INDEX TO STRUCTURAL DRAWINGS

DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	37C-02-23	2024	69	115

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DESCRIPTION



STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

INDEX TO STRUCTURAL DRAWINGS

KULA HIGHWAY,
INTERSECTION IMPROVEMENTS
AT OMAOPIO ROAD
Project No. 37C-02-23

Scale: None

None Date: Apr. 2024
SHEET No. S0.1 OF 8 SHEETS

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SHEET NO.

STRUCTURAL GENERAL NOTES

DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	37C-02-23	2024	70	115

1. General Specifications: Hawaii Department of Transportation, Standard Specifications for Road and Bridge Construction, 2005, and Standard Special Provisions containing the most current revisions together with Special Provisions prepared for this contract.

2. Design Specifications:

- A. American Association of State Highway and Transportation Officials (AASHTO) 2020 "LFRD Bridge Design Specifications", 9th Edition as amended by HDOT document dated August 8, 2014 with subject title "Design Criteria for Bridges and Structures".
- B. HDOT Document dated August 8, 2014 with subject title "Design Criteria for Bridges and Structures" and HDOT memorandum dated January 8, 2018 with subject title "Changes to Design Criteria for Bridges and Structures".
- C. AASHTO 2015 "LRFD Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals", 1st Edition including all subsequent interim revisions and editions as amended by the HDOT document dated August 8, 2014 with subject title "Design Criteria for Bridge and Structures".

3. Materials:

A. Concrete structures shall be as noted below:

Item No.	Structural Parts	Compressive Strength, f'c (28 Days)	Maximum Water/ Cementitious (W/C)	Maximum Cementitious Material Content (Ibs./cy)	Included Admixtures (See Notes Below in This Section)
(1)	Final CIP Facing	5,000 psi	0 . 45	670	C, D, E
(2)	Concrete Ditch "A"	5,000 psi	0.45	700	C, D, E
(3)	All Others, except as noted otherwise	4,000 psi	0.49	700	С

- B. The use of any calcium chloride in any concrete is prohibited.
- C. Amine carboxylate corrosion inhibiting water-based admixture such as Cortec MCI 2005 NS or approved equal shall be added at a dosage of 24 ounces per cubic yard of concrete.
- D. Shrinkage reducing admixture such as Masterlife SRA 35 or approved equal shall be added at a dosage of 128 ounces per cubic vard of concrete or as recommended by the Manufacturer.

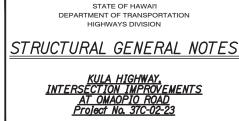
Materials (Cont.):

- E. Alkali resistant structural glass fiber such as CEMFIL, ANTI-CRAK HP67/36 or approved equal shall be added to the concrete mix. The dosage rate shall be 15 lbs. per cubic yard of concrete for CEMFIL or the equivalent amount of approved equal to achieve similar properties as the glass dosage. The fiber shall be added to the concrete as recommended by the Manufacturer during the mixing process.
- F. The Contractor has the option to design the concrete for all items to be pumpable and flowable. All concrete shall be designed for minimum segregation and separation.
- G. Shotcrete for wall:
 - (1) Shotcrete for walls shall have a minimum compressive strength at 28-days of 5000 psi and have a maximum 0.45 water to cement ratio and contain 24 oz. per cubic yard of migrating amine carboxylate corrosion inhibiting water-based admixture. Cortec MCI-2005 NS or approved equivalent. A shrinkage reducing admixture, such as Eclipse or Master Life SRA 20 or approved equivalent shall be added at a dosage of 128 oz. per cubic vard. Shotcrete shall contain 7.5 lbs of Strux 85/50 synthetic structural fiber per cubic yard or approved equal.
 - (2) Temperature of shotcrete shall not exceed 90°F at the point of placement.
 - (3) Shotcrete shall be cured using Sinak Lithium Cure or approved equivalent at a coverage rate of 100 sq. ft. per aallon.
- n) H. Grout for soil nails shall be a pre-bagged neat cement, non-shrink grout with silica fume admixture and anti-washout additive in conformance with ASTM C1107. Grout shall achieve minimum 8,000 psi compressive strength at 28-days such as Target 1118 Grout or approved equal. Additionally, 10 grams of an Amine Carboxylate Corrosion Inhibiting Admixture, such as Cortec Mini MCI Grenades or approved equal shall be added to each bag of grout. Grout shall be stable (bleed less than 2%) per ASTM C940. Temperature of grout shall not exceed 85°F at the end of the grouting hose coupling to fill tube.
 - I. Soil nails shall be triple corrosion protected. Each threaded steel bar shall be ASTM A615-Grade 75 with ASTM A934 Epoxy Coating, and pregrouted in a corrugated PVC or HDPE sheathing. Pregrout, provided by manufacturer, shall contain Cortec MCI-2005 NS or approved equivalent corrosion inhibitor and shall be added at a dose of 24 oz. per cubic yard. Soil nails that are damaged shall either not be used or shall be repaired in accordance with Manufacturer's recommendations.
 - J. Geocomposite drain strips shall be American Wick Drain Sitedrain C-184 or approved equal. Drain strips shall be taped at edges to prevent shotcrete from entering drain during placement.

Materials (Cont.):

- K. At the base of every chimney drain, the Contractor shall install a manufactured Geo Outlet, provided by the same manufacturer as the chimney drain, and solvent weld the Geo Outlet to the PVC drain pipe (weep hole). The Contractor shall not attempt to connect the chimney drain directly to the weep hole.
- L. PVC drain pipe shall be ASTM D 1785 Schedule 40, solid and perforated wall, cell classification 12454-B or 12354-C, wall thickness SDR 35, with solvent welded joints.
- M. If formwork is removed/stripped exposing concrete within 7 days of placement then concrete shall be cured. All exposed surfaces shall be cured using Sinak Lithium Cure 1000 or approved equal at a coverage rate of no less than 400 sq. ft. per gallon.
- N. Cure concrete as specified in the Contract Documents. Remove curing that may affect binding from all areas requiring future bonding unless a curing agent such as Sinak Lithium Cure or accepted equal that does not affect bond and provide equal or better curina is used.
- O. The final CIP facing shall receive an anti-graffiti coating such as Protectosil Antigraffiti, or approved equal, Coating shall be applied in three coats at a coverage rate of 125 sq. ft. per gallon. Anti-graffiti coating shall be incidental to respective concrete work.





Scale: None

Date: Apr. 2024 SHEET No. SO.2 OF 8 SHEETS

STRUCTURAL GENERAL NOTES

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4. Structural Steel:

- A. All structural steel shall be hot-dip galvanized.
- B. Zinc (hot-dip galvanizing) coatings for structural iron and steel products made from rolled, pressed and forged shapes, castings, bars and plates including unfabricated and fabricated products shall conform to ASTM A123/A123M-17.
- C. Structural steel plates and bars shall conform to ASTM A36/A36M-14.
- D. Bearing plates, nuts, and welded shear connectors
 - (1) Bearing plates: AASHTO M183/ASTM A36
 - (2) Nuts: AASHTO M291, Grade B, hexagonal, fitted with beveled washers or spherical seat to provide uniform bearing
 - (3) Shear Connectors: AASHTO LFRD Bridge Construction Specifications 3rd Edition Section 11.3.3.1

5. Reinforcement:

- A. All reinforcing steel shall be ASTM A615, Grade 60 unless otherwise noted.
- B. Reinforcing steel shall be ASTM A706, Type W, Grade 60 where welded connections are required.
- C. Reinforcing bars shall be placed and installed in accordance with the CRSI Manual of Standard Practice and CRSI Placing Reinforcing Bars, unless otherwise noted.
- D. The covering measured from the surface of the concrete to the face of any reinforcing bars shall be as follows, except as otherwise shown:
 - (1) Shotcrete or concrete cast against and permanently exposed to earth = 3" unless otherwise noted
 - (2) All others unless otherwise noted = 2"
- E. Minimum clear spacing between parallel bars shall be 1 1/2 times the diameter of bars (for nonbundled bars). In no case shall the clear distance between the bars be less than 1 1/2 times the maximum size of the coarse aggregate or 1 1/2".
- F. All dimensions relating to reinforcing bars are to centers of bars unless otherwise noted.
- G. Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of intersections is less than one foot in each direction, in which case alternate intersections shall be tied.
- H. For steel reinforcing, stagger all splices where possible.

Reinforcement (Cont.):

- I. Minimum lap splice length for steel reinforcing shall be 40 bar diameters or 2'-0", whichever is greater, for #8 bars or smaller. Minimum lap splice length for #9 bars or larger shall be 50 bar diameters. Increase lap length by multiplying the minimum lap splice length by 1.3 for bars having more than 12" of fresh concrete below bars. Increase lap length by 1.25 for ASTM A1035 reinforcing. Lap splices for bundled bars shall consist of individual bar splices within a bundle that do not overlap. Entire bundles shall not be lap spliced.
- J. Unless otherwise noted, reinforcing splices shall be staggered. Minimum distance between staggered lap splice shall be equal to the length required for the lap splice. Number of bars spliced at sections normal to axis of member shall not exceed 50 percent of the total main reinforcing in the member.
- K. All welding shall conform to AWS D1.5 Bridge Welding Code. All welds shall be ground smooth. Unless noted otherwise, all welding shall be shielded arc welding done with E70 electrodes.

6. Glass Fiber Reinforced Polymer Bar (ASTM D7957):

- A. Glass Fiber Reinforced Polymer (GFRP) rebar shall conform to ASTM D7957.
- B. Concrete cover for the GFRP bars shall be 1 1/2" unless otherwise noted.
- C. Minimum lap splice lengths for the GFRP bars shall be 42 bar diameter unless otherwise noted.
- D. All GFRP bars shall be securely tired in place. Tie wire shall be allow 302 or 304 stainless steel or non-metallic.
- E. The GFRP bars may be cut in the field with a masonry or diamond blade.
- F. All work including materials and bends shall follow Manufacturer's recommendations.

7. Construction Notes:

- A. The Contractor shall verify all dimensions and site conditions and shall report any discrepancies in writing to the Engineer before commencing work or ordering materials.
- B. The Contractor shall verify all site conditions and not rely upon these plans for existing dimensions, elevations and azimuths, stream channel location, roads, roadway gutters, curbs and sidewalks, etc. Conditions may differ from those shown.

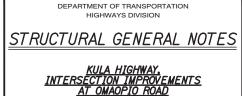
7. Construction Notes (Cont.):

- C. The Contractor shall be solely responsible for the protection of adjacent properties, utilities and existing and new structures from damage due to construction. Repairing any damage shall be at the Contractor's own expense, to the satisfaction of the Engineer.
- D. The Contractor shall verify the location of all utility lines and notify the respective owners before commencing with excavation, and any temporary piling or sheeting.
- E. Except as otherwise noted, all vertical dimensions are measured plumb.
- F. For concrete finish see Standard Specifications and Special Provisions. Construction joints may be relocated or additional ones added subject to the approval of the Engineer.
- G. Unless otherwise noted, all exposed concrete edges shall be chamfered 3/4" x 3/4".
- H. The Contractor shall unplug, clean and maintain existing drains during construction of the project.
- I. All footings and slabs shall bear on firm undisturbed natural soils or properly compacted structural fill.
- J. The Contractor shall conduct his work in such a manner and provide such temporary shoring or other measures as may be necessary to insure the safety of all concerned and to protect existing structures.
- K. In the event of over excavation, the space between the footing or footing key and the ground shall be filled with a minimum of Class D concrete at the Contractor's expense at no cost to the city.

8. <u>General:</u>

- A. GFRP bars shall be paid for under reinforcing steel.
- B. The cost of the waterproofing shall be considered as included in the cost of the concrete and shall not be paid for directly.
- C. Cost of CMP band and concrete collar shall be incidental to the cost of new 54" CMP.





STATE OF HAWAI'I

Scale: None

Date: Apr. 2024

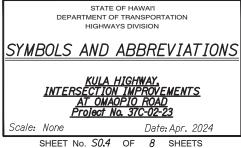
SHEET No. SO.3 OF 8 SHEETS

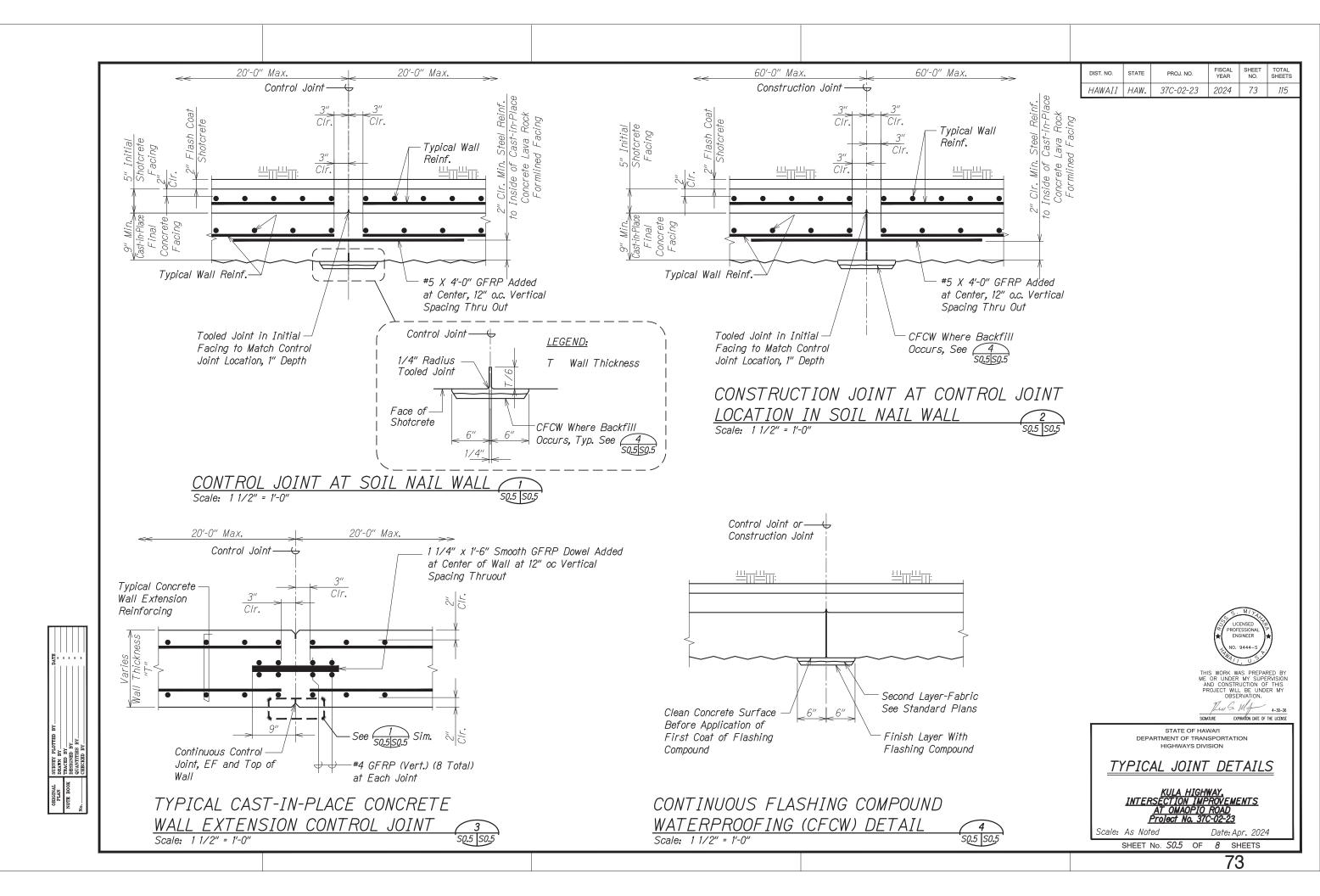
SYMBOLS AND ABBREVIATIONS

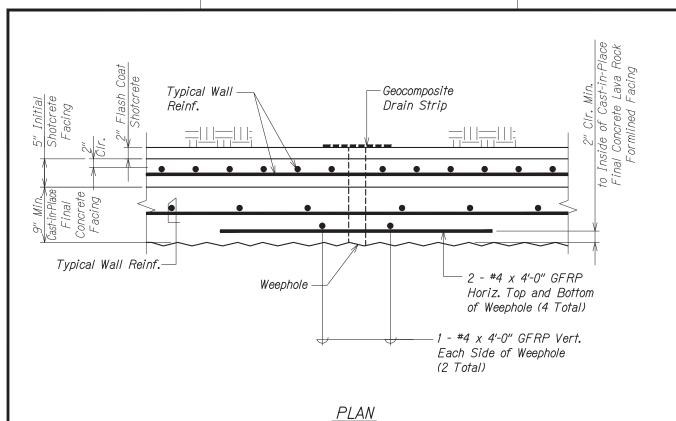
DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
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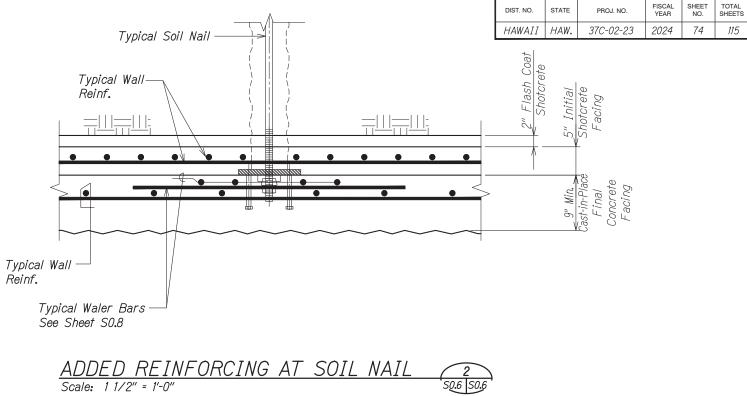
,		_					0 "
\$	And	E	East	L	Length	S	South
@	At Daniel	Ea.	Each Face	Lb., Lbs., Lbs.	Pound, Pounds	Sch.	Schedule
<u>#</u> ©	Baseline	EF.	Each Face	LF	Linear Feet/Foot	SDMH	Sewer Drain Manhole
<u>(L</u>	Centerline	EFH	Each Face Horizontal	Lin. Ft.	Linear Feet/Foot	SE	Super Elevation
Ø #	Diameter	EFV	Each Face Vertical	Longit.	Longitudinal	Sect.	Section
#	Number, Pound	EJ	Expansion Joint	LS	Lump Sum	SF	Square Feet
4.5		El., Elev.	Elevation	Ltg. Std.	Lighting Standard	Sht.	Sheet
AB	Anchor Bolt	Elec.	Electrical			SI.	Slope
Abbr.	Abbreviation	Emb.	Embankment_	M	Modified	Spc	Spacing
Abut.	Abutment	Embed.	Embedded, Embedment	Max.	Maximum	Sprd.	Spread
AC	Asphaltic Concrete	EMH	Electrical_Manhole	Mech.	Mechanical	Spec.	Specification
Add.	Additional, Added	EP	Edge of Pavement	MH	Manhole	ŚRA	Shrinkage Reducing Admixture
Alt.	Alternate	<u>E</u> PS	Expanded Polystyrene	Min.	Minimum	SS, SStl	Stainless Steel
Approx.	Approximate	<u>E</u> q.	Equal	Misc.	Miscellaneous	Sta.	Station
Az.	Azimuth	Est.	Estimated	MPH	Miles per Hour	Stagg.	Staggered
		EVC	End of Vertical Curve			Std.	Standard
B, Bot., Bott.	Bottom	EW	Each Way	N	North _	Stiff.	Stiffener
Bal.	Balance	Exc.	Excavation	NF	Near Face	Stirr.	Stirrup
Bet.	Between	Excl.	Excluding	NIC	Not In Contract	Str.	Straight
BF	Both Faces, Back Face	Exist., Ex.	Existing	No.	Number	Struct.	Structure
BFE	Bottom of Footing Elevation	Exp., (E)	Expansion	NTS	Not To Scale	SY	Square Yard
Bk.	Back	Ext.	Exterior			Symm.	Symmetrical
BIt.	Bolt and the second sec			OB, Outbnd.	Outbound	•	•
Bm.	Beam	(F)	Fixed	OC	On Center	Τ	Тор
BMP	Best Management Practices	FA	Force Account	OD	Outside Diameter	T ¢ B	Top And Bottom
Br.	Bridge	FB	Flat Bar	Opn'g	Opening	Tan.	Tangent
Brg., Brgs.	Bearing, Bearings	F'c	Specified Strength	0/5	Offset	Temp.	Temporary
BVC	Beginning of Vertical Curve	, •	of Concrete	5 , 5		TFÉ	Top of Footing Elevation
BW	Both Ways	F'ci	Strength of Concrete at	PB	Pull Box	Thk.	Thick
5.,	Bom wayo	, 0,	Time of Initial Prestress	PC	Point of Curvature	TOD	Top of Deck
Cant.	Cantilever	FF	Far Face, Front Face	PCC	Portland Cement Concrete	Tot.	Total
CB	Catch Basin	Fig.	Figure	PCF	Pounds per Cubic Foot	TOW	Top of Wall
CC	Center to Center	Fin. Gr.	Finish Grade	P(e)	Effective Or Working	Transv.	Transverse
CF	Cubic Feet	Ft.	Feet, Foot	7 (0)	Prestressing Force	TS	Structural Tubing
CFCW	Continuous Flashing	r r. Ftg.	Footing	Perf.	Perforated	Тур .	Typical
C/ CW	Compound Waterproofing	1 1y.	i ooiiiig	PI	Point of Intersection	ryp.	rypicar
00		00	Cogo Cougo	ΓI	of Tangents	Undorard	Undorground
CG	Center of Gravity	Ga.	Gage, Gauge	DIVO		Undergrd.	Underground
CGS	Center to Gravity of Strands	Galv.	Galvanized	PIVC	Point of Intersection of	UNO	Unless Noted Otherwise
CIP	Cast-In-Place	GFRP	Glass Fiber Reinforced Polymer	DI	Vertical Curve	1/	Varian
CJ	Control Joint	Gr.	Grade	PL DI C	Plate	Var.	Varies
CI.	Class	Grd.	Ground	PLF	Pounds per Linear Foot	VC	Vertical Curve
CLSM	Controlled Low Strength	GRP	Grouted Rubble Pavement	PP	Precast Plank	Vert., V	Vertical
0.1	Material			PRC	Point of Reverse Curvature	147	
CIr.	Clearance	Н	Height	Prestr.	Prestressed	W	West
Col.	Column	(H)	Hinge	P/S	Prestressed Strands_	W/	With
Conc.	Concrete	HECO	Hawaiian Electric Company	PSF	Pounds per Square Foot	W/C	Water/Cement Ratio
Conn.	Connection	Horiz.	Horizontal	PSI	Pounds per Square Inch	W <i>P</i>	Work Point, Working Point
Const.	Construction	HS	High Strength	PT	Point of Tangency,	WS	Water Surface
Const. Jt.	Construction Joint	HSS	Hollow Structural Section		Post Tensioned	WW	Wing Wall
Cont.	Continuous			Pt., Pts.	Point, Points	WWR	Welded Wire Reinforcing
CSL	Crosshole Sonic Logging	IB, Inbnd.	Inbound	PVC	Polyvinyl Chloride		
CY, Cu. Yd.	Cubic Yard	ID	Inside Diameter			Yr.	Year
		IF	Inside Face	Q	Flow Rate		
DЫ.	Double	In.	Inch				
Demo	Demolish, Demolition	Int.	Interior	R, Rad.	<i>Radius</i>		
Det.	Detail	Inv.	Invert	Ŕdwy.	Roadway		
DI	Drain Inlet, Ductile Iron	=::::		Rebar	Reinforcing Bar		G. M/V
Dia.	Diameter	Jt.	Joint	Ref.	Reference		LICENSED
Dim.	Dimension	···		Reinf.	Reinforced, Reinforcing,		PROFESSIONAL Y
Dist.	Distance	K	Kips		Reinforcement		* SYMBOLS
Dron.	Down	KF	Kip Foot	Reg'd.	Required		NO. 9444-S
Do.	Ditto	KSF	Kips per Square Foot	Ret.	Retaining		WATT. U.S.
DS DS	Drilled Shaft	KSI	Kips per Square Inch	RF	Rear Face		THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION
Dwg., Dwgs.	Drawing, Drawings	KLF	Kips per Square Thon Kips per Linear Foot	ROW	Right of Way		THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION AND CONSTRUCTION OF THIS PROJECT WILL BE UNDER MY
Dwys, Dwys. Dwls.	Dowels	1141	Mpo por Errour Foor	11011	rugin or way		PROJECT WILL BE UNDER MY OBSERVATION.
DW10.	DOWGIO						Thu S. Mf 4-30-26 Scale: None
							15-10 - 101 J 4-50-26











1 - #4 x 4'-0" GFRP Vert. -Each Side of Weephole (2 Total) Bend as Required to Weephole Not Extend into Next Lift

2 - #4 x 4'-0" GFRP — Horiz. Top and Bottom _2" CIr. of Weephole (4 Total) Тур.

ELEVATION

NOTE:

Shift bars as required to fit added bars.

ADDED REINFORCING AT WEEPHOLES (
Scale: 1 1/2" = 1'-0"



Trus Mf 4-30-26

EXPIRATION DATE OF THE LICENSE

STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

ADDED REINFORCING DETAILS

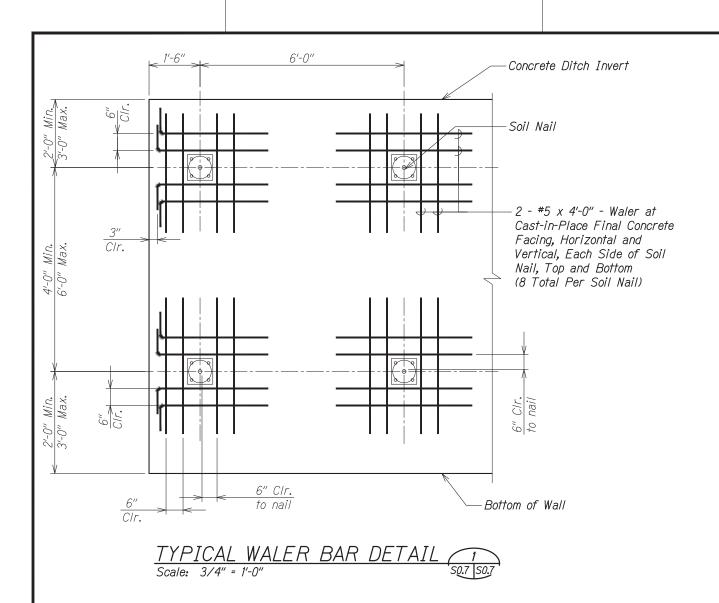
KULA HIGHWAY.

INTERSECTION IMPROVEMENTS
AT OMAOPIO ROAD
Project No. 37C-02-23

Scale: As Noted

Date: Apr. 2024

SHEET No. SO.6 OF 8 SHEETS



 DIST. NO.
 STATE
 PROJ. NO.
 FISCAL YEAR
 SHEET NO.
 TOTAL SHEETS

 HAWAII
 HAW.
 37C-02-23
 2024
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 115

STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

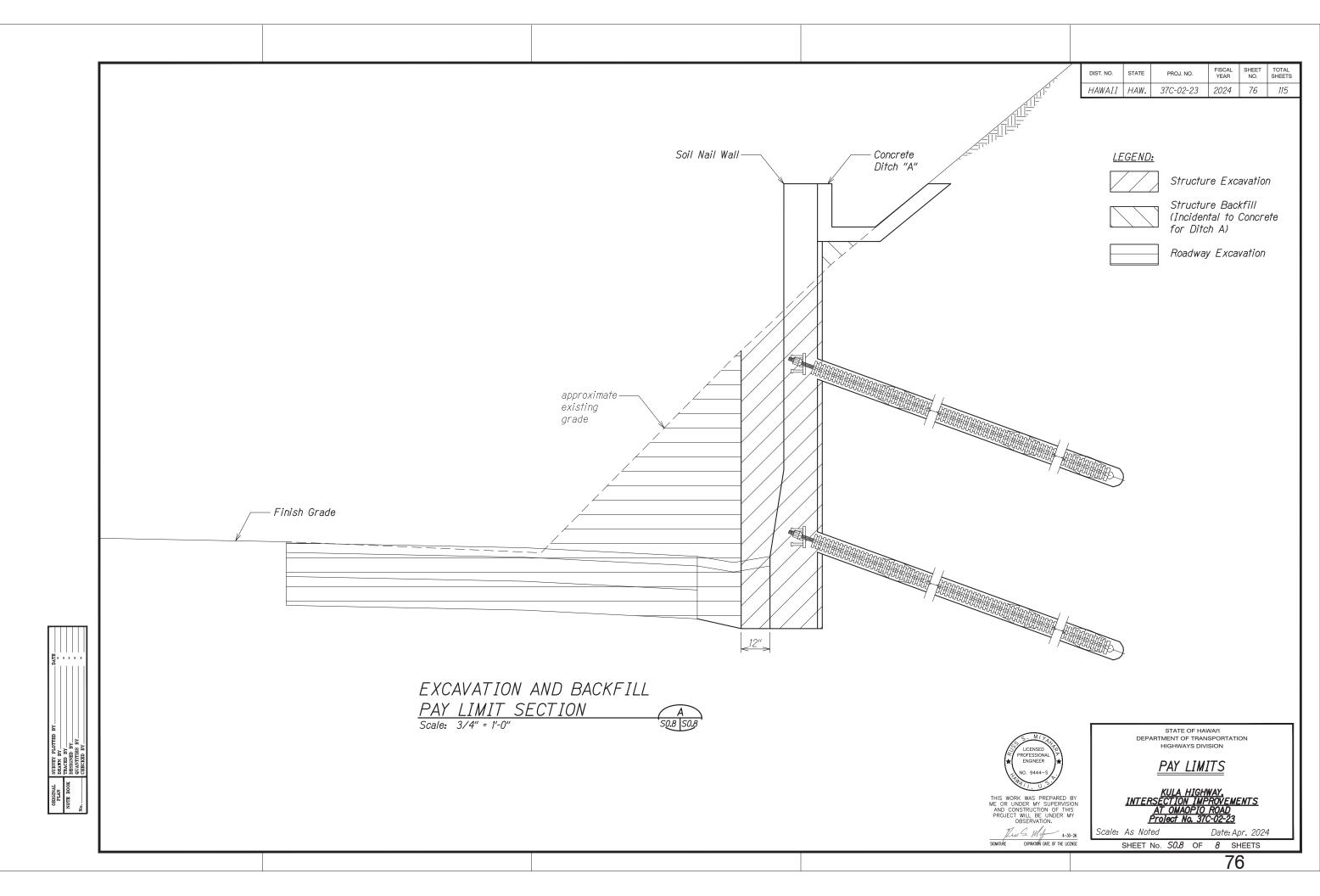
TYPICAL WALER BAR DETAIL

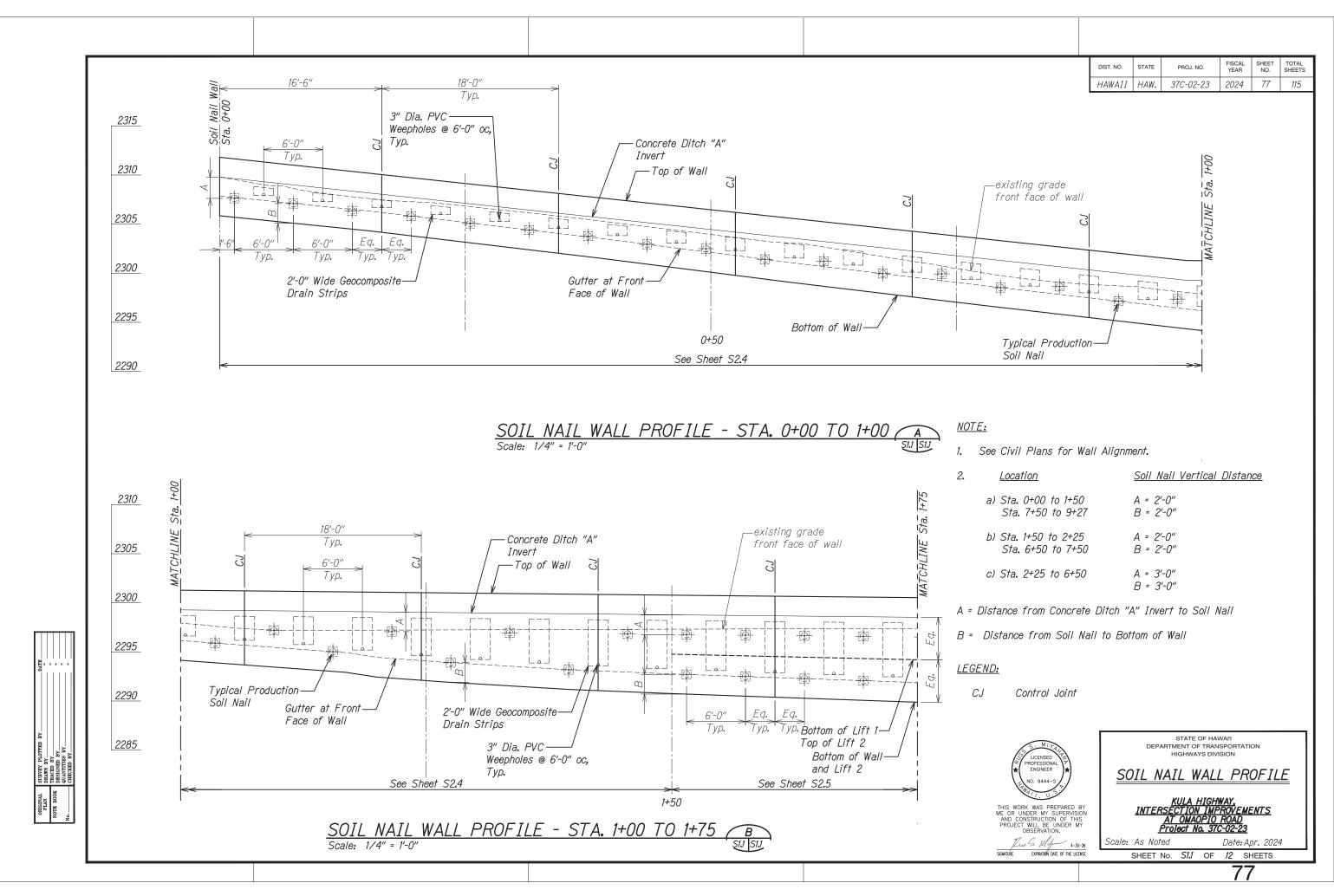
KULA HIGHWAY, INTERSECTION IMPROVEMENTS AT OMAOPIO ROAD Project No. 37C-02-23

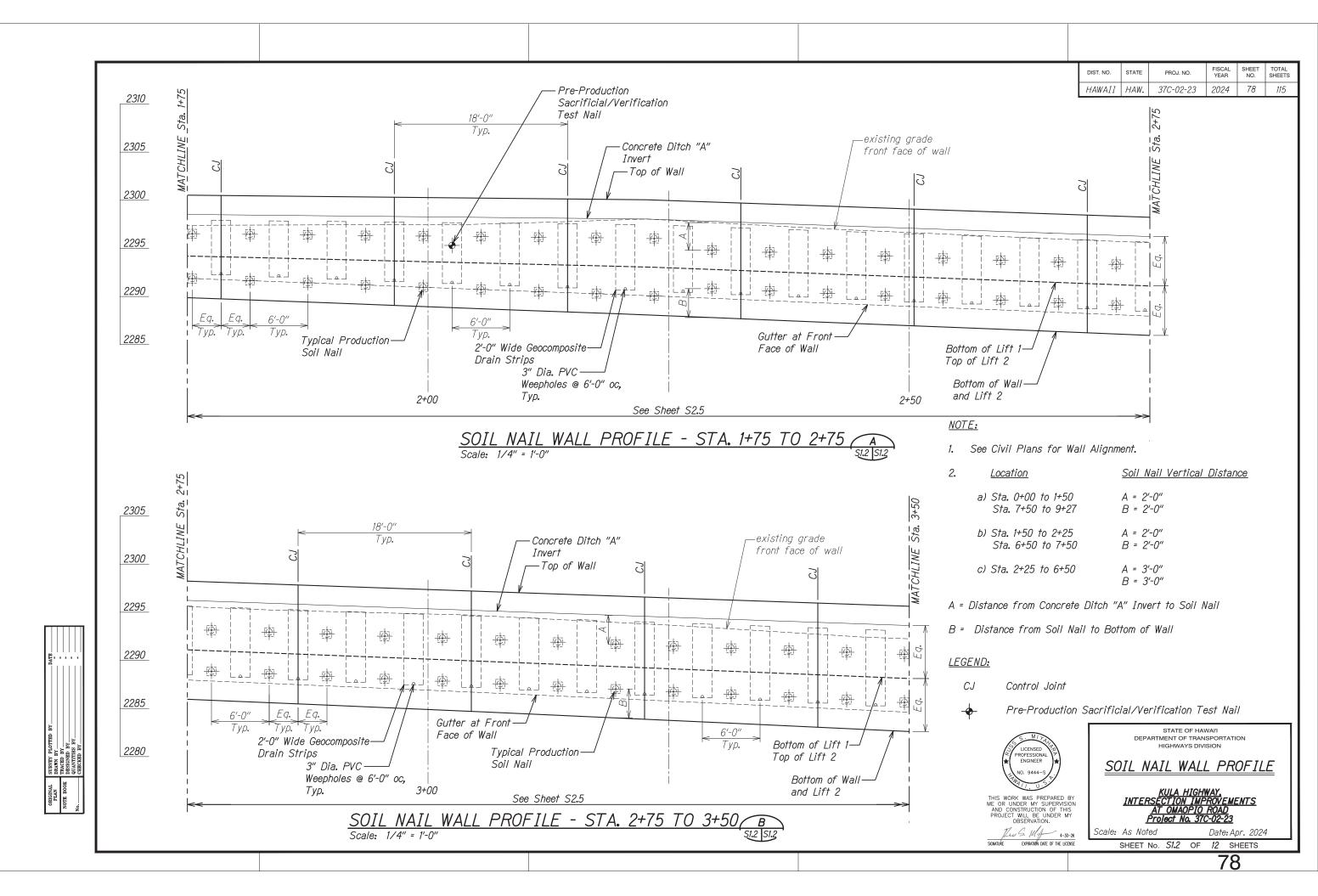
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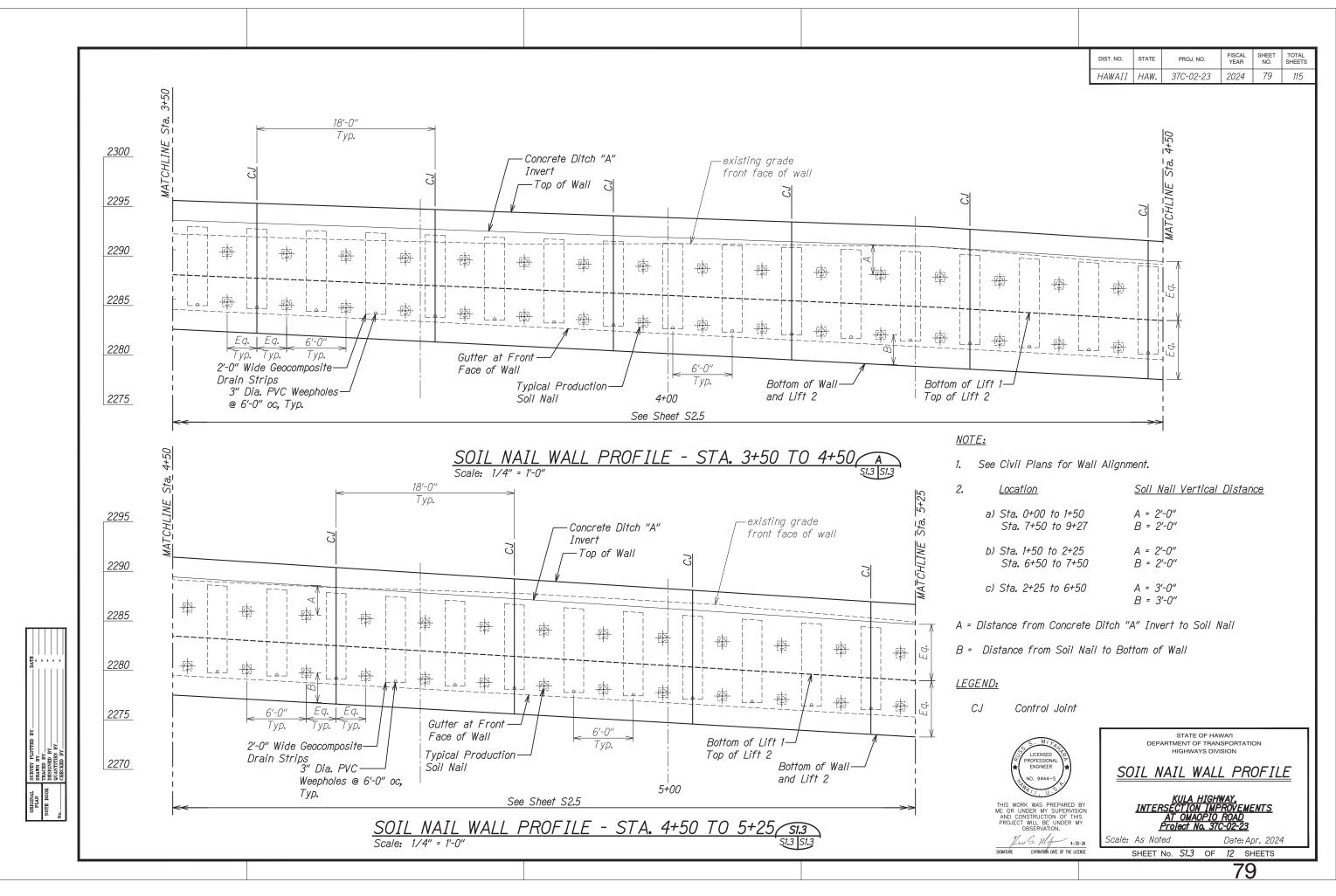
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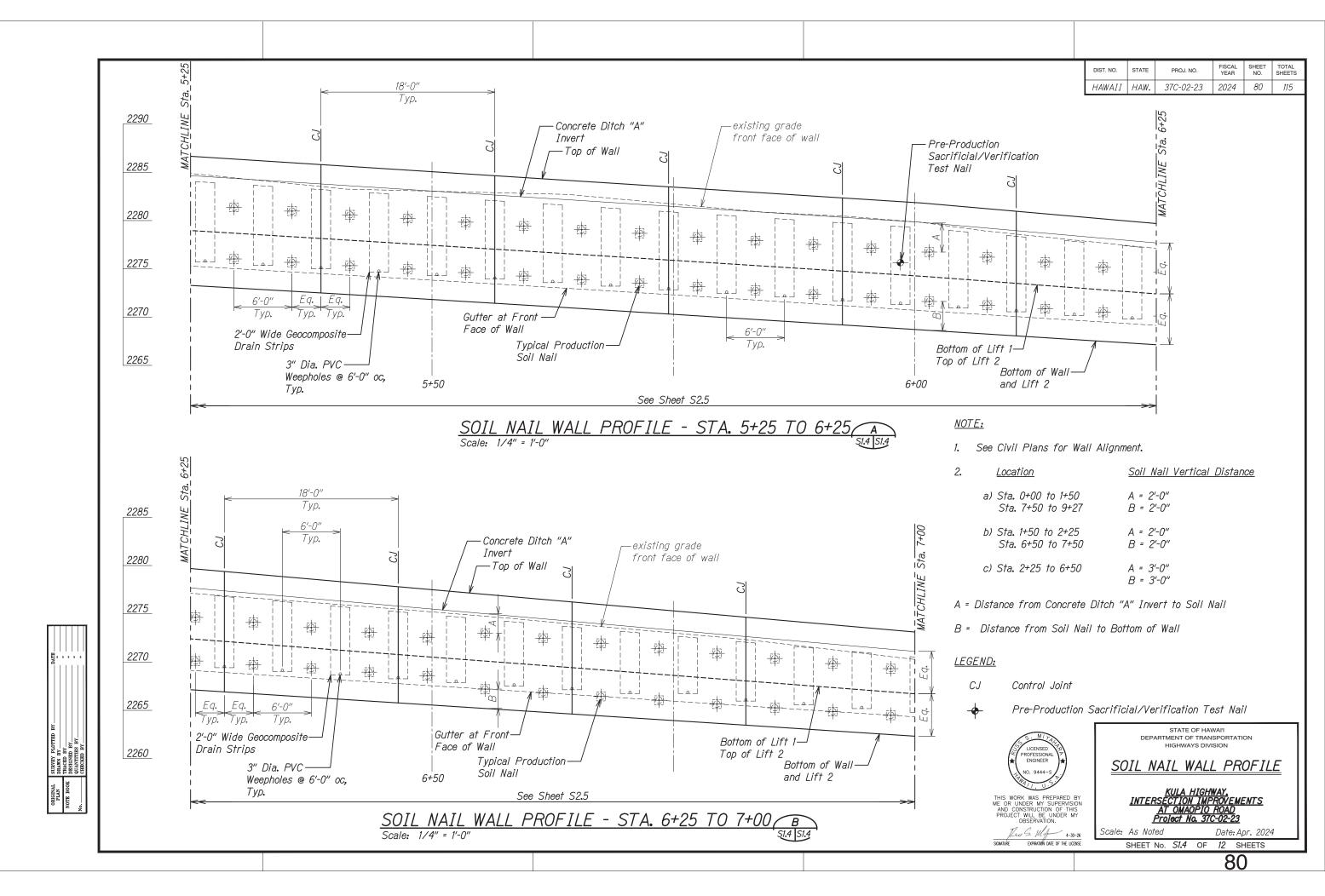
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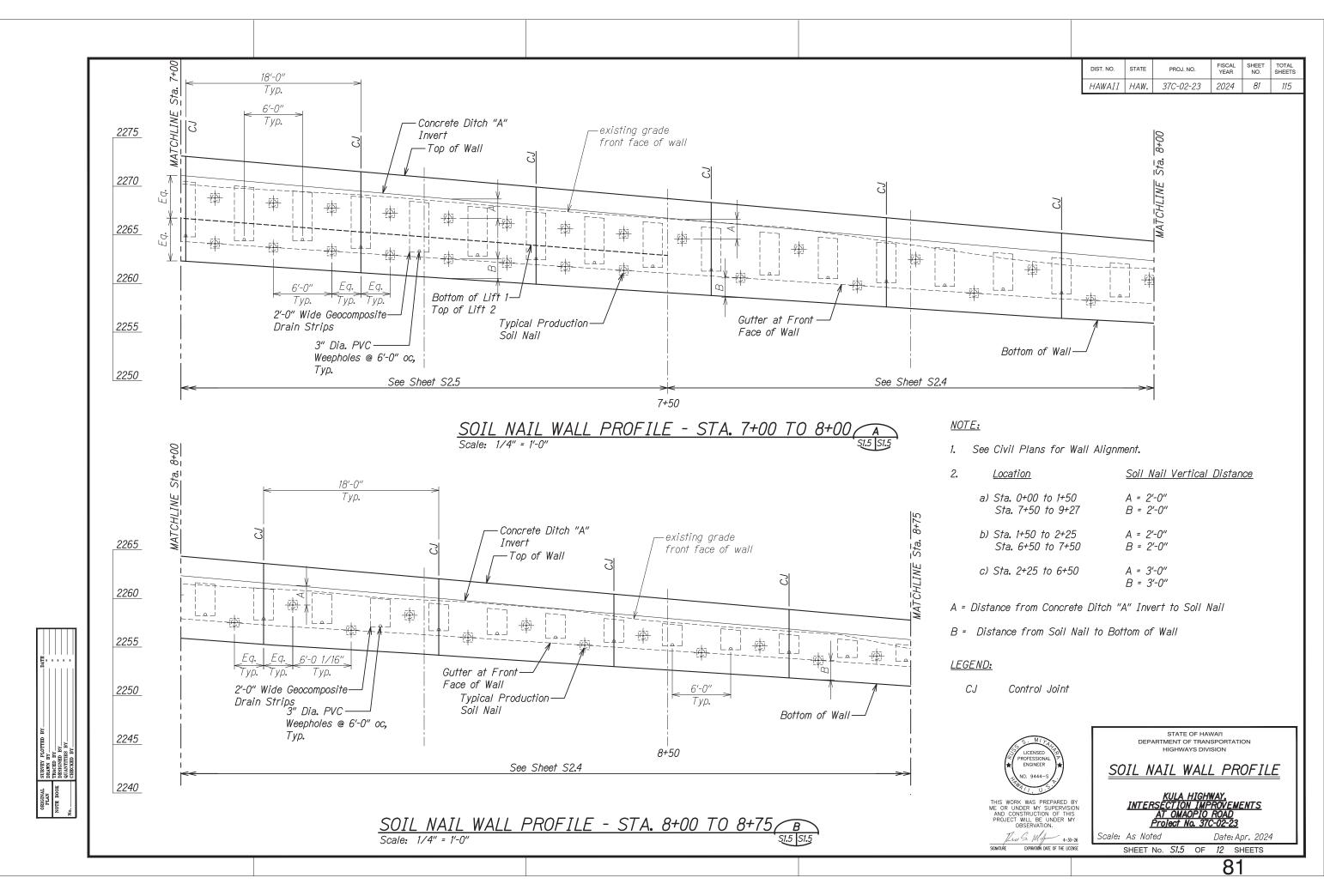




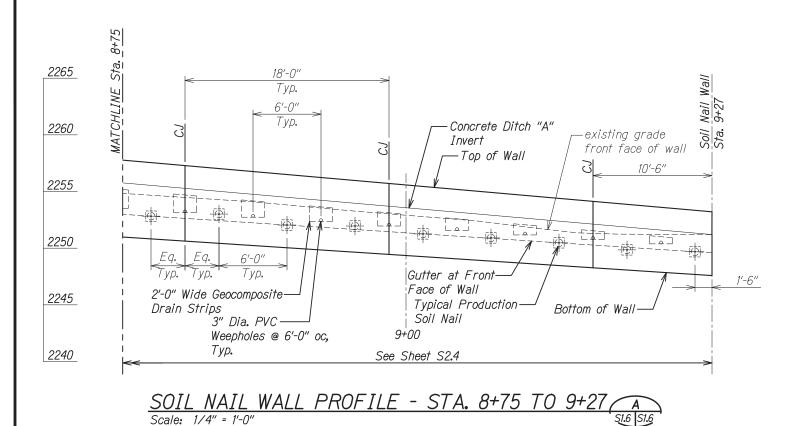








DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	37C-02-23	2024	82	115



<u>NOTE:</u>

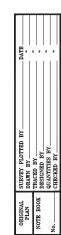
1. See Civil Plans for Wall Alignment.

A = Distance from Concrete Ditch "A" Invert to Soil Nail

B = Distance from Soil Nail to Bottom of Wall

LEGEND:

CJ Control Joint







SOIL NAIL WALL PROFILE

KULA HIGHWAY.

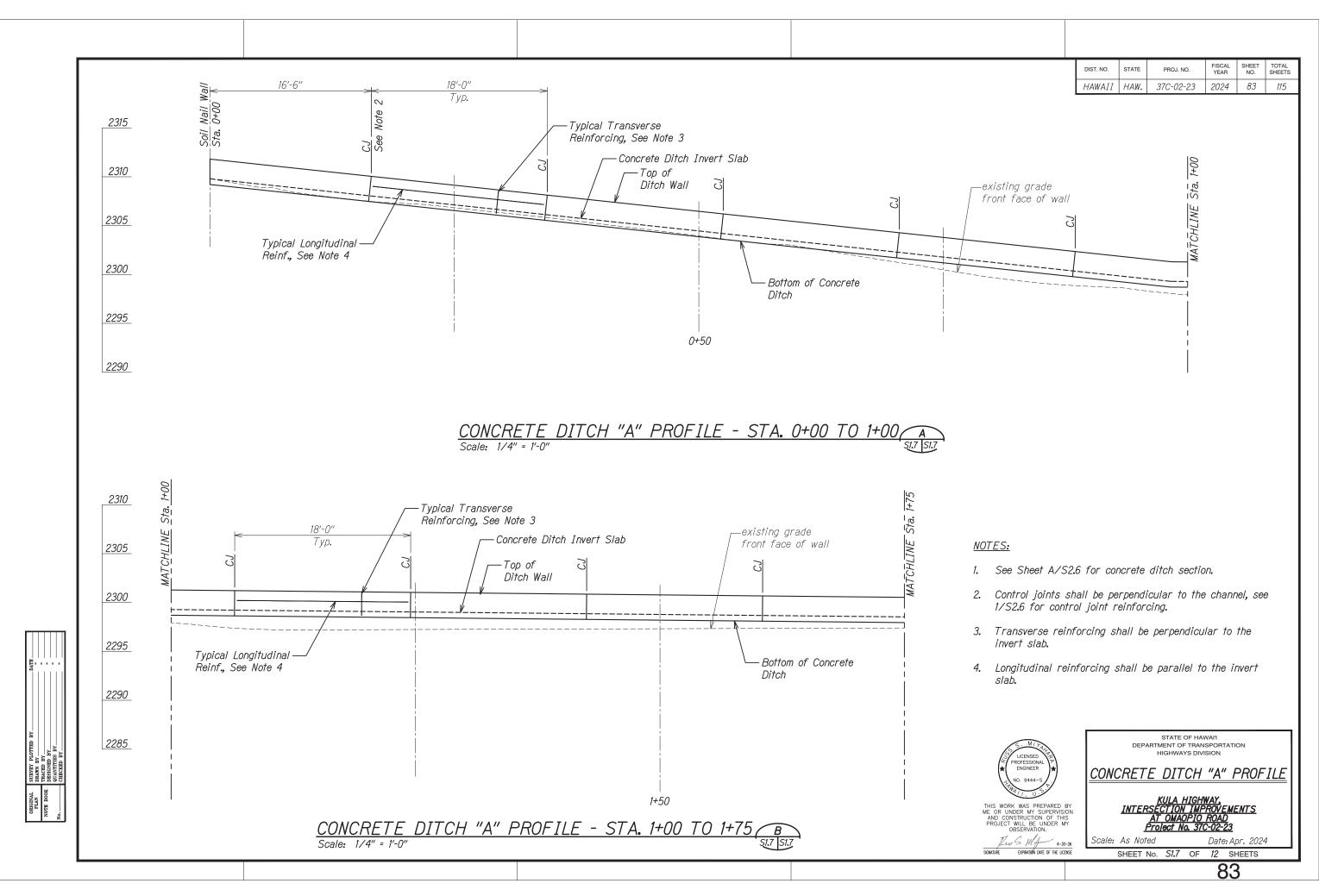
INTERSECTION IMPROVEMENTS

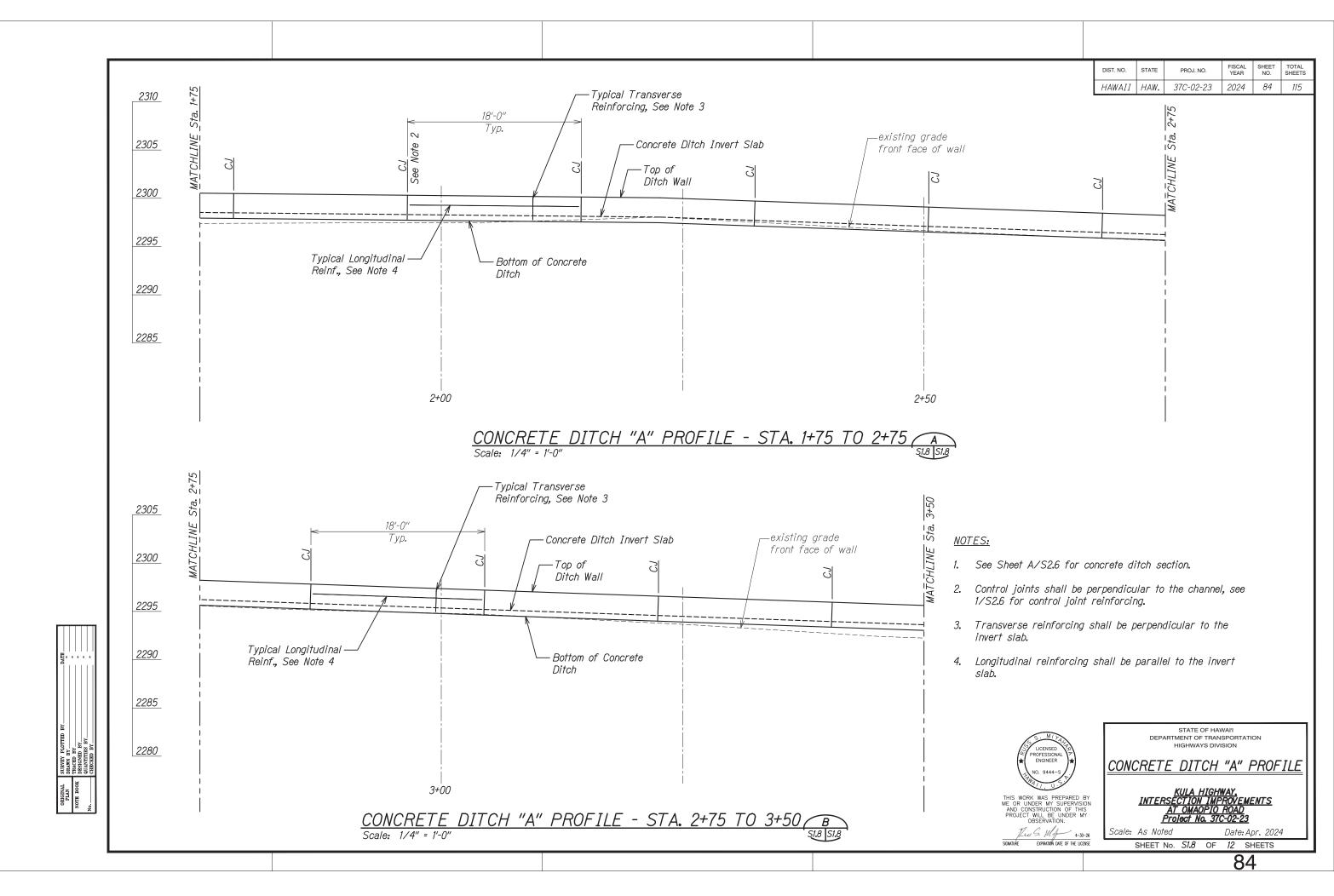
AT OMAOPIO ROAD

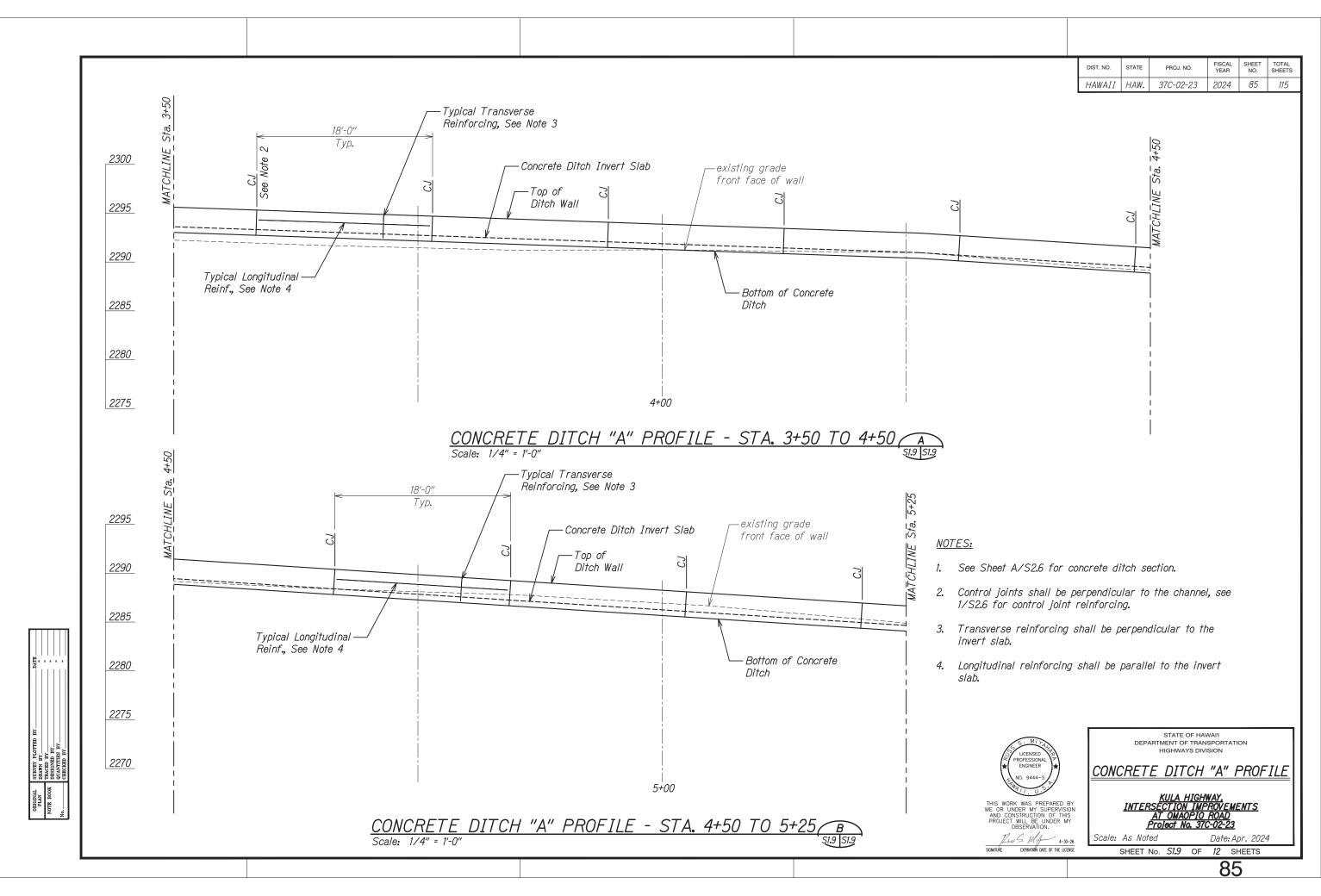
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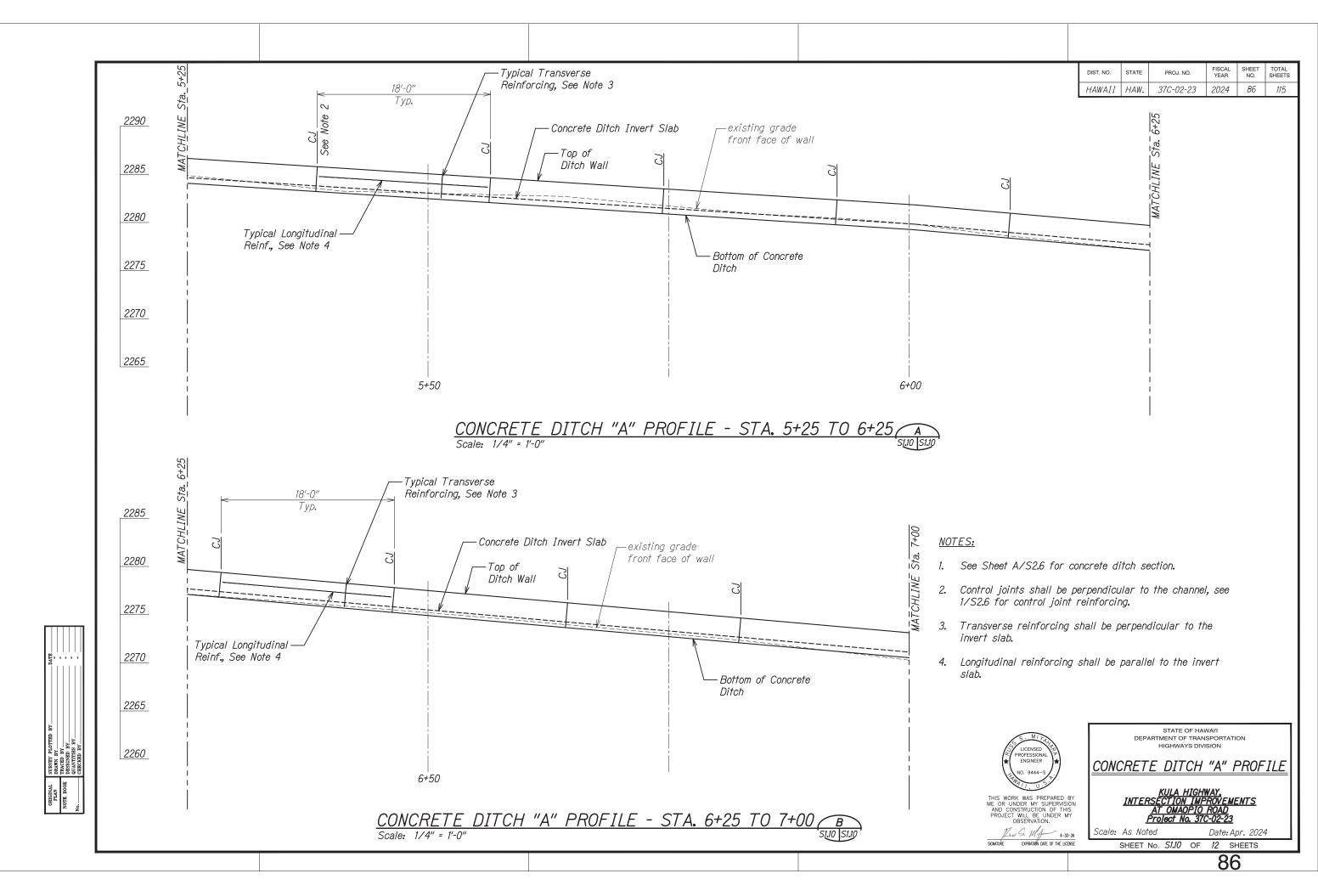
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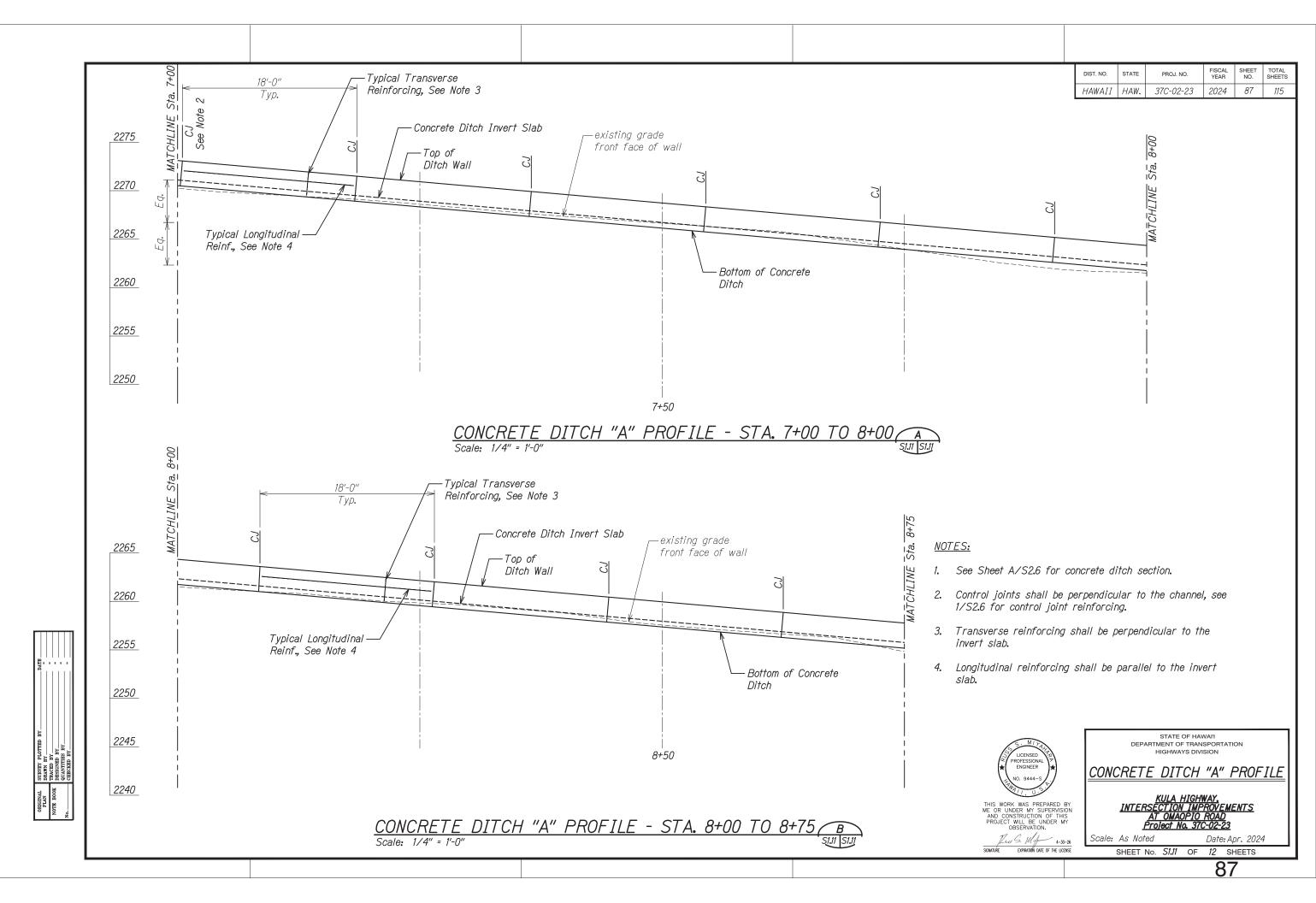
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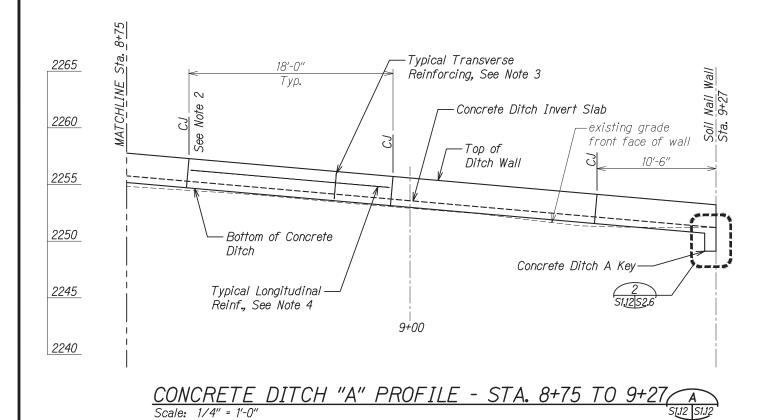








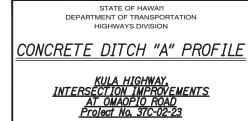
DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	37C-02-23	2024	88	115



NOTES:

- 1. See Sheet A/S2.6 for concrete ditch section.
- 2. Control joints shall be perpendicular to the channel, see 1/S2.6 for control joint reinforcing.
- 3. Transverse reinforcing shall be perpendicular to the invert slab.
- 4. Longitudinal reinforcing shall be parallel to the invert slab.

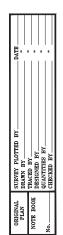


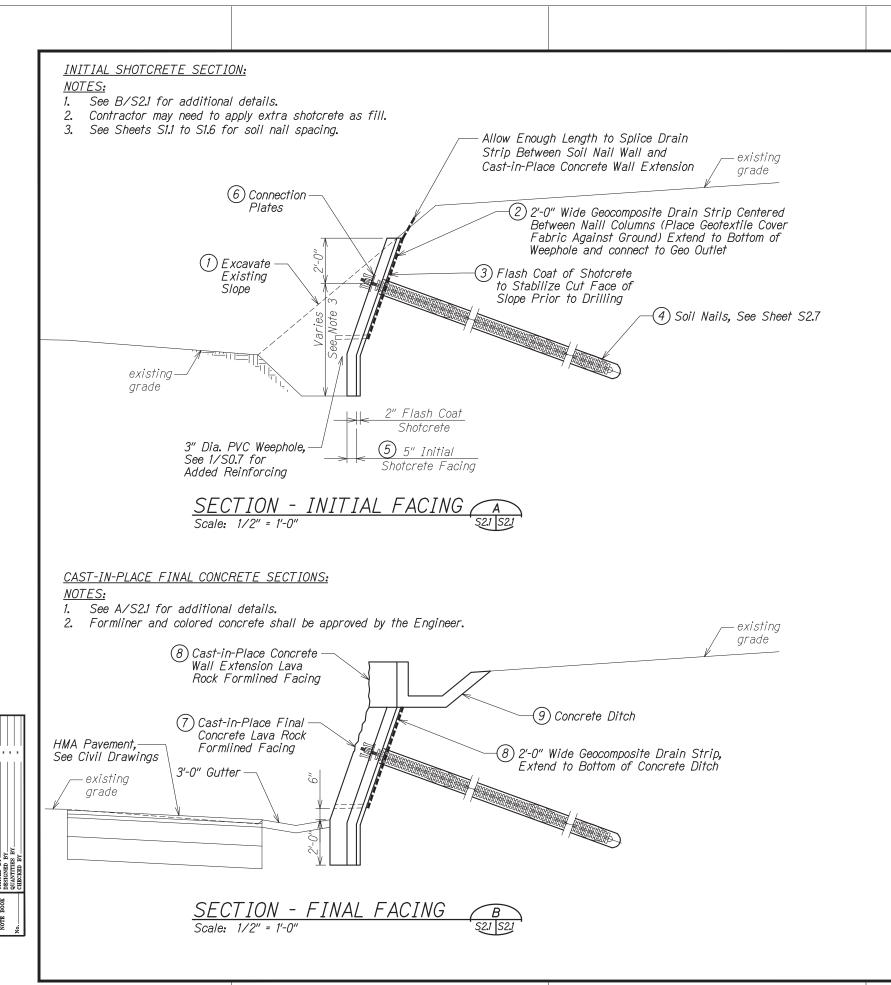


Scale: As Noted

Date: Apr. 2024

SHEET No. S1.12 OF 12 SHEETS



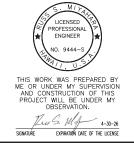


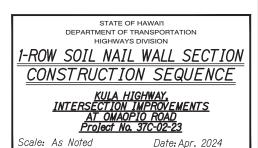
 DIST. NO.
 STATE
 PROJ. NO.
 FISCAL YEAR
 SHEET NO.
 TOTAL SHEETS

 HAWAII
 HAW.
 37C-02-23
 2024
 89
 115

CONSTRUCTION SEQUENCE:

- 1) Excavate to Bottom of Wall.
- 2) Install Geocomposite Drain Strips to Bottom of Weepholes. Cover Weepholes with Tape Before Applying Shotcrete Wall Facing.
- (3) Apply Flash Coat Shotcrete.
- 4) Drill, Install, and Grout Soil Nails; Run Any Necessary Performance/Proof Tests.
- 5) Place Reinforcing and Apply Initial Shotcrete Facing.
 Reinforcing Shall Extend into Cast-in-Place Concrete Wall Extension.
- (6) Install Studded Connection Plates.
- (7) Place Reinforcing and Pour Cast-in-Place Final Concrete Facing.
- (8) Extend Geocomposite Drain Strips to Bottom of Concrete Ditch, Place Reinforcing and Cast-in-Place Concrete Wall Extension.
- 9) Place Reinforcing and Pour Concrete Ditch.

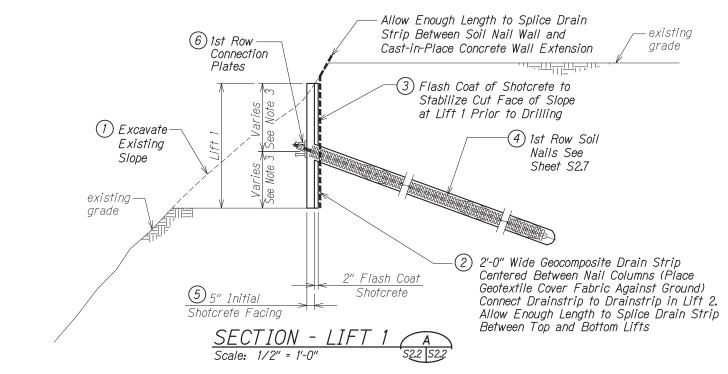




SHEET No. S2.1 OF 7 SHEETS

INITIAL SHOTCRETE SECTION (LIFT 1): NOTES:

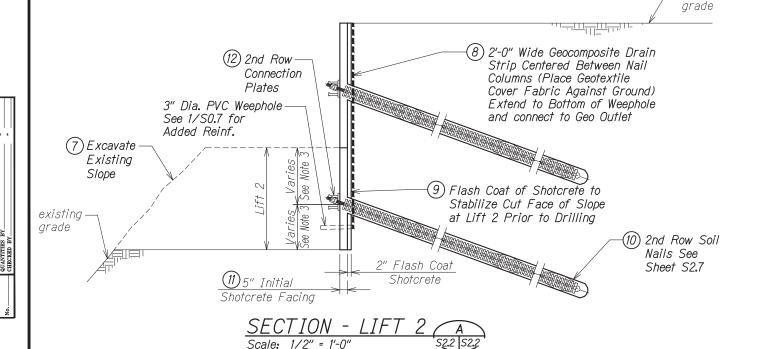
- 1. See B/S2.2 and A/S2.3 for additional details.
- 2. Contractor may need to apply extra shotcrete as fill in case the face of the cut slope is not vertical.
- 3. See Sheets S1.1 to S1.6 for soil nail spacing.



INITIAL SHOTCRETE SECTION (LIFT 2):

NOTES:

- 1. See A/S2.2 and A/S2.3 for additional details.
- 2. Contractor may need to apply extra shotcrete as fill in case the face of the cut slope is not vertical.
- 3. See Sheets S1.1 to S1.6 for soil nail spacing.



CONSTRUCTION SEQUENCE:

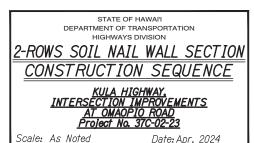
DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	37C-02-23	2024	90	115

- (1) Excavate to Required 1st Lift.
- (2) Install Geocomposite Drain Strips.
- (3) Apply Flash Coat Shotcrete.
- 4 Drill, Install, and Grout 1st Row of Soil Nails; Run Any Necessary Performance/Proof Tests.
- (5) Place Reinforcing and Apply Lift 1 Initial Shotcrete Facing.
 Reinforcing Shall Extend into Cast-in-Place Concrete Wall Extension.
- (6) Install Studded Connection Plates on 1st Row.
- (7) Excavate to Required 2nd Lift.
- (8) Extend Geocomposite Drain Strips to Location Shown on Sheets S1.1 to S1.6 and Install PVC Weepholes. Cover Weepholes with Tape Before Applying Shotcrete Wall Facing.
- (9) Apply Flash Coat Shotcrete.

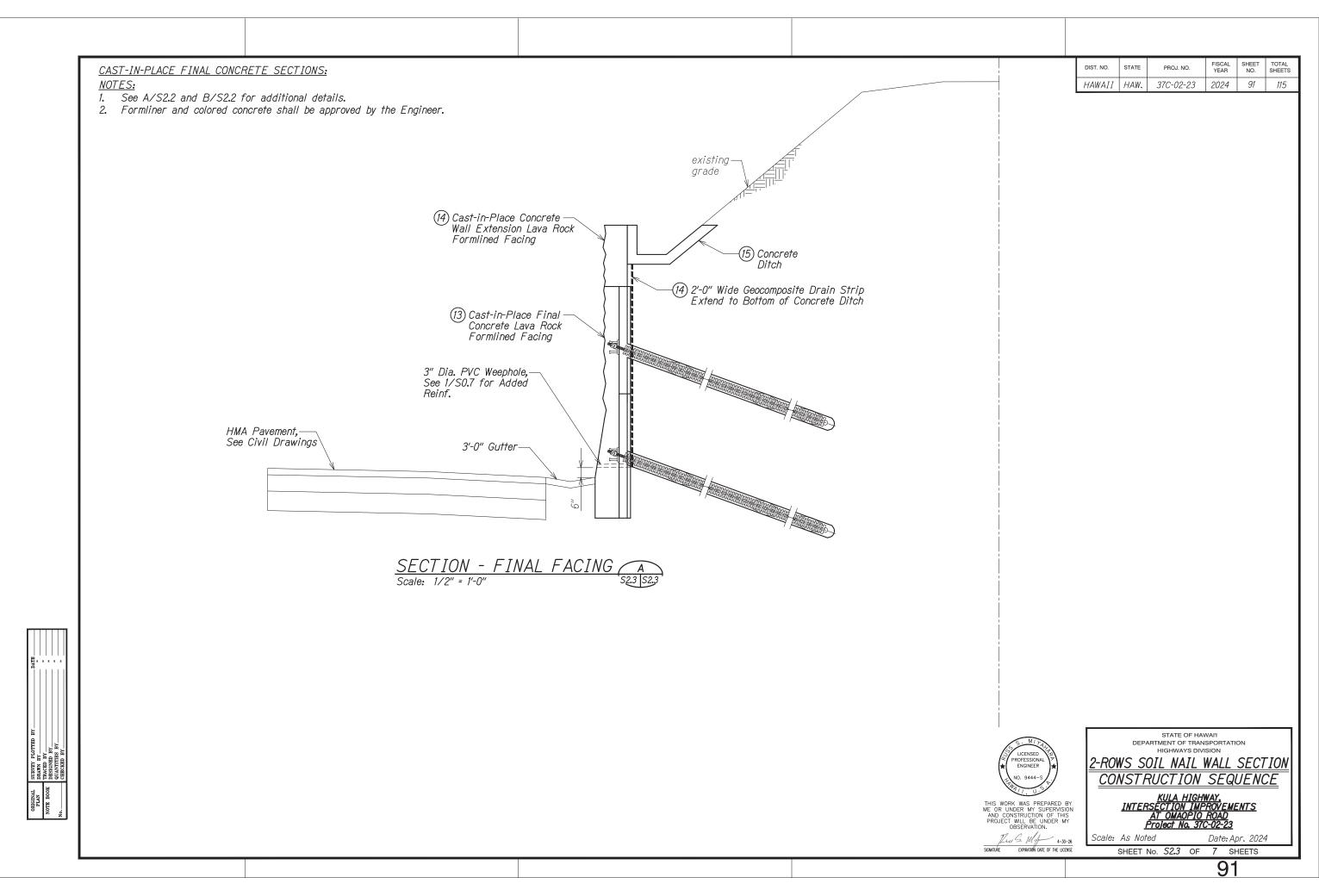
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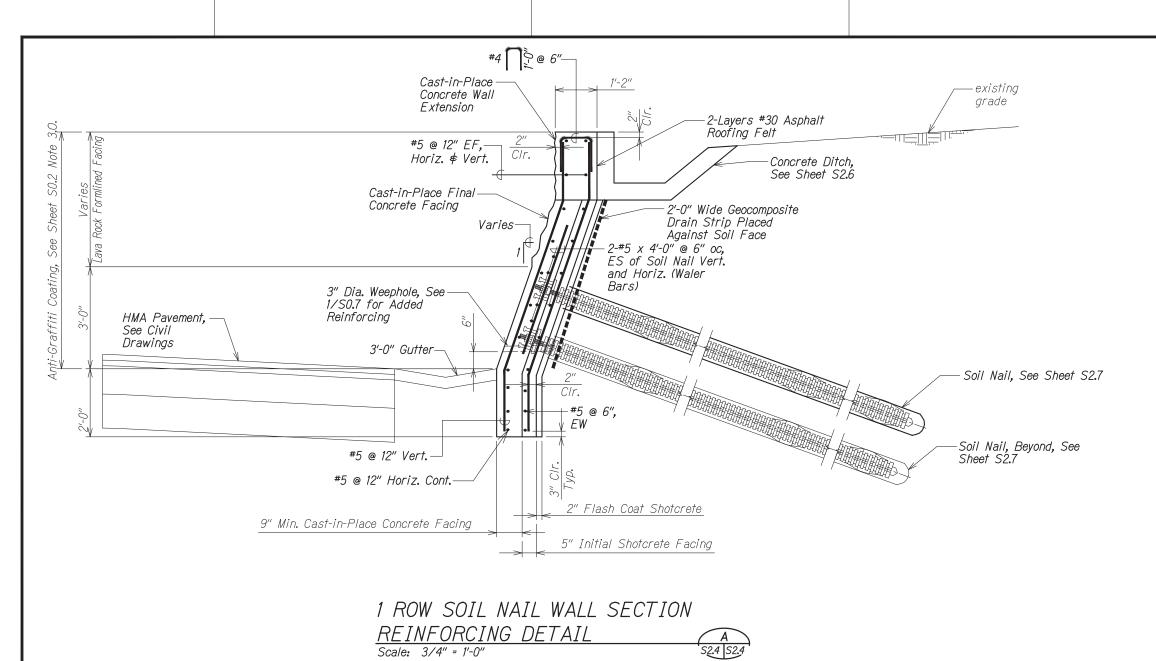
- (10) Drill, Install, and Grout 2nd Row of Soil Nails; Run Any Necessary Performance/Proof Tests.
- (11) Place Reinforcing and Apply Lift 2 Initial Shotcrete Facing.
- (12) Install Studded Connection Plates on 2nd Row.
- (13) Place Reinforcing and Pour Cast-in-Place Concrete Facing.
- (14) Extend Geocomposite Drain Strips to Bottom of Concrete Ditch,
 Place Reinforcing and Cast-in-Place Final Concrete Wall Extension.
- (15) Place Reinforcing and Pour Concrete Ditch.





SHEET No. S2.2 OF 7 SHEETS





DIST. NO. STATE PROJ. NO. FISCAL NO. SHEET NO. SHEETS

HAWAII HAW. 37C-02-23 2024 92 115

THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION OF THIS PROJECT WILL BE UNDER MY OBSERVATION.

Thu S. M.J. 4-30-26
SIGNATURE EXPIRATION DATE OF THE LICENSE

STATE OF HAWAPI
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

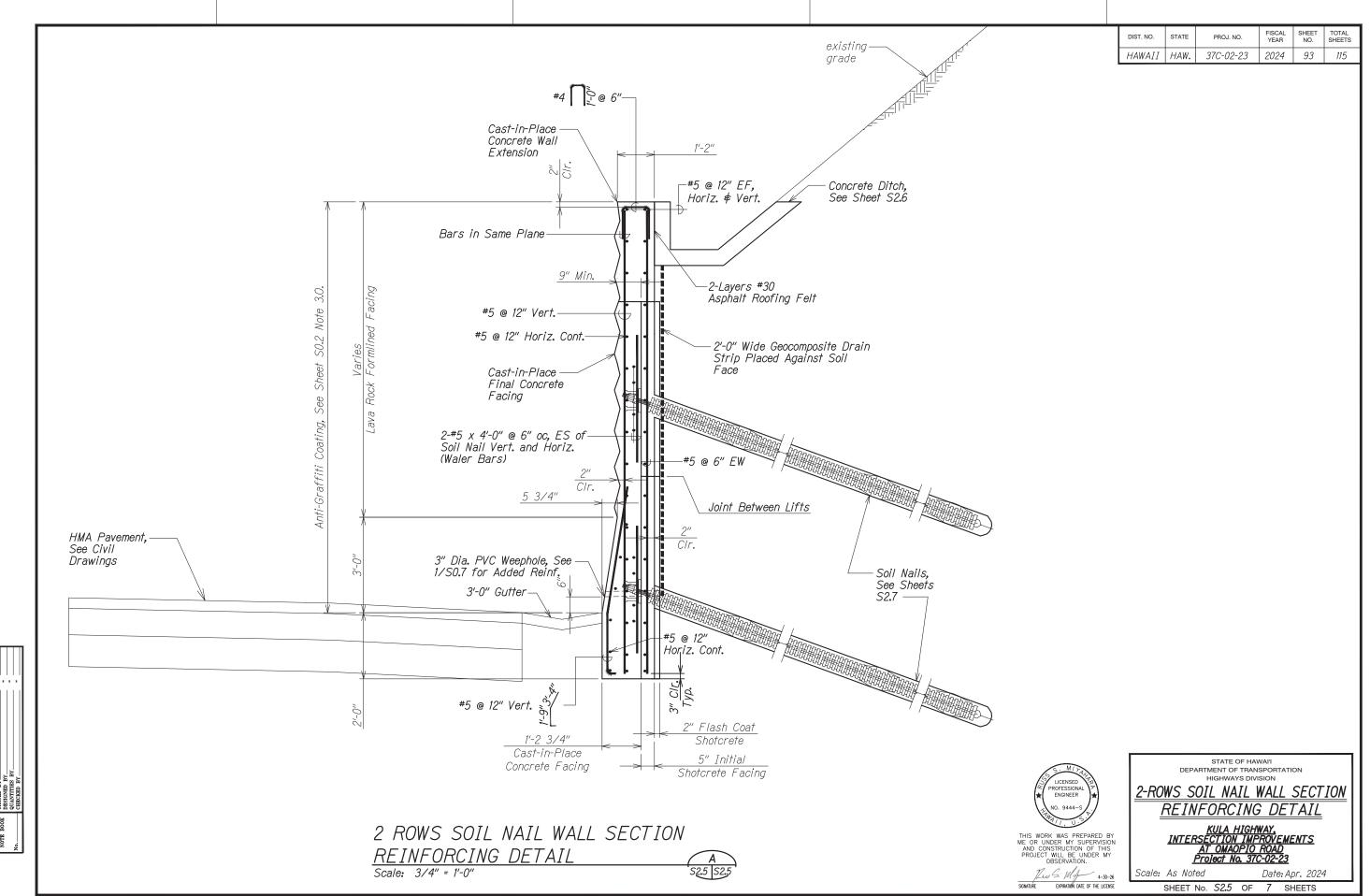
1-ROW SOIL NAIL WALL SECTION

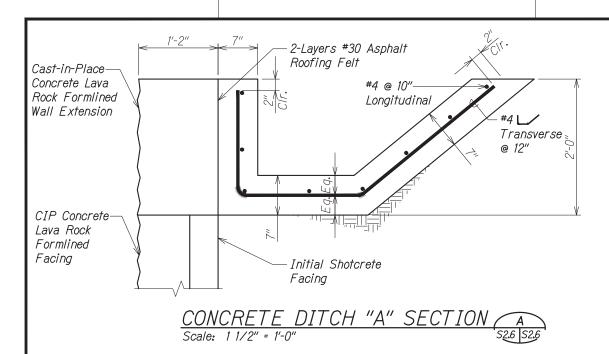
REINFORCING DETAIL

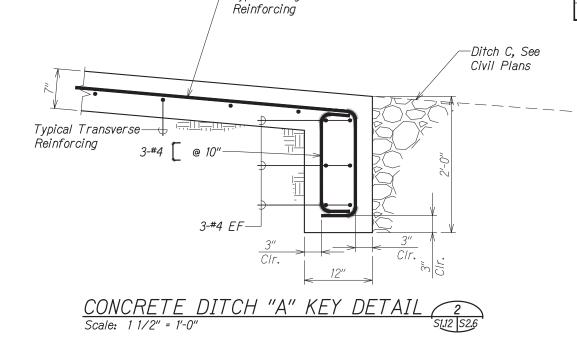
KULA HIGHWAY.
INTERSECTION IMPROVEMENTS
AT OMAOPIO ROAD
Project No. 37C-02-23

Scale: As Noted Date: Apr. 2024

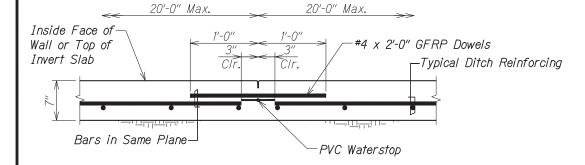
SHEET No. S2.4 OF 7 SHEETS







-Typical Longitudinal





NOTES:

- 1. Concrete ditch control joints shall match cast-in-place concrete wall extension control joints.
- 2. Concrete Ditch "A" shall be poured monolithic.
- 3. See Civil Drawings for additional information.



STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

CONCRETE DITCH "A" DETAILS

KULA HIGHWAY. INTERSECTION IMPROVEMENTS AT OMAOPIO ROAD Project No. 37C-02-23

Scale: As Noted

Date: Apr. 2024

FISCAL YEAR

37C-02-23

DIST. NO.

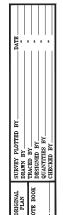
HAWAII HAW.

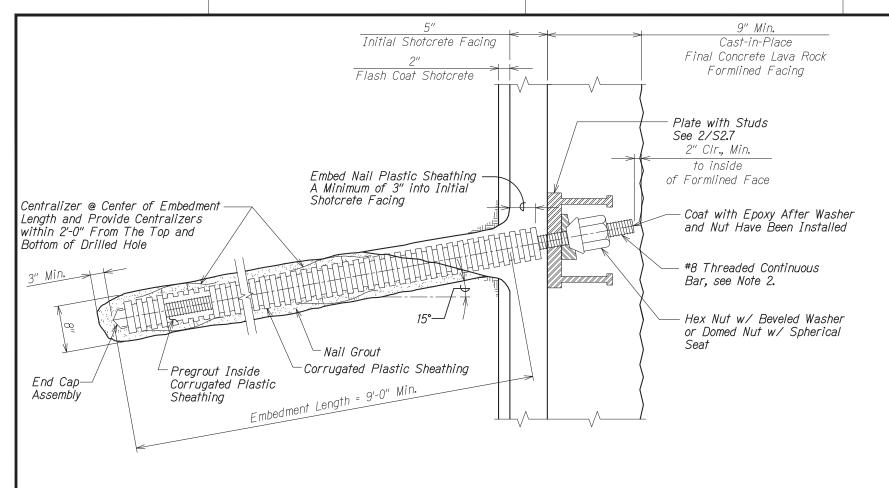
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SHEET TOTAL SHEETS

2024 | 94 | 115

SHEET No. S2.6 OF 7 SHEETS



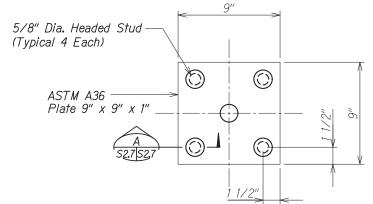


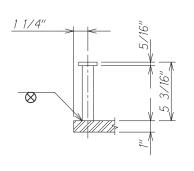
DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	37C-02-23	2024	95	115

NOTES:

- 1. Reinforcing not shown for clarity. See Sheets S2.4 and S2.5.
- 2. Soil nail shall conform to ASTM A615 Grade 75, ASTM A934 - Epoxy Coated, Deformed Bar.
- 3. Contractor shall handle all epoxy coated soil nails with care and shall avoid damaging in accordance with ASTM D3963. All damages shall be patched using a two-part epoxy repair material, approved by the coating Manufacturer.
- 4. Design Test Load : 20 Kips







CONNECTION PLATE WITH STUD DETAIL
Scale: 3" = 1'-0"



NOTE:

All plates, nuts, washers, and shear connectors shall be hot dip galvanized after fabrication.





Scale: As Noted

SHEET No. S2.7 OF 7 SHEETS

TRAFFIC SIGNAL AND LIGHT POLE FOUNDATION GENERAL NOTES:

DIST. NO. STATE PROJ. NO. FISCAL YEAR SHEET NO. TOTAL SHEETS HAWAII HAW. 37C-02-23 2024 96 115

1. <u>Design Specifications:</u>

- A. American Association of State Highway and Transportation Officials (AASHTO) 2020 LRFD Bridge Design Specifications, 9th Edition, as amended by Hawaii Department of Transportation (HDOT) document dated August 8, 2014 with subject title "Design Criteria for Bridges and Structures" and HDOT memorandum dated January 8, 2018 with subject title "Changes to Design Criteria for Bridges and Structures".
- B. Design shall conform with the AASHTO LRFD Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals, First Edition 2015 with 2020 Interim Revisions.
- C. See Structural General Notes on Sheet S0.2 to S0.4 for additional information.

. Loads:

- A. Basic Wind Speed: 145 mph.
- B. Recurrence Interval of 1700 years.
- C. Fatigue importance factor, $I_{\rm f}$, shall be based on Fatigue Category I for cantilevered traffic signal structures.
- D. Vortex shedding induced loads shall be considered for cantilevered mast arms and pole shafts that do not have tapers or have tapers of less than 0.14 in/ft.
- E. Traffic signal and light pole structures shall be designed for a truck induced gust based on a truck speed of 20 mph over the posted speed.
- F. Galloping and natural wind gusts shall be considered for cantilevered traffic signal and light pole structures.
- G. Natural Wind Gusts shall be considered for all traffic signal structures.

3. <u>Materials</u>

- A. Concrete for traffic signal and light pole foundation shall develop a minimum 28-day compressive strength of 4,500 psi with a maximum w/c ratio of 0.45.
- B. All concrete shall contain a water-based amine carboxylate migrating corrosion inhibitor or approved equal. Dosage shall be 24 ounces per cubic yard of concrete or recommended by the manufacturer.
- C. All reinforcing steel shall be ASTM A615 Grade 60 deformed bars unless otherwise noted.
- D. All connection bolts shall be AASHTO M164 bolts and anchor bolts shall be AASHTO M314-105 bolt.
- E. Aluminum members and surfaces in contact with structural steel shall be isolated with neoprene material as approved by the Engineer.

4. <u>General</u>

- A. The recommendations of the traffic pole manufacturer shall be followed.

 Manufacturer shall select pole, anchor bolts, etc. based on criteria given in the contract documents. The Contractor shall submit catalog cuts and calculations to the Engineer for approval.
- B. The Contractor shall use templates while installing the anchor bolts. Anchor bolts shall be vertical.
- C. The Contractor shall adjust the spiral vertical spacing to allow installation of anchor bolts and plates.
- D. For all traffic signals, wire cloth shall be galvanized steel standard grade plain weave 2x2 mesh 0.063 inch diameter wires. Wrap around base plate perimeter with 3" minimum lap. Fasten with 1/4" diameter capscrew ASTM F593 with SS washer spaced at approximately 9" o.c., drilled and tapped into perimeter of base plate.
- E. Drilled shaft diameters were determined based on assumed pole designs with parameters such as the bolt circles and base plate diameters. The Contractor is responsible for verifying that the drilled shaft designs, as shown on this sheet, are compatible with the manufacturer's provided pole designs. If it is found incompatible, alternate pole designs are recommended. Otherwise, the contractor is responsible for alternate drilled shaft designs, and is required to submit alternate drilled shaft designs to the Engineer for approval. The shafts shall not be installed until the pole and shaft designs have been finalized and verified for their compatibility.



DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

TRAFFIC SIGNAL AND LIGHT POLE

FOUNDATION NOTES

KULA HIGHWAY.
INTERSECTION IMPROVEMENTS
AT OMAOPIO ROAD
Project No. 37C-02-23

STATE OF HAWAI'I

Thus S. May 4-30-26

E DIPPRINTING DATE OF THE LICENSE

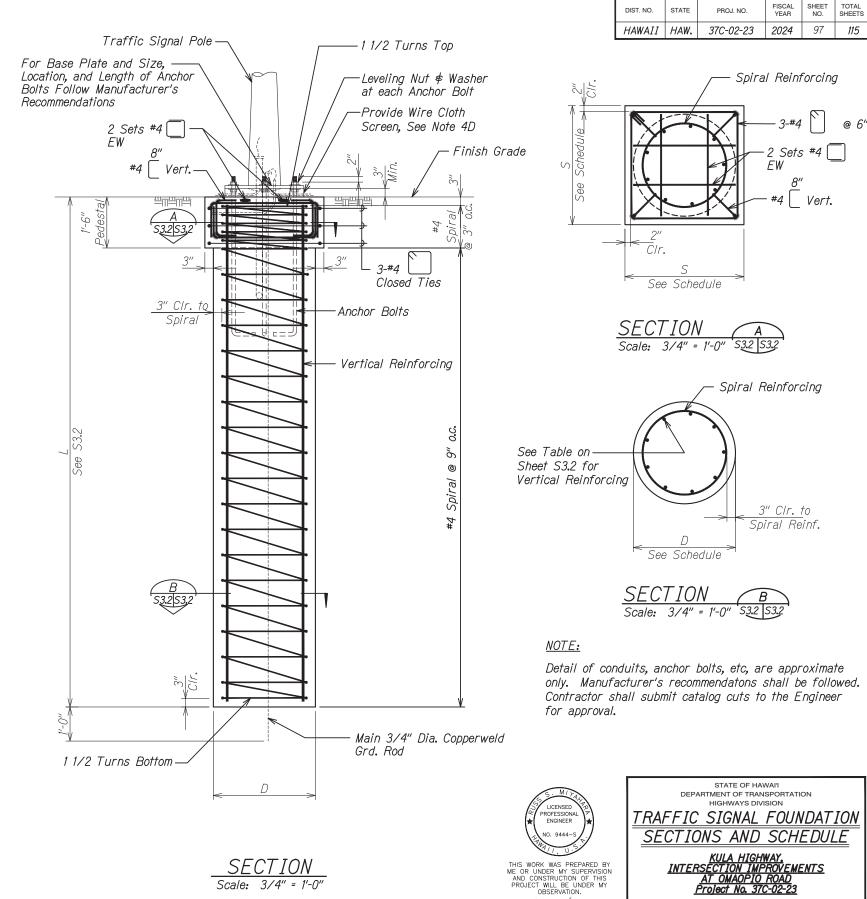
SCAIRE: AS NOTED

SCHEET NO.

As Noted Date: Apr. 2024
SHEET No. S31 OF 3 SHEETS



TRAFFIC SIGNAL FOUNDATION SCHEDULE						
Mast Arm Length (ft.)	D	S	L	Vertical Reinforcing		
20	3'-0"	3'-6"	8'-0"	6-#10		
25	3'-0"	3'-6"	8'-0"	6-#10		
35	3'-0"	3'-6"	9'-0"	9-#10		





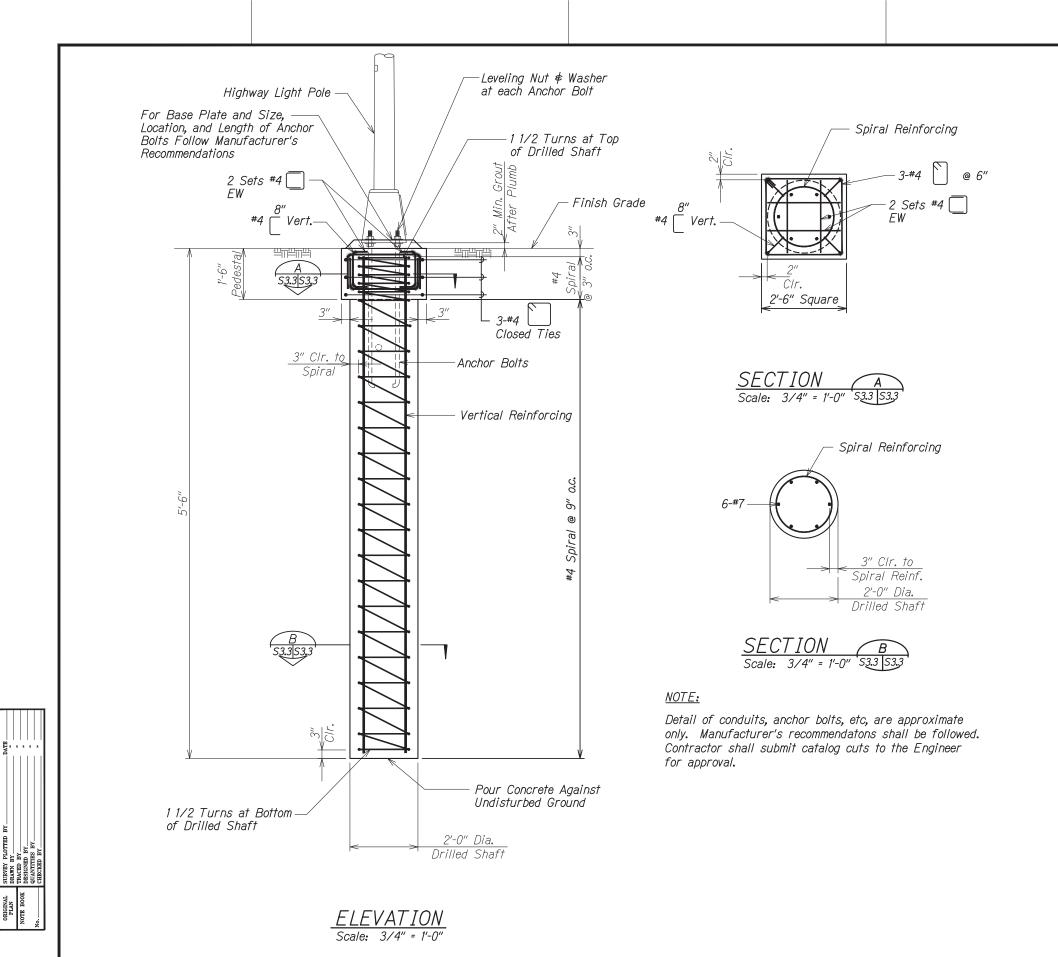
SHEET No. S3.2 OF 3 SHEETS

Date: Apr. 2024

July S. M.J. 4-30-26 SIGNATURE EXPIRATION DATE OF THE LICENSE

Scale: None

SHEET TOTAL SHEETS



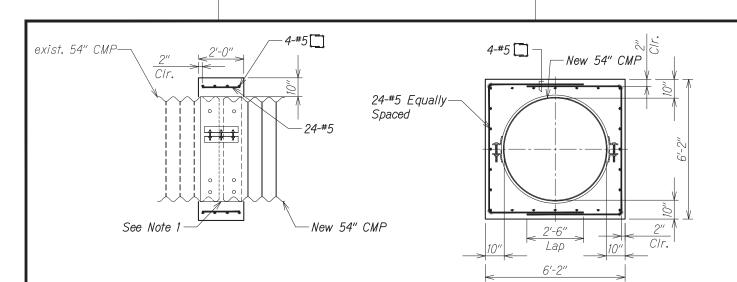
TOTAL SHEETS FISCAL YEAR SHEET NO. DIST. NO. STATE 37C-02-23 2024 98 115 HAWAII HAW.

> STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION LIGHT POLE

FOUNDATION SECTION

Date: Apr. 2024

SHEET No. S3.3 OF 3 SHEETS



DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	37C-02-23	2024	99	115

NOTES:

- 1. Joint shall be externally banded and meet AASHTO R82 6.2.3 and AASHTO LRFD Bridge Construction Specifications, Section 26 for Erodible Soils.
- 2. See Civil Drawings for additional information.

PLAIN

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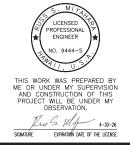
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STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

CMP JOINT DETAIL

KULA HIGHWAY.

INTERSECTION IMPROVEMENTS
AT OMAOPIO ROAD
Project No. 37C-02-23

Scale: As Noted

Date: Apr. 2024

SHEET No. S4.1 OF 1 SHEETS