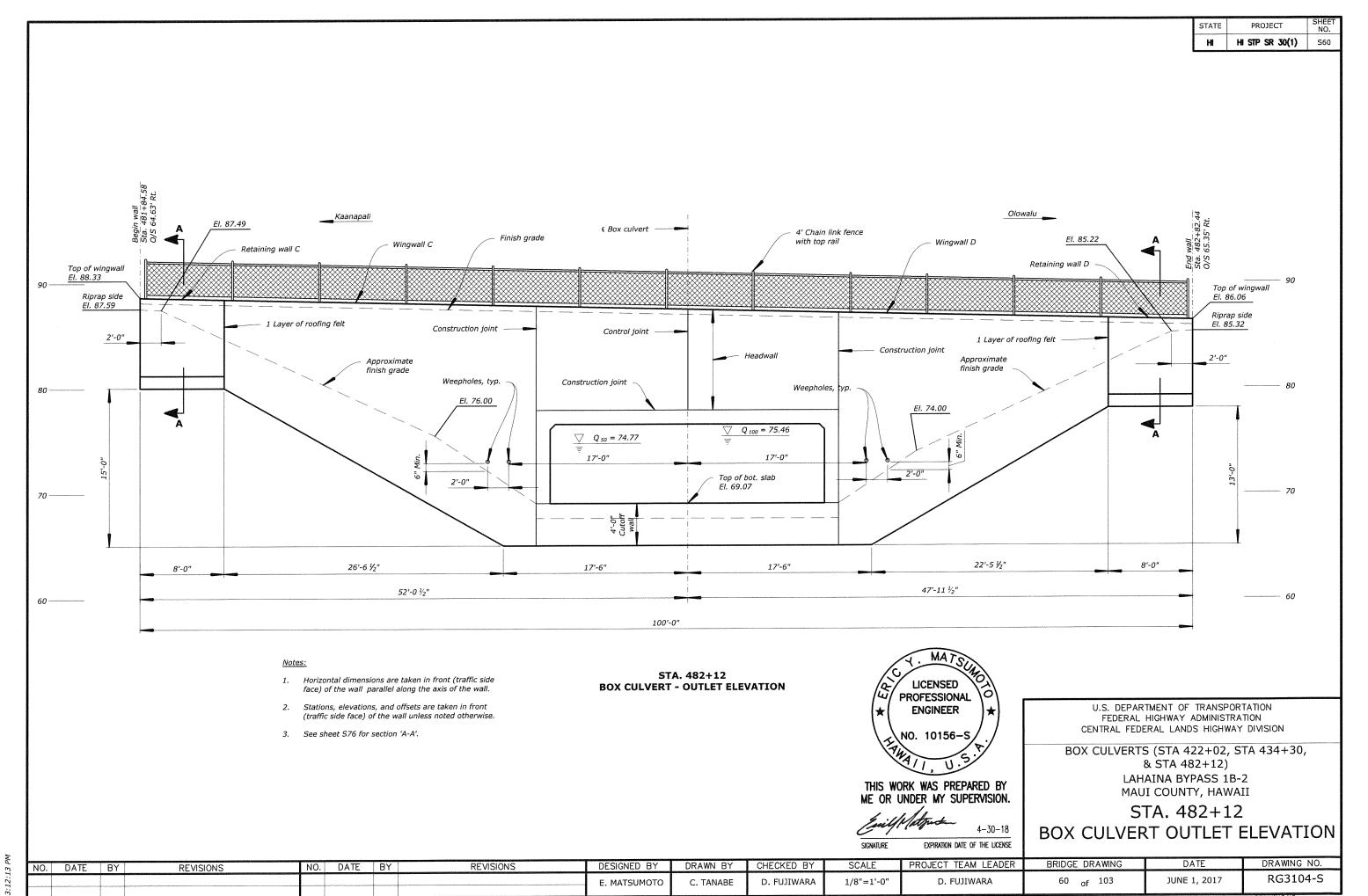


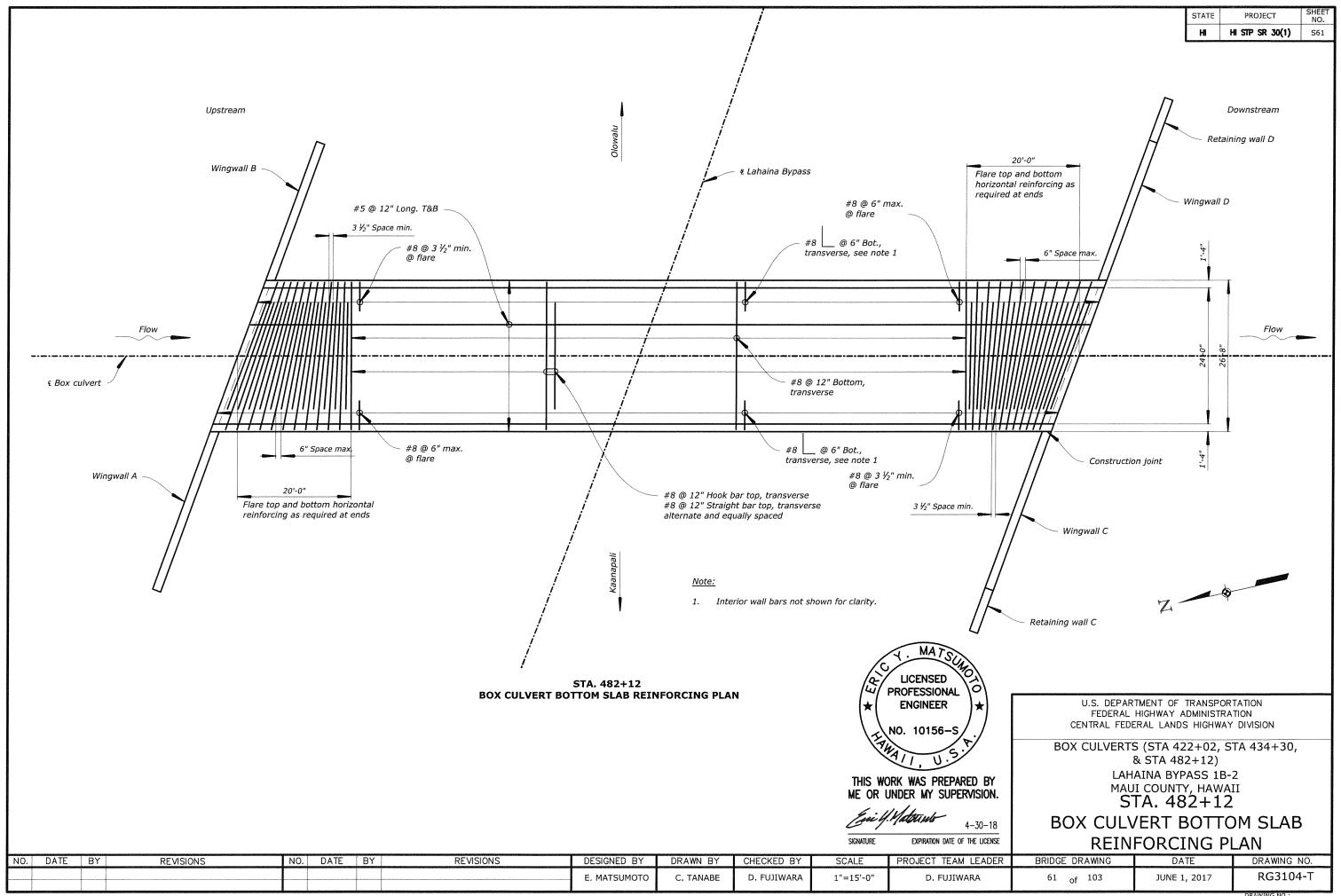


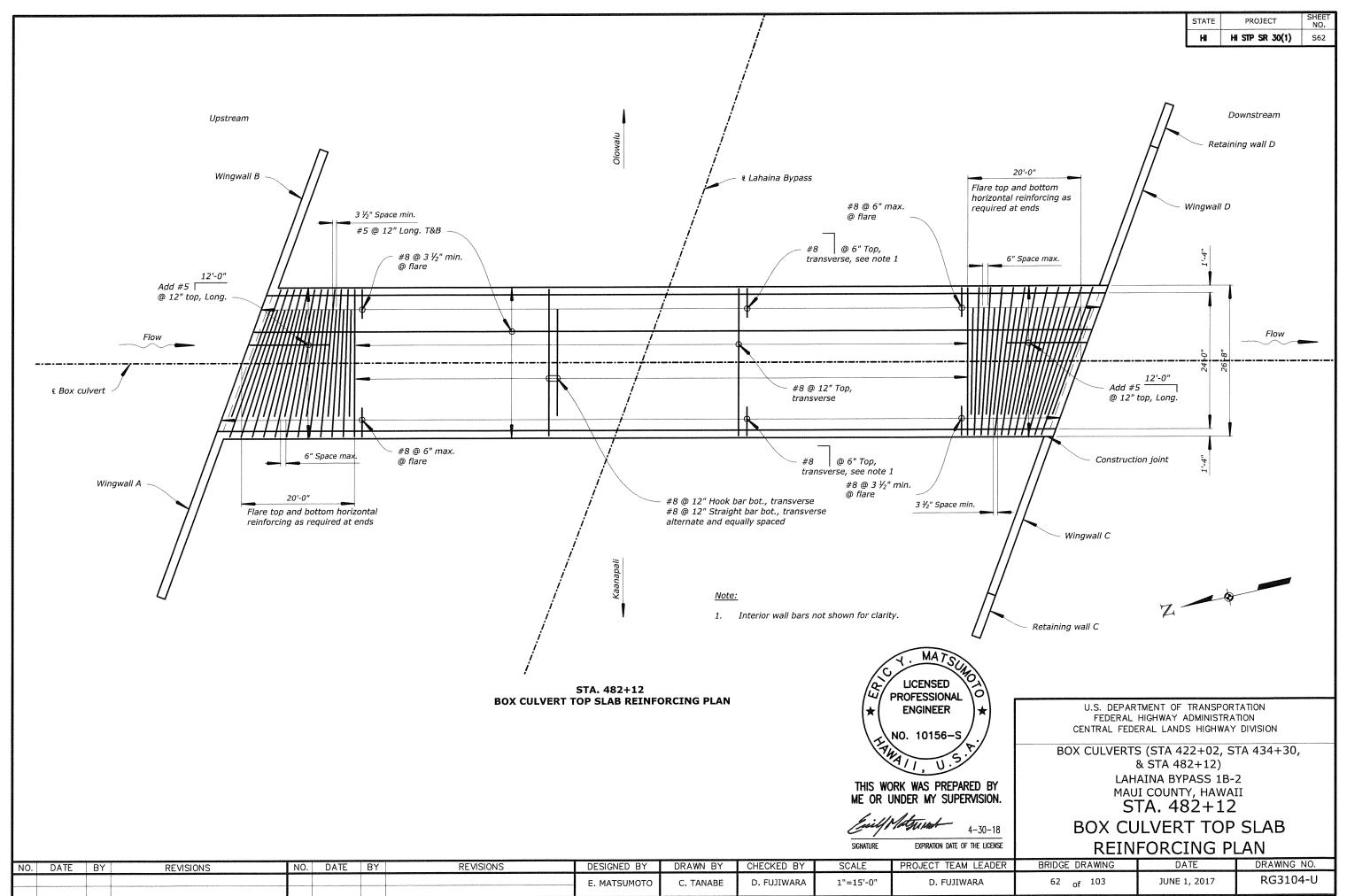


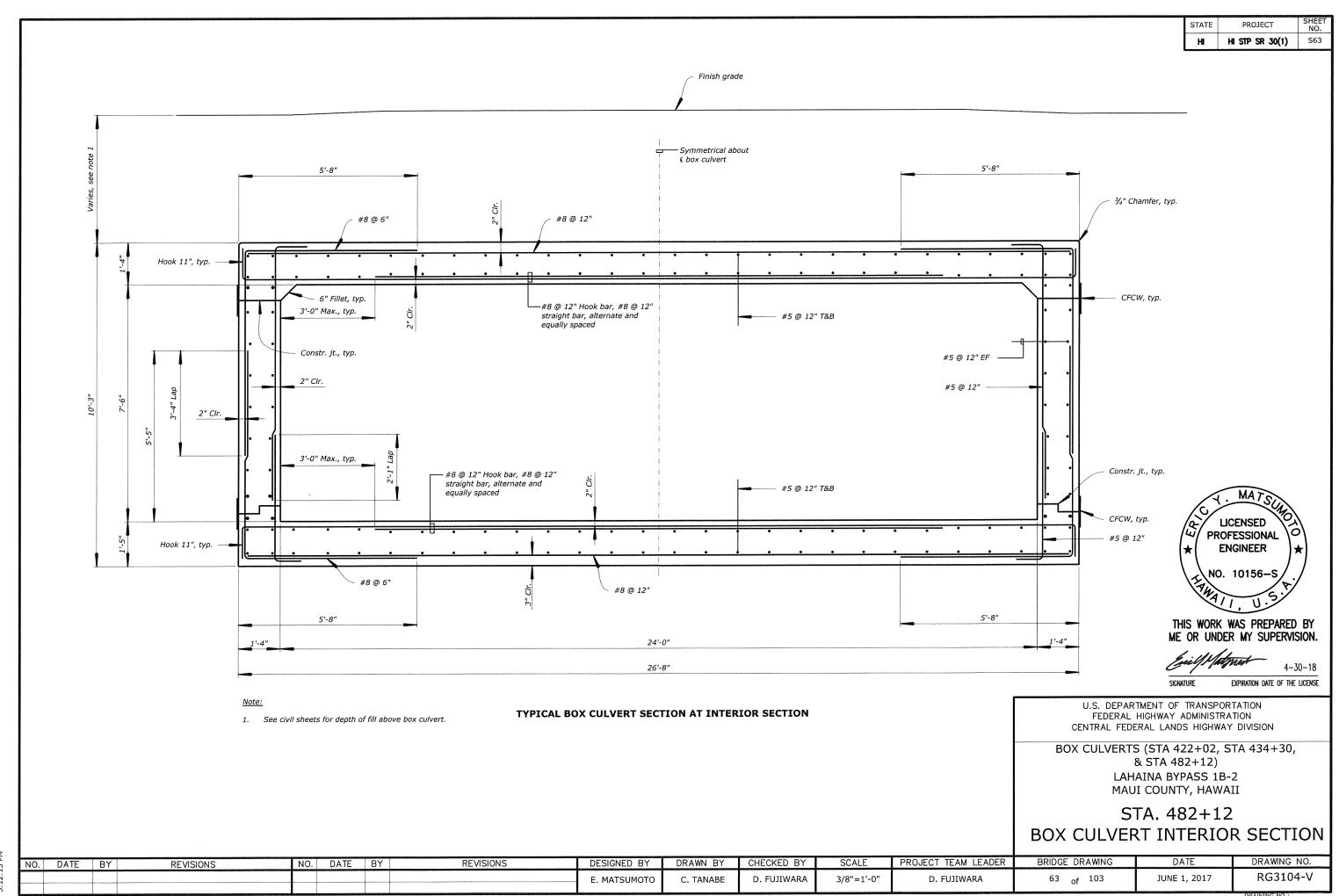


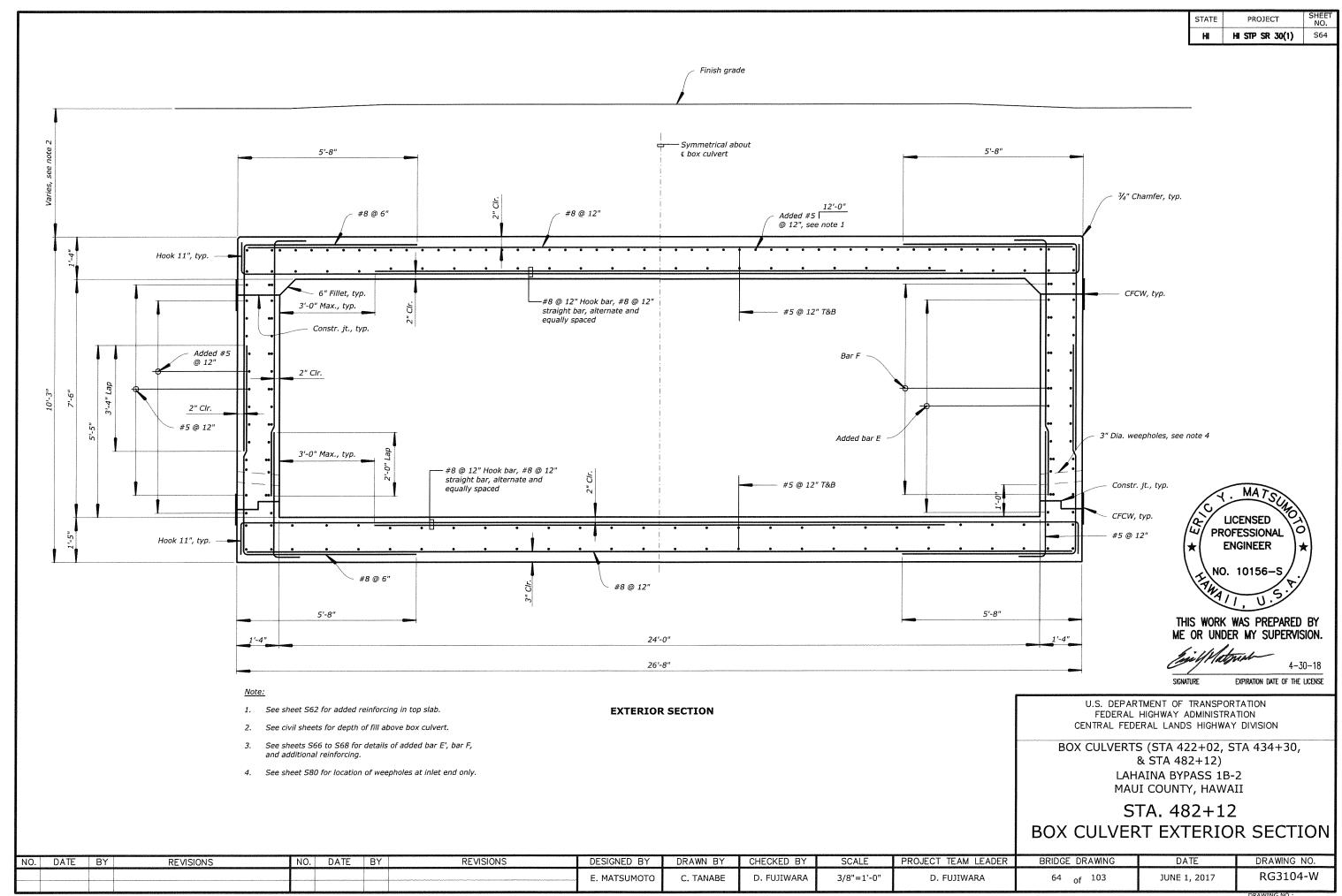


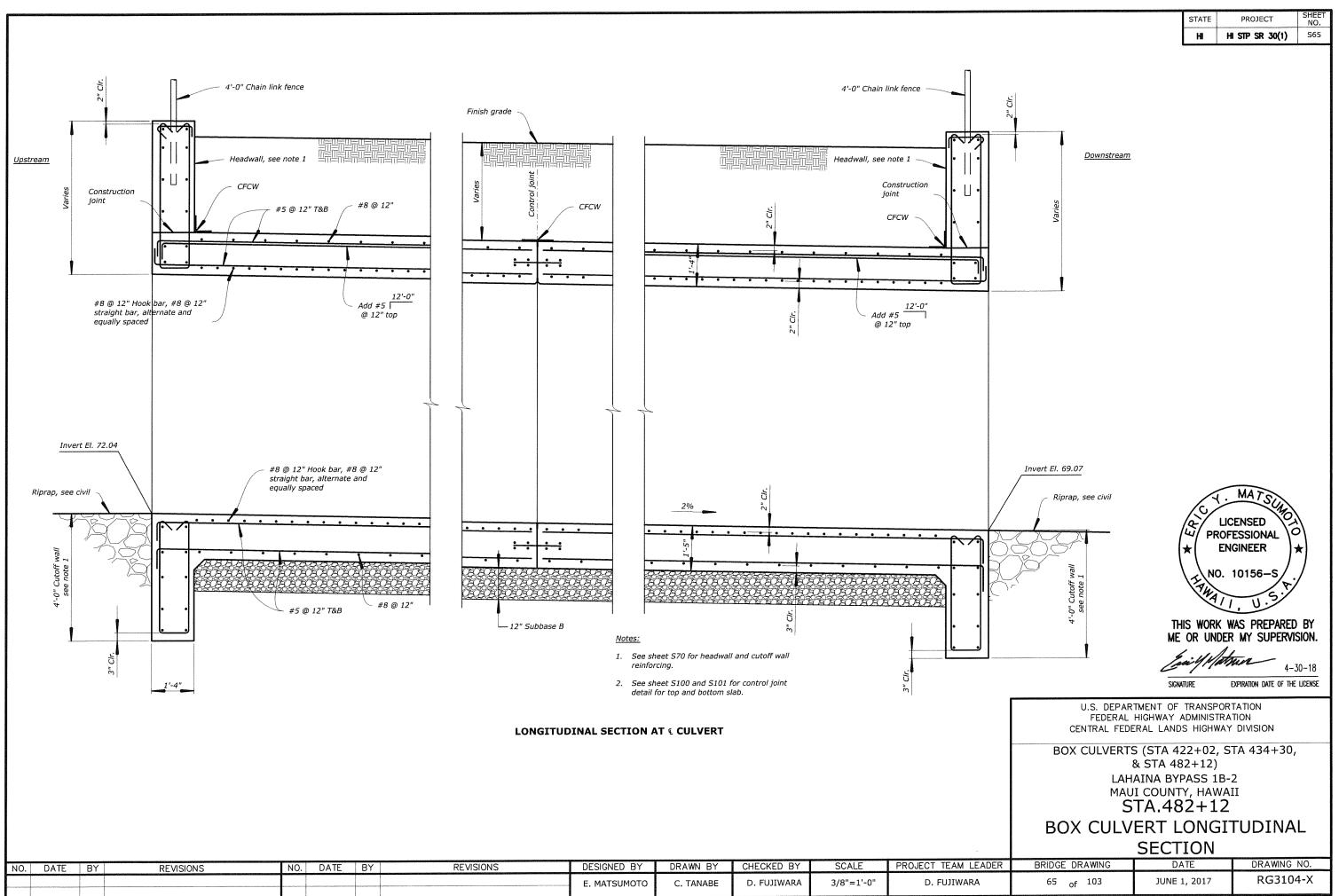


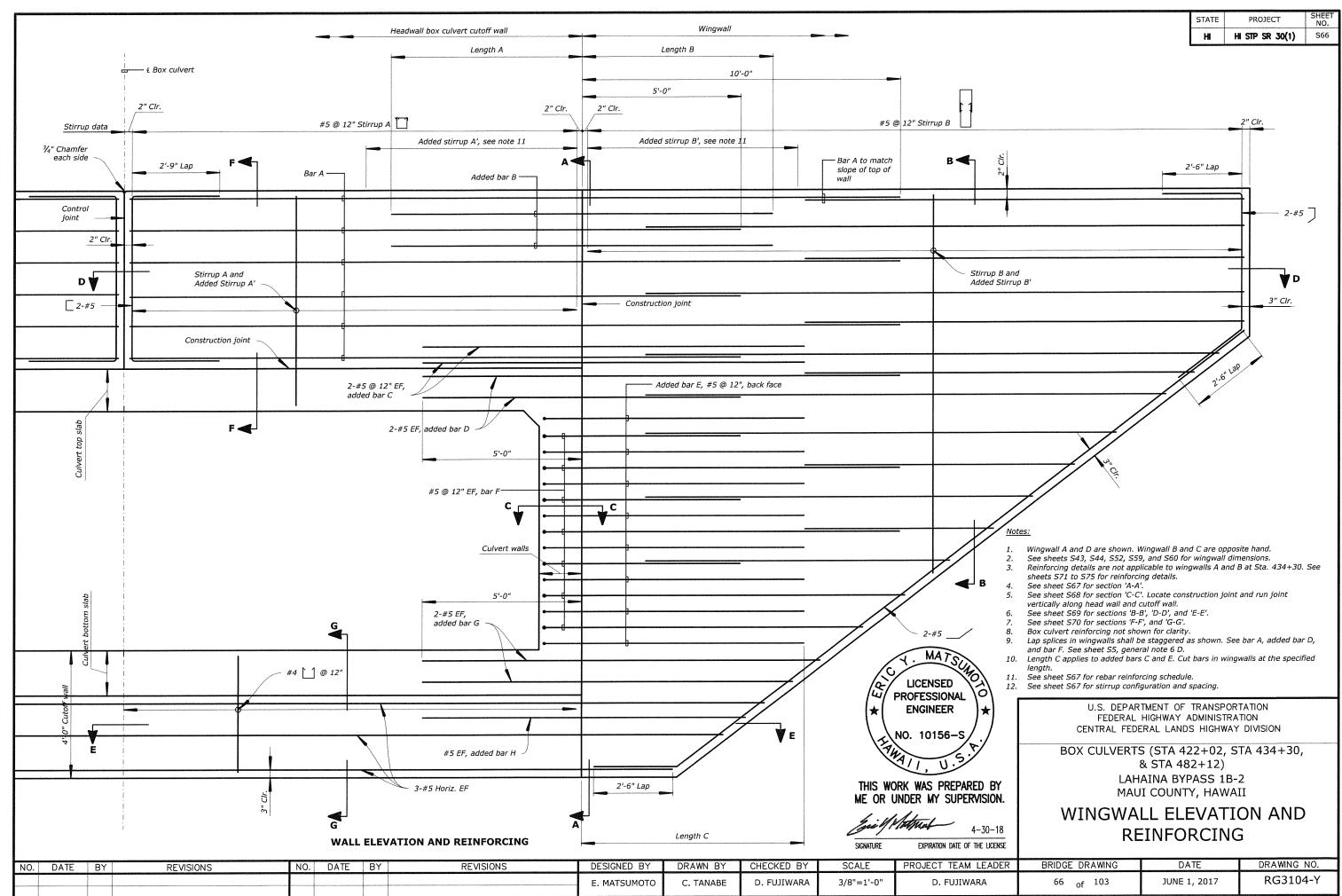








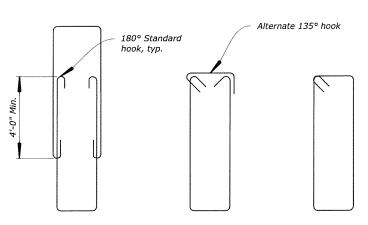






HORIZONTAL WALL REINFORCING SCHEDULE													
Station	Wingwall	Bar A	Bar B	Length A	Length B	Length C							
422+02	Α	6-#5 @ 12" EF	2-#5 @ 12" EF	6'-0"	5'-0"	6'-0"							
	В	6-#5 @ 12" EF	2-#5 @ 12" EF	6'-0"	5'-0"	6'-0"							
	С	6-#5 @ 12" EF	2-#5 @ 12" EF	6'-6"	5'-0"	6'-0"							
	D	6-#5 @ 12" EF	2-#5 @ 12" EF	6'-6"	5'-0"	6'-0"							
434+30	С	6-#5 @ 12" EF	2-#5 @ 12" EF	6'-0"	5'-0"	6'-0"							
	D	6-#5 @ 12" EF	2-#5 @ 12" EF	6'-0"	5'-0"	6'-0"							
482+12	А	5-#5 @ 12" EF	2-#5 @ 12" EF	5'-0"	5'-0"	6'-0"							
	В	5-#5 @ 12" EF	2-#5 @ 12" EF	5'-0"	5'-0"	6'-0"							
	С	10-#5 @ 12" EF	2-#6 @ 12" EF	11'-0"	6'-0"	8'-0"							
	D	10-#5 @ 12" EF	2-#6 @ 12" EF	11'-0"	6'-0"	8'-0"							

WALL	STIRRUP	REINFORCIN	G SCHEDULE
Station	Wingwall	Stirrup A'	Stirrup B'
422+02	Α	8-#5 @ 12"	4-#5 @ 12"
	В	8-#5 @ 12"	4-#5 @ 12"
	С	8-#5 @ 12"	4-#5 @ 12"
***************************************	D	8-#5 @ 12"	4-#5 @ 12"
434+30	С	9-#5 @ 12"	5-#5 @ 12"
	D	9-#5 @ 12"	5-#5 @ 12"
482+12	Α	6-#5 @ 12"	3-#5 @ 12"
····	В	6-#5 @ 12"	3-#5 @ 12"
v.****	С	12-#5 @ 12"	6-#5 @ 12"
,	D	12-#5 @ 12"	6-#5 @ 12"



Note:

 The following stirrups are permitted for use with stirrup A, A', B, B', and in Sta. 434+30 box culvert wingwalls A and B.

WINGWALL AND HEADWALL STIRRUP DETAIL

No Scale



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EXPIRATION DATE OF THE LICENSE

4-30-18

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

WINGWALL SECTION AND SCHEDULE

2-#5 ---REVISIONS PROJECT TEAM LEADER BRIDGE DRAWING DRAWING NO. DRAWN BY CHECKED BY SCALE DESIGNED BY DATE REVISIONS NO. DATE BY JUNE 1, 2017 RG3104-Z D. FUJIWARA 67 _{of} 103 D. FUJIWARA AS NOTED E. MATSUMOTO C. TANABE

3" Clr.

Bar A to match slope -

Front face

Added Bar D, 2-#5 EF

Stirrup B and added stirrup B

Bar F, typ., — #5 @ 12" EF

Add Bar G, 2-#5 EF -

3-#5 EF ___

Bar A, typ._

of top of wall

Finish grade, see

Box culvert top slab

Box culvert bottom slab

Legend

Denotes typical Bar

o Denotes added Bar

civil drawings

bar B, typ.

- Added Bar C, 2-#5 @ 12" EF

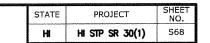
Add Bar E,

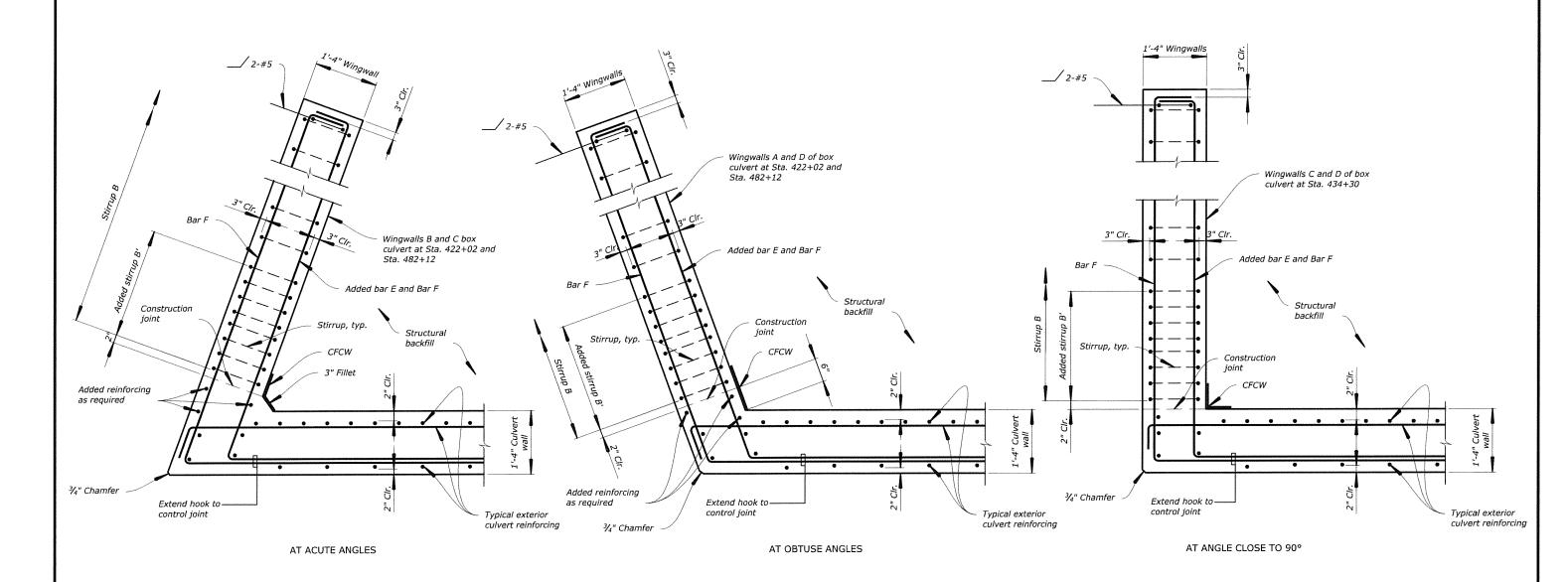
— Add Bar H, #5 EF

SECTION 'A-A'

Scale: ½"=1'-0"

#5 @ 12" back face





SECTION 'C-C' AT CULVERT WALLS



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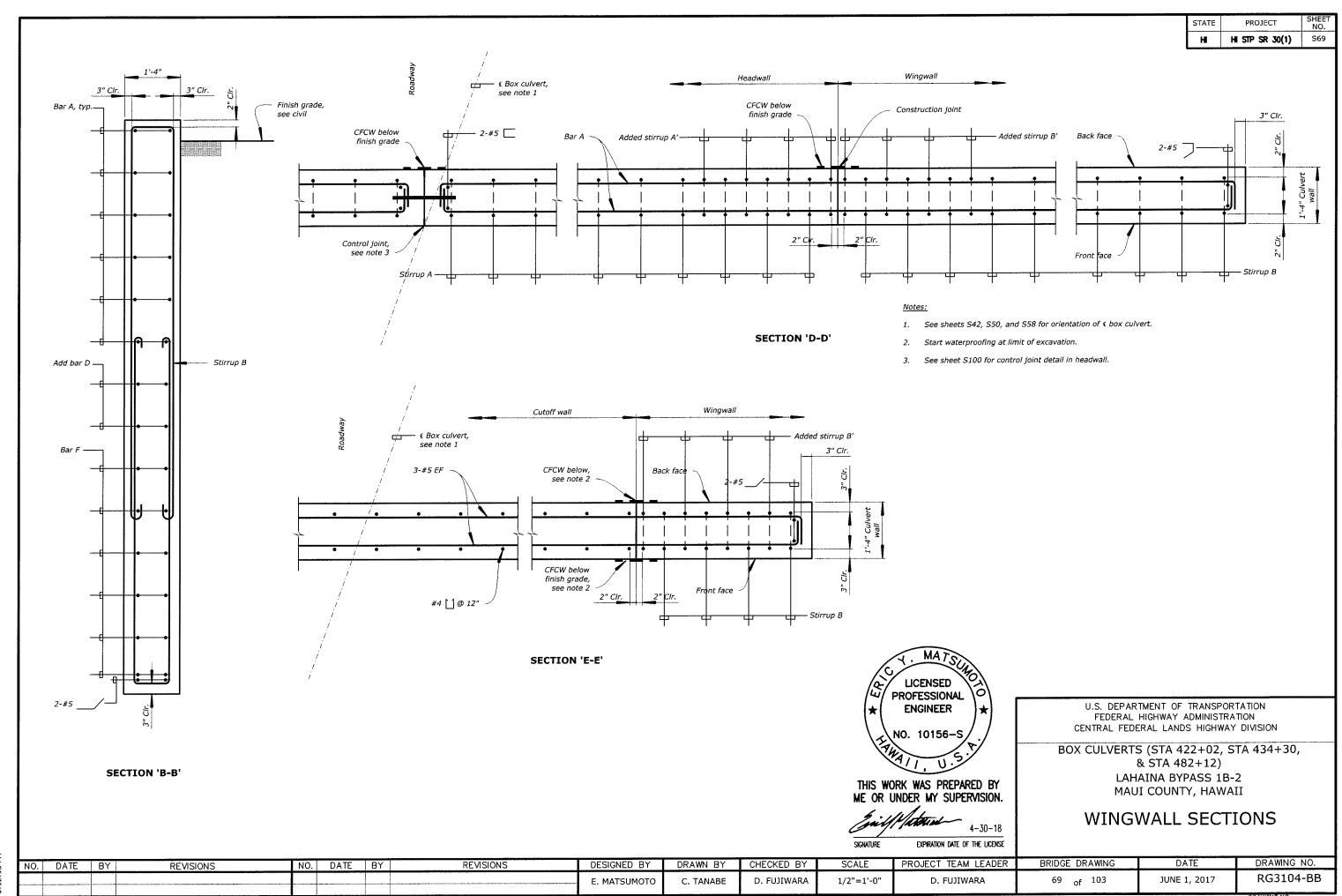
SIGNATURE EXPIRATION DATE OF THE LICENSE

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

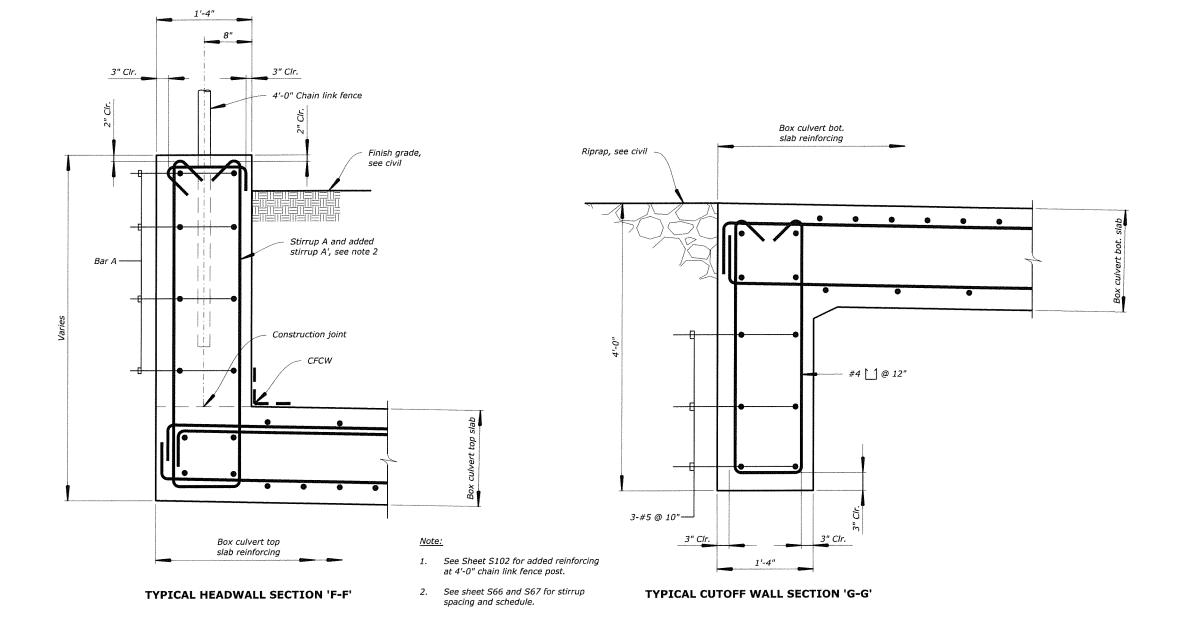
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

WINGWALL SECTIONS

REVISIONS DESIGNED BY DRAWN BY CHECKED BY SCALE PROJECT TEAM LEADER BRIDGE DRAWING DATE DRAWING NO. NO. DATE BY NO. DATE BY REVISIONS JUNE 1, 2017 RG3104-AA D. FUJIWARA 1/2"=1'-0" D. FUJIWARA 68 _{of} 103 E. MATSUMOTO C. TANABE



SHEET NO. STATE PROJECT S70 Н HI STP SR 30(1)



LICENSED **PROFESSIONAL ENGINEER** NO. 10156-S

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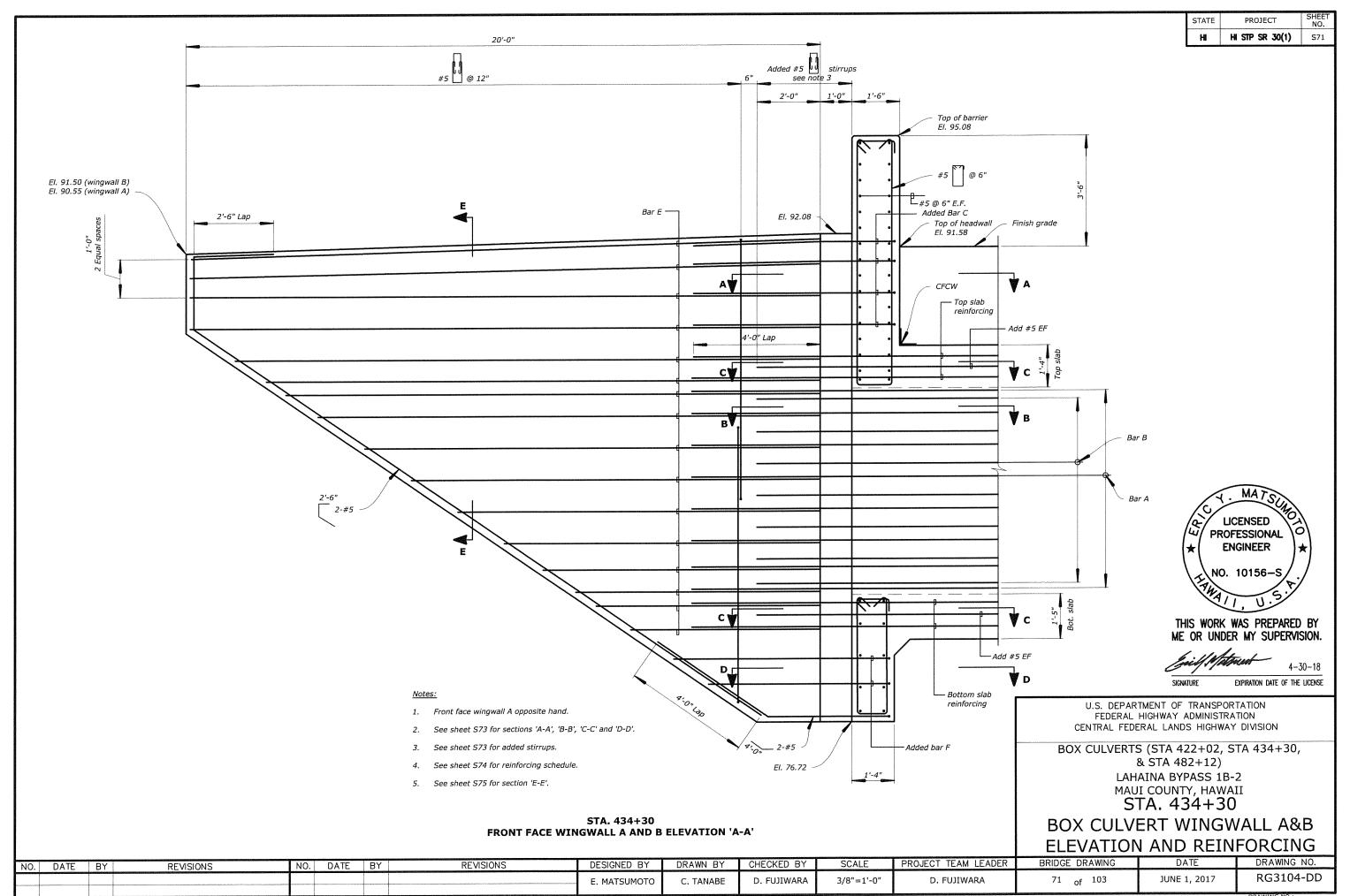
4-30-18 SIGNATURE EXPIRATION DATE OF THE LICENSE

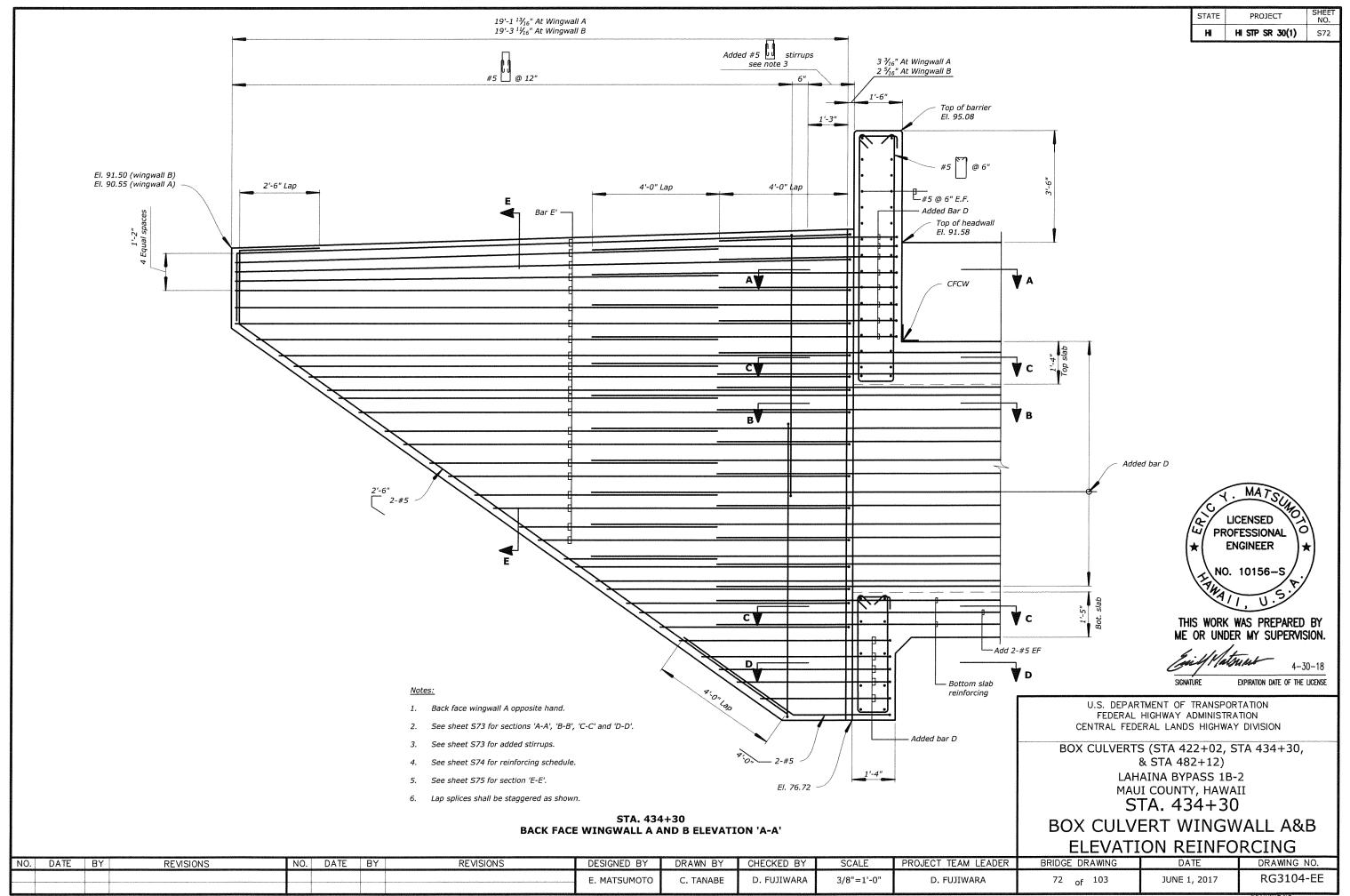
U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION
CENTRAL FEDERAL LANDS HIGHWAY DIVISION

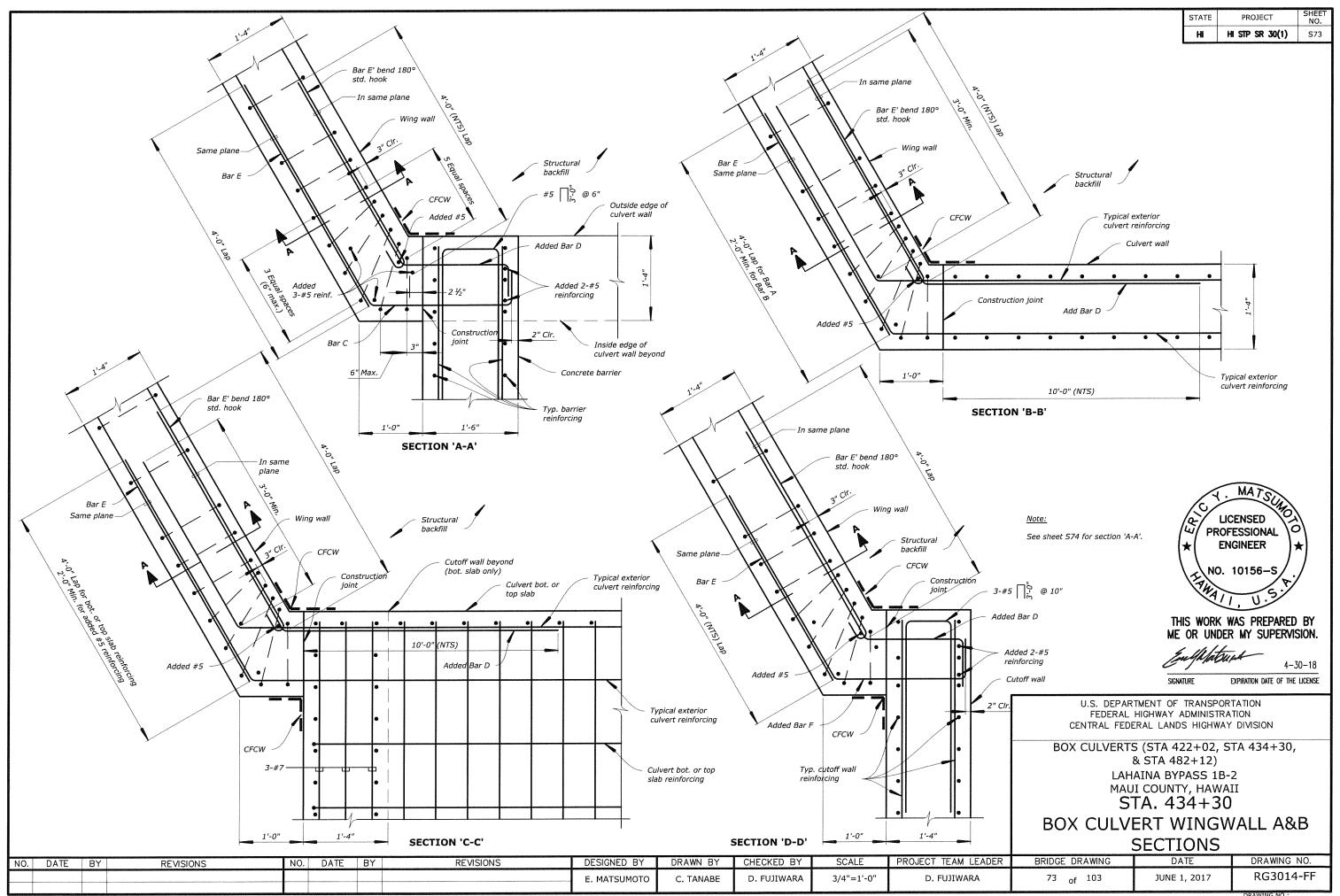
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

CULVERT SECTIONS

O. DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
							E. MATSUMOTO	C. TANABE	D. FUJIWARA	3/4"=1'-0"	D. FUJIWARA	70 _{of} 103	JUNE 1, 2017	RG3104-CC







STATE PROJECT SHEET NO.

HI HI STP SR 30(1) S74

REINFORCING SCHEDULE												
Sta. 434+30	Wingwall	Bar A	Bar A'	Bar B	Bar B'	Bar C	Bar D	Bar E	Bar E'	Bar F		
	A & B	#5 @ 12"	#5 @ 12"	#5 @ 12"	#5 @ 12"	#5 @ 12"	#5 @ 6"	#5 @ 12"	#5 @ 6"	#5 @ 12"		

LICENSED PROFESSIONAL ENGINEER

NO. 10156-S

AND S. MATSUMO.

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CICHATIDE CYCLICATION DATE

SCALE

1/2"=1'-0"

CHATURE EXPIRATION DATE OF THE LICENSE

PROJECT TEAM LEADER

D. FUJIWARA

4-30-18

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII STA. 434+30

BOX CULVERT WINGWALL A&B

JUNE 1, 2017

SECTION

BRIDGE DRAWING

74 of 103

DRAWING NO.

RG3104-GG

NO. DATE BY REVISIONS NO. DATE BY REVISIONS DESIGNED BY
E. MATSUMOTO

SECTION 'A-A'

Bottom slab reinforcing

3" Clr.

Bar C

Add 2-#5-

reinforcing

• Bar E 🦠

o Bar B'

• Bar B

Chain link fence

Bottom of headwall beyond

#5 Stirrup bend 180° standard hook, typ.

Structural

backfill

Top of cutoff wall beyond

o Bar D

Finish grade

∽•Bar E'

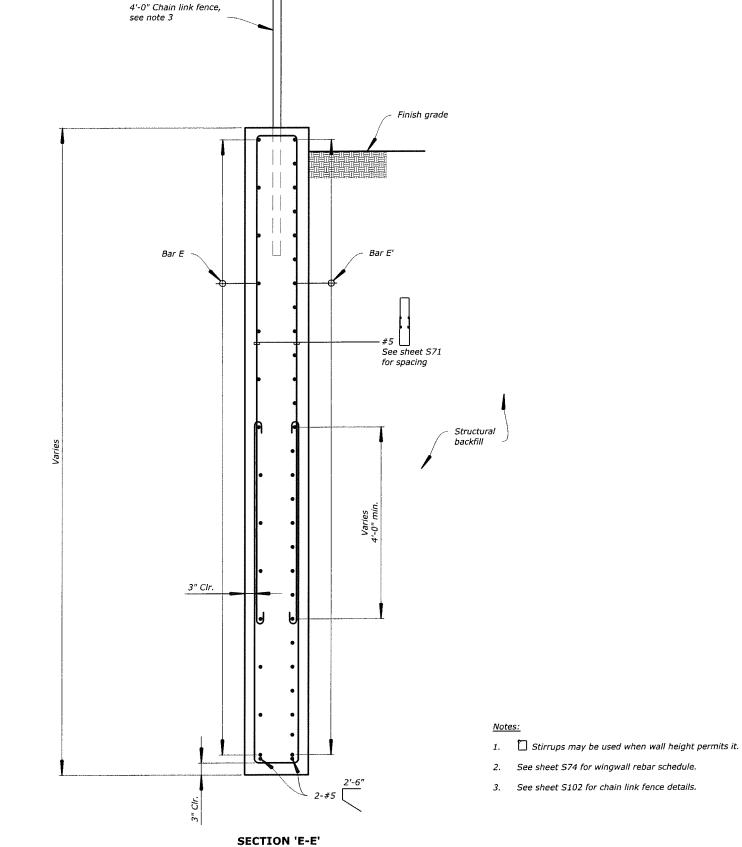
DRAWN BY

C. TANABE

CHECKED BY

D. FUJIWARA

STATE SHEET NO. PROJECT HI STP SR 30(1) S75



LICENSED **PROFESSIONAL ENGINEER** NO. 10156-9

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BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2

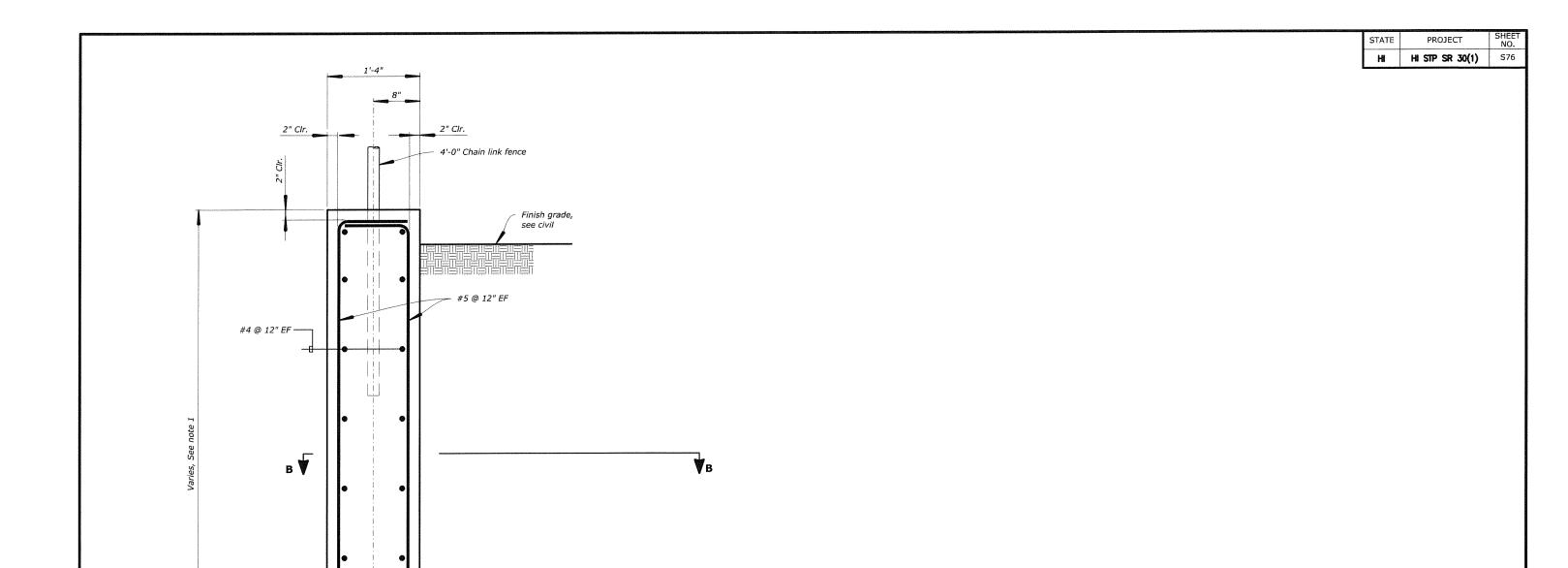
MAUI COUNTY, HAWAII

STA. 434+30

BOX CULVERT WINGWALL A&B

SECTION DATE

PROJECT TEAM LEADER BRIDGE DRAWING DRAWING NO. CHECKED BY SCALE DESIGNED BY DRAWN BY NO. DATE BY REVISIONS NO. DATE BY REVISIONS RG3104-HH 75 _{of} 103 JUNE 1, 2017 D. FUJIWARA 1/2"=1'-0" D. FUJIWARA E. MATSUMOTO



Notes:

Construction joint

CFCW

5'-0"

SECTION 'A-A'

9-#5 Top & bottom

6-#4

Top and bottom

Vc

CFCW

Bars in samevertical plane

- 1. See Sheet S60 for elevations of retaining walls C and D.
- 2. See sheet S92 for retaining wall backfill section.
- 3. See sheet S77 for section 'B-B' and 'C-C'.
- 4. See sheet S102 for chain link fence details.



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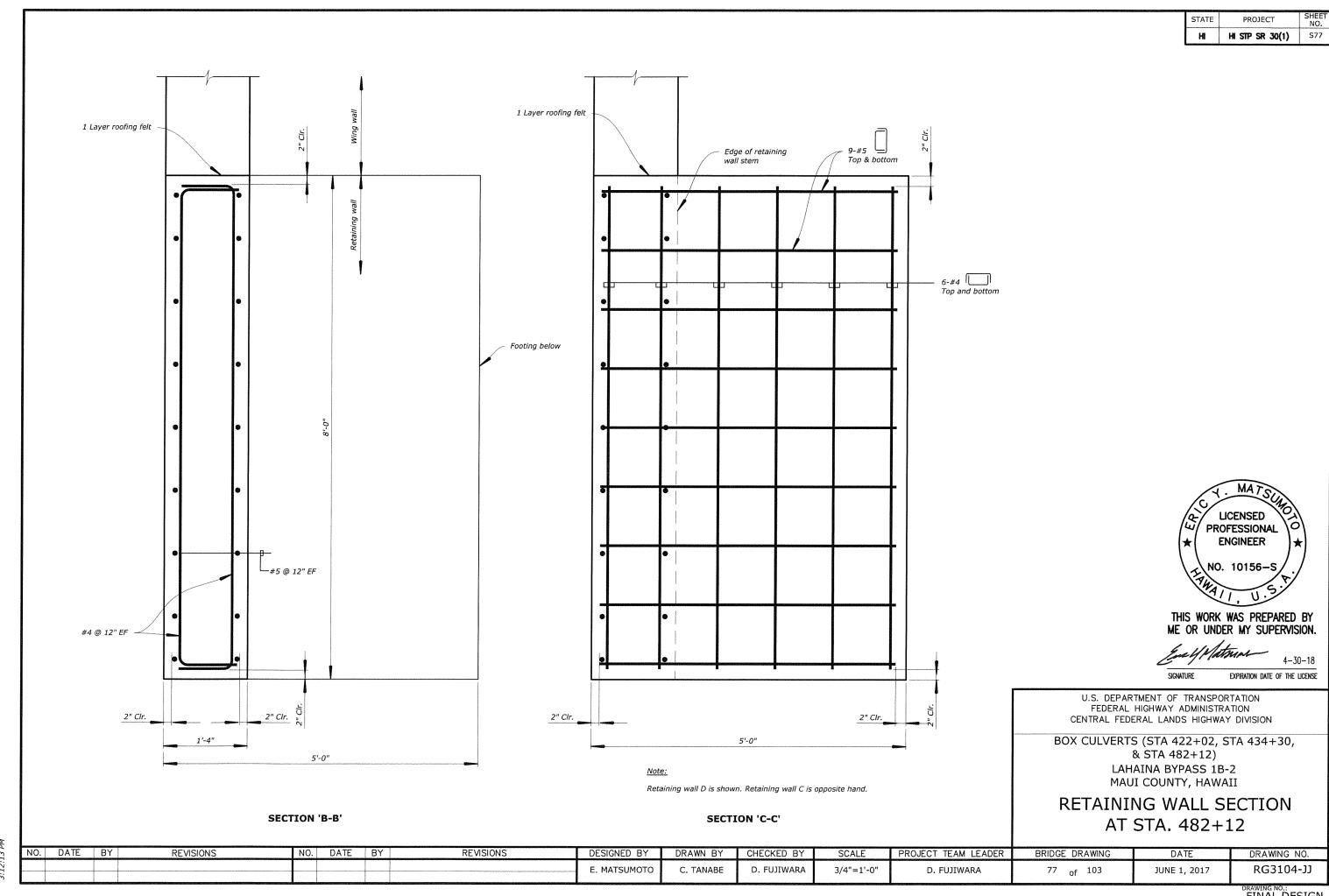
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SIGNATURE EXPIRATION DATE OF THE LICENSE

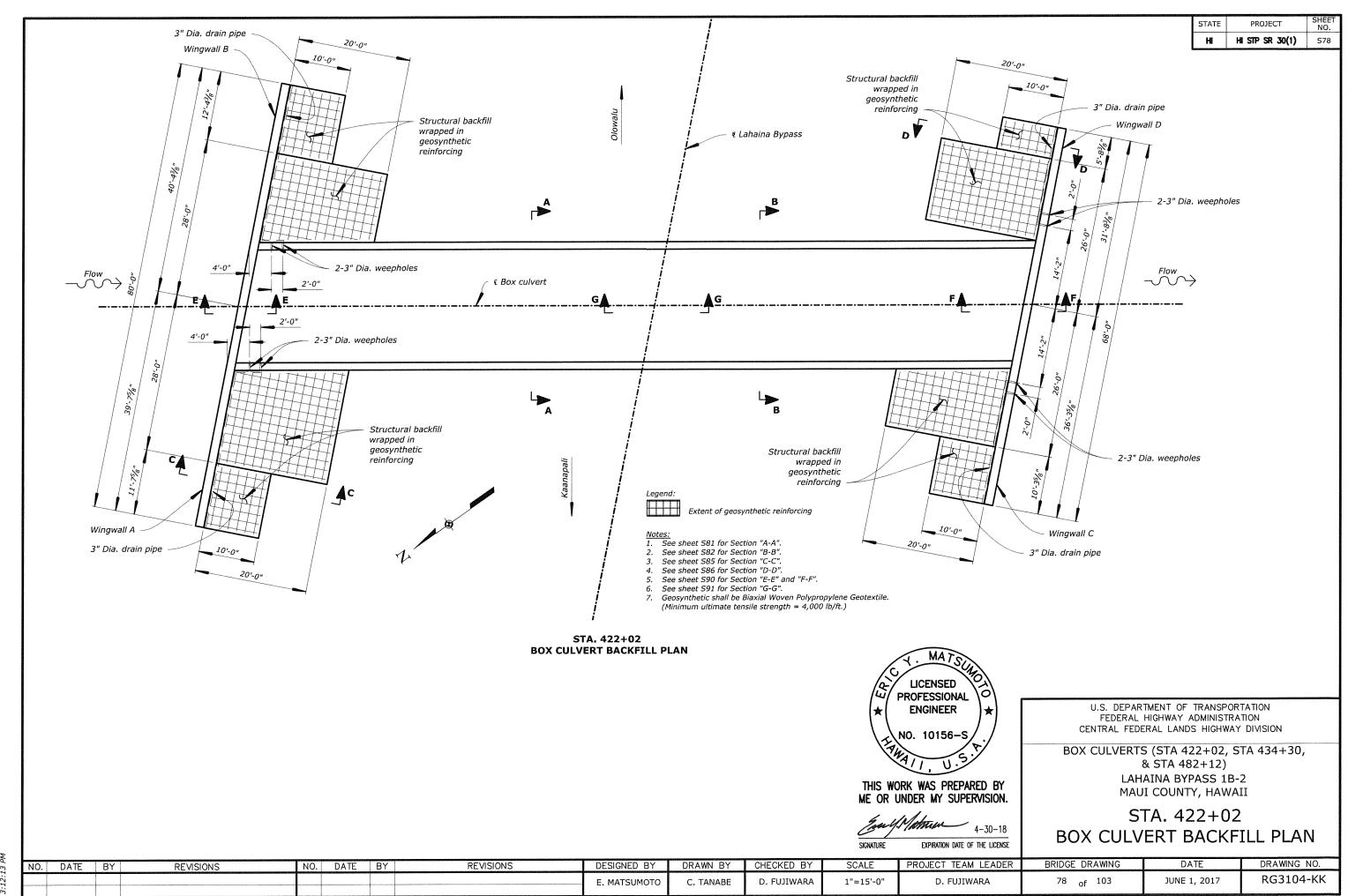
U.S. DEPARTMENT OF TRANSPORTATION
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CENTRAL FEDERAL LANDS HIGHWAY DIVISION

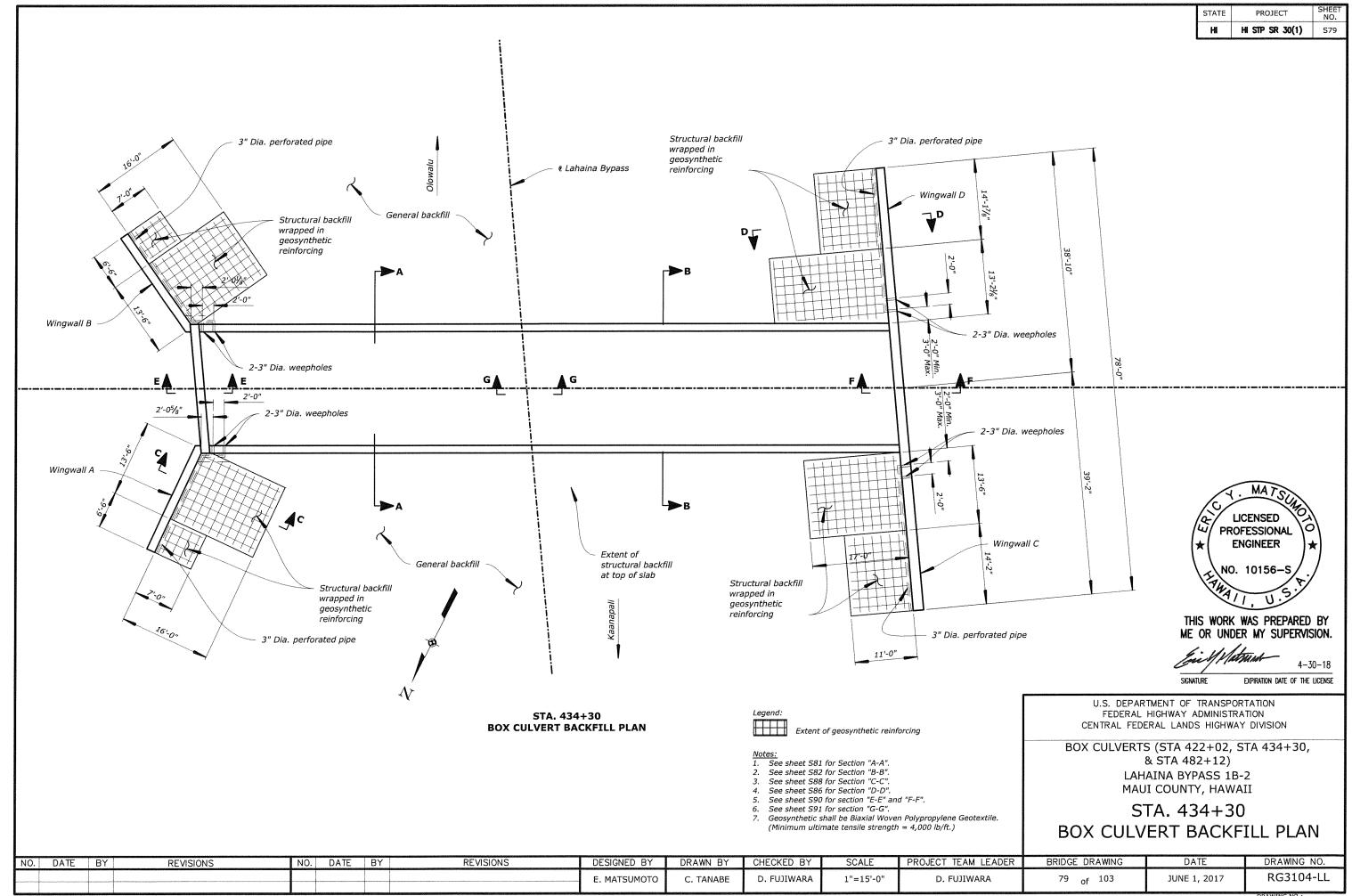
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

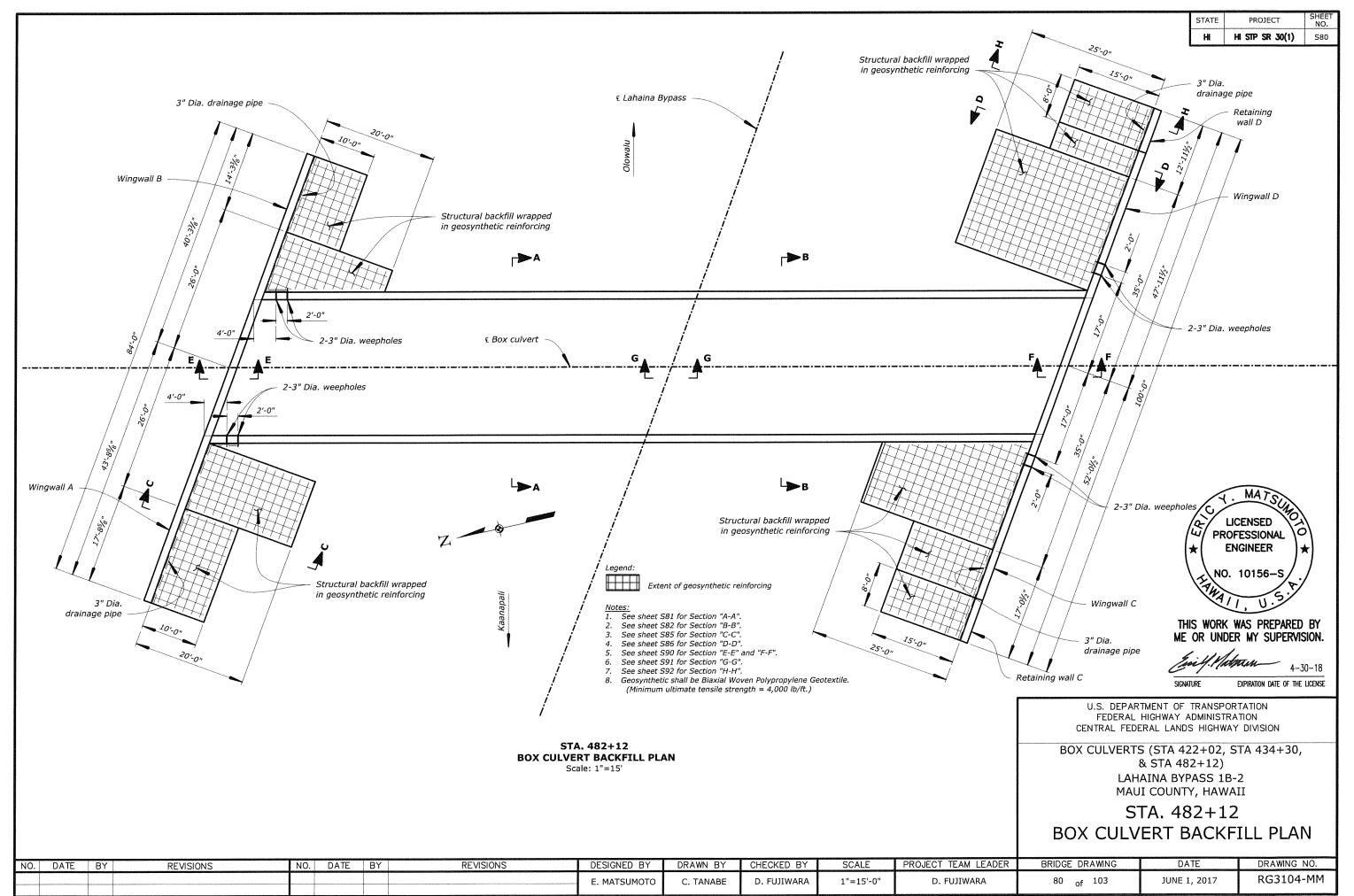
RETAINING WALL SECTION AT STA. 482+12

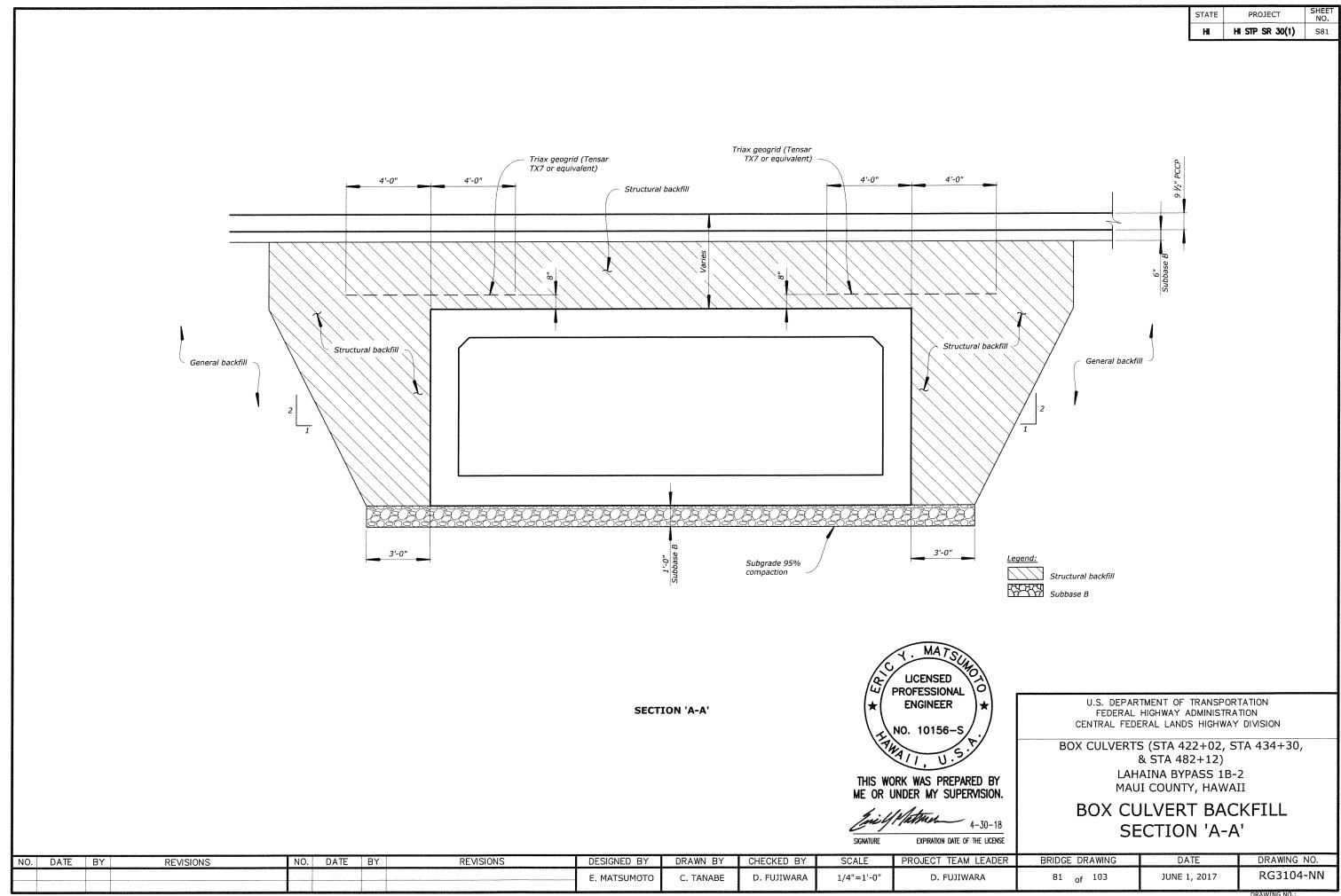
BRIDGE DRAWING DRAWING NO. CHECKED BY SCALE PROJECT TEAM LEADER DATE DATE BY REVISIONS NO. DATE BY REVISIONS DESIGNED BY DRAWN BY RG3104-II JUNE 1, 2017 D. FUJIWARA 76 _{of} 103 E. MATSUMOTO C. TANABE D. FUJIWARA 3/4"=1'-0"



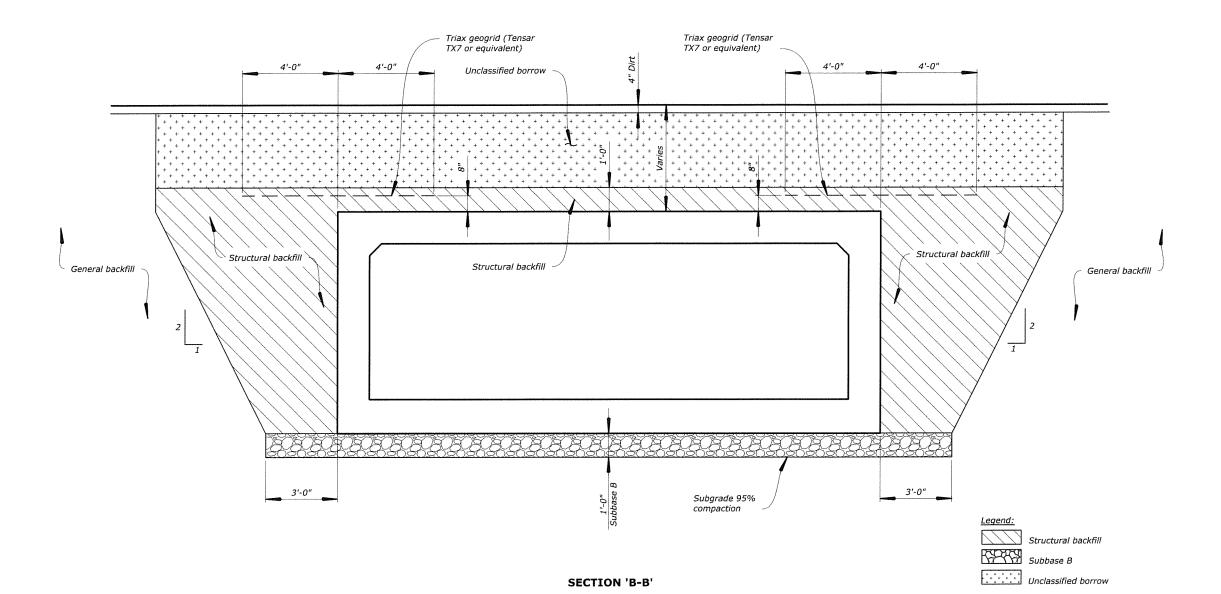








SHEET NO. STATE PROJECT HI STP SR 30(1) S82



LICENSED **PROFESSIONAL ENGINEER** NO. 10156-

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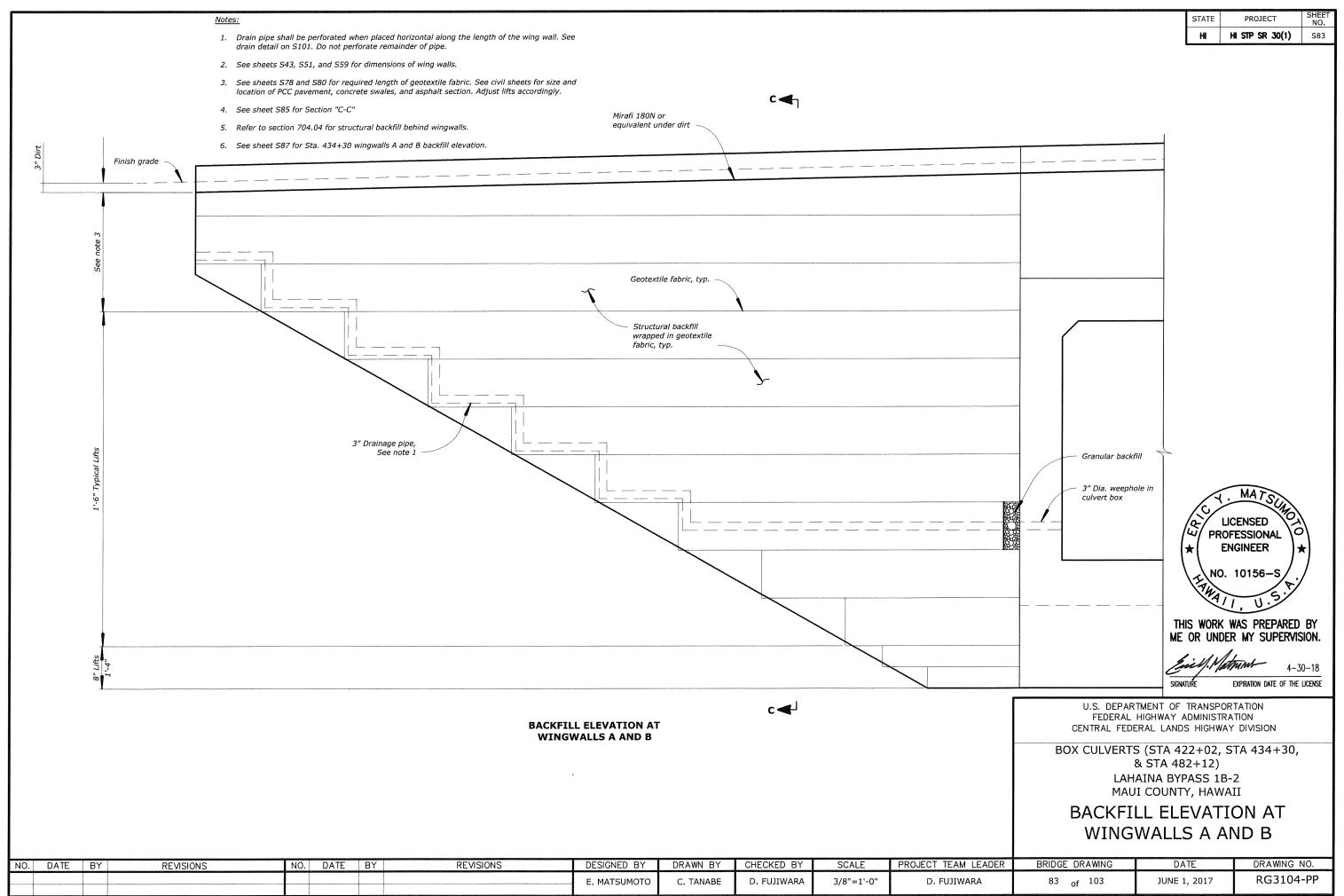
EXPIRATION DATE OF THE LICENSE

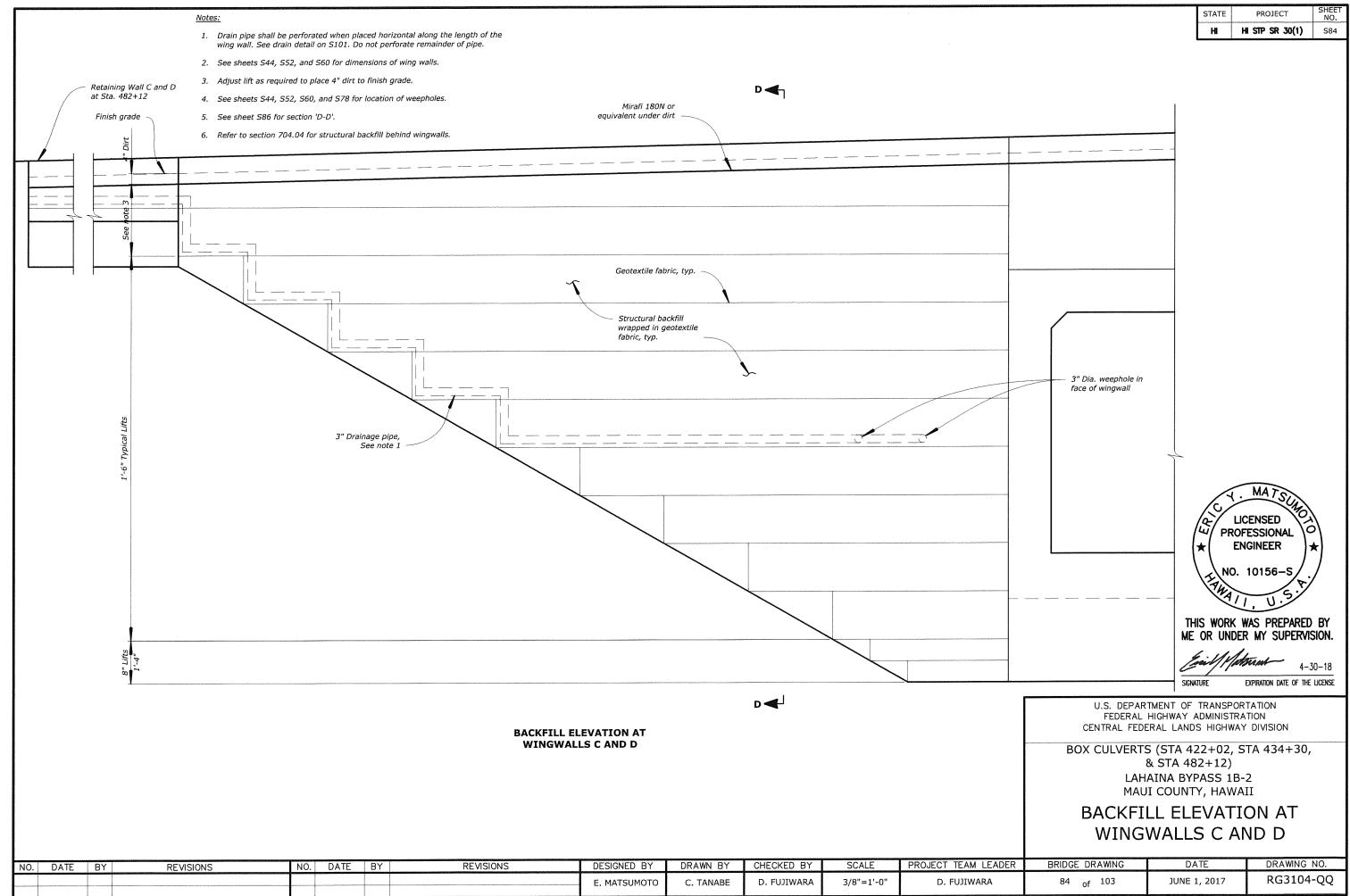
U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION
CENTRAL FEDERAL LANDS HIGHWAY DIVISION

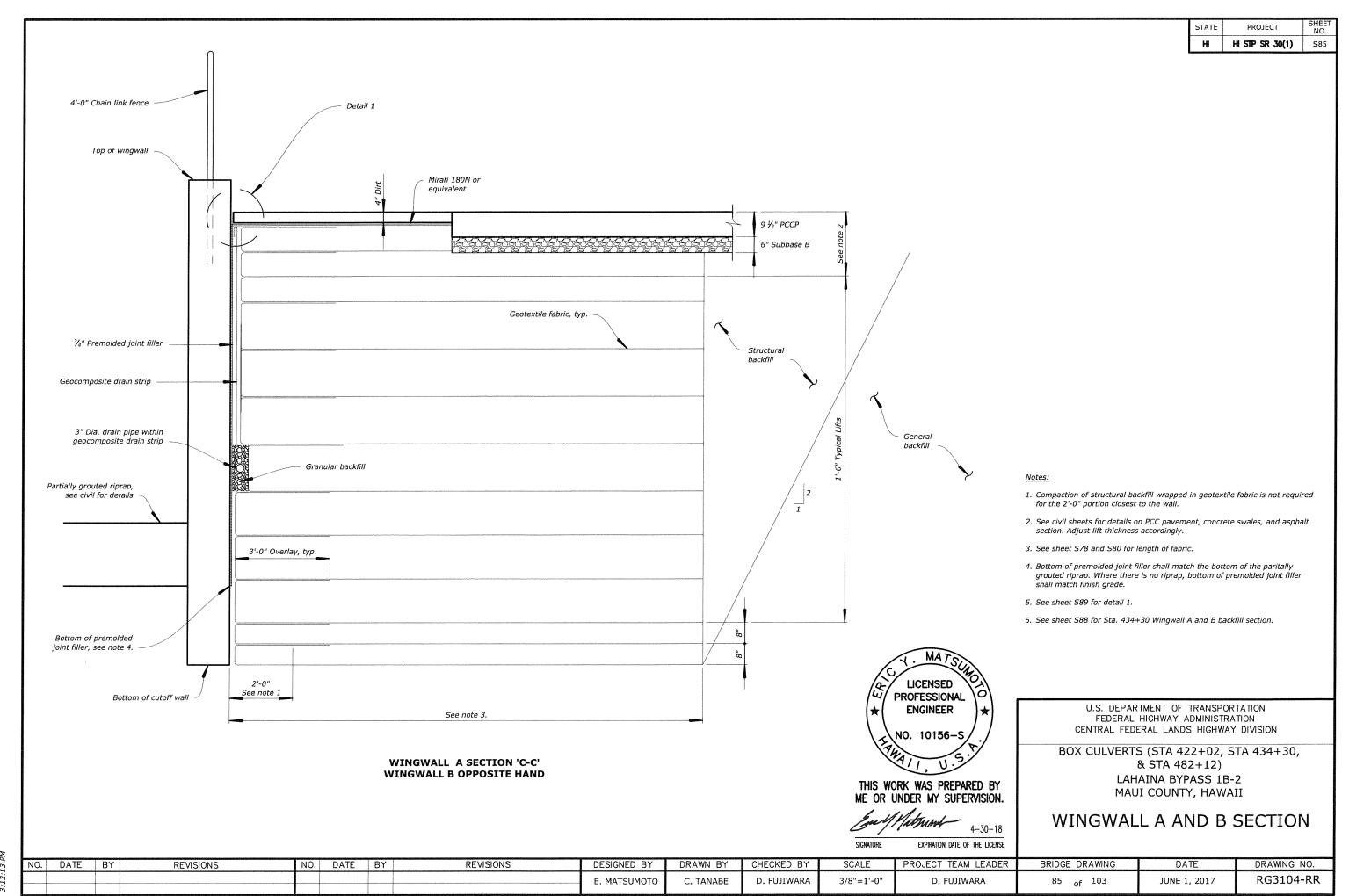
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

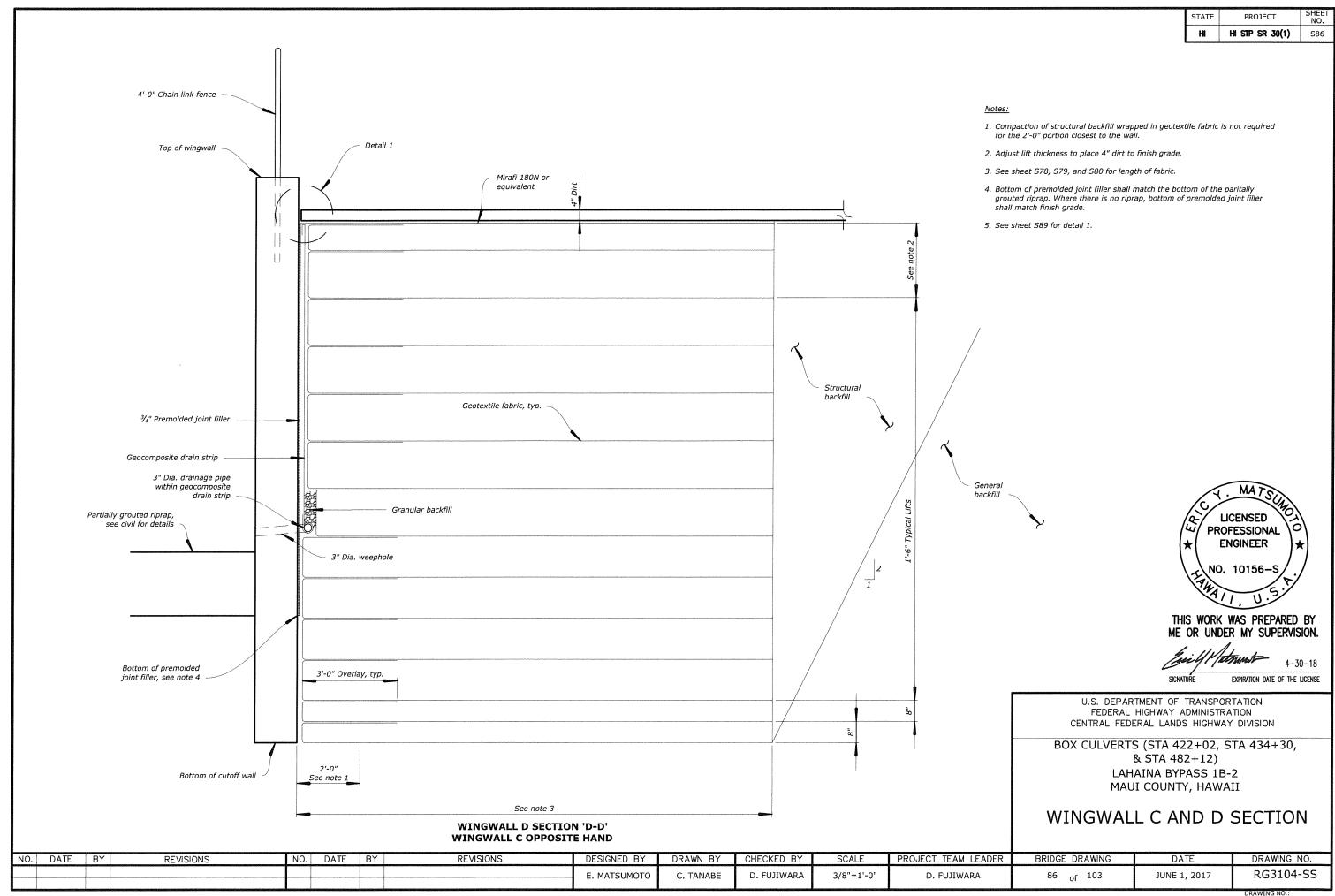
BOX CULVERT BACKFILL SECTION 'B-B'

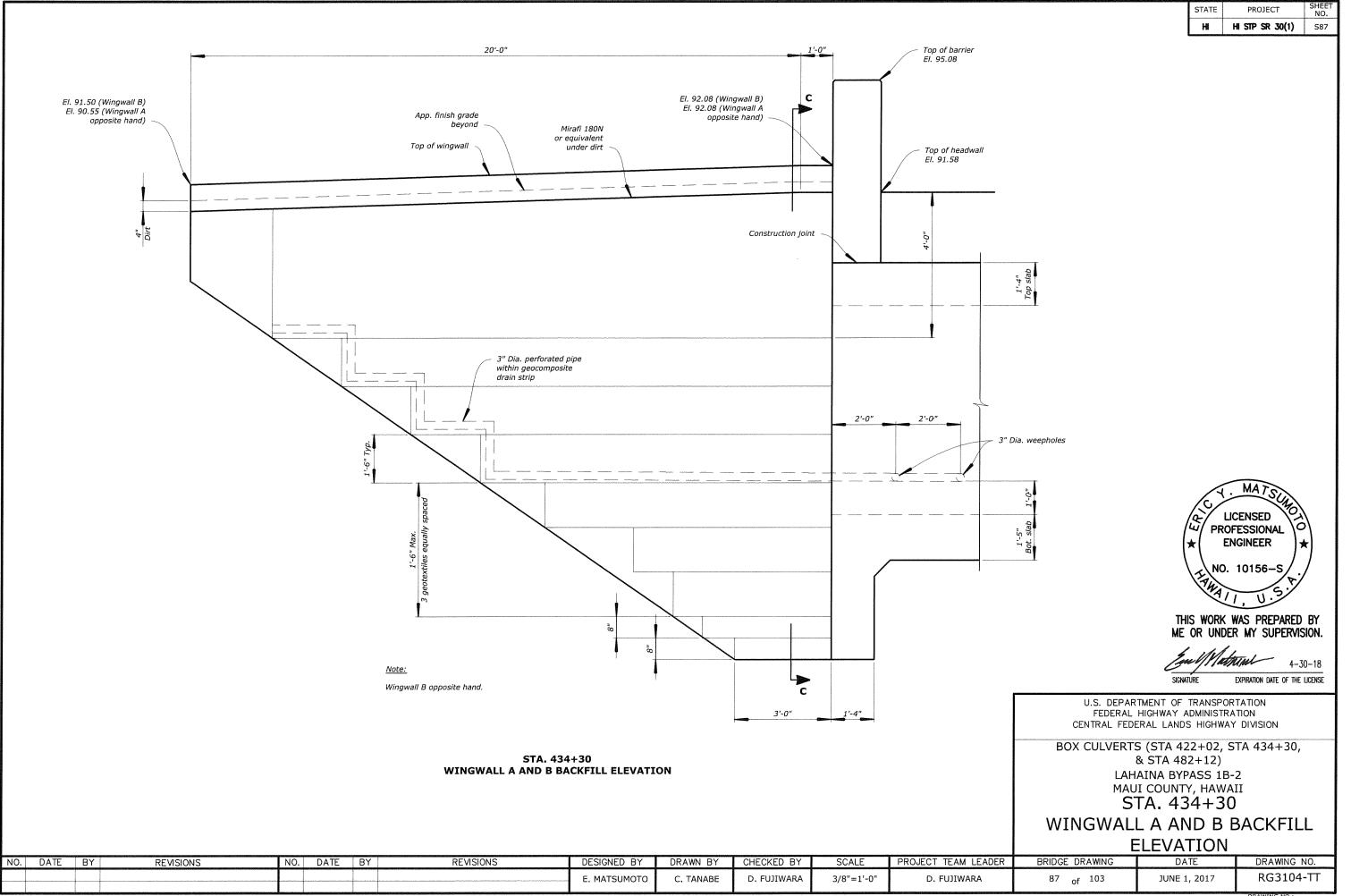
3 PI	NO. DA	ATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
3:12:1	- topic								E. MATSUMOTO	C. TANABE	D. FUJIWARA	1/4"=1'-0"	D. FUJIWARA	82 _{of} 103	JUNE 1, 2017	RG3104-00

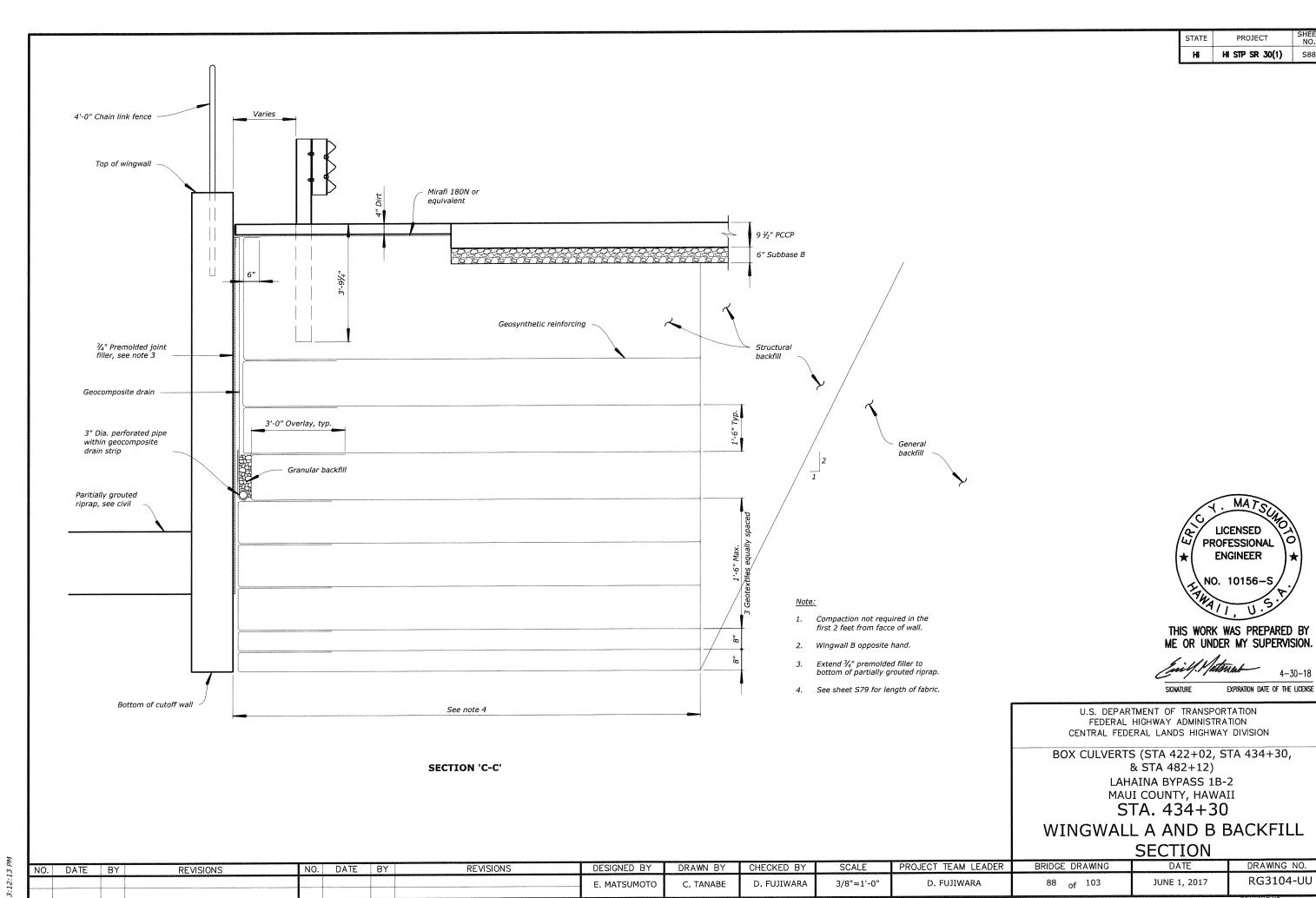












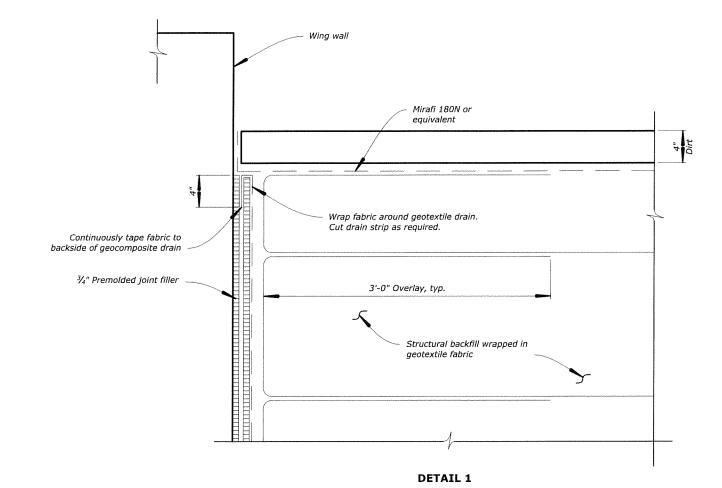
FINAL DESIGN

SHEET NO.

S88

 STATE
 PROJECT
 SHEET NO.

 HI
 HI STP SR 30(1)
 S89





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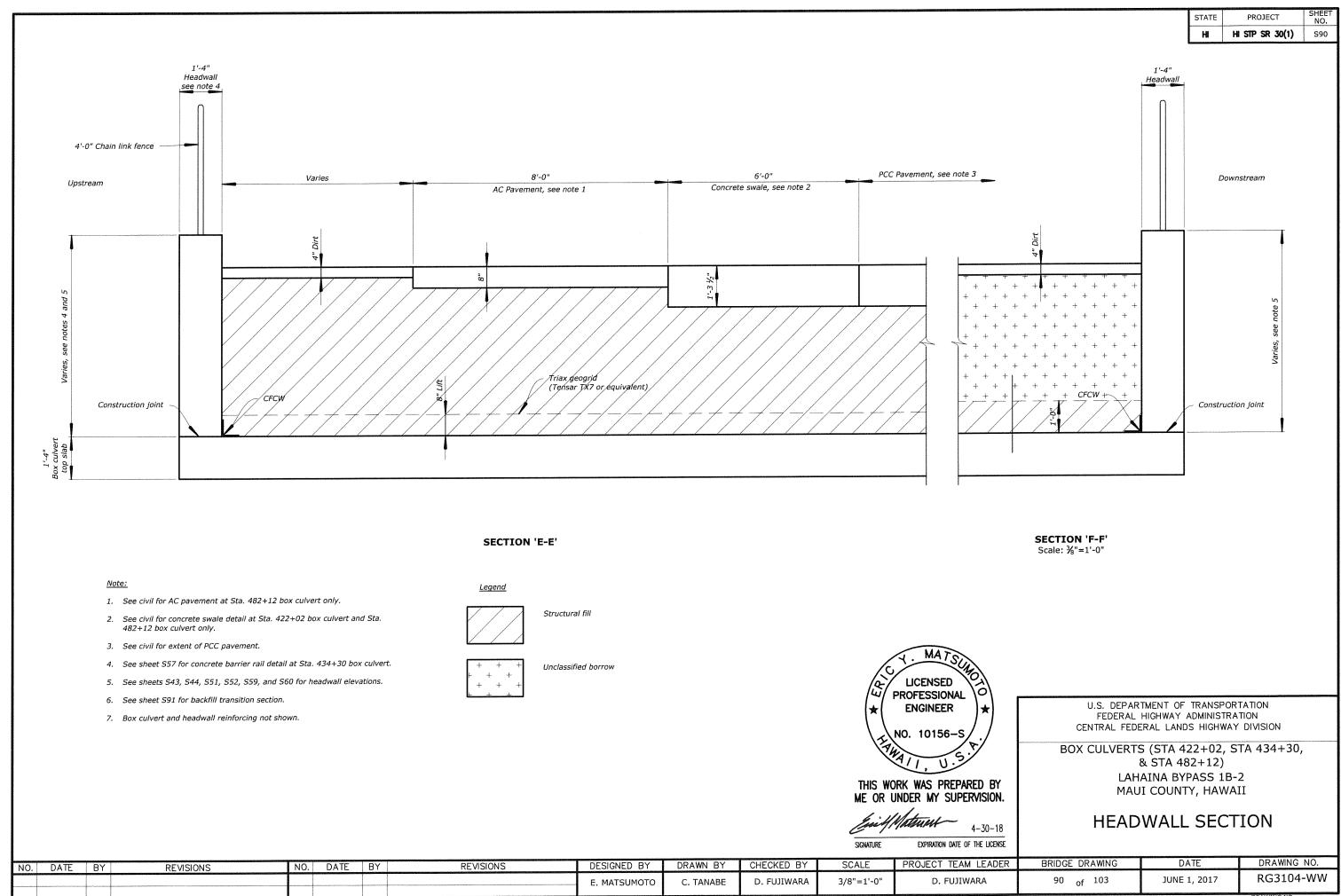
SIGNATURE EXPIRATION DATE OF THE LICENSE

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

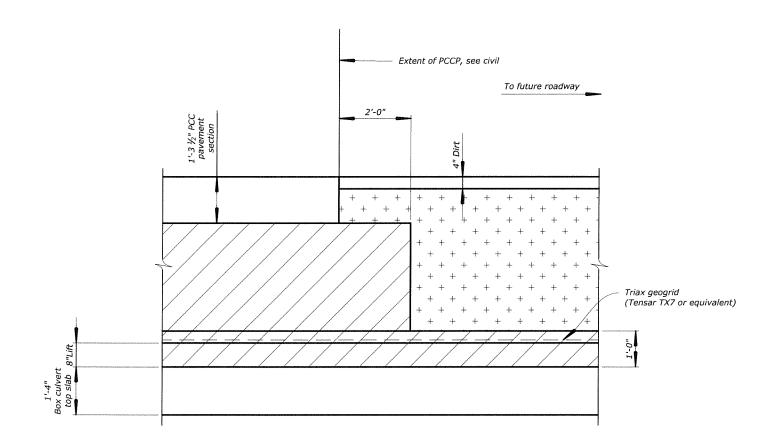
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

BACKFILL DETAILS

,	NO. DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
;		-						E. MATSUMOTO	C. TANABE	D. FUJIWARA	1"=1'-0"	D. FUJIWARA	89 _{of} 103	JUNE 1, 2017	RG3104-VV



SHEET NO. STATE PROJECT HI STP SR 30(1) S91



SECTION 'G-G'

SECTION 'E-E' Scale: 3/8"=1'-0"

Geogrids shall extend to the full length of the box culvert. See sheet S81 to S82 for details.

<u>Legend</u> Structural fill Unclassified borrow



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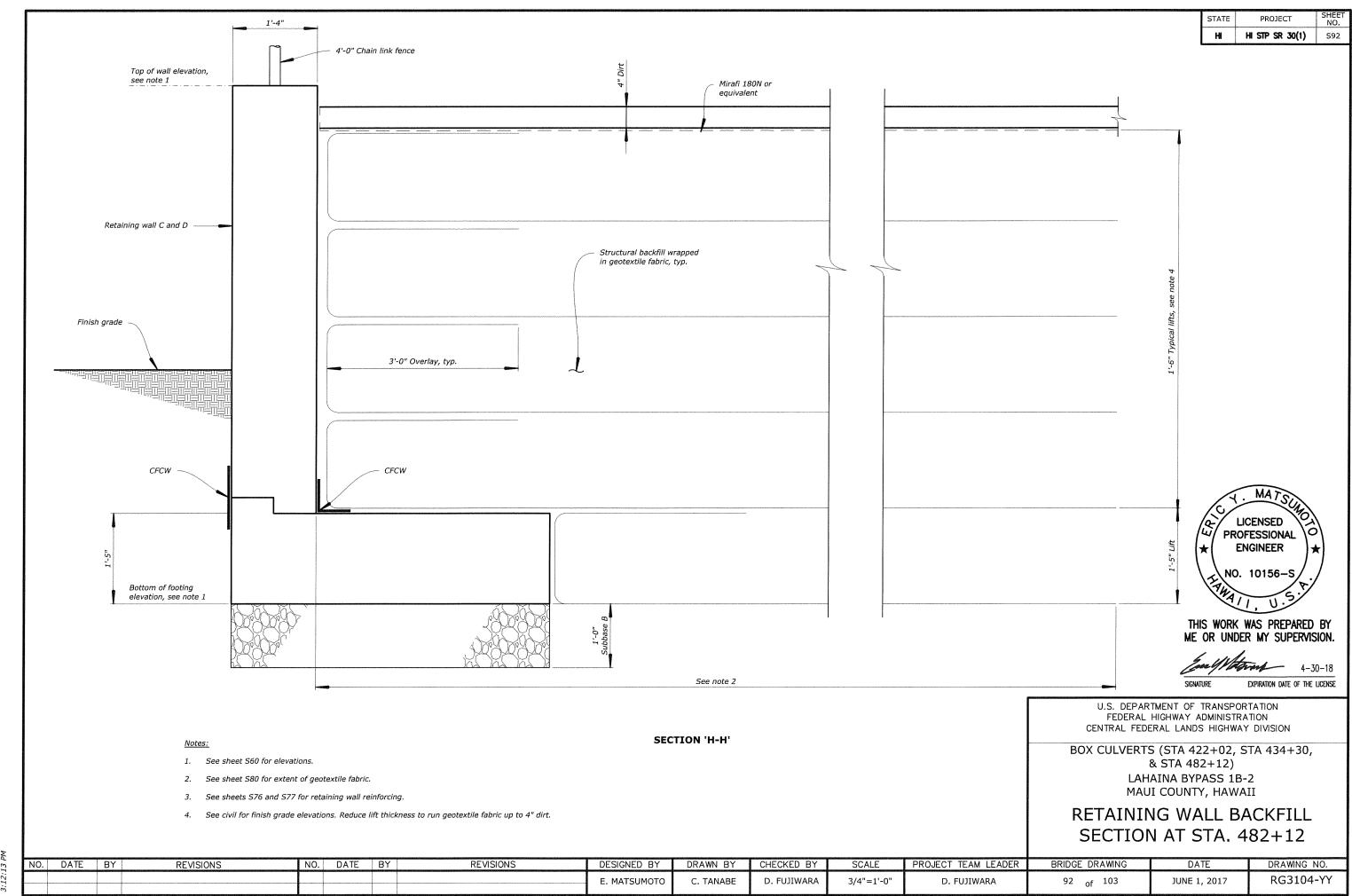
EXPIRATION DATE OF THE LICENSE

U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

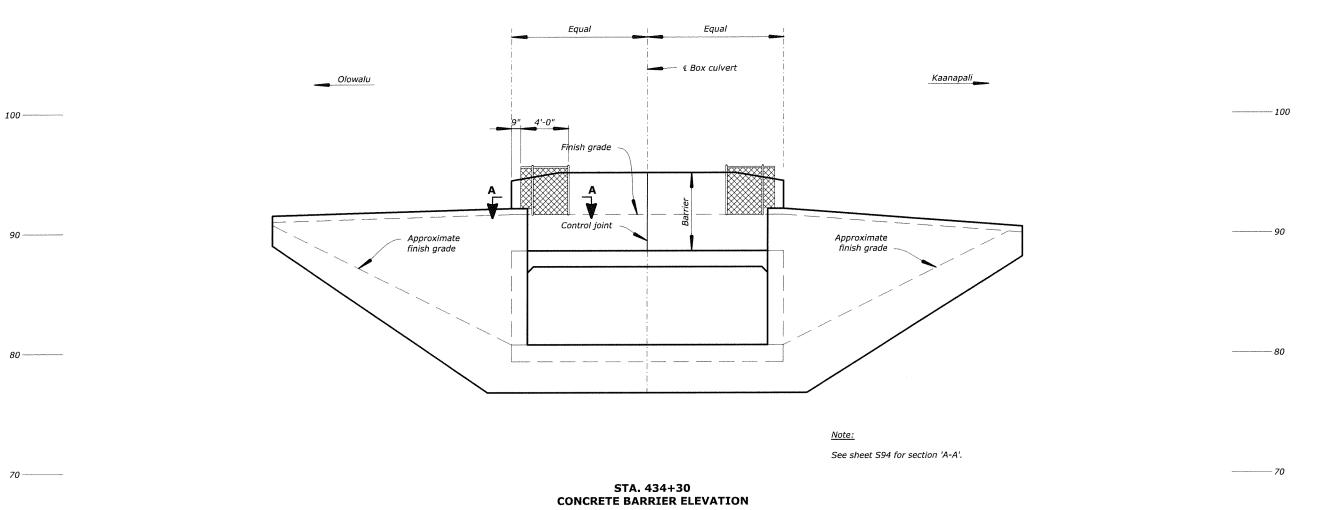
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

BACKFILL TRANSITION SECTION

13 54	NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
3:14.						-			E. MATSUMOTO	C. TANABE	D. FUJIWARA	3/8"=1'-0"	D. FUJIWARA	91 _{of} 103	JUNE 1, 2017	RG3104-XX







LICENSED PROFESSIONAL ENGINEER

NO. 10156-S

THAT SUMO A CONTRACT OF THE PROFESSIONAL ENGINEER

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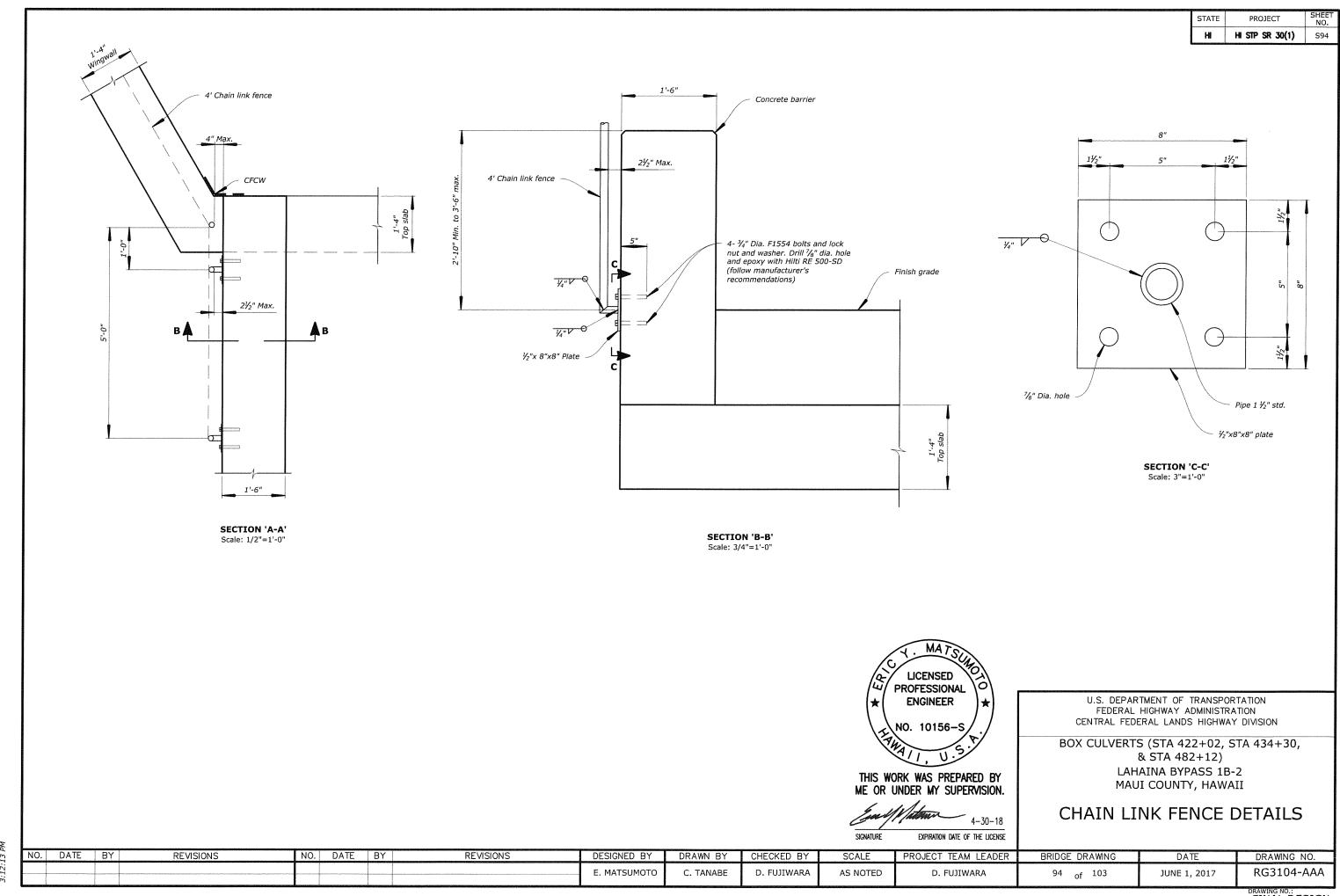
SIGNATURE EXPIRATION DATE OF THE LICENSE

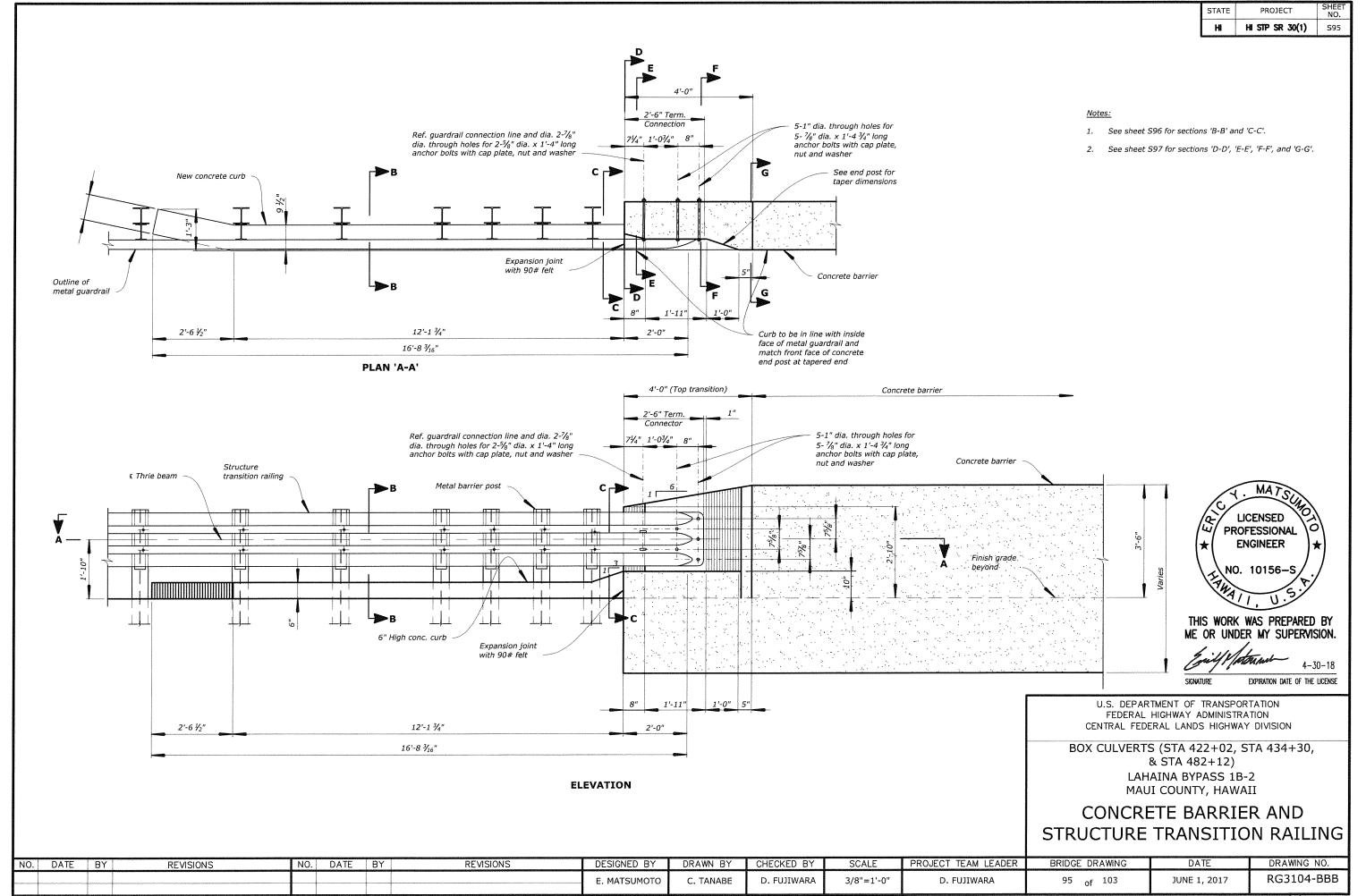
U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

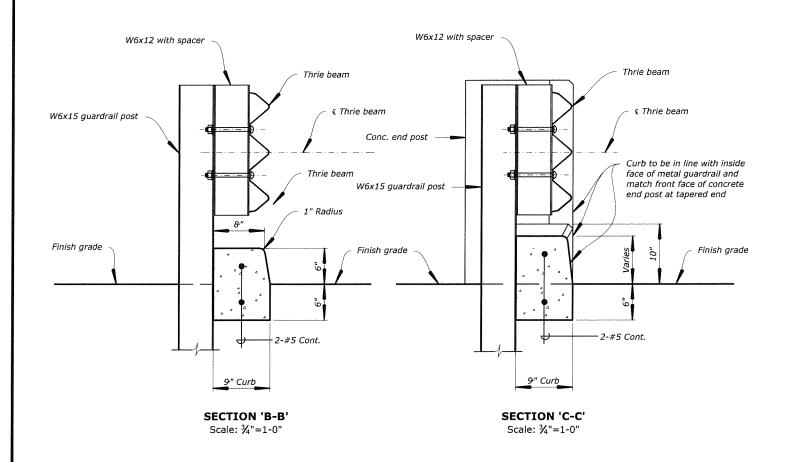
CONCRETE BARRIER ELEVATION

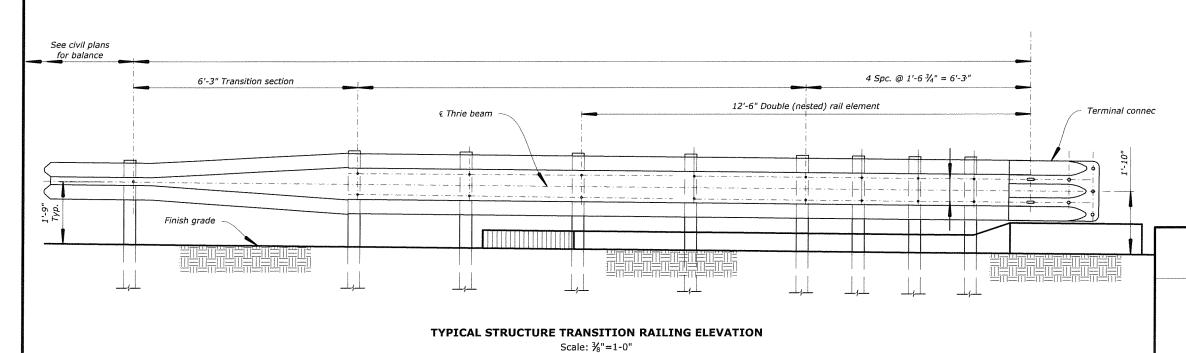
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3.0	NO. DATE BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
3:12:1							E. MATSUMOTO	C. TANABE	D. FUJIWARA	1/8"=1'-0"	D. FUJIWARA	93 _{of} 103	JUNE 1, 2017	RG3104-ZZ













U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

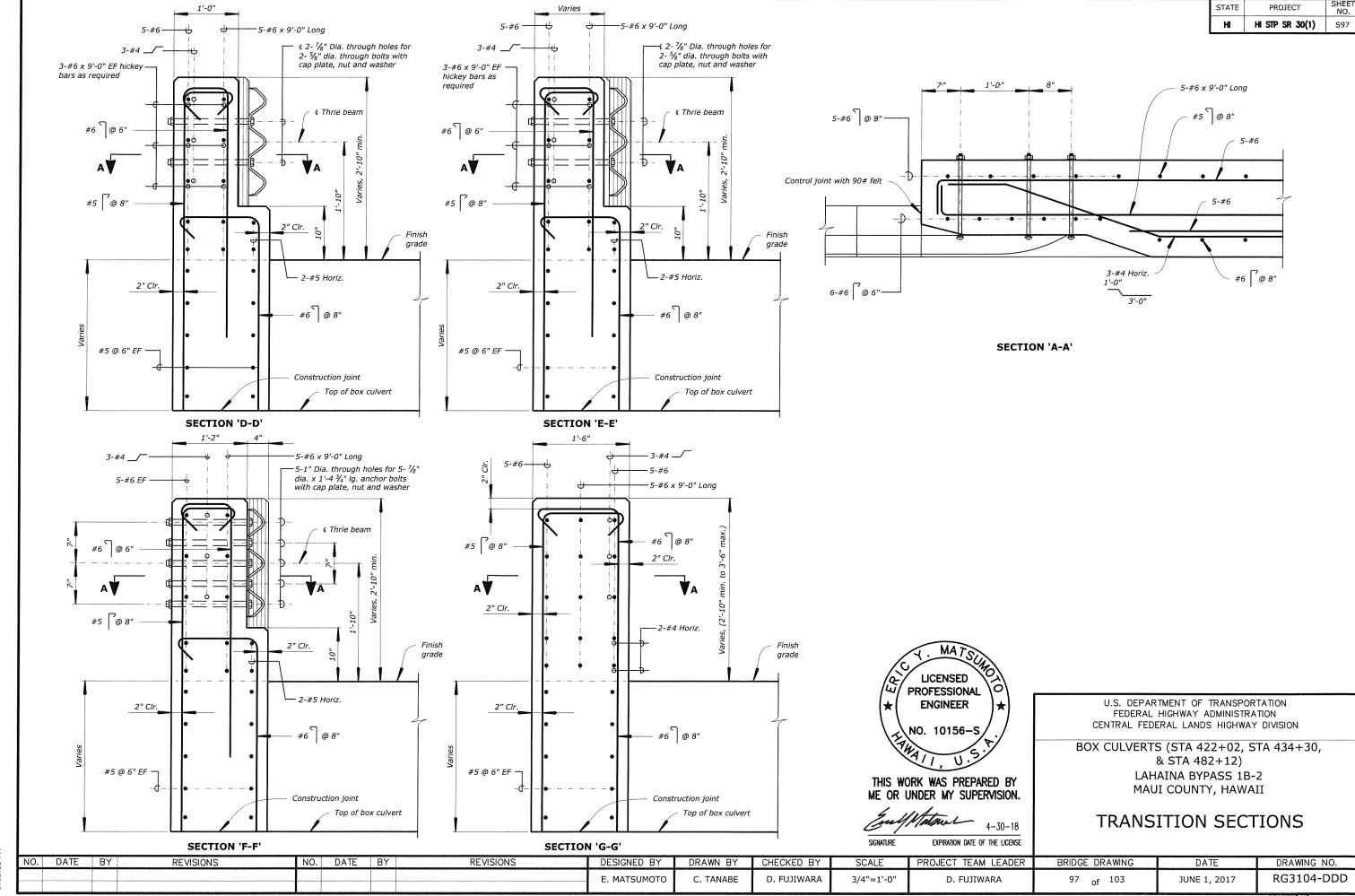
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

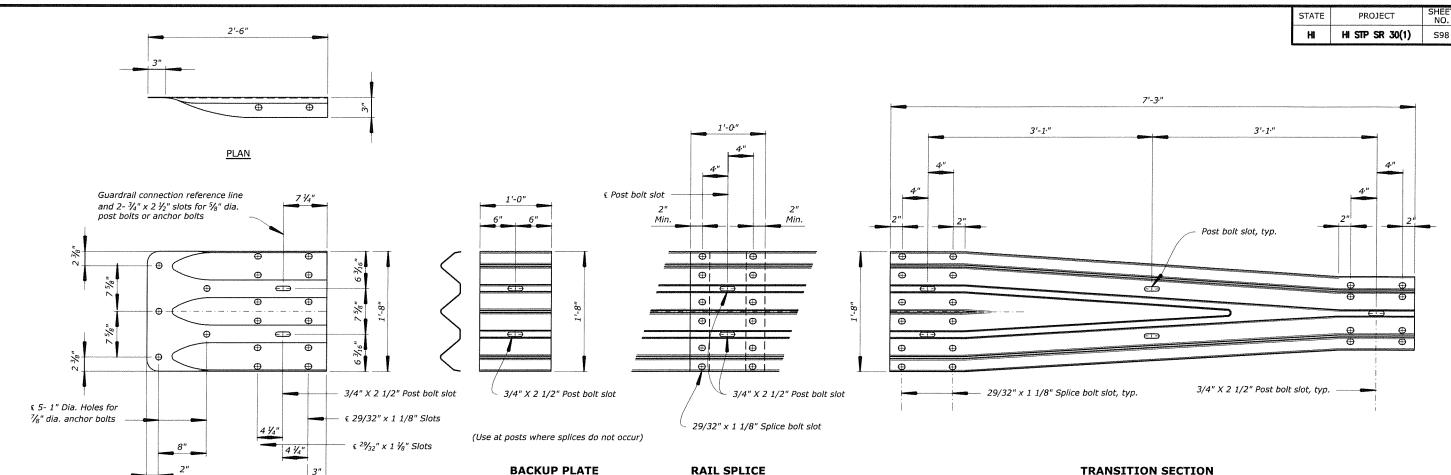
CONCRETE BARRIER TRANSITION RAILING

NO. DATE BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
						E. MATSUMOTO	C. TANABE	D. FUJIWARA	AS NOTED	D. FUJIWARA	96 _{of} 103	JUNE 1, 2017	RG3104-CCC

4-30-18

EXPIRATION DATE OF THE LICENSE





ELEVATION

2'-6"

TERMINAL CONNECTOR

NOTES:

- A. The work necessary to connect guardrail to concrete end post shall include all labor, materials, tools, equipment and incidentals necessary to complete the work and will not be paid for separately.
- B. Lap terminal connector and rail element in direction of traffic to prevent snagging.
- C. Splice bolt (FBB01), rail bolt (FBB02), and recessed nut shall conform to AASHTO M180 and be hot-dip galvanized in accordance with AASHTO M232, Class C.
- D. Post hex bolt and nut shall conform to Section 617.
- E. "Terminal Connector" and standard spacer, including all anchor bolts, nuts and washers, shall be hot-dip galvanized after fabrication.
- F. "Terminal Connector", "Transition Section" and thrie beam shall be fabricated from 10 gauge steel conforming to the requirements of AASHTO M180, Type II, Class B. All other rail shall conform to the requirements of AASHTO M180, Type II, Class A.
- G. First 25'-0" of guardrail adjoining "Terminal Connector" shall be galvanized steel and supports spaced as shown on the detail drawings. This section of rail shall be placed on tangent to end post or parallel to roadway, unless conditions at site renders it impossible to do so. Flare point to be determined in field.
- H. Double (nest 1st panel) thrie beam elements at all end post connections, except on highways with one-way traffic pattern, use single thrie beam elements at end post on trailing end only.
- I. Where double (nested) beam occur, 12" "Back-Up Plate" not required.
- J. Heads of through anchor bolts shall be placed on the traffic side of the rail.
- K. W-beam structural steel shall conform to ASTM A992. Structural steel plates and bars shall conform to ASTM A36.
- L. No bolts are to be cut after manufacturing and galvanizing.

IRANSTITON SECTION



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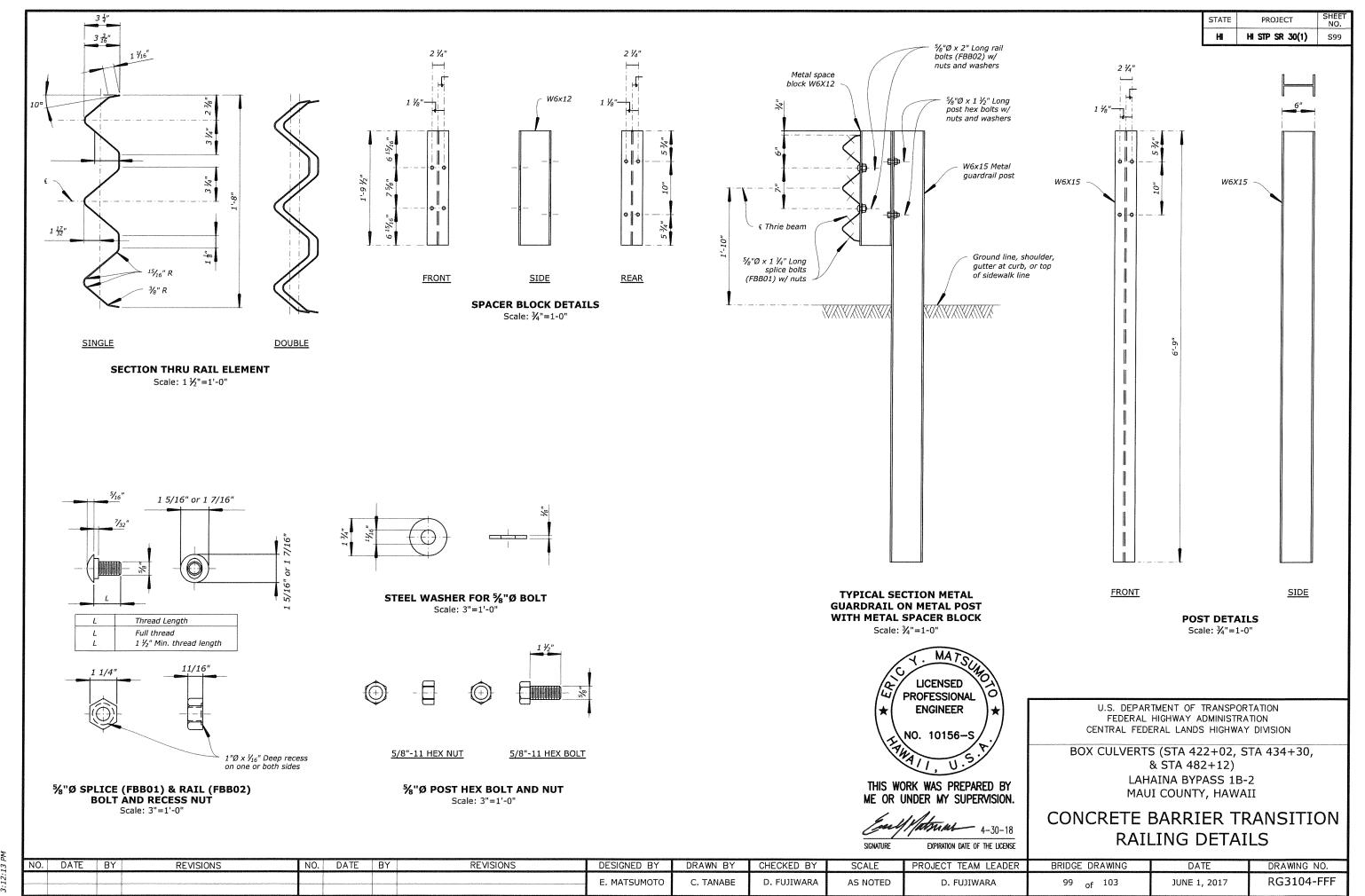
SIGNATURE EXPIRATION DATE OF THE LICENSE

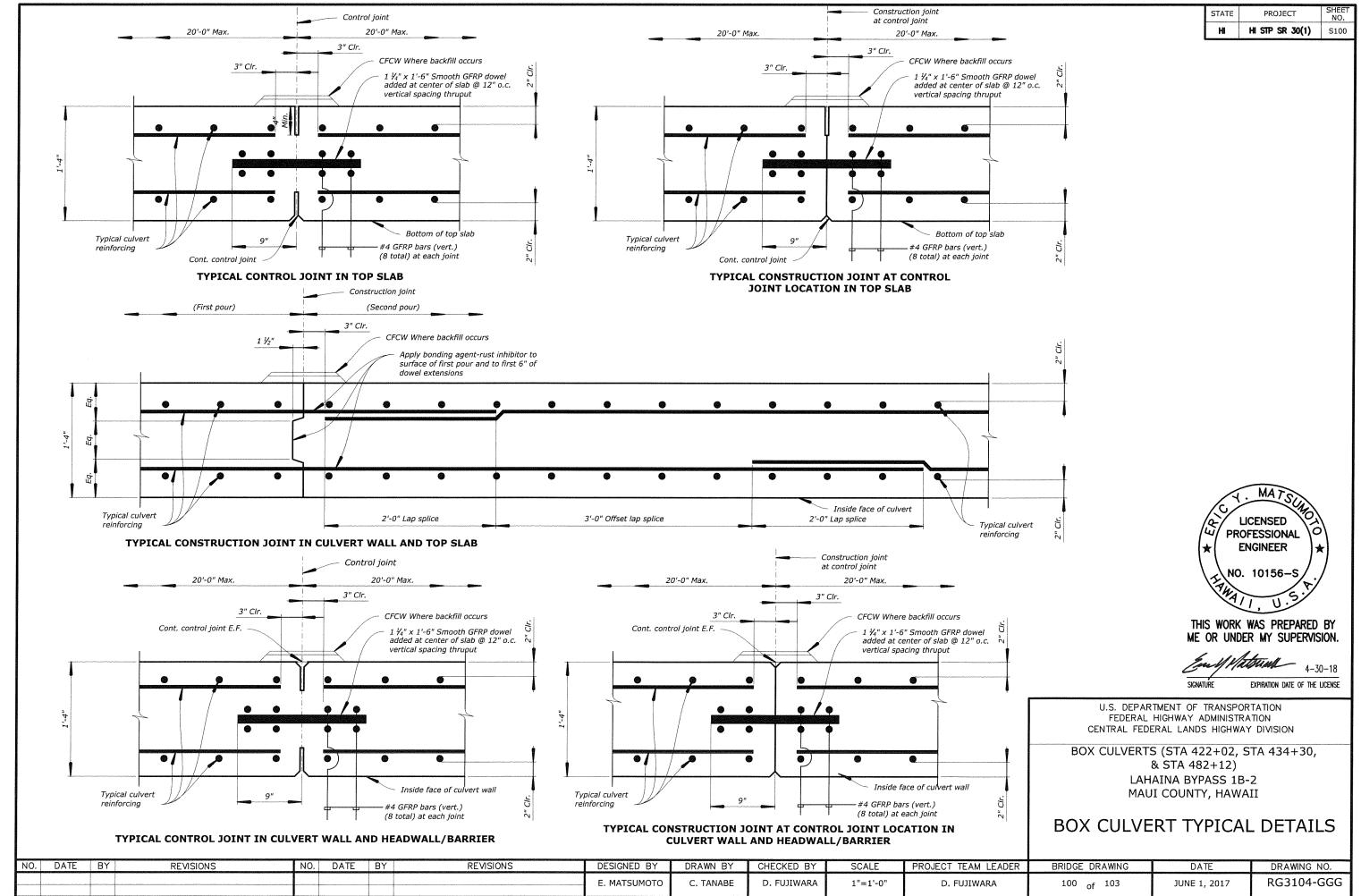
U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION
CENTRAL FEDERAL LANDS HIGHWAY DIVISION

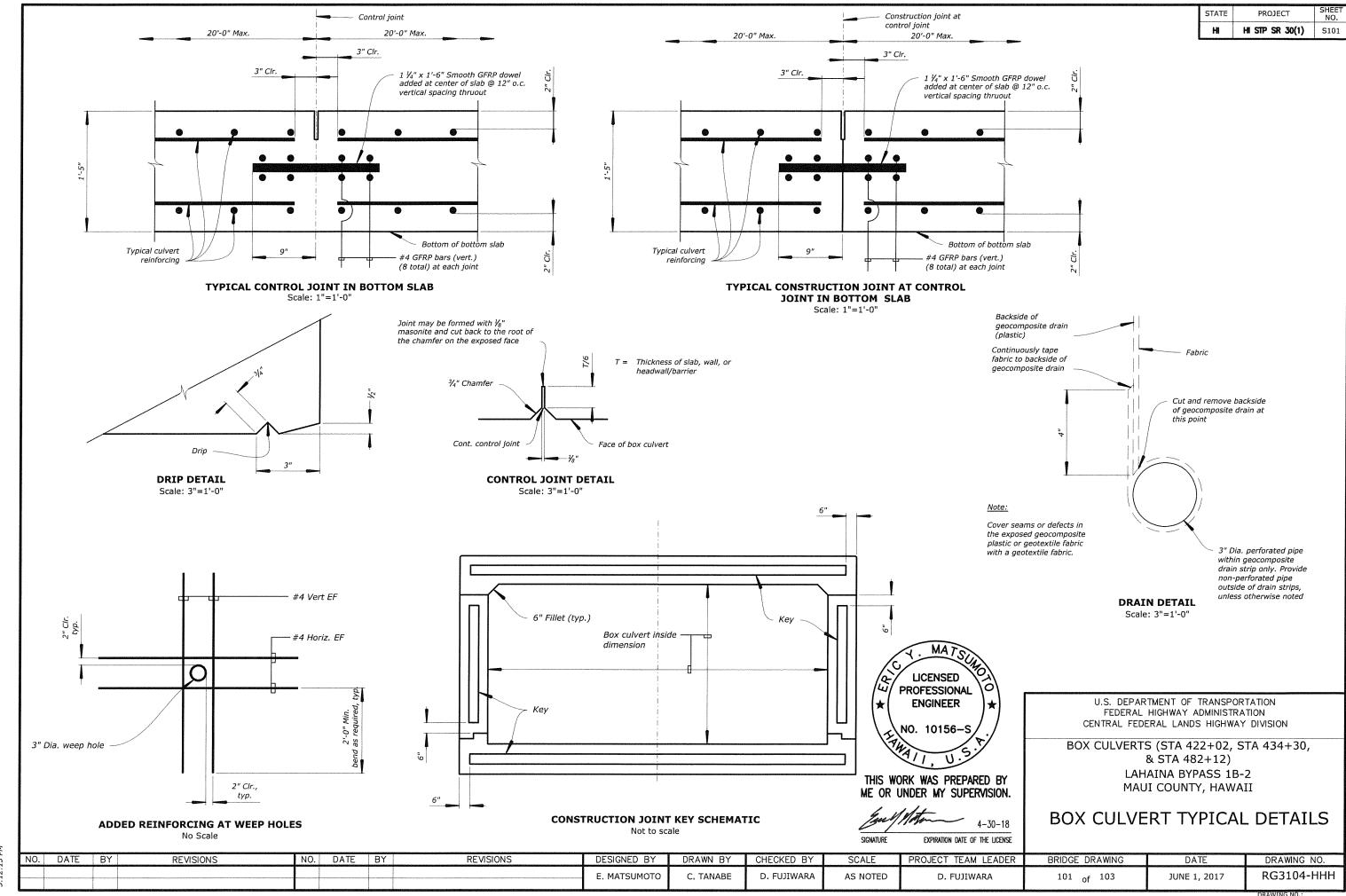
BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

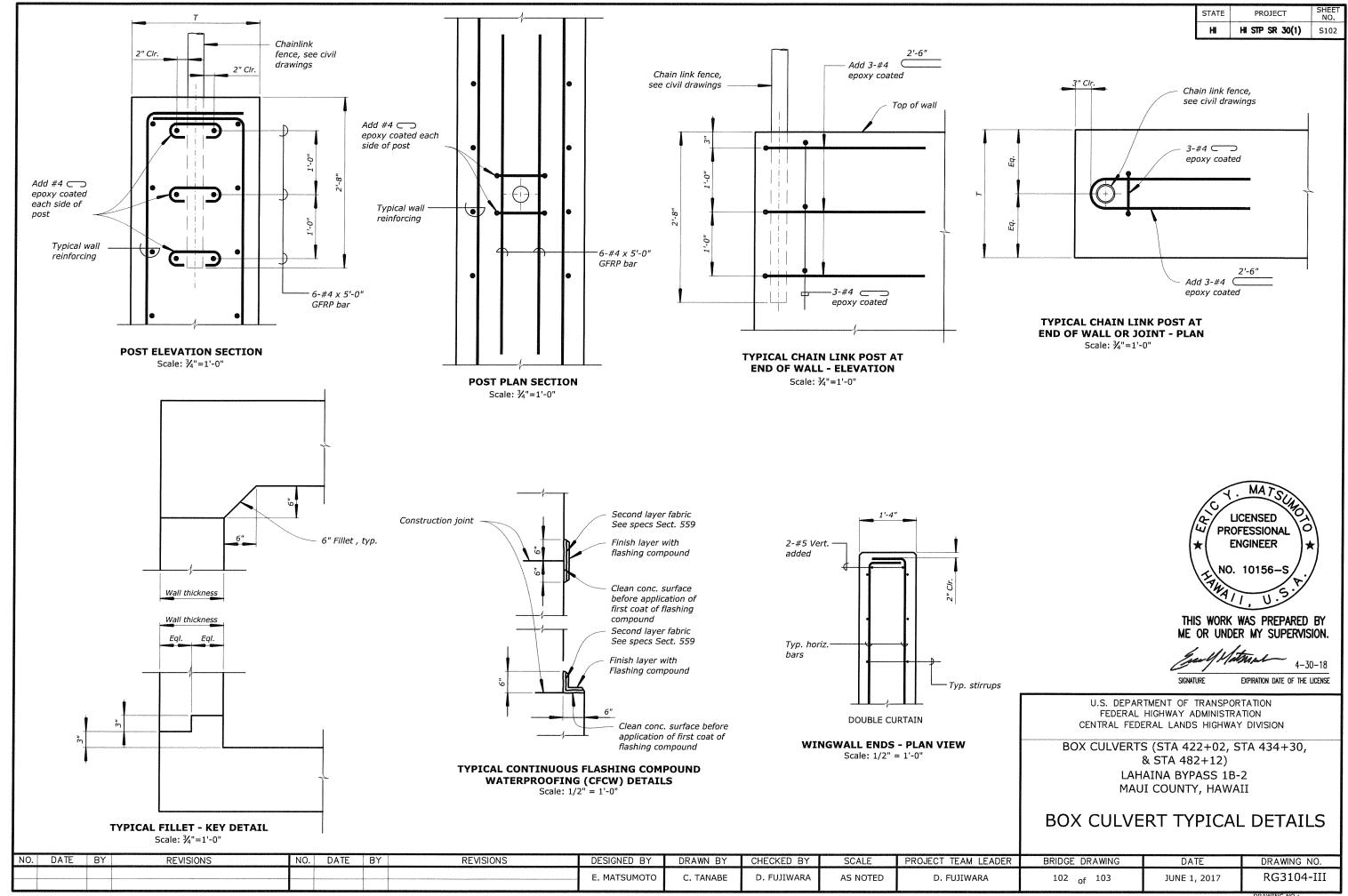
CONCRETE BARRIER TRANSITION RAILING NOTES AND DETAILS

NO. DATE BY REVISIONS NO. DATE BY REVISIONS DESIGNED BY DRAWN BY CHECKED BY SCALE PROJECT TEAM LEADER BRIDGE DRAWING DATE DRAWING NO. RG3104-EEE E. MATSUMOTO C. TANABE D. FUJIWARA 3/4"=1'-0" D. FUJIWARA JUNE 1, 2017 98 of 103



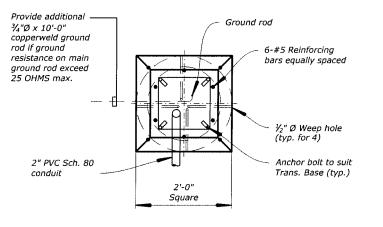




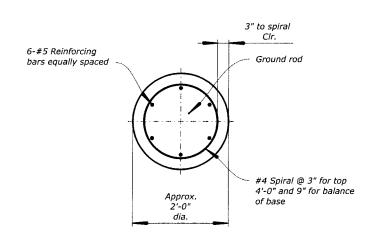


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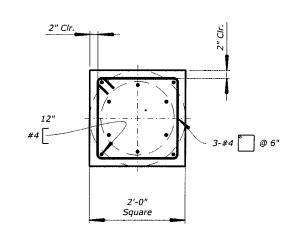
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PLAN



SECTION 'A-A'



SECTION 'B-B'

Notes

- 1. See civil and electrical drawings for additional details
- Contractor shall design pole, base, anchor bolts, and all appurtenances for the light pole.
- 3. Light pole manufacturer's recommendations shall be followed.
- 2. Separate dissimilar metals with teflon tape at contact points only.



U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION CENTRAL FEDERAL LANDS HIGHWAY DIVISION

BOX CULVERTS (STA 422+02, STA 434+30, & STA 482+12) LAHAINA BYPASS 1B-2 MAUI COUNTY, HAWAII

HIGHWAY LIGHT STANDARD FOUNDATION

13 PI	NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE DRAWING	DATE	DRAWING NO.
3:12:									E. MATSUMOTO	C. TANABE	D. FUJIWARA	1/2"=1'-0"	D. FUJIWARA	103 _{of} 103	JUNE 1, 2017	RG3104-JJJ
																DDAWING NO

Light pole

в

3" Clr.

to spiral

Shaft diameter

TYPICAL POLE FOUNDATION

— 3-#4 💟 Closed ties @ 6"

411411411

Drilled shaft

1 $\frac{1}{2}$ " Turn at top and bottom of drilled shaft

Pour concrete against

undisturbed ground

Non-shrink grout

(4) 1" Ø x 40" Anchor bolts (stainless steel)

Finish grade or

1'-6" Squar section top of pavement

4-30-18

EXPIRATION DATE OF THE LICENSE