GENERAL SPECIFICATIONS: HAWAII STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND PUBLIC WORKS CONSTRUCTION, 1994, TOGETHER WITH SPECIAL PROVISIONS PREPARED FOR THIS CONTRACT. DESIGN SPECIFICATIONS: "AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS," SECOND EDITION, 1998. LOADS: A. DEAD LOAD: AN ALLOWANCE OF 2 INCHES FOR FUTURE WEARING SURFACE OF ASPHALT CONCRETE HAS BEEN PROVIDED FOR IN THE DESIGN. HL-93 LIVE LOAD: SEISMIC LOAD: ACCELERATION COEFFICIENT = 0.25 SEISMIC PERFORMANCE ZONE = 3 IMPORTANCE CATEGORY = ESSENTIAL BRIDGE SOIL PROFILE TYPE I D. RAILING TEST LEVEL = TL-4**MATERIALS:** A. MINIMUM CONCRETE COMPRESSIVE STRENGTH (AT 28 DAYS) SHALL BE AS FOLLOWS, UNLESS OTHERWISE NOTED: COLUMNS = 5.000 PSIALL OTHER CONCRETE = 4,000 PSI. B. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS OTHERWISE NOTED. C. ALL COLUMN AND DRILLED SHAFT SPIRAL REINFORCING SHALL BE DEFORMED REINFORCING BARS CONFORMING TO ASTM A615, GRADE 60. ALL MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A36 AND BE HOT DIP GALVANIZED AFTER FABRICATION, UNLESS OTHERWISE NOTED. E. ALL PIPE SECTIONS SHALL CONFORM TO ASTM A53 (TYPE E), GRADE B, AND BE HOT DIP GALVANIZED AFTER FABRICATION. F. ALL STRUCTURAL TUBE SECTIONS SHALL CONFORM TO ASTM A500, GRADE B AND BE HOT DIP GALVANIZED AFTER FABRICATION. ALL ANCHOR BOLTS, WASHERS AND NUTS SHALL CONFORM TO ASTM A307 AND BE HOT DIP GALVANIZED AFTER FABRICATION, UNLESS OTHERWISE SPECIFIED. H. FOR MATERIALS OF PRESTRESSED CONCRETE GIRDERS. SEE APPLICABLE PRESTRESSED CONCRETE NOTES. ELASTOMERIC BEARING PADS SHALL BE 60 HARDNESS. STEEL LAMINATIONS SHALL BE 1/8 INCHES THICK ASTM A36 STEEL PLATES. REINFORCEMENT: A. UNLESS OTHERWISE NOTED. THE COVERING MEASURED FROM THE SURFACE OF THE CONCRETE TO THE FACE OF ANY REINFORCING BARS SHALL BE AS FOLLOWS: 1) DECK SLABS TOP BARS = 2" BOTTOM BARS = 1-1/4" RAILINGS AND PARAPETS = 2"FOR PRESTRESSED CONCRETE GIRDERS, SEE PRESTRESSED CONCRETE GIRDER DETAILS. COLUMNS = 3" (TO SPIRALS)ABUTMENT WALLS AND WING WALLS = 2" CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH = 3" DRILLED SHAFTS = 3" (TO SPIRALS) REINFORCING BARS SHALL BE DETAILED IN ACCORDANCE WITH ACI DETAILING MANUAL FOR REINFORCED CONCRETE HIGHWAY STRUCTURES UNLESS OTHERWISE NOTED. MINIMUM CLEAR SPACING BETWEEN PARALLEL BARS SHALL BE 1-1/2 TIMES THE DIAMETER OF BARS (FOR NON BUNDLED BARS) OR 1-1/2 TIMES THE DIAMETER DERIVED FROM THE EQUIVALENT TOTAL AREA OF THE BARS (FOR BUNDLED BARS) BUT IN NO CASE SHALL THE CLEAR DISTANCE BETWEEN THE PARALLEL BARS BE LESS THAN 1-1/2 TIMES THE MAXIMUM SIZE OF THE COARSE AGGREGATE OR 1-1/2 INCHES. ALL DIMENSIONS RELATING TO REINFORCING BARS (E.G. SPACING OF BARS, ETC.) ARE TO CENTERS OF BARS UNLESS OTHERWISE NOTED. E. REINFORCING BARS SHALL BE SECURELY TIED AT ALL INTERSECTIONS AND LAP SPLICES EXCEPT WHERE THE SPACING OF INTERSECTIONS IS LESS THAN ONE FOOT IN EACH DIRECTION, IN WHICH CASE ALTERNATE INTERSECTIONS SHALL BE TIED. F. ALL BARS SHOWN WITH BENDS SHALL CONFORM TO STANDARD ACI HOOKS UNLESS OTHERWISE NOTED. VERTICAL DRILLED SHAFT BARS SHALL BE ARRANGED IN SUCH A MANNER AS TO AVOID INTERFERENCE WITH FOOTING BARS. TOP BENDS OF VERTICAL BARS SHALL BE ARRANGED TO FIT WITHIN LIMITS OF THE FOOTING. MINIMUM EMBEDMENT SHOWN SHALL BE PROVIDED. NO BARS SHALL BE CUT.

H. COLUMN SPIRALS SHALL BE SPLICED WITH MECHANICAL CONNECTORS, UNLESS OTHERWISE NOTED.

DEPTH, AND THE CALCULATED OR, IF AVAILABLE, THE ACTUAL CAMBER OF THE GIRDERS.

CONCRETE FOR DECK SLAB AND DIAPHRAGMS. ALSO, SEE PRESTRESSED GIRDER NOTES.

THE CONTRACTOR SHALL VERIFY ALL SITE CONDITIONS BEFORE COMMENCING WITH WORK.

SURFACE WITH ADHESIVES OR OTHER MEANS NECESSARY AS APPROVED BY THE ENGINEER.

THE RESPECTIVE OWNERS BEFORE COMMENCING THE WORK OF EXCAVATION OR DRILLED SHAFTS.

A. BEARING SURFACE SHALL BE SMOOTH AND LEVEL. GIRDERS SHALL BE SET ON A

GIRDER SEAT ELEVATIONS SHALL BE VERIFIED BY THE CONTRACTOR. SEAT

DIFFERENTIAL, DECK SLAB THICKNESS, MINIMUM HAUNCH REQUIRED, GIRDER

TOP OF CONCRETE DECK SLAB SHALL BE CONSTRUCTED TO FOLLOW THE

CONSTRUCTION JOINTS MAY BE RELOCATED OR ADDITIONAL ONES ADDED

UNLESS OTHERWISE NOTED, ALL EXPOSED CONCRETE EDGES SHALL BE

J. THE CONTRACTOR SHALL RESTRAIN THE GIRDERS IN THE LONGITUDINAL

ELEVATIONS SHALL TAKE INTO CONSIDERATION THE VERTICAL ROADWAY CURVE

THE CONCRETE SEAT AND ELASTOMERIC BEARING PAD SHALL BE PERPENDICULAR

ROADWAY FINISH GRADES. THE CONTRACTOR SHALL SET DECK SLAB FORMS TO

THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UTILITY LINES AND NOTIFY

ACCOUNT FOR ANTICIPATED DEFLECTION OF GIRDERS DURING PLACEMENT OF

UNLESS OTHERWISE NOTED, ALL VERTICAL DIMENSIONS ARE MEASURED PLUMB.

ELASTOMERIC BEARING PADS SHALL BE SECURED TO THE CONCRETE BEARING

FRESH LAYER OF MORTAR TO INSURE FULL BEARING.

SEE STANDARD SPECIFICATIONS AND SPECIAL PROVISIONS.

FOR CONCRETE FINISH, SEE STANDARD SPECIFICATIONS.

SUBJECT TO THE APPROVAL OF THE ENGINEER.

CHAMFERED 3/4" X 3/4".

DIRECTION DURING PLACEMENT.

TO THE CENTERLINE OF THE GIRDER.

GIRDER BEARINGS:

CONSTRUCTION NOTES:

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SUR DRA TRA DES

UAOA BRIDGE GENERAL

K. SHORING SHALL BE PROVIDED FOR THE EXISTING BRIDGE FOOTINGS DURING CONSTRUCTION OF THE NEW PIER FOOTING.

L. JUST PRIOR TO CONSTRUCTION OF THE PIER NO. 2 FOOTING, REMOVE THE PORTION OF THE EXISTING BENT NO. 8 FOOTING THAT INTERFERES WITH THE NEW FOOTING. EXISTING FOOTING SHALL BE VERIFIED AND REMOVED AS DIRECTED BY THE ENGINEER.

M. JUST PRIOR TO CONSTRUCTION OF THE PIER NO. 1 FOOTING, REMOVE THE PORTION OF THE EXISTING CRM WALL, THAT INTERFERES WITH THE NEW FOOTING. EXISTING CRM WALL SHALL BE VERIFIED. REMOVED AND REPLACED AS DIRECTED BY THE ENGINEER.

GENERAL:

ALL ITEMS NOTED INCIDENTAL WILL NOT BE PAID FOR SEPARATELY. STANDARD DETAIL DRAWINGS REFER TO ALL STRUCTURES IN GENERAL, EXCEPT FOR MODIFICATIONS AS MAY BE REQUIRED FOR SPECIAL CONDITIONS. FOR SUCH MODIFICATIONS. REFER TO THE CORRESPONDING DETAILED DRAWINGS.

THE CONTRACTOR SHALL COMPLY WITH ALL CONSTRUCTION PERMITS FOR THIS PROJECT. IN ADDITION, THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING AND COMPLYING WITH ANY NPDES PERMITS AND LICENSES THAT MAY BE REQUIRED FOR THE WORK.

D. THE CONTRACTOR SHALL UTILIZE APPROPRIATE EROSION CONTROL MEASURES DURING CONSTRUCTION AND SHALL PERFORM CONSTRUCTION ACTIVITIES AFFECTING OR AFFECTED BY THE STREAM ONLY DURING PERIODS OF LOW STREAM FLOW. THE CONTRACTOR SHALL PREVENT DEBRIS AND CONSTRUCTION MATERIALS, INCLUDING CEMENT OR CONCRETE, PETROLEUM PRODUCTS, AND OTHER POLLUTANTS FROM ENTERING THE STREAM. WASH AND DUST CONTROL WATER SHALL BE PROPERLY DISPOSED.

IN THE EVENT THAT SUBSURFACE CULTURAL REMAINS SUCH AS ARTIFACTS. BURIALS OR DEPOSITS OF SHELLS OR CHARCOAL ARE ENCOUNTERED DURING EXCAVATION WORK, THE CONTRACTOR SHALL STOP WORK IN THE AREA OF THE FIND AND CONTACT THE ENGINEER IMMEDIATELY. WORK MAY RECOMMENCE ONLY WITH ENGINEER'S APPROVAL.

9. <u>FOUNDATION:</u>

THESE FOUNDATION NOTES WERE BASED ON RECOMMENDATIONS CONTAINED IN A FOUNDATION INVESTIGATION REPORT BY ERNEST K. HIRATA AND ASSOCIATES, INC. DATED DECEMBER 10, 1998. THE REPORT SHALL BE CONSIDERED AS PART OF THE CONSTRUCTION DOCUMENTS AND ITS RECOMMENDATIONS SHALL BE IMPLEMENTED UNLESS OTHERWISE DIRECTED BY THE SOILS ENGINEER.

A. DRILLED SHAFTS:

THE VERTICAL LOAD CAPACITY FOR THE 48 INCHES DIAMETER DRILLED SHAFTS ARE FROM FRICTION BETWEEN THE SHAFT AND THE SURROUNDING SOILS. THE FOLLOWING NET VERTICAL LOAD PARAMETERS WERE USED FOR DESIGN:

A) ABUTMENT NO. 1: • ESTIMATED SHAFT TIP ELEVATION = 373.00'

-STRENGTH LIMIT STATE = 2070 KIPS

-EXTREME EVENT LIMIT STATE = 3120 KIPS • ESTIMATED SHAFT TIP ELEVATION = 453.00'

-STRENGTH LIMIT STATE = 617 KIPS -EXTREME EVENT LIMIT STATE = 950 KIPS

B) ABUTMENT NO. 2:

-STRENGTH LIMIT STATE = 775 KIPS-EXTREME EVENT LIMIT STATE = 1230 KIPS

C) PIER NO. 1 AND NO. 2:

-STRENGTH LIMIT STATE = 1072 KIPS -EXTREME EVENT LIMIT STATE = 1650 KIPS

2) THE VERTICAL UPLIFT LOAD CAPACITY FOR THE 48 INCHES DIAMETER DRILLED SHAFTS ARE FROM FRICTION BETWEEN THE SHAFT AND SURROUNDING SOIL AND BASALT. THE FOLLOWING NET VERTICAL UPLIFT LOAD PARAMETERS WERE USED FOR DESIGN:

A) ABUTMENT NO. 1:

-STRENGTH LIMIT STATE = -350 KIPS -EXTREME EVENT LIMIT STATE = -350 KIPS

B) ABUTMENT NO. 2:

-EXTREME EVENT LIMIT STATE = -200 KIPS

3) THE FOLLOWING LATERAL LOAD CAPACITIES FOR THE 48 INCHES DIAMETER DRILLED SHAFTS (FIXED — HEAD CONDITION) WERE USED FOR DESIGN:

A) ABUTMENT NO. 1 = 147 KIPS ABUTMENT NO. 2 = 159 KIPS

C) PIER NO. 1 AND NO. 2 = 160 KIPS

THE DRILLED SHAFT ESTIMATED TIP ELEVATION SHOWN ON THE PLANS ARE BASED ON THE BORING DATA. THE ACTUAL TIP ELEVATION COULD CHANGE DUE TO VARYING SUBSURFACE CONDITIONS. SOILS ENGINEER OF RECORD SHALL BE PRESENT DURING THE DRILLING OPERATION TO DETERMINE THAT THE ACTUAL SUBSURFACE CONDITIONS ARE CONSISTENT WITH THE CONDITIONS ASSUMED FOR DESIGN. BASED ON THE RECOMMENDATIONS, THE TIP ELEVATIONS COULD CHANGE. THE CONTRACTOR SHALL MAKE PROVISIONS TO ACCOUNT FOR VARIATIONS IN THE FINAL TIP ELEVATIONS.

THE CONTRACTOR SHALL EXERCISE CARE IN DRILLING THE SHAFT HOLES AND IN PLACING CONCRETE INTO THE HOLES. ROCK DRILLING/CORING EQUIPMENT WILL BE REQUIRED FOR DRILLED SHAFTS EXTENDING THROUGH HARD BASALT LAYERS. TEMPORARY CASING MAY BE NEEDED TO REDUCE THE POTENTIAL FOR CAVING IN OF THE HOLES. THE USE OF PERMANENT CASING WILL NOT BE ALLOWED. DRILLING SHALL NOT BE CONDUCTED BY METHODS UTILIZING DRILLING FLUIDS.

CONCRETE FOR DRILLED SHAFTS SHALL BE PLACED WITHIN 24 HOURS AFTER DRILLING TO REDUCE THE POTENTIAL FOR CAVING IN.

SINCE GROUNDWATER WILL LIKELY BE ENCOUNTERED IN THE DRILLED HOLES, PLACEMENT OF CONCRETE SHALL BE BY TREMIE METHODS. A MINIMUM OF 10 FEET OF CONCRETE HEAD SHALL BE MAINTAINED ABOVE THE BOTTOM OF THE

TREMIE PIPE DURING PLACEMENT OF CONCRETE.

FED. ROAD STATE FISCAL SHEET TOTAL FED. AID DIST. NO. PROJ. NO. YEAR NO. SHEETS C.O. 45 HAW. BR-036-1(14) 2000 70 HAWAII

B. ABUTMENTS AND WING WALLS:

1) DESIGN EQUIVALENT FLUID PRESSURE:

A) LEVEL BACKFILL:

- ACTIVE = 40 PCF

- AT-REST = 55 PCF

B) 2H: 1V SLOPING BACKFILL: - ACTIVE = 50 PCF - AT-REST = 55 PCF

PASSIVE EARTH PRESSURE = 300 PCF (3,000 PSF MAXIMUM)

BACKFILL MATERIAL:

THE ONSITE CLAYEY SILT AND HIGHLY TO COMPLETELY WEATHERED BASALT MAY BE REUSED AS BACKFILL PROVIDED ALL ROCK FRAGMENTS LARGER THAN 6 INCHES IN MAXIMUM DIMENSION ARE REMOVED PRIOR TO COMPACTION.

IMPORTED STRUCTURAL BACKFILL SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS FOR STRUCTURE BACKFILL MATERIAL A AS INDICATED IN SECTION 703.20 OR THE HAWAII STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND PUBLIC WORKS CONSTRUCTION, 1994.

C) PLACEMENT OF THE BACKFILL MATERIAL SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND THE SPECIAL PROVISIONS.

MECHANICALLY STABILIZED EARTH RETAINING WALL SYSTEM:

INTERNAL FRICTION ANGLE = 42 DEGREES

COHESION = 800 PSF

ACTIVE EARTH PRESSURE = 40 PCF

SEE SECTION 520 - MECHANICALLY STABILIZED EARTH RETAINING WALL SYSTEMS OF THE SPECIAL PROVISIONS.

10. TESTING AND DISPOSAL OF HAZARDOUS MATERIAL:

A. A TEST FOR HAZARDOUS MATERIALS SHALL BE CONDUCTED ON THE EXISTING BRIDGE TO BE DEMOLISHED. DISPOSAL OF HAZARDOUS MATERIALS SHALL BE IN ACCORDANCE WITH ALL FEDERAL, STATE, AND COUNTY REQUIREMENTS.

A TEST FOR CREOSOTE TIMBERS SHALL BE CONDUCTED ON THE EXISTING BRIDGE TO BE DEMOLISHED. TESTING AND DISPOSAL SHALL BE IN ACCORDANCE WITH THE COUNTY OF MAUI, DEPARTMENT OF PUBLIC WORKS AND WASTE MANAGEMENT REQUIREMENTS.

FOR BIDDING PURPOSES, THE CONTRACTOR SHALL ASSUME THE DISPOSAL OF

CREOSOTE TIMBERS AT A LINED LANDFILL, SUCH AS MAUI DEMOLITION & CONSTRUCTION LANDFILL, INC. REFER TO SECTION 107.25 - CONTAMINATED AND/OR HAZARDOUS MATERIAL OF THE SPECIAL PROVISIONS.

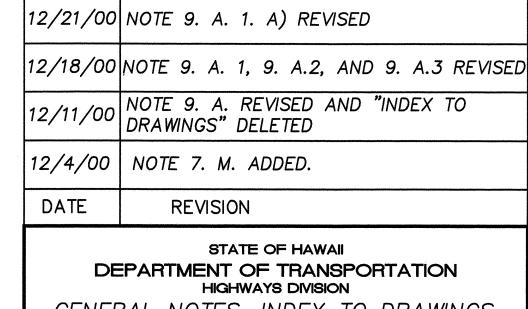
PROTECTION OF EXISTING TUNNEL

CONTRACTOR SHALL VERIFY THE LOCATION OF THE EXISTING TUNNEL. CONTRACTOR SHALL TAKE THE NECESSARY PRECAUTIONS TO PROTECT THE

EXISTING TUNNEL DURING CONSTRUCTION WORK.

ALL BRACING TO SUPPORT THE EXISTING TUNNEL SHALL BE DESIGNED AND STAMPED BY A LICENSED PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR.

D. ALL COSTS SHALL BE BORNE BY THE CONTRACTOR.



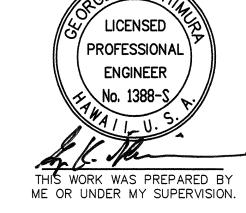
GENERAL NOTES, INDEX TO DRAWINGS

HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES DISTRICT OF MAKAWAO

Federal-Aid Project No. BR-036-1(14) Date: May, 1999 Scale: As Noted

SHEET No. S-0 OF 26 SHEETS

2/28/01 NOTE 9. C. REVISED



	SUMMARY OF ESTIMATED QUANTIT	IES	
ITEM NO.	CONTRACT ITEM	QUANTITY	UNIT
202.0440	REMOVAL OF BRIDGE	(1)	(EACH) L.S.
206.6000	STRUCTURE EXCAVATION FOR BRIDGE	5890	C.Y.
206.7000	STRUCTURE BACKFILL	4120	C.Y.
206.8000	FILTER MATERIAL	5	C.Y.
503.1090	CONCRETE IN BRIDGE	(3070)	(C.Y.) L.S.
504.4100	KEEHI IV PRESTRESSED CONCRETE GIRDERS	(1245)	(L.F.) L.S.
504.6100	KEEHI VI PRESTRESSED CONCRETE GIRDERS	(820)	(L.F.) L.S.
507.1501	METAL RAILING - TWO RAILS ON PARAPET FOR BRIDGE	645	L.F.
507.7001	CONCRETE PARAPET FOR BRIDGE (INCLUDING END POST)	680	L.F.
511.0020	LOAD TEST	1	EACH
511.0100	DRILLED SHAFT - 48 INCH DIAMETER	3430	L.F.
511.0200	UNCLASSIFIED SHAFT EXCAVATION	3430	L.F.
511.0300	UNCLASSIFIED EXTRA DEPTH EXCAVATION	150	L.F.
511.0400	DRILLED SHAFT SIDEWALL OVERREAMING	150	L.F.
511.0500	TRIAL SHAFT HOLES	70	L.F.
602.0091	REINFORCING STEEL IN BRIDGE	(839,000)	(LBS.) L.S.

Α	В	В	R	E	V	1	Α	Т	0	Ν	S
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<u>A</u>	BBKFVIA	<u> </u>	<u>S</u>
ф #	DIAMETER NUMBER OR POUND	LB., LBS. L.F. L.S.	POUND, POUNDS LINEAR FEET LUMP SUM
A.B. A.C. AZ.	ANCHOR BOLT ASPHALT CONCRETE AZIMUTH	MAX. MIN.	MAXIMUM MINIMUM
вот., вотт., в	ВОТТОМ	NO., #	NUMBER NOT TO SCALE
C.J. Q. C.G.	CONSTRUCTION JOINT CENTERLINE CENTER OF GRAVITY	N.T.S. O.C.	NOT TO SCALE ON CENTER
CLR., CL. CONC. CONT. C.Y.	CLEAR CONCRETE CONTINUOUS CUBIC YARD	PCF PL., PL PSF PSI	POUNDS PER CUBIC FEET PLATE POUNDS PER SQUARE FEET
DBL.	DOUBLE	PVC.	POUNDS PER SQUARE INCH POLYVINYL CHLORIDE
DET. D.I. DIA. DN. DWG.	DETAIL DUCTILE IRON DIAMETER DOWN DRAWING	R, RAD. REBAR REF. REINF.	RADIUS REINFORCING BAR REFERENCE REINFORCED, REINFORCING, REINFORCEMENT
E.F. ELEV., EL.	EACH FACE ELEVATION	R.O.W.	RIGHT-OF-WAY
E.W. EXP.	EACH WAY EXPANSION	SHT. SL.	SHEET SLOPE
F.B.	FLAT BAR	STA. STD.	STATION STANDARD
G GALV.	GIRDER GALVANIZED	STIRR. SYM., SYMM.	STIRRUP SYMMETRICAL
HORIZ., H	HORIZONTAL	THK., TH.	THICK
IN.	INCH	TYP.	TYPICAL
JT.	JOINT	VERT., V	VERTICAL
K KSI	KIPS KIPS PER SQUARE INCH	W/	WITH

FED. ROAD STATE FISCAL SHEET TOTAL YEAR NO. SHEETS FED. AID PROJ. NO. HAWAII HAW. BR-036-1(14) 2000 70

Depressed "V" Letters 3/8" Deep—



<u>Use Correct Name Of Bridge</u>

LICENSED

PROFESSIONAL

<u>Date Of Year Built</u>

- Note:

 1. Name And Date Shall Be Placed At An End Post
- On Each Side Of The Roadway.

 2. Exact Details And Spacing Of Letters And Figures
 And Location Shall Be As Directed By The Engineer.
 Gothic Letters And Figures Approximating Dimensions
 Shown Will Be Acceptable If Approved By The Engineer.

TYPICAL DETAIL OF LETTERS AND FIGURES AT CONCRETE END POST

Not To Scale

	1101511111111111	
45	5 S-0	GENERAL NOTES
46	S-1	INDEX TO DRAWINGS, ABBREVIATIONS, SUMMARY OF ESTIMATED QUANTITIES
47	7 S-2	BRIDGE LAYOUT PLAN, LONGITUDINAL SECTION/ELEVATION, TYPICAL CROSS SECTION
48	3 S-3	EXISTING BRIDGE DEMOLITION PLAN, NORMAL DECK SECTION
49	9 5-4	FOUNDATION PLAN, WING WALL DETAILS, DRILLED SHAFT DETAILS
50	S-5	ABUTMENT NO. 1, PLAN AT ABUTMENT SHELF, DRILLED SHAFT LAYOUT PLAN
51	S-6	ABUTMENT NO. 1 ELEVATION AND SECTION, WING WALL ELEVATIONS
51S	-1 S-6A	ABUTMENT NO. 1 SECTIONS
52	2 S-7	PLAN/SECTION AT ABUTMENT SHELF, EXPANSION JOINT DETAIL
53	3 S-8	ABUTMENT NO. 2, PLAN AT ABUTMENT SHELF, DRILLED SHAFT LAYOUT PLAN, WING WALL ELEVATION
54	4 S-9	ABUTMENT NO. 2 ELEVATION AND SECTION
55	5 S-10	PIER FOUNDATION PLAN AND SECTIONS
56	S-11	PIER SECTION
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58	S-13	DECK LAYOUT PLAN, DECK FRAMING PLAN, DIAPHRAGM DETAIL AT KEEHI VI GIRDER
59	9 S-14	DECK FRAMING PLANS, INTERMEDIATE DIAPHRAGM AT KEEHI IV GIRDER
60	S-15	NORMAL DECK SECTIONS, CONCRETE PARAPET DETAILS
61	S-16	DECK REINFORCING LAYOUT PLANS
62	2 S-17	KEEHI IV GIRDER DETAILS
63	3 S-18	KEEHI VI GIRDER DETAILS
64	1 S-19	TYPICAL END DIAPHRAGM GIRDER DETAILS
65	5 S-20	BEARING PAD, CONCRETE SEAT AND CREEP BLOCK DETAILS
66	5 S-21	TYPICAL GUARDRAIL TO END POST NO. 1 AND 2 CONNECTION DETAIL
67	7 S-22	TYP. PARAPET AND METAL RAIL DET., TYP. GUARDRAIL TO END POST NO. 3 CONNECTION DET.
68	3 S-23	PLAN AT MSERW RETAINING WALL, ELEVATION AT MSERW RETAINING WALL
69	9 S-24	MSERW RETAINING WALL DETAILS
70	S-25	TYPICAL GUARDRAIL TO END POST NO. 4 CONNECTION DETAIL

INDEX TO DRAWINGS

DESCRIPTION

SHEET NO DRAW NO.

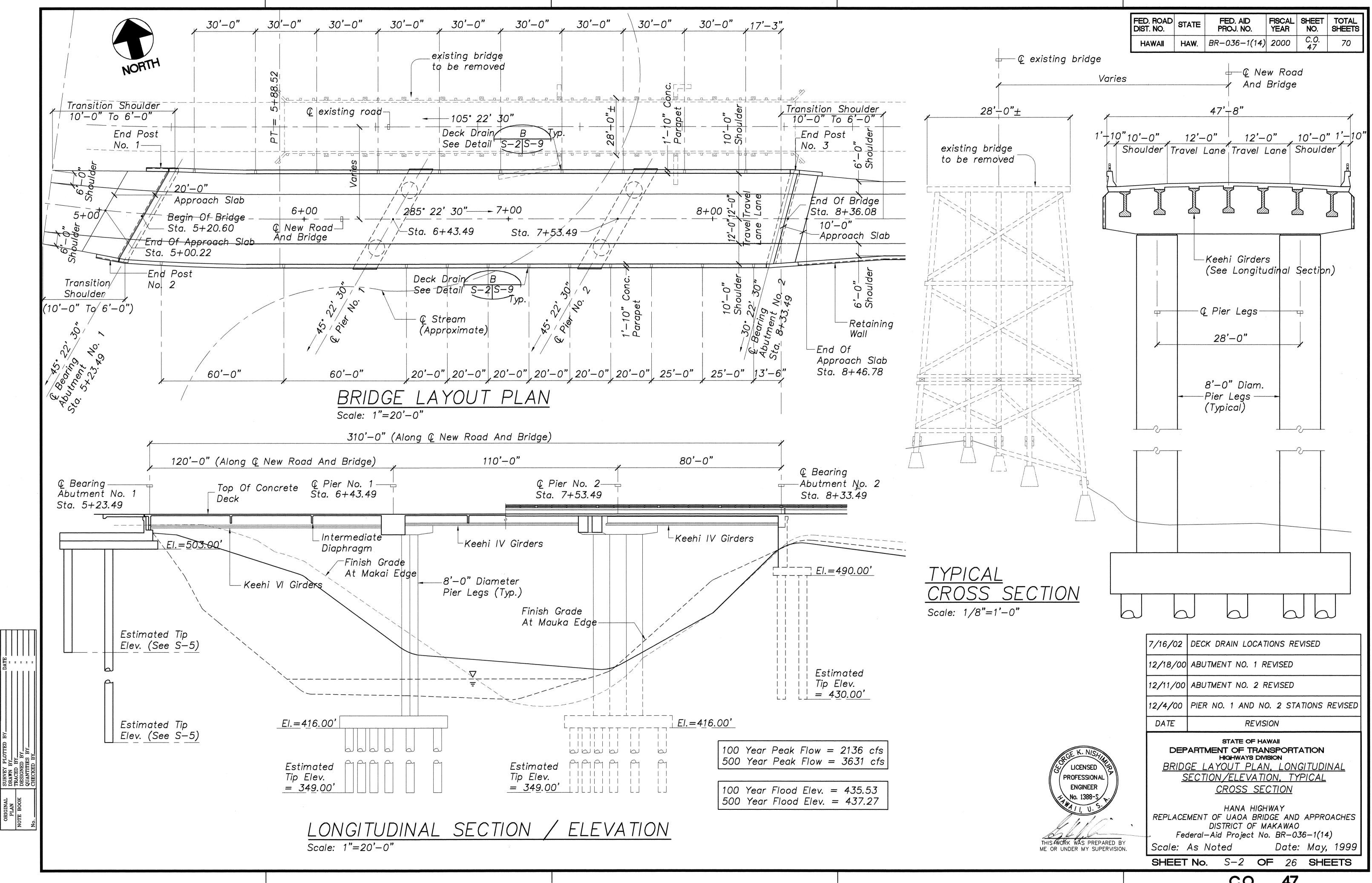
2/28/01	"INDEX TO DRAWINGS" REVISED
1/23/01	REVISED "SUMMARY OF ESTIMATED QUANTITIES"
12/18/00	"INDEX TO DRAWINGS" REVISED
12/11/00	ADDED AND REVISED "INDEX TO DRAWINGS"
DATE	REVISION

STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

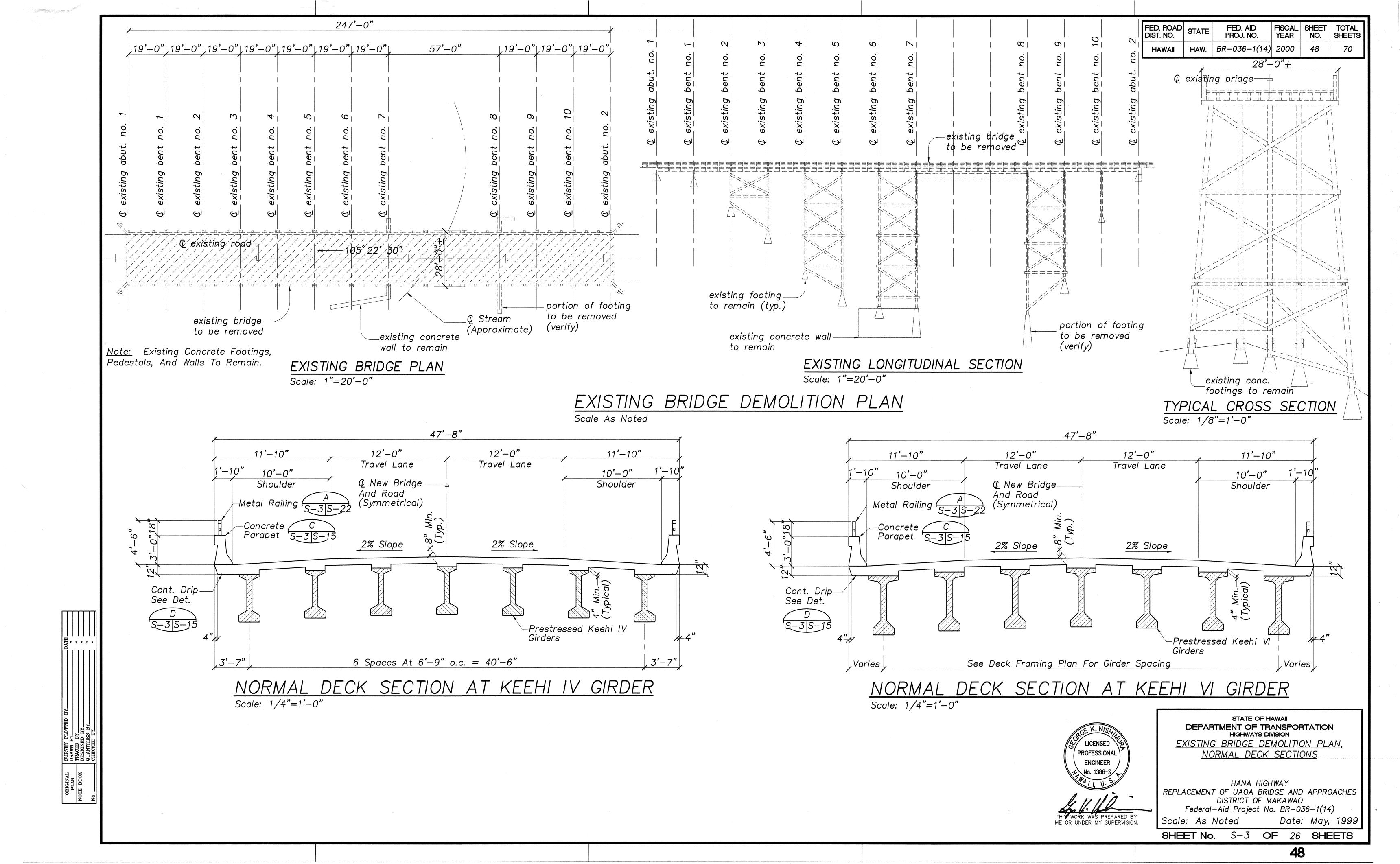
GENERAL NOTES, ABBREVIATIONS SUMMARY OF ESTIMATED QUANTITIES

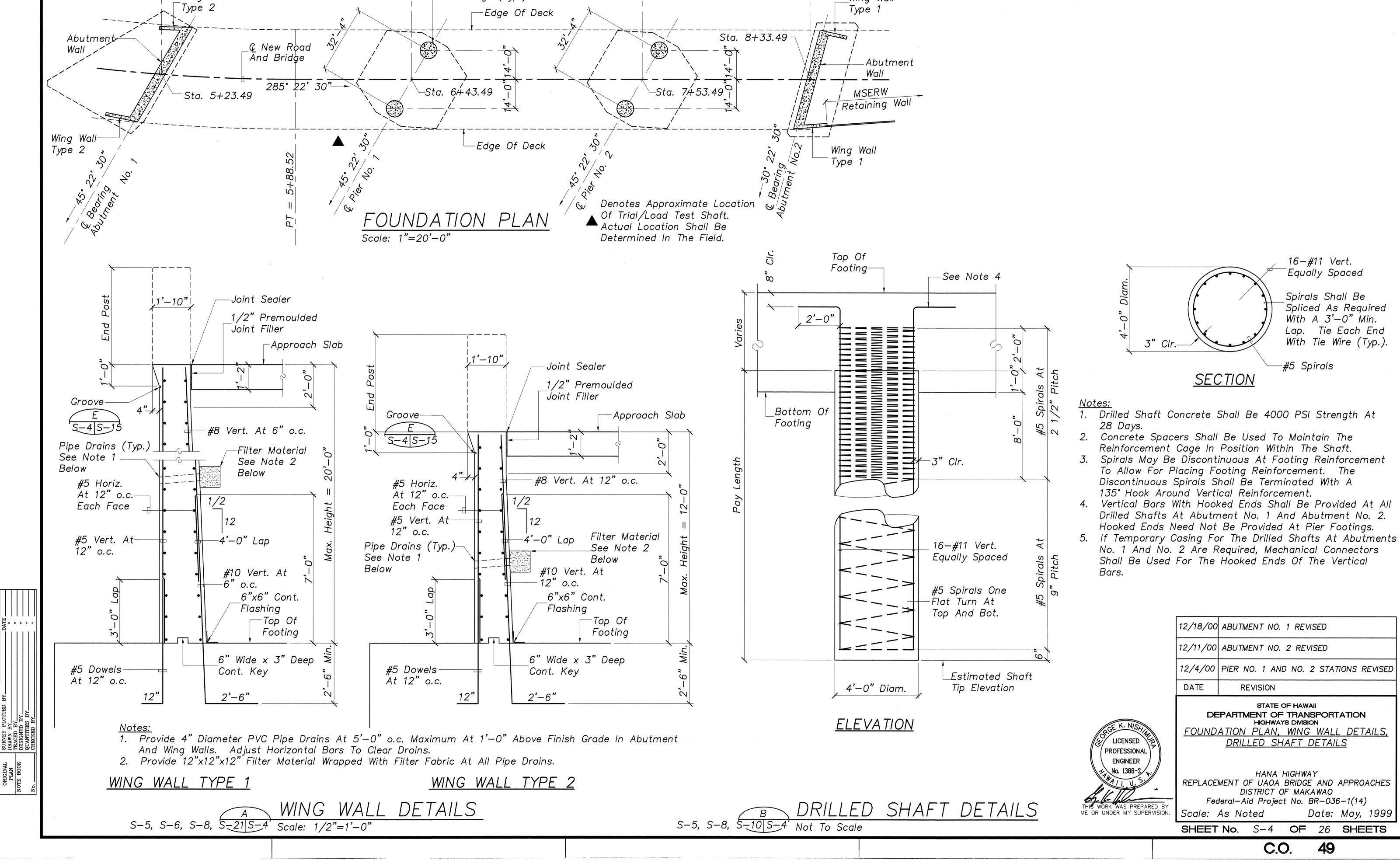
HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES DISTRICT OF MAKAWAO Federal—Aid Project No. BR—036—1(14) Date: May, 1999 Scale: As Noted

SHEET No. S-1 OF 26 SHEETS



C.O.





80'-0"

_Wing Wall

110'-0"

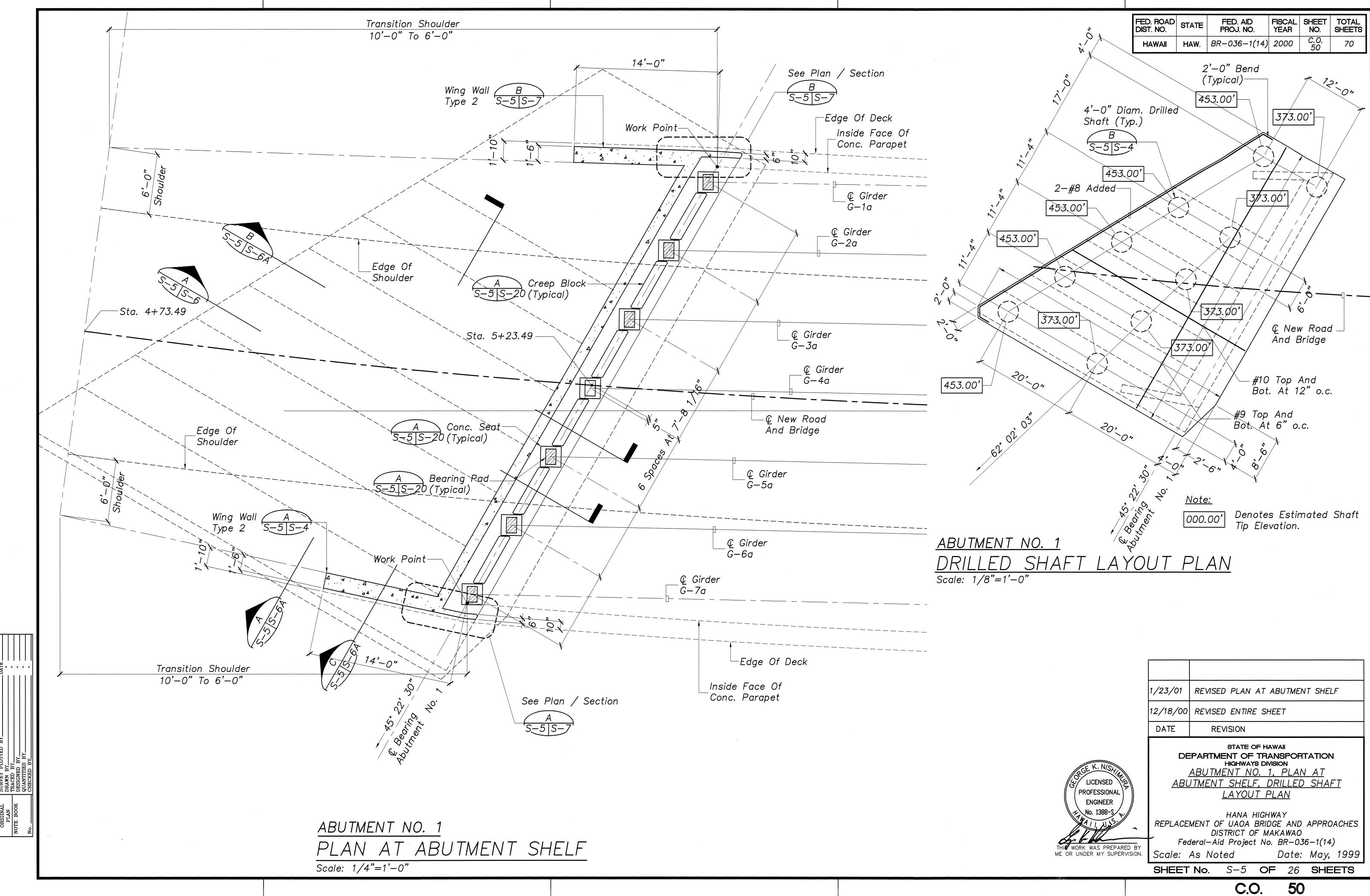
8'-0" Diameter

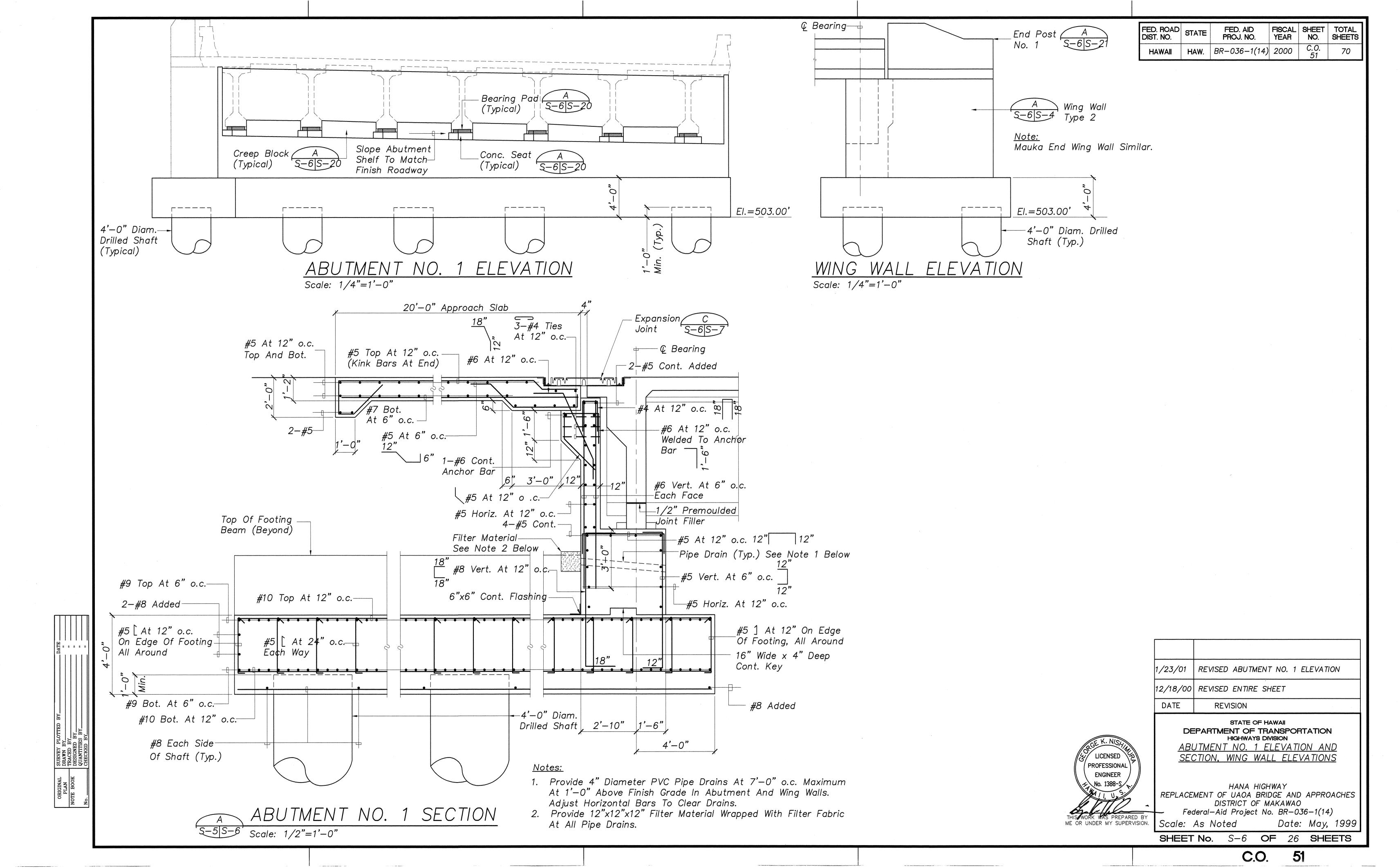
Pier Legs (Typ.)

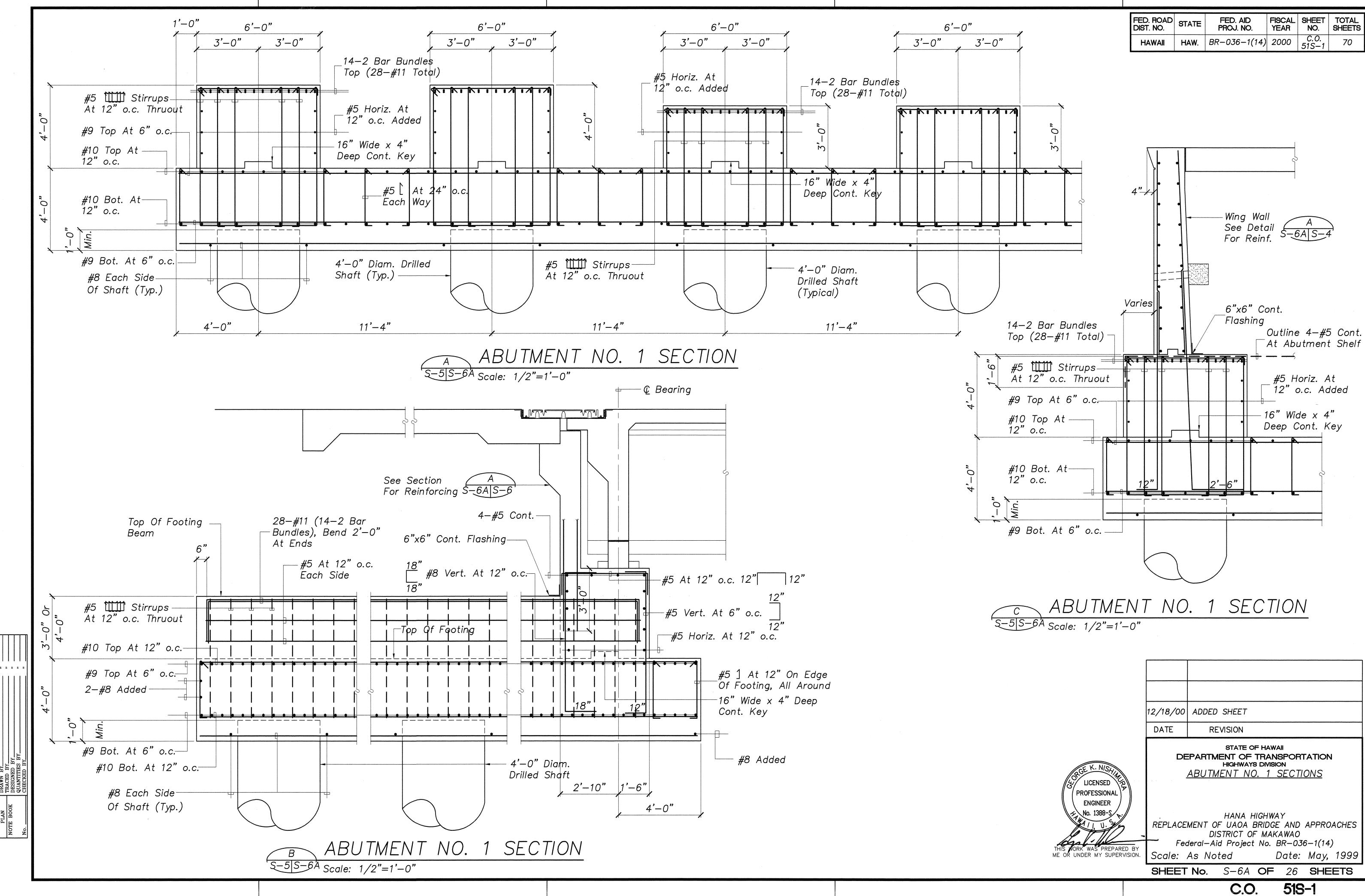
120'-0" (Along ℚ New Road And Bridge)

_Wing Wall

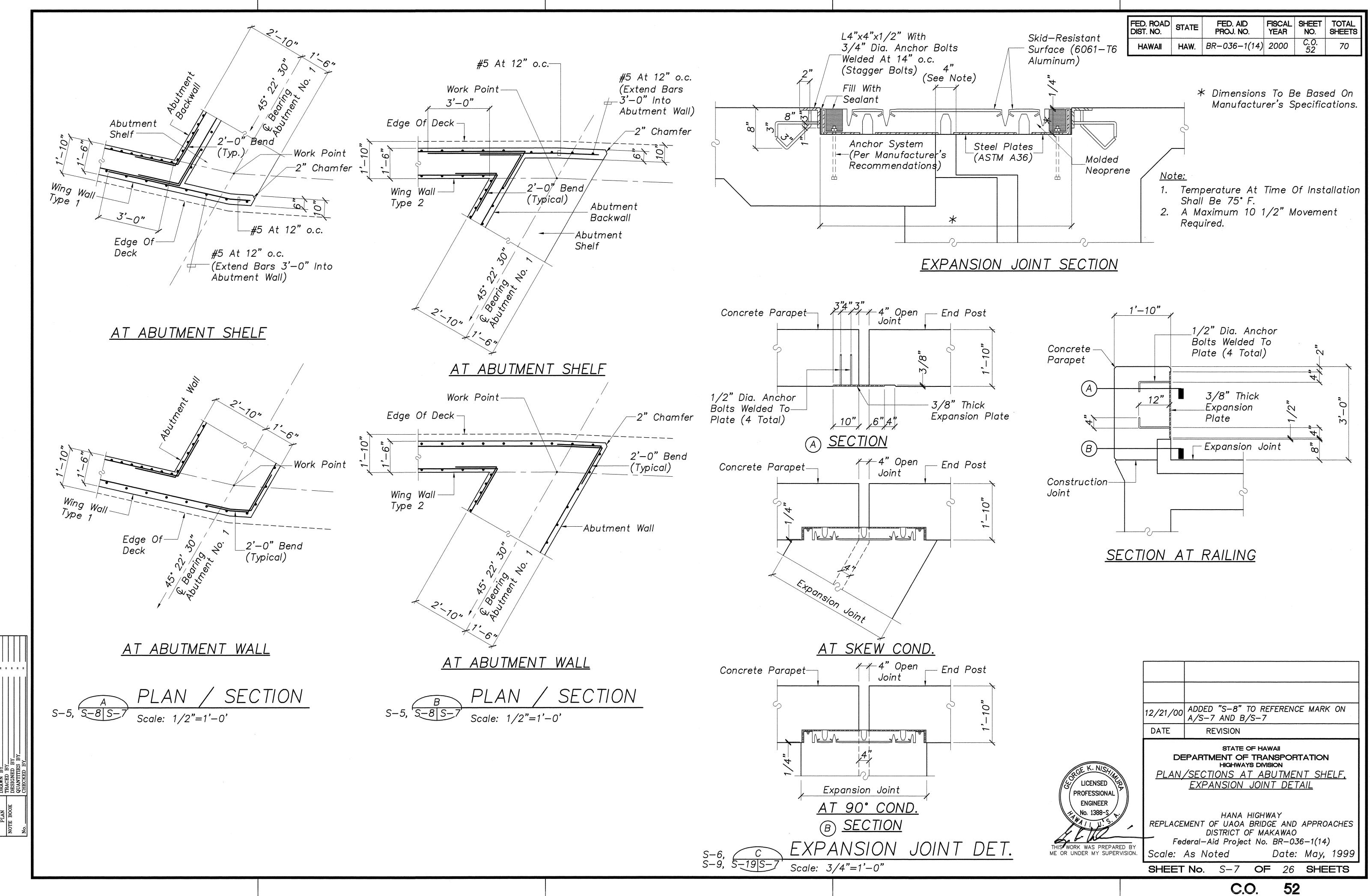
FISCAL SHEET TOTAL YEAR NO. SHEETS FED. ROAD STATE FED. AID PROJ. NO. HAW. BR-036-1(14) 2000 HAWAII

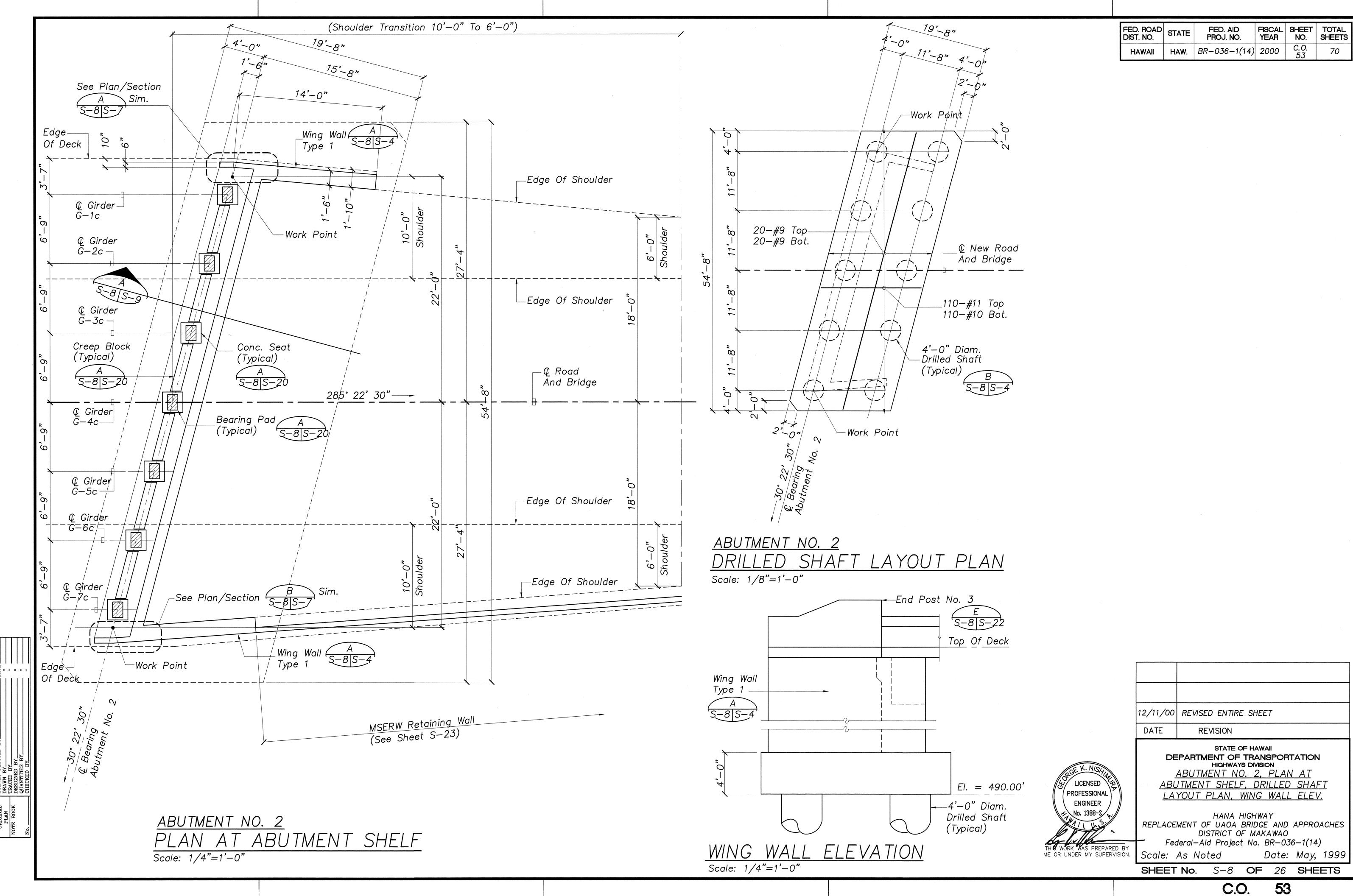


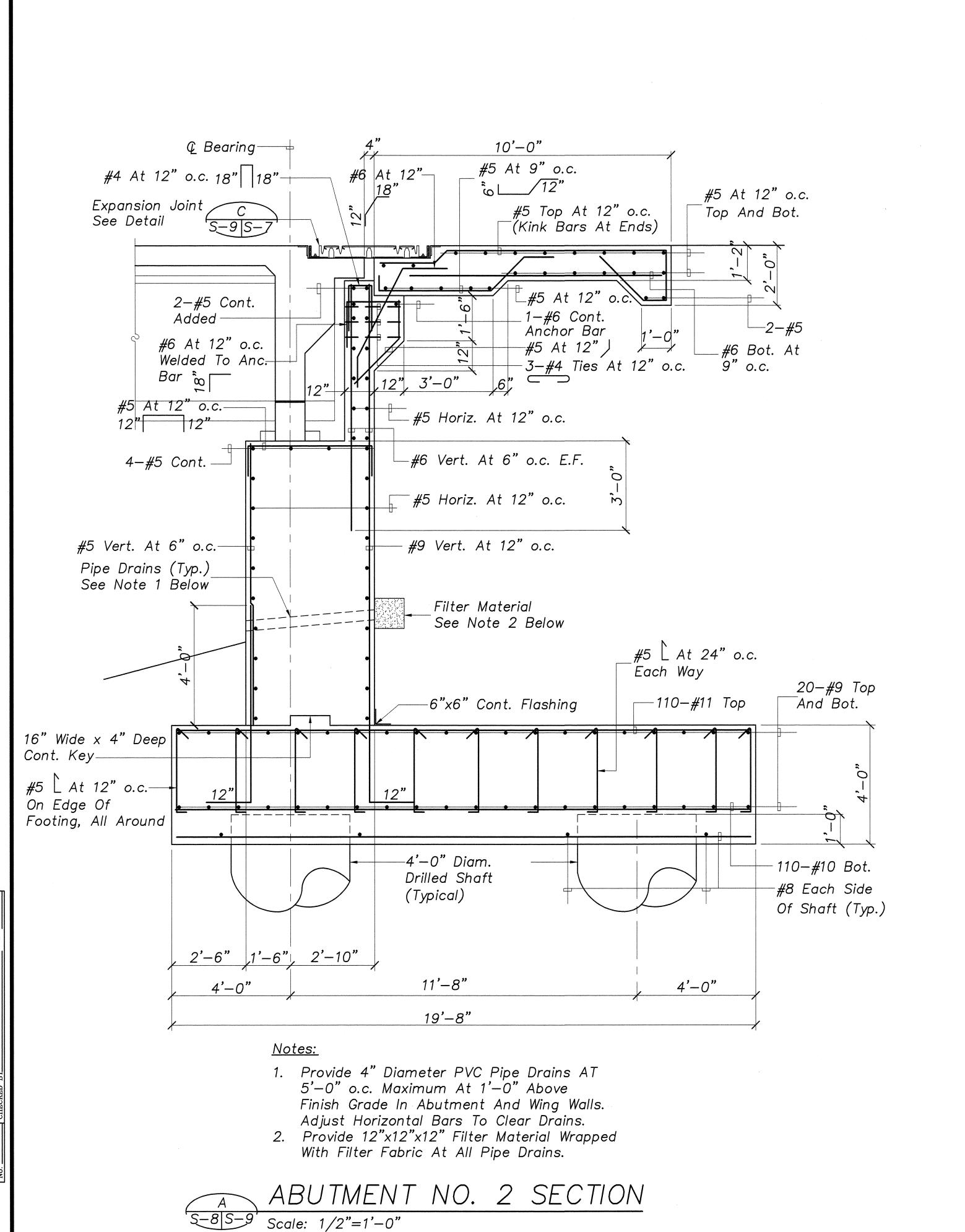




518-1







1/2"x1/2"x2'-0" F.B. Welded To Top Of 1'-0" Rectangular Tube 12"x2"x1/4" Rectangular Tube— SI. 1/2" ×1"

1. Kink deck slab top reinforcing <u>SECTION</u> at deck drain, as required.

2. Provide 2 sets of vertical bars (4" on center) in concrete parapet on each side of deck drain.

<u>Notes:</u>

—Concrete Parapet 1/2"x1/2"x2'-0" F.B. Welded To Top Of Rectangular Tube— 12"x2"x1/4" Rectangular Tube

<u>PLAN</u>

FED. ROAD STATE

HAWAII

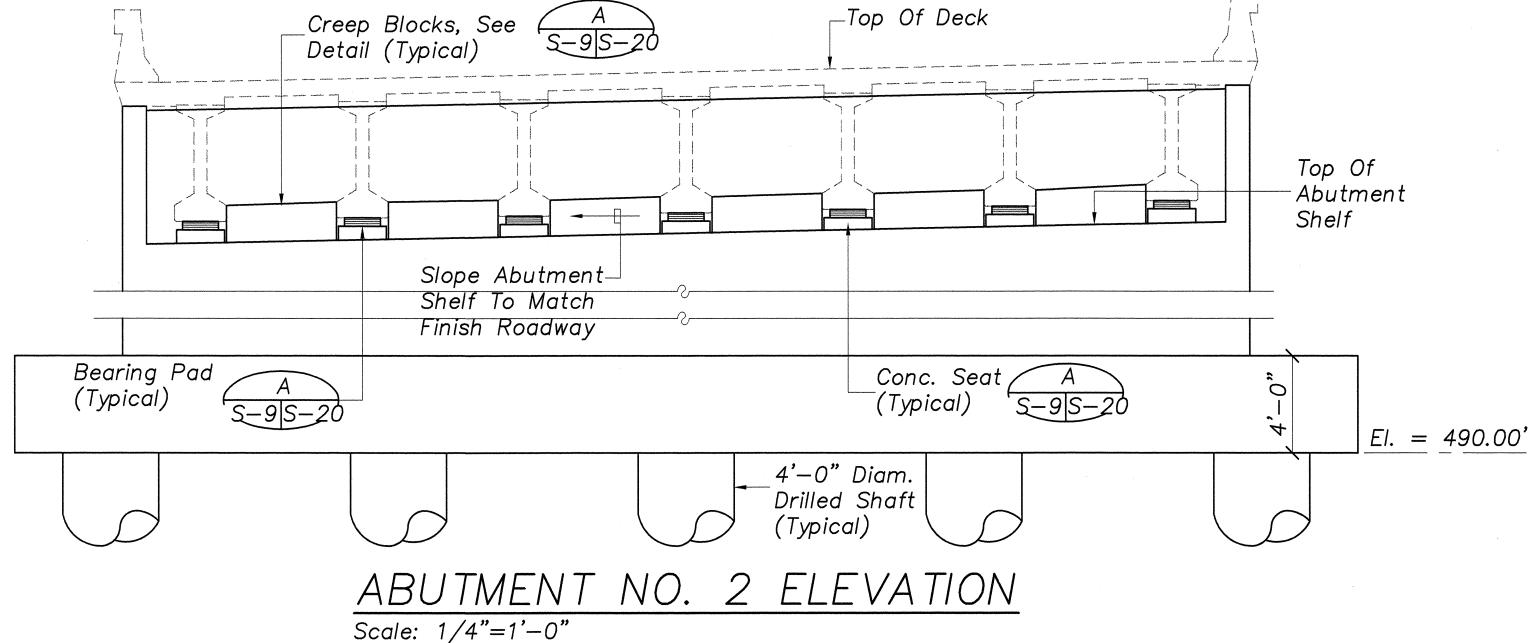
FED. AID PROJ. NO.

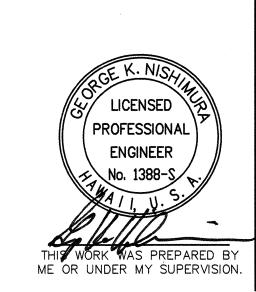
HAW. | BR-036-1(14) | 2000 |

FISCAL SHEET TOTAL YEAR NO. SHEETS

C.O. 54

TYPICAL DECK DRAIN DETAIL S-2, S-23 S-9Scale: 3/4"=1'-0"





1/23/01 | REVISED ABUTMENT NO. 2 ELEVATION 12/11/00 ABUTMENT NO. 2 ELEVATION AND SECTION REVISED DATE REVISION

> STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

ABUTMENT NO. 2 ELEVATION AND SECTION

HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES DISTRICT OF MAKAWAO Federal-Aid Project No. BR-036-1(14) Scale: As Noted Date: May, 1999

SHEET No. S-9 **OF** 26 **SHEETS**

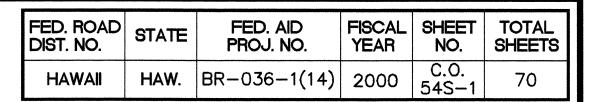
C.O. 54

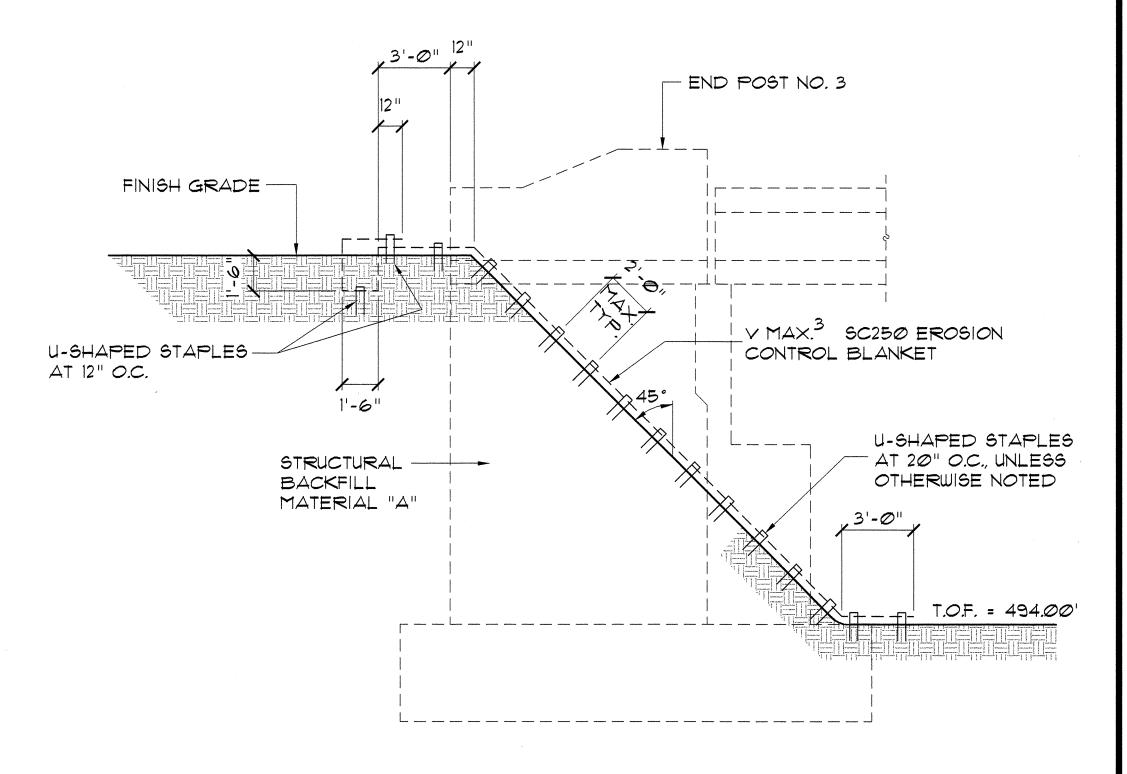
1. MATERIALS:

- A. GEOWEB WALL SECTIONS (GW30V) SHALL BE 6-INCHES DEEP AND HAVE A NONPERFORATED WALL FACE AND PERFORATED INTERIOR CELLS. THE COLOR OF THE WALL FACE SHALL BE APPROVED BY THE ENGINEER. THE GEOWEB WALL SECTIONS (GW30V) SHALL BE AS SUPPLIED BY PRESTO PRODUCTS COMPANY OR AN APPROVED EQUIVALENT.
- GEOSYNTHETIC REINFORCEMENT SHALL BE "STRATAGRID 300" AS SUPPLIED BY STRATA SYSTEMS, INC. OR AN APPROVED EQUIVALENT.
- EROSION CONTROL BLANKET SHALL BE "VMAX3 SC250" AS SUPPLIED BY NORTH AMERICAN GREEN OR AN APPROVED EQUIVALENT.
- STAPLES FOR EROSION CONTROL BLANKET SHALL BE 11 GAUGE WIRE MINIMUM WITH A 1-INCH CROWN AND 6-INCH LEGS, AS SUPPLIED BY NORTH AMERICAN GREEN OR AN APPROVED EQUIVALENT. INSTALLATION OF THE STAPLES SHALL BE AS RECOMMENDED BY THE MANUFACTURER.
- STRUCTURE BACKFILL MATERIAL A SHALL BE AS SPECIFIED IN SECTION 703.20 - STRUCTURE BACKFILL MATERIAL OF THE HAWAII STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND PUBLIC WORKS CONSTRUCTION. 1994.
- GEOWEB REINFORCED WALL INSTALLATION:
 - EXCAVATE AND SHAPE FOUNDATION SOILS TO THE GRADES, ELEVATIONS AND DIMENSIONS SHOWN ON THE DRAWINGS.
 - B. UNLESS OTHERWISE NOTED, GEOWEB SECTIONS AND GEOSYNTHETIC REINFORCEMENT INSTALLATION SHALL BE AS RECOMMENDED BY THE MANUFACTURERS.
 - C. GEOWEB INSTALLATION:
 - 1) EXPAND GEOWEB SECTIONS, DIMENSIONED ACCORDING TO THE CONSTRUCTION DRAWINGS INTO POSITION USING STRETCHER FRAMES. TEMPORARY STAKES OR OTHER SUITABLE METHOD TO TEMPORARILY HOLD IN PLACE. INTERLEAF OR OVERLAP EDGES OF ADJACENT SECTIONS IN EACH LAYER, ACCORDING TO WHICH SIDE WALL PROFILES ABUT. ENSURE THAT ALL ADJOINING SECTIONS ARE FLUSH AT THE JOINTS AND ADJOINING CELLS ARE FULLY STAPLED.
 - PLACE SPECIFIED INFILL MATERIAL TO APPROXIMATELY 2-INCHES ABOVE THE CELL WALLS. COMPACT BACKFILL MATERIALS TO A MINIMUM 95% RELATIVE COMPACTION AND REMOVE EXCESS MATERIAL ABOVE THE CELL WALLS.
 - 3) PLACE SUBSEQUENT LAYERS WITH THE SPECIFIED SET BACK AND ALIGN GEOWEB SECTIONS TO AVOID OVERHANGING OF UPPER SECTIONS OVER LOWER SECTIONS.
 - WHERE DIFFERENT INFILL MATERIALS ARE SPECIFIED FOR THE OUTER CELLS (TOPSOIL AND VEGETATED), THE FOLLOWING PROCEDURES MAY BE USED.
 - A) COVER OUTER CELLS WITH REMOVABLE BOARD WHILE FILLING BACK CELLS. PLACE SPECIAL INFILL IN OUTER CELLS BEFORE ADVANCING TO THE NEXT LAYER.
 - LEAVE OUTER CELLS OPEN BUT USE EXTRA CARE TO FILL BACK CELLS AND AVOID EXCESSIVE SPILLAGE INTO OUTER CELLS. WHERE THE SPECIFIED OUTER CELLS INFILL AND VEGETATION WILL TOLERATE IT, SOME SPILLAGE OF GRANULAR INFILL INTO THE OUTER CELLS IS ACCEPTABLE.
 - GEOSYNTHETIC REINFORCEMENT INSTALLATION:
 - 1) INSTALL GEOSYNTHETIC REINFORCEMENT BETWEEN GEOWEB SECTIONS AT THE SPECIFIED ELEVATIONS. PLACE THE LEADING EDGE OF THE GEOSYNTHETIC AT THE FRONT FACE OF THE GEOWEB SECTION TO BE PLACED AND EXTEND THE SPECIFIED LENGTH OVER THE COMPACTED BACKFILL. ENSURE THAT THE GEOSYNTHETIC REINFORCEMENT IS PLACED WITH THE HIGH STRENGTH DIRECTION PERPENDICULAR TO THE WALL FACE.
 - THE GEOSYNTHETIC REINFORCEMENT SHALL BE CONTINUOUS IN THE LONGITUDINAL DIRECTION (FROM THE FRONT FACE OF THE GEOWEB SECTION TO THE BACK OF THE REINFORCED SOIL ZONE). NO SPLICES IN THE GEOSYNTHETIC REINFORCEMENT WILL BE PERMITTED IN THE LONGITUDINAL DIRECTION. IN THE TRANSVERSE DIRECTION, THE GEOSYNTHETIC REINFORCEMENT SHALL BE PLACED ADJACENT TO EACH OTHER WITHOUT OVERLAPPING.
 - PLACE AND INFILL THE NEXT GEOWEB SECTION ON TOP OF THE GEOSYNTHETIC REINFORCEMENT.
 - MANUALLY TENSION THE GEOSYNTHETIC AND HOLD TAUT WITH TEMPORARY STAKES (OR ANY OTHER METHOD) DURING PLACEMENT OF THE SPECIFIED BACKFILL SOIL.
 - 5) PLACE BACKFILL MATERIAL WITHIN REINFORCED SOIL ZONE IN 10-INCH LOOSE LIFTS AND COMPACT TO A MINIMUM 95% RELATIVE COMPACTION. ENSURE THAT THE GEOSYNTHETIC REINFORCEMENT REMAINS TAUT AND IS NOT DISPLACED WHILE PLACING THE BACKFILL MATERIAL. THE BEST METHOD FOR PLACING THE BACKFILL MATERIAL IS TO START AT THE BACK OF THE GEOWEB SECTIONS AND SPREADING TOWARDS THE BACK OF THE REINFORCED SOIL ZONE. SHAPE AND COMPACT THE BACKFILL LEVEL WITH THE TOP OF THE GEOWEB SECTIONS PRIOR TO PLACING THE NEXT LAYER.
 - PLACE AND COMPACT THE RETAINED SOILS BEHIND THE REINFORCED SOIL ZONE AS SPECIFIED IN THE STANDARD SPECIFICATIONS AND SPECIAL PROVISIONS.

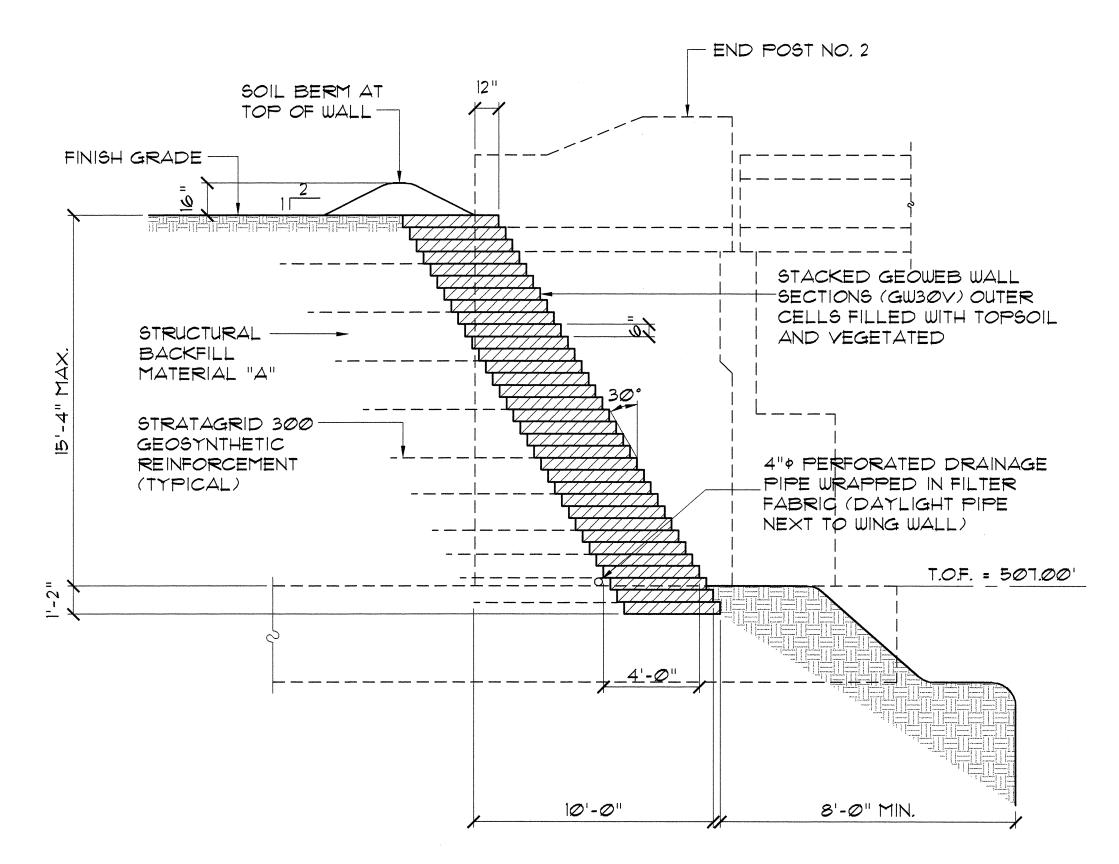
EROSION CONTROL BLANKET INSTALLATION:

- A. PREPARE THE SOIL BEFORE INSTALLING THE BLANKETS, INCLUDING ANY NECESSARY APPLICATION OF LIME, FERTILIZER, AND SEED.
 - BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN A 1'-6" DEEP BY 1'-6" WIDE TRENCH WITH APPROXIMATELY 2'-6" OF BLANKET EXTENDED BEYOND THE UP-SLOPE PORTION OF THE TRENCH. ANCHOR THE BLANKET WITH A ROW OF STAPLES SPACED 12-INCHES APART IN THE BOTTOM OF THE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING. APPLY SEED TO COMPACTED SOIL AND FOLD REMAINING 2'-6" PORTION OF BLANKET OVER SEED AND COMPACTED SOIL. SECURE BLANKET OVER COMPACTED SOIL WITH A ROW OF STAPLES SPACED 12-INCHES APART ACROSS THE WIDTH OF THE BLANKET.
 - ROLL THE BLANKETS DOWN THE SLOPE. BLANKETS WILL UNROLL WITH APPROPRIATE SIDE AGAINST THE SOIL SURFACE. ALL BLANKETS MUST BE SECURELY FASTENED TO THE SOIL SURFACE WITH THE SPACING AS SHOWN ON THE DRAWING.
 - THE EDGES OF PARALLEL BLANKETS SHALL BE PLACED WITH APPROXIMATELY 2-INCHES TO 5-INCHES OVERLAP AND SHALL BE STAPLED THROUGH OVERLAPPED AREA.
- CONSECUTIVE BLANKETS SPLICED DOWN THE SLOPE MUST BE PLACED END OVER END (SHINGLE STYLE) WITH AN APPROXIMATE 6-INCHES OVERLAP. STAPLE THROUGH THE OVERLAPPED AREA. APPROXIMATELY 12-INCHES APART ACROSS THE ENTIRE WIDTH OF THE BLANKET.
- ADDITIONAL STAPLES SHALL BE PLACED AS REQUIRED TO ENSURE THAT THE EROSION CONTROL BLANKET IS IN CONTACT WITH THE SOIL.
- THE SOILS ENGINEER OF RECORD SHALL BE NOTIFIED TO OBSERVE THE EXCAVATION AND BACKFILL PLACEMENT FOR THE GEOWEB REINFORCED WALL.

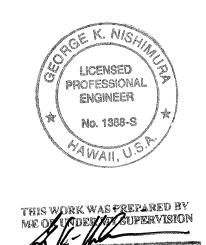




ABUTMENT NO. 2 SECTION AT EROSION CONTROL BLANKET SCALE: 1/4"=1'-0" S-1



ABUTMENT NO. 1 SECTION AT GEOWEB REINFORCED WALL SCALE: 1/4"=1'-0"S-1



04 30 04

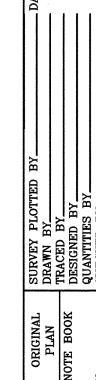
STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

GEOWEB REINFORCED WALL

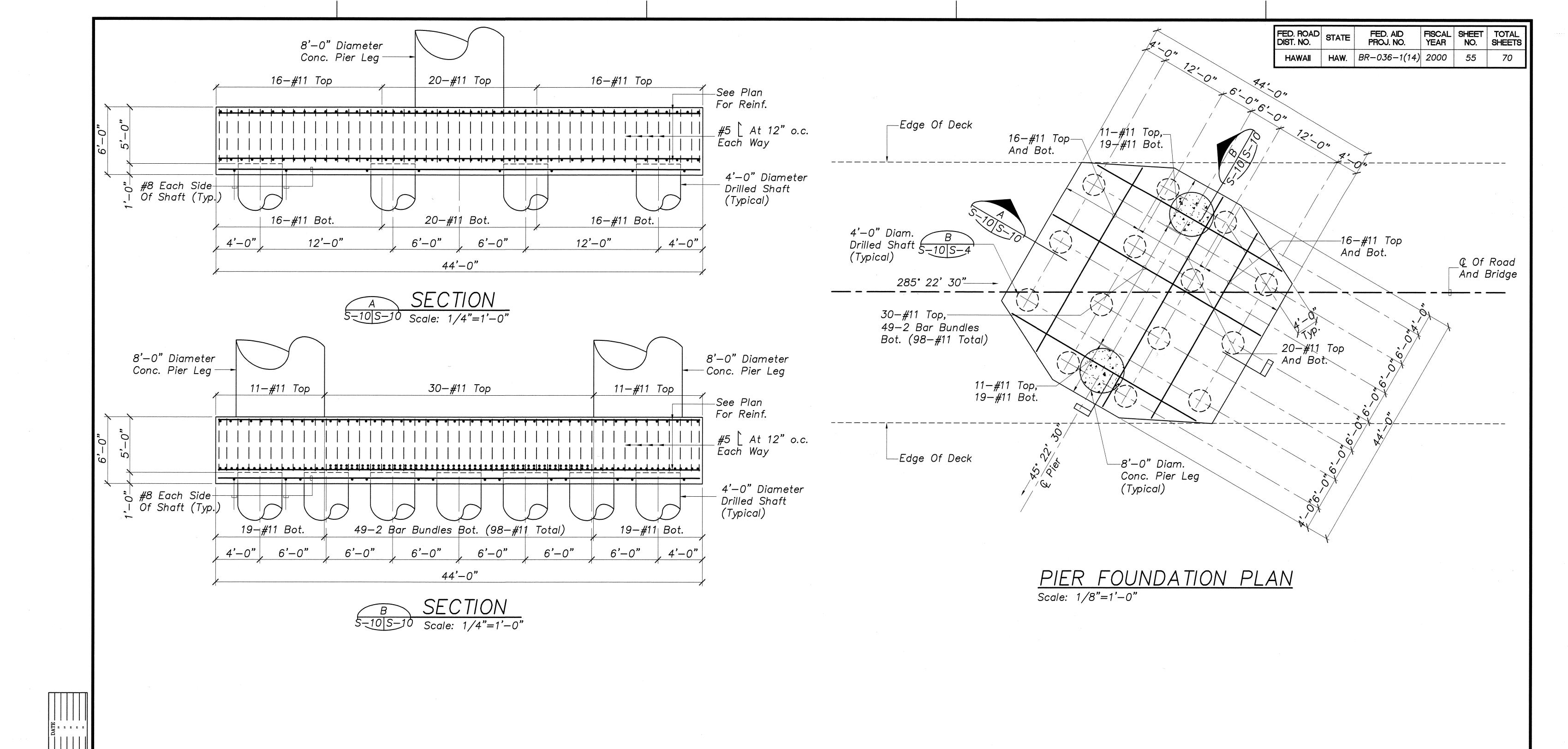
HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES DISTRICT OF MAKAWAO Federal-Aid Project No. BR-036-1(14)

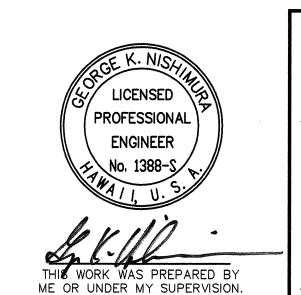
Scale: As Noted Date: May, 2002

SHEET No. S-9A OF 26 SHEETS



E::::





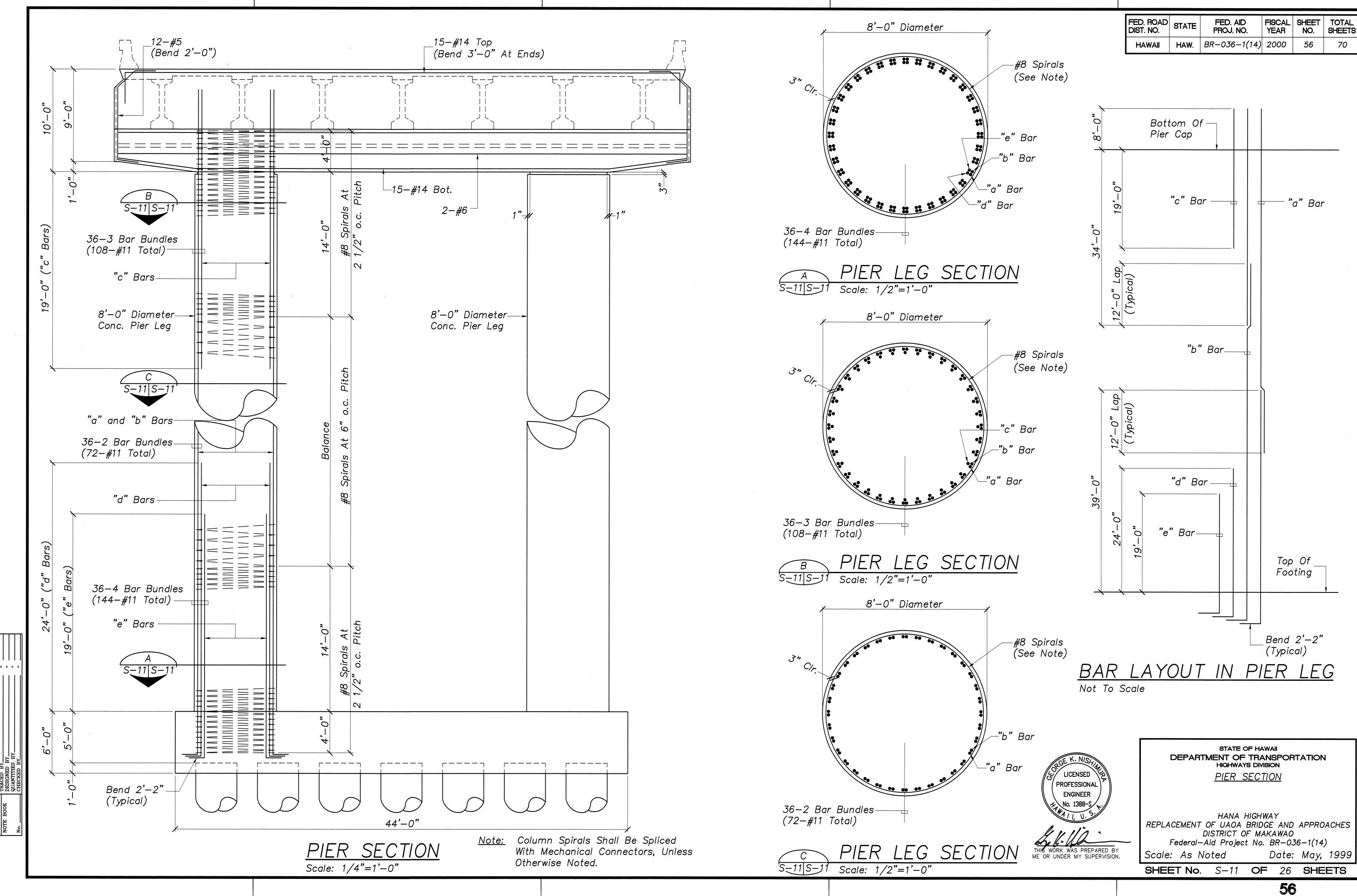
STATE OF HAWAII

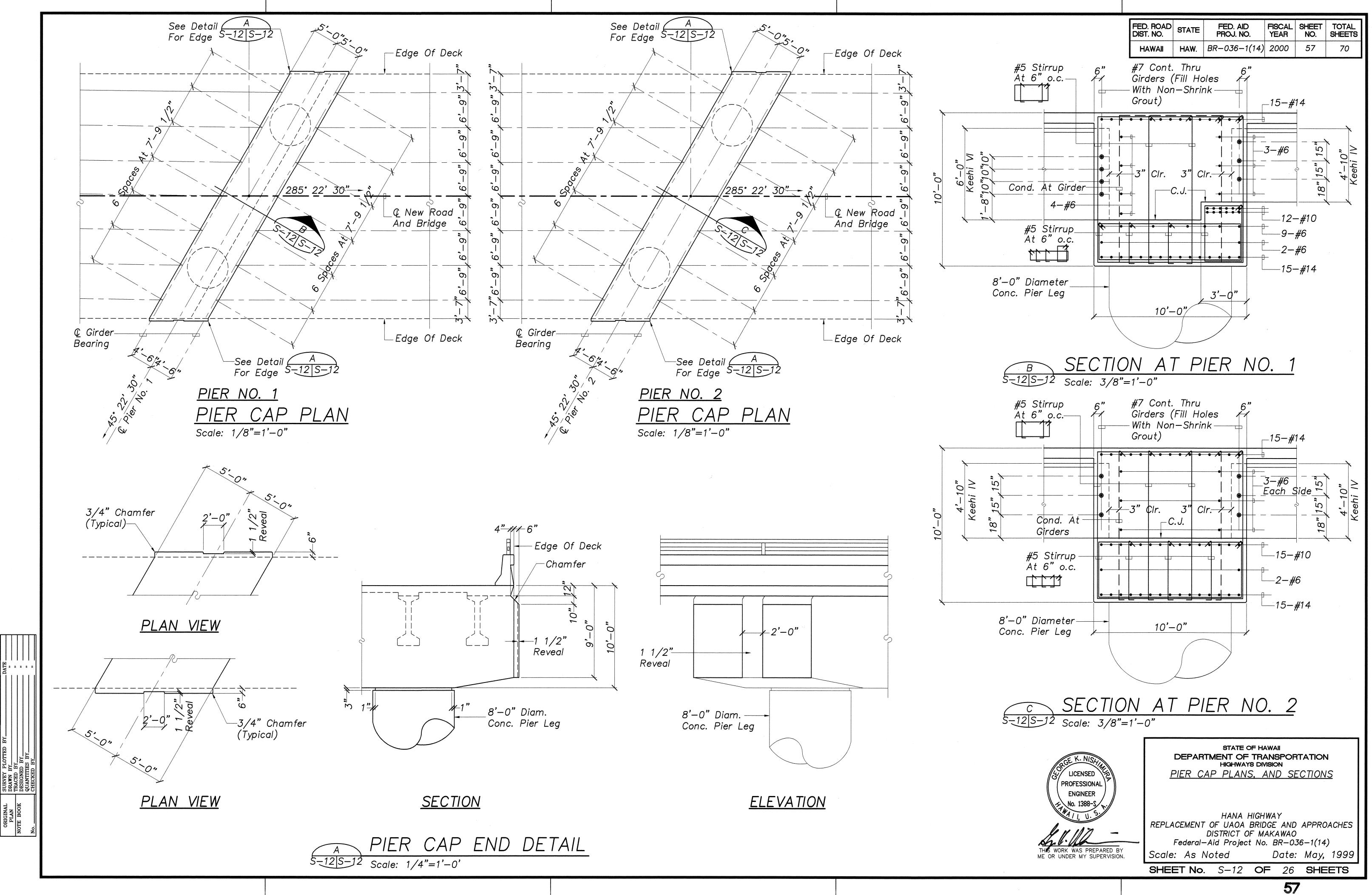
DEPARTMENT OF TRANSPORTATION

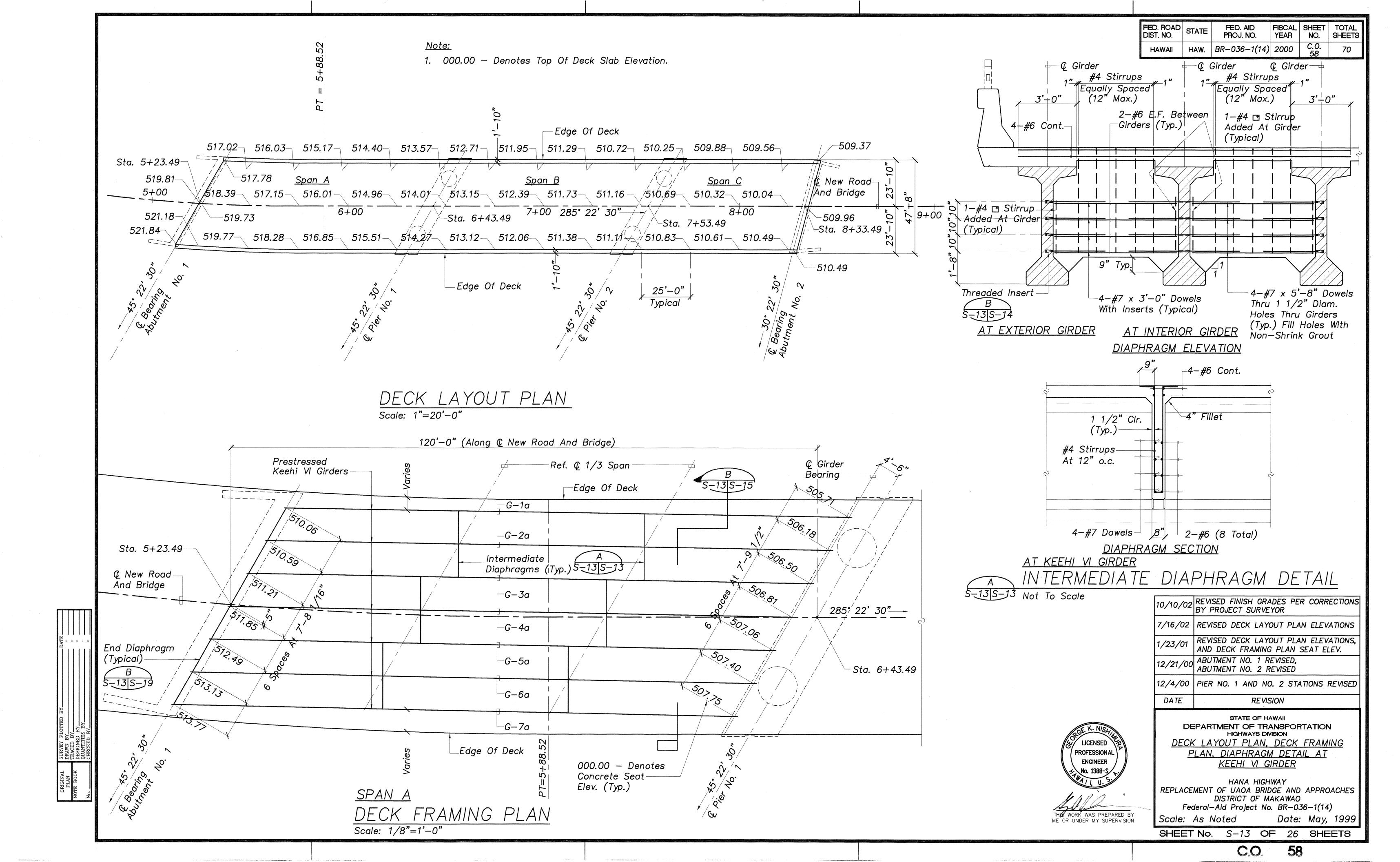
HIGHWAYS DIVISION

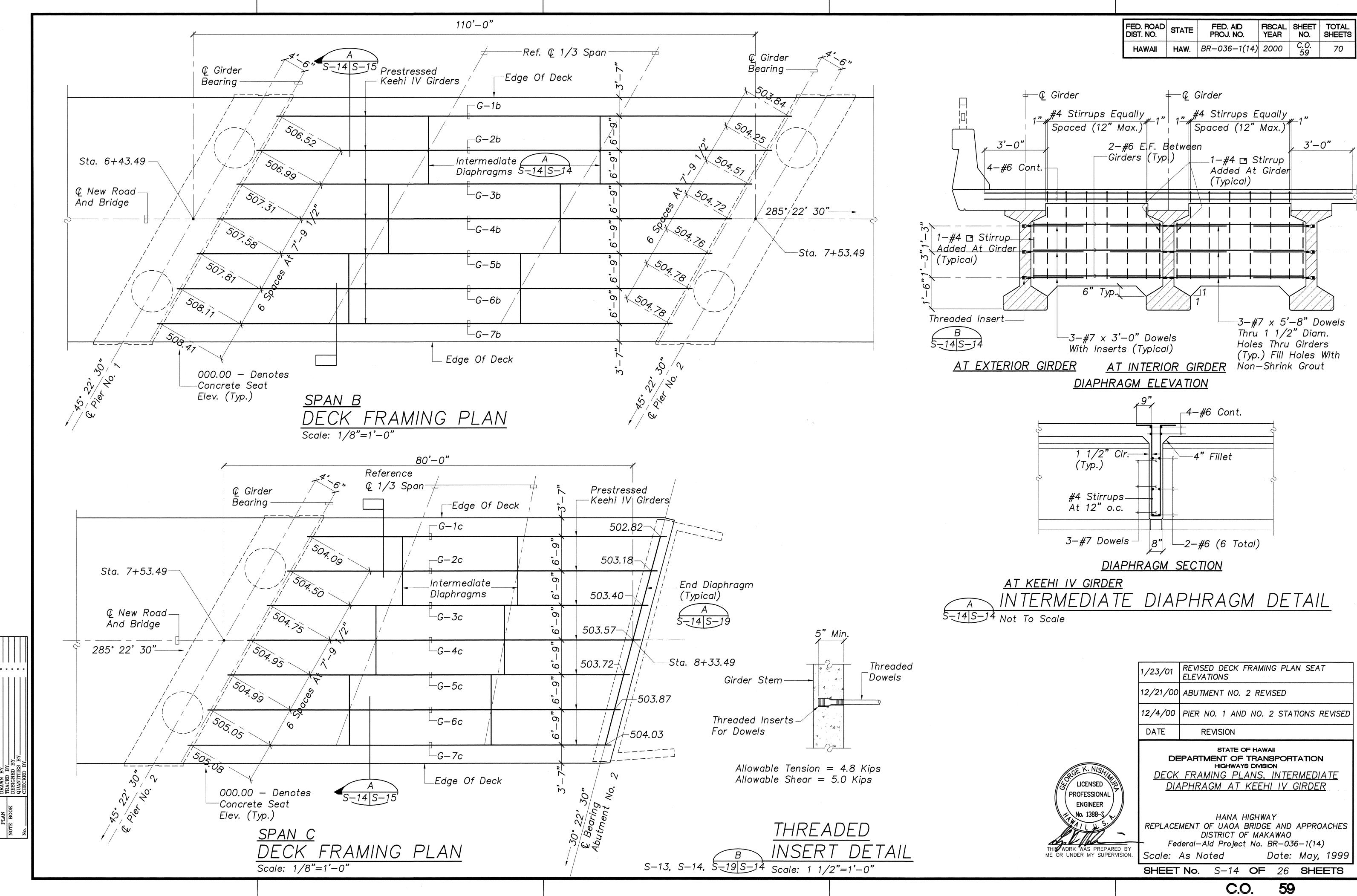
PIER FOUNDATION PLAN AND SECTIONS

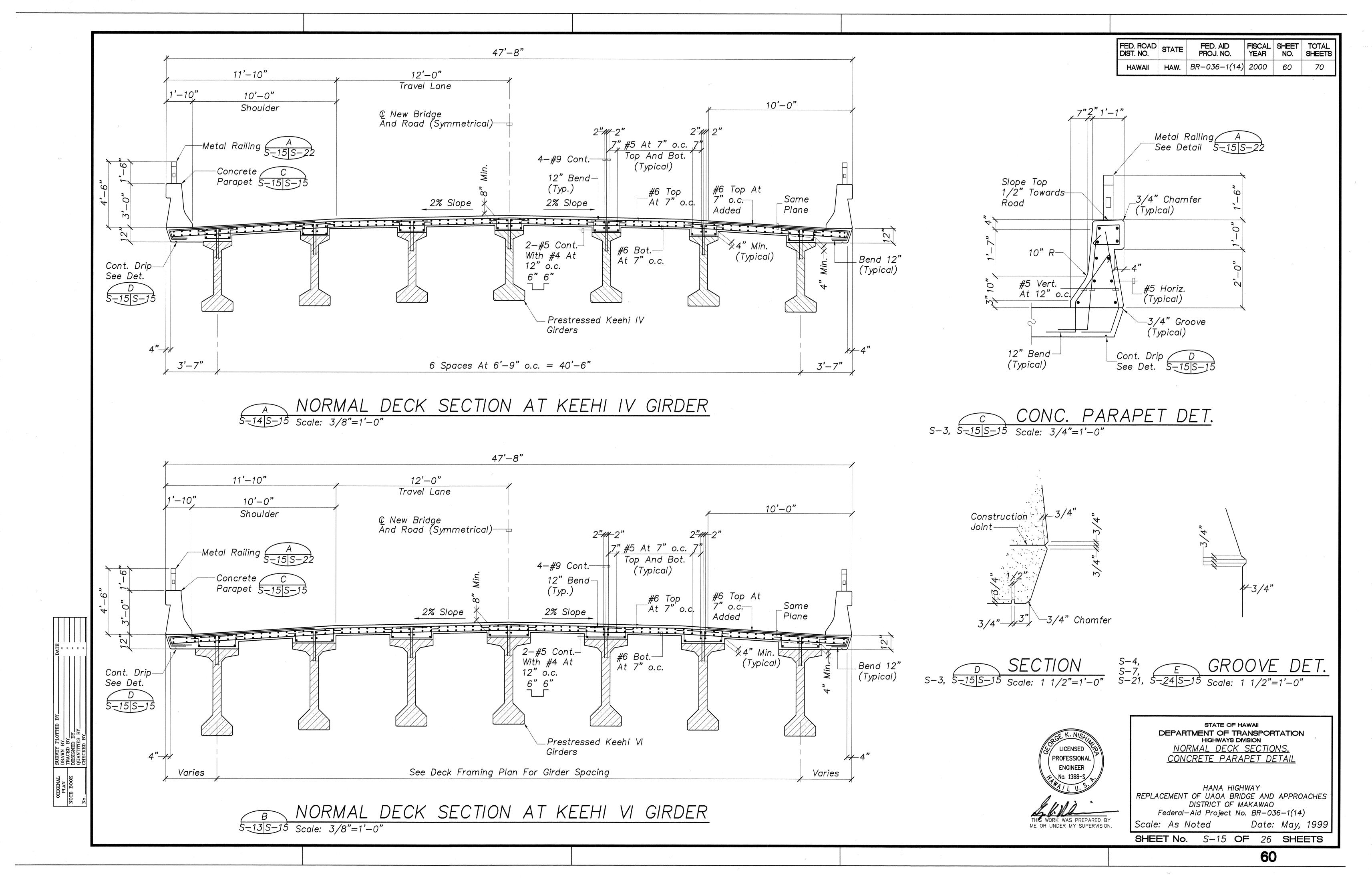
HANA HIGHWAY
REPLACEMENT OF UAOA BRIDGE AND APPROACHES
DISTRICT OF MAKAWAO
Federal-Aid Project No. BR-036-1(14)
Scale: As Noted Date: May, 1999
SHEET No. S-10 OF 26 SHEETS

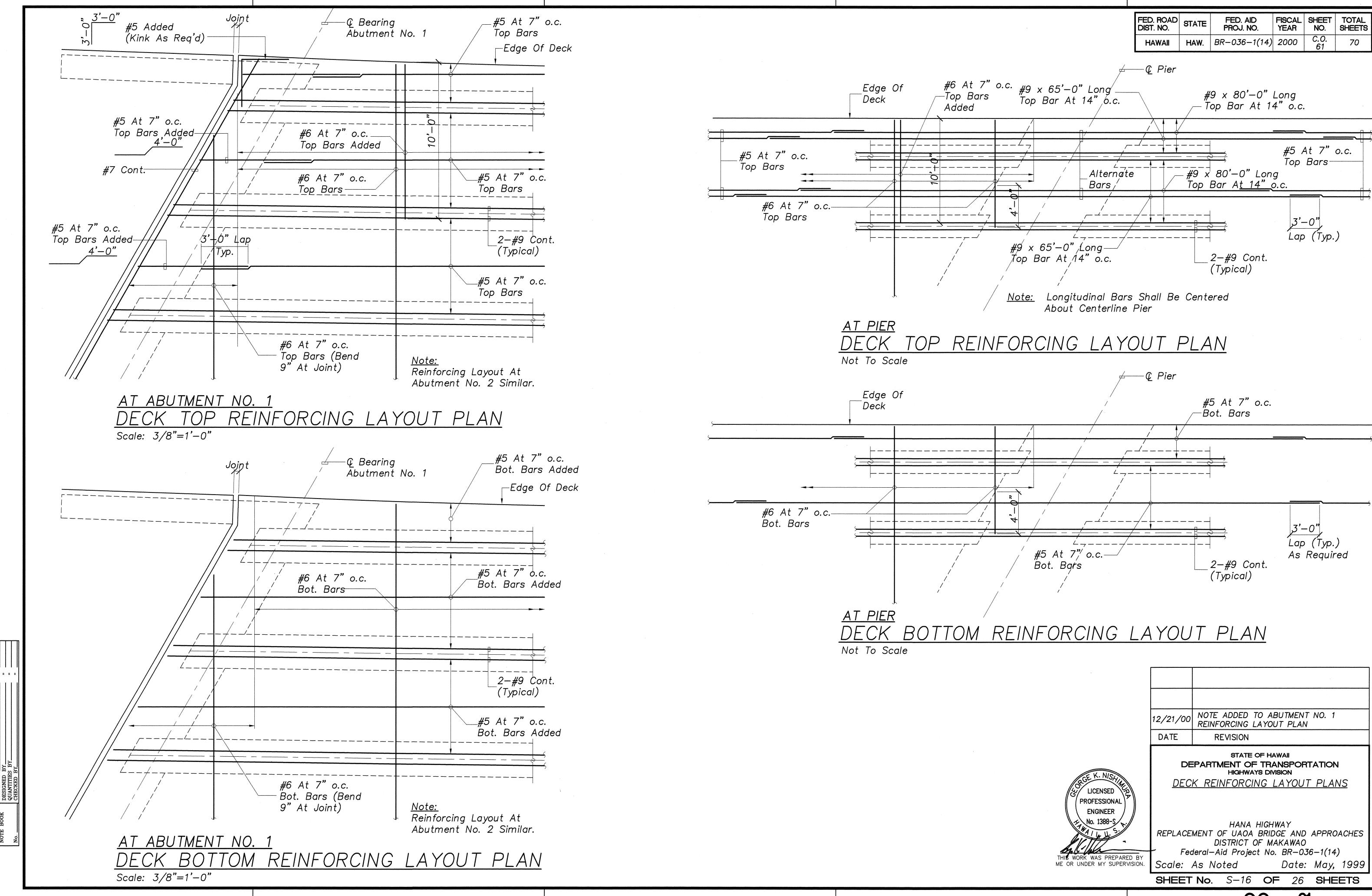




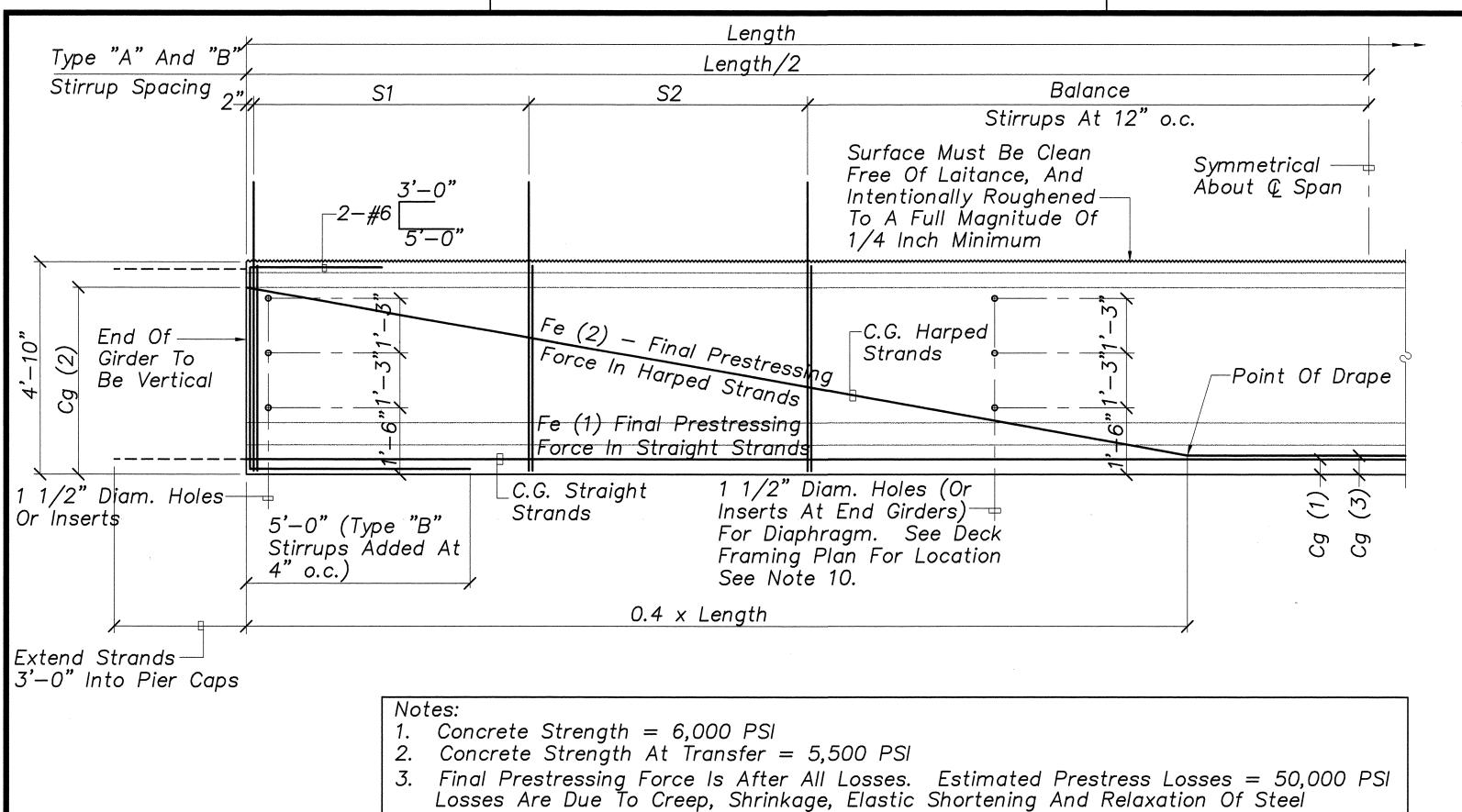








C.O. 61



TYPICAL PRESTRESSED GIRDER ELEVATION (KEEHI IV) Not To Scale

−See Note "1a" #5 ← Ties At 12" -4-#6 Cont. #5 Stirrups In 8 3/4" 1" Clr. (*Typ.*) -2-#6 At Ends[4,-10" -3/4" R Same-Plane Type "B" 2'-0" Same Plane −3/4" Chamfer Bend 1'-0" (Typ.)× 6 1/2" 9 3/4" 9 3/4" 2'-2"

2'-0"

Note "1a" Bend Bars In Field 2" Below Top Of Deck. All Reinforcing Dimensions Are Out To Out

TYPICAL GIRDER SECTION Scale: 3/4"=1'-0"

FED. ROAD STATE FISCAL SHEET TOTAL YEAR NO. SHEETS FED. AID PROJ. NO. HAW. BR-036-1(14) 2000 HAWAII

Prestress Girder Notes:

Girders.

- 1. Prestress Strands Shall Be Seven Wire 1/2" Diameter Low Relaxation Steel Strands (Area = 0.153 Sq. In.), With An Ultimate Strength Of 270 KSI. For Properties, See State Standard Specifications.
- 2. Non-Prestressed Reinforcing Steel Shall Be Grade 60, Unless Otherwise Noted On Plans, For Properties See State Standard Specifications.
- 3'-0" 3. Dead Load Deflection Includes The Combine Effects Of The Weight Of Slab, Haunch And Diaphragms.
 - Strand Pattern Shall Be Symmetrical About The Longitudinal Centerline Of The Girders.
 - 5. Strand Release Sequence Shall Not Include Any Lateral Deflection Of The Girder.
 - 6. The Contractor Shall Submit His Proposed Strand Pattern And Releasing Sequence To The Engineer For Approval.
 - 7. During Curing, Care Shall Be Taken To Avoid Any Lateral Deflection Of the Girder Due To Improper Orientation.
 - 8. Lifting Devices Shall Be Placed As Close As Possible To The Centerline Bearing Of The Girder. Details And Locations Of Lifting Devices Shall Be Submitted To The Engineer For Approval. Such Approval Does Not Relieve The Contractor Of His Responsibilities If The Girder Is Damaged Due To Failure Of The Lifting Devices.
 - 9. Length Of Girders Do Not Include Effect Of Elastic Shortening. 10. The Contractor Shall Incorporate All Holes, Inserts And Other Embedded Items Required In Girders During Fabrication Of The
 - 11. The Calculated Camber Includes The Effects Of The Initial Prestress Force And The Weight Of The Girder After Removal From The Bed. Negative Values Indicate A Net Upward Deflection. The Calculated Camber Value Has Been Multiplied By A Factor Of 2.0 To Approximate The Effect Of Camber Growth And Concrete Creep. The Actual Camber Shall Not Exceed The Calculated Camber By More Than 1/2".

© Bearing	Calculated Camber Dead Load See Note 11 See Note 3	ring

	KEEHI	IV PRES	STRESSEL) GIRDER	SCHEDU	JLE	
Girder Mark	Fe (1)	Fe (2)	Cg (1)	Cg (2)	Cg (3)	S1	S2
G-1b G-2b G-3b G-4b G-5b G-6b G-7b	744k	326k	10.25"	46.00"	8.00"	60 At 4"	32 At 6"
G-5c G-6c G-7c	552k	322k	8.00"	48.00"	8.00"	36 At 4"	30 At 6"
G-1c G-2c G-3c G-4c	506k	276k	8.18"	49.00"	7.00"	36 At 4"	30 At 6"

GIF	GIRDER CAMBER AND DEAD LOAD DEFLECTION TABLE							
Girder Mark	Calculated Camber	Dead Load Deflection	Girder Mark	Calculated Camber	Dead Load Deflection			
G-1b	-2.68"	1.54"	G-1c	-1.24"	0.47"			
G-2b	-2.68"	1.54"	G-2c	-1.26"	0.45"			
G-3b	-2.68"	1.54"	G-3c	-1.28"	0.50"			
G-4b	<i>−2.68</i> "	1.54"	G-4c	-1.32"	0.54"			
G-5b	-2.68"	1.54"	G-5c	-1.56 "	0.59"			
G-6b	-2.68"	1.54"	G-6c	-1.60"	0.64"			
G-7b	<i>−2.68</i> "	1.54"	G-7c	-1.60"	0.79"			

GIRDER CAMBER DIAGRAM

Not To Scale

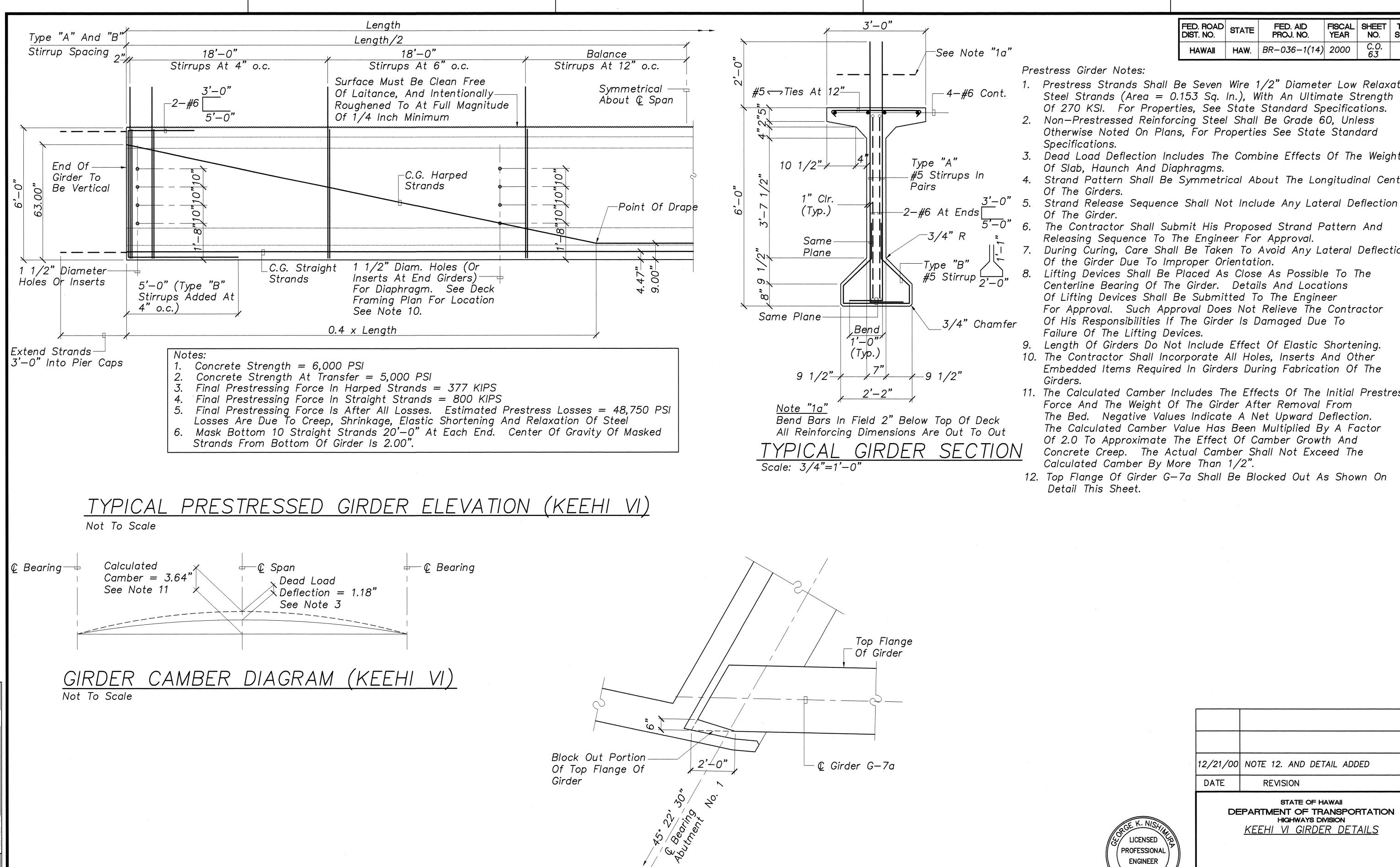
LICENSED THIS WORK WAS PREPARED BY

12/21/00 GIRDER G-1C TO G-7C REVISED **REVISION**

> STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION KEEHI IV GIRDER DETAILS

HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES DISTRICT OF MAKAWAO Federal-Aid Project No. BR-036-1(14) Date: May, 1999 Scale: As Noted

SHEET No. S-17 OF 26 SHEETS



Not To Scale

BLOCK OUT DETAIL AT GIRDER G-7a

FED. ROAD STATE FISCAL SHEET TOTAL YEAR NO. SHEETS FED. AID PROJ. NO. HAW. | BR-036-1(14) 2000

1. Prestress Strands Shall Be Seven Wire 1/2" Diameter Low Relaxation Steel Strands (Area = 0.153 Sq. In.), With An Ultimate Strength Of 270 KSI. For Properties, See State Standard Specifications.

2. Non-Prestressed Reinforcing Steel Shall Be Grade 60, Unless Otherwise Noted On Plans, For Properties See State Standard

3. Dead Load Deflection Includes The Combine Effects Of The Weight Of Slab, Haunch And Diaphragms.

4. Strand Pattern Shall Be Symmetrical About The Longitudinal Centerline

The Contractor Shall Submit His Proposed Strand Pattern And Releasing Sequence To The Engineer For Approval.

7. During Curing, Care Shall Be Taken To Avoid Any Lateral Deflection Of the Girder Due To Improper Orientation.

Lifting Devices Shall Be Placed As Close As Possible To The Centerline Bearing Of The Girder. Details And Locations Of Lifting Devices Shall Be Submitted To The Engineer For Approval. Such Approval Does Not Relieve The Contractor Of His Responsibilities If The Girder Is Damaged Due To Failure Of The Lifting Devices.

9. Length Of Girders Do Not Include Effect Of Elastic Shortening. 10. The Contractor Shall Incorporate All Holes, Inserts And Other Embedded Items Required In Girders During Fabrication Of The

11. The Calculated Camber Includes The Effects Of The Initial Prestress Force And The Weight Of The Girder After Removal From The Bed. Negative Values Indicate A Net Upward Deflection. The Calculated Camber Value Has Been Multiplied By A Factor Of 2.0 To Approximate The Effect Of Camber Growth And Concrete Creep. The Actual Camber Shall Not Exceed The Calculated Camber By More Than 1/2".

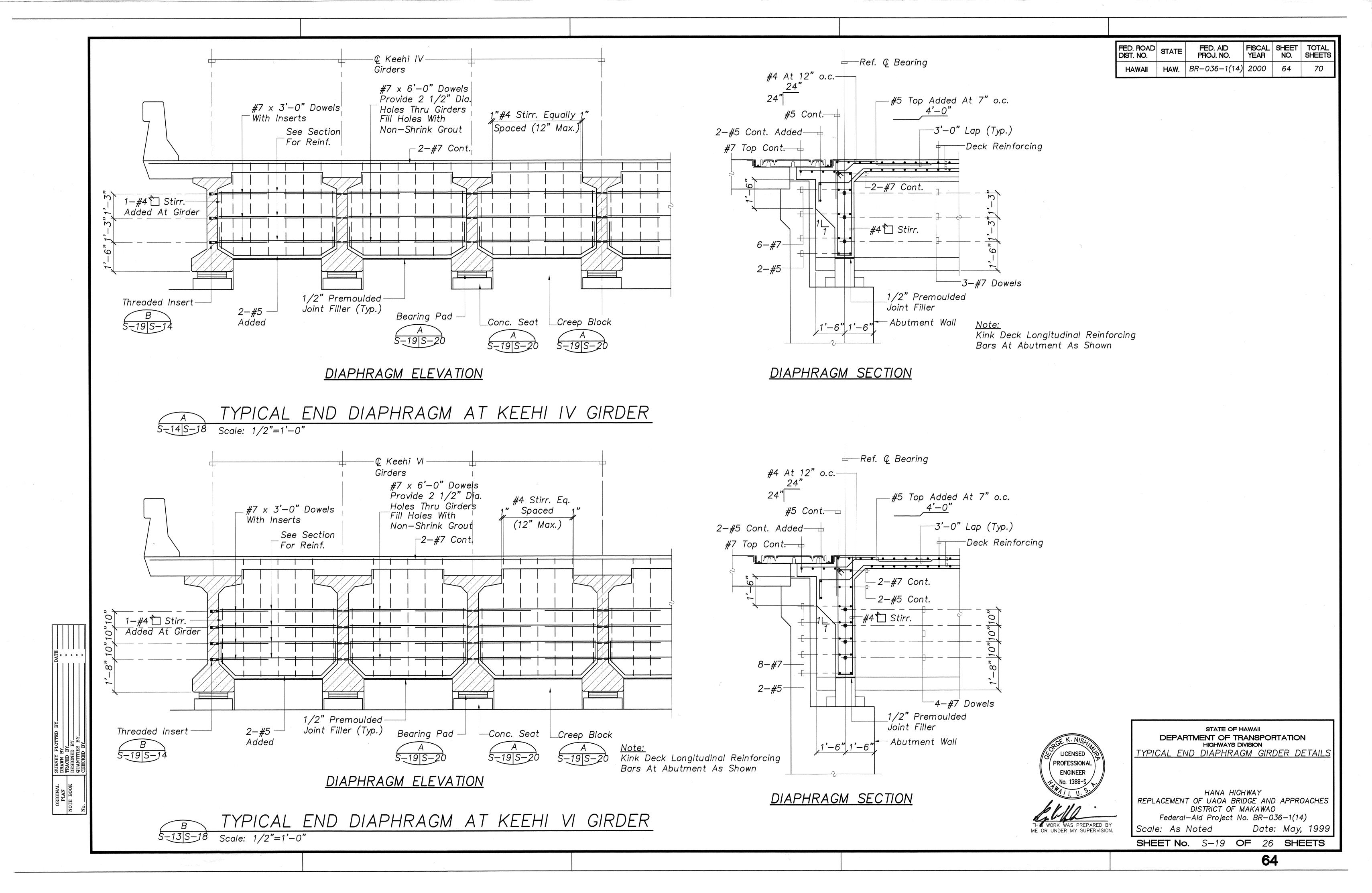
12. Top Flange Of Girder G-7a Shall Be Blocked Out As Shown On Detail This Sheet.

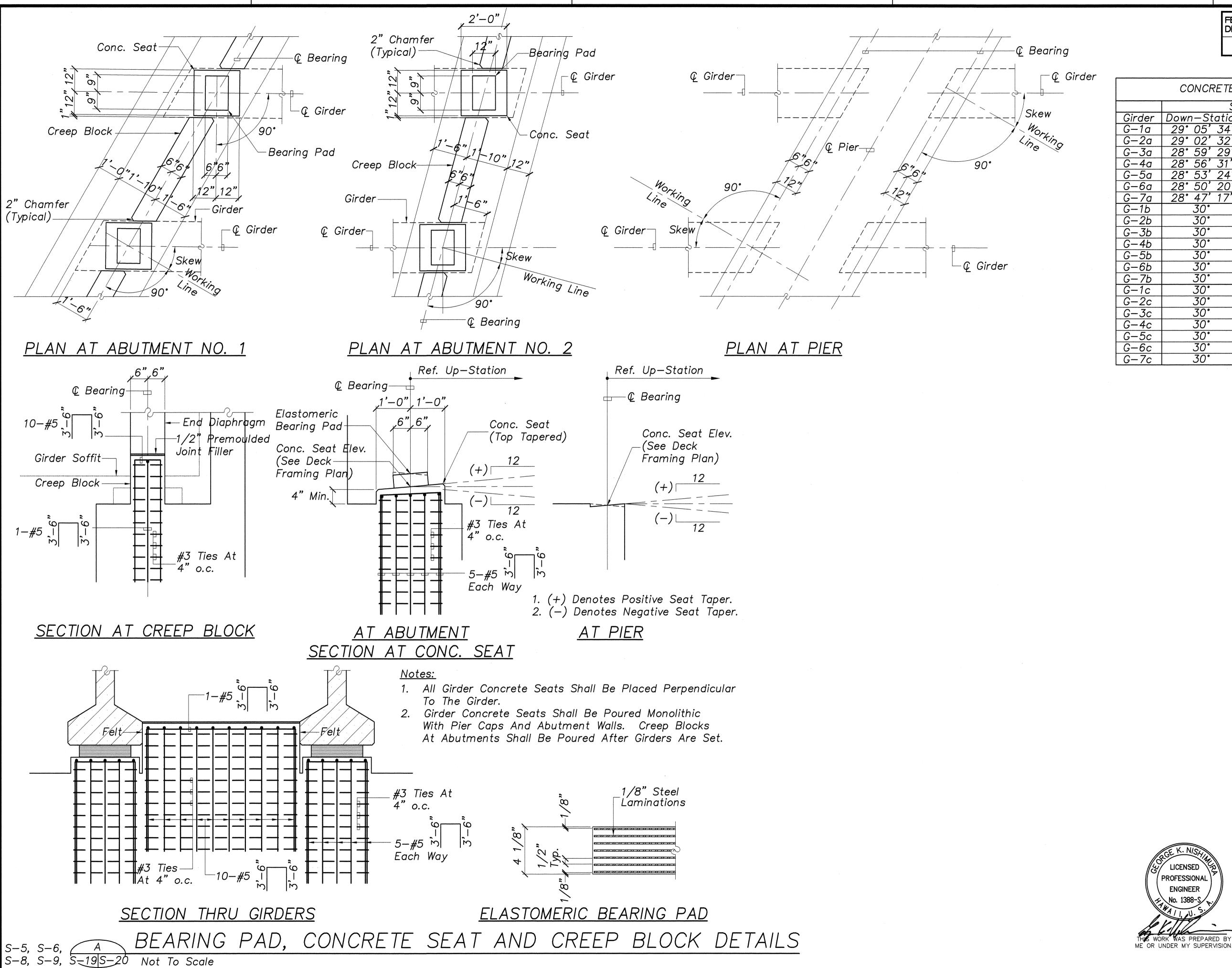
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

12/21/00 NOTE 12. AND DETAIL ADDED DATE REVISION STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION KEEHI VI GIRDER DETAILS

HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES DISTRICT OF MAKAWAO Federal-Aid Project No. BR-036-1(14) Date: May, 1999 Scale: As Noted

SHEET No. S-18 OF 26 SHEETS





FED. ROAD	STATE	FED. AID	FISCAL	SHEET	TOTAL
DIST. NO.		PROJ. NO.	YEAR	NO.	SHEETS
HAWAII	HAW.	BR-036-1(14)	2000	C.O. 65	70

CONCRETE SEAT SKEW AND TAPER TABLE							
	Sk	ew	Taper				
Girder	Down-Station		Down-Station	Up-Station			
G-1a	29° 05' 34"	29° 05' 34"	-7/16 : 12	<i>−7/</i> 16 : 12			
G-2a	29° 02' 32"	29° 02' 32"	-7/16:12	-7/16:12			
G-3a	28° 59' 29"	28° 59' 29"	-1/2:12	-1/2:12			
G-4a	28° 56′ 31″	28° 56′ 31″	-1/2:12	-1/2:12			
G-5a	28° 53′ 24″	28° 53′ 24″	-9/16:12	-9/16:12			
G-6a	28° 50' 20"	28° 50′ 20″	-5/8:12	-5/8:12			
G-7a	28° 47' 17"	28° 47' 17"	-5/8:12	-5/8:12			
G-1b	30°	30°	-5/16:12	-5/16:12			
G-2b	30°	30°	-5/16:12	-5/16:12			
G-3b	30°	30°	-5/16:12	-5/16:12			
G-4b	30°	30°	-3/8:12	-3/8:12			
G-5b	30°	30°	-3/8:12	-3/8:12			
G-6b	30°	30°	-3/8:12	-3/8:12			
G-7b	30°	30°	-7/16:12	-7/16:12			
G-1c	30°	15°	-1/4:12	-1/4:12			
G-2c	30°	15°	-1/4:12	-1/4:12			
G-3c	30°	15°	-1/4:12	-1/4:12			
G-4c	30°	15°	-1/4:12	-1/4:12			
G-5c	30°	15°	-3/16:12	-3/16:12			
G-6c	30°	15°	-3/16:12	-3/16:12			
G-7c	<i>30°</i>	15°	-3/16 : 12	-3/16:12			

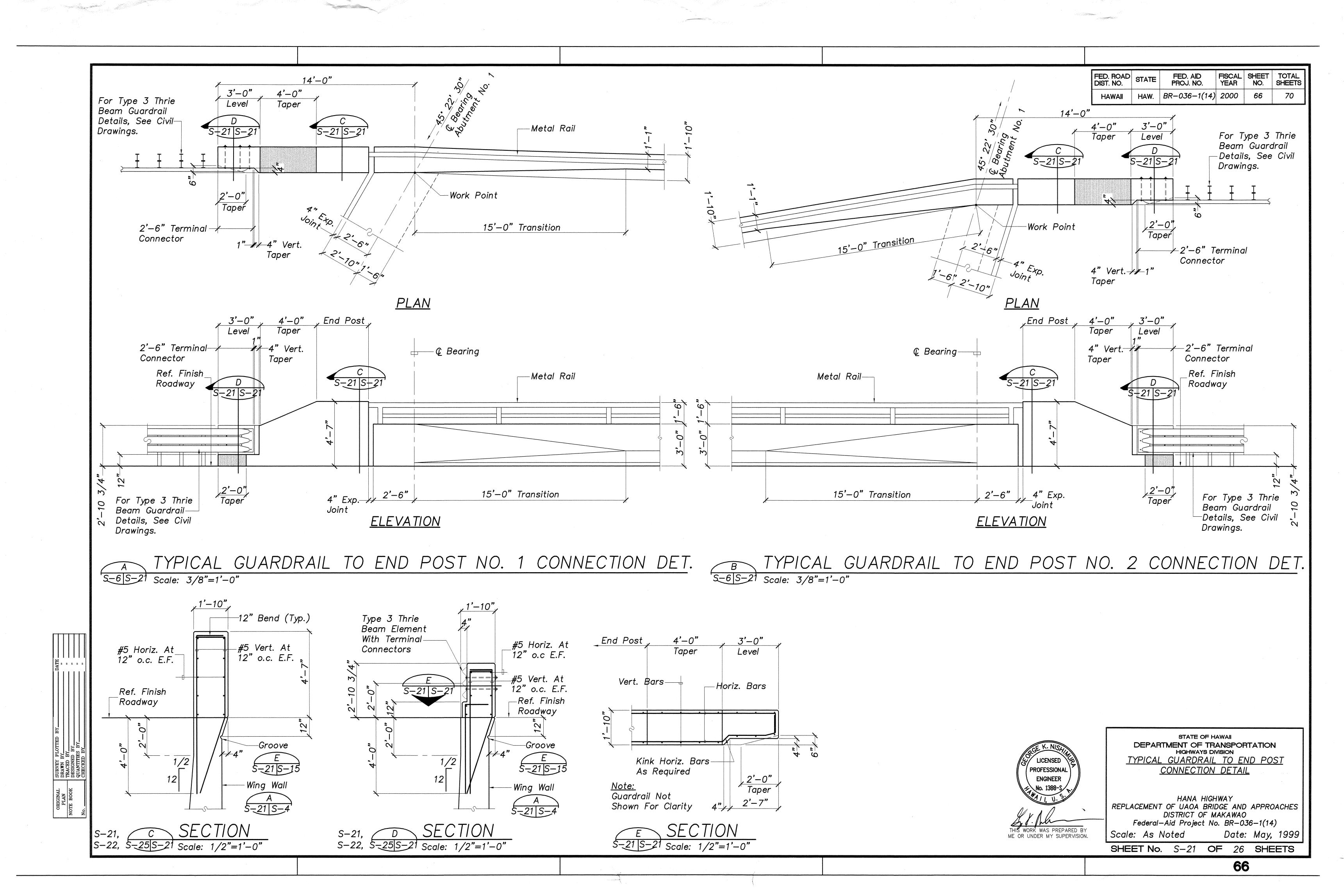
CONCRETE SEAT SKEW AND TAPER
TABLE REVISED 12/21/00 PLAN AT ABUTMENT NO. 2 REVISED, CONC. SEAT SKEW AND TAPER TABLE REVISED DATE REVISION

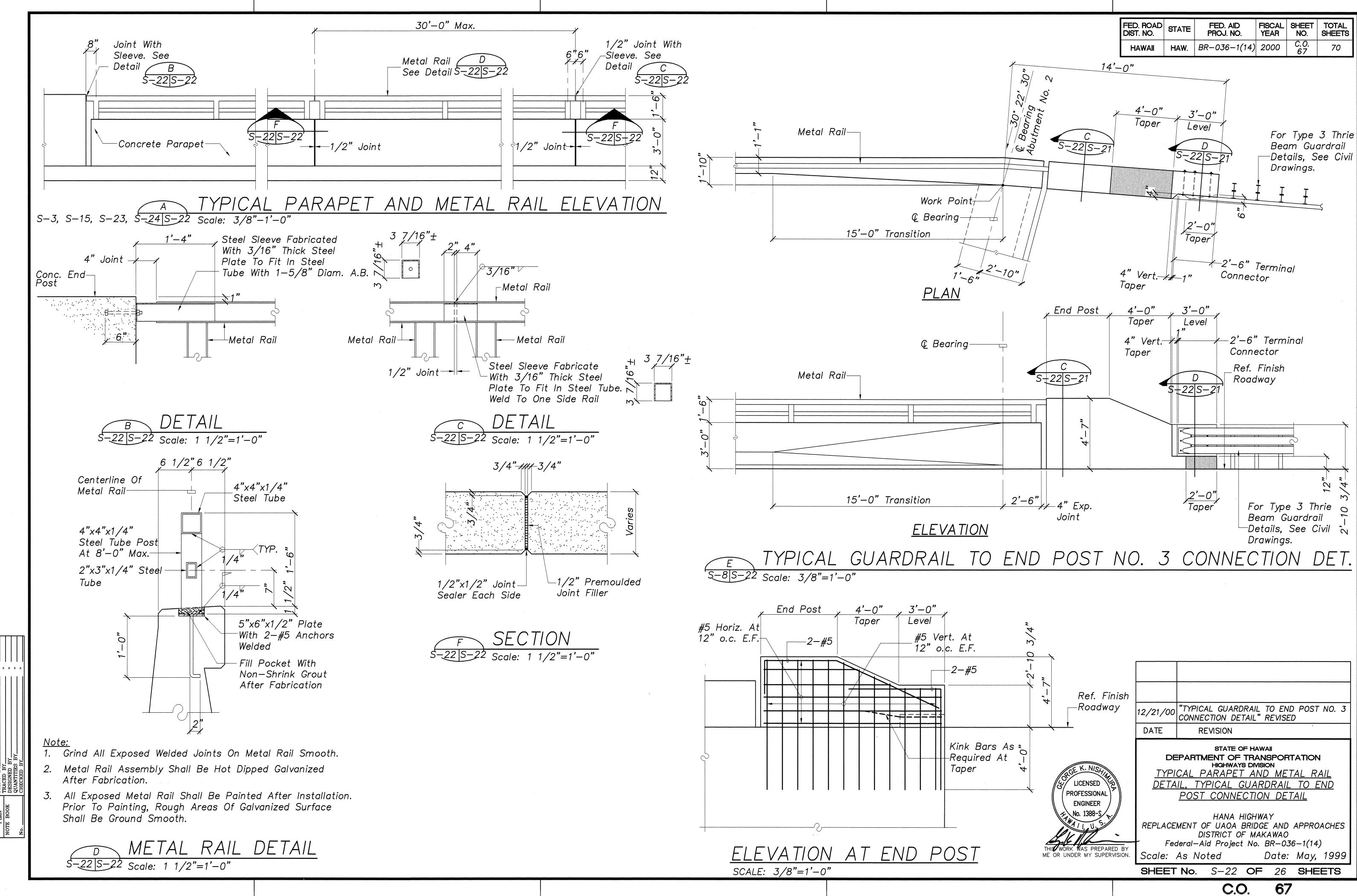
STATE OF HAWAII DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION
BEARING PAD, CONCRETE SEAT AND CREEP BLOCK DETAILS

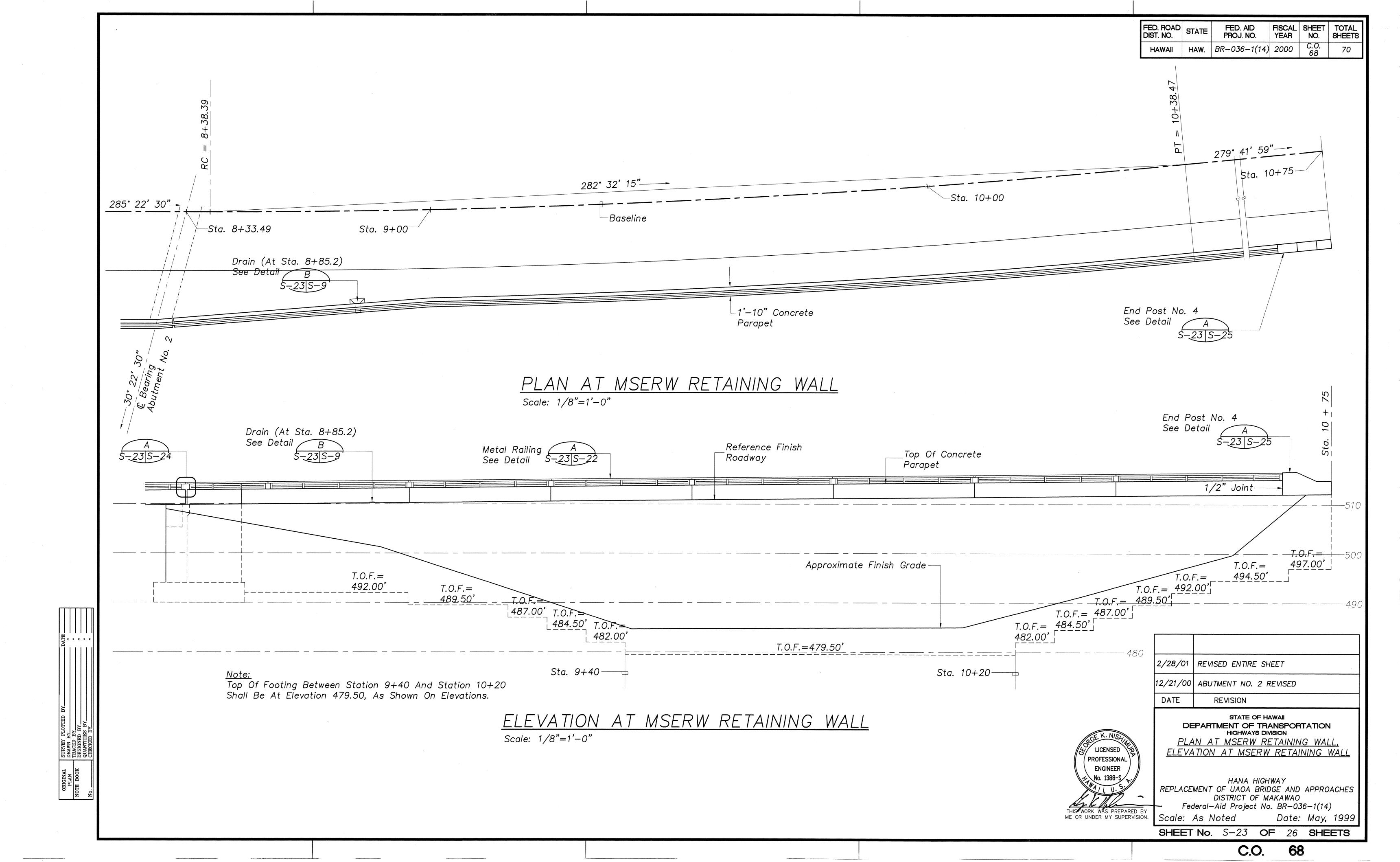
HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES

DISTRICT OF MAKAWAO Federal-Aid Project No. BR-036-1(14) Date: May, 1999 Scale: As Noted

SHEET No. S-20 OF 26 SHEETS







<u>Notes:</u>

- 1. Retaining wall shall be constructed as a design—build MSERW. Refer to section 520-Mechanically Stabilized Earth Retaining Wall System of the special provisions.
- 2. The reinforced zone behind the MSERW shall be founded on level ground. The area of the reinforced zone and the MSERW shall be excavated such that there is a minimum lateral distance of 4 feet between the lower outside edge of the wall and the compacted slope face.
- 3. The actual design of the MSERW shall be performed in accordance with the Manufacturer's requirements.
- 4. Reference stations are measured along baseline of road, unless noted otherwise.
- 5. The assumed size for the Mechanically Stabilized Earth Retaining Wall (MSERW) Panels is $5'-0" \times 5'-0"$ actual size and shape (square "cruciform", hexagonal, etc.) will be determined by the actual MSERW System chosen.
- 6. MSERW Panels along curves will be formed by a series of chords to match, as closely as possible, the desired wall alignment, except as noted and approved by the Engineer.
- 7. Top of Footing (T.O.F.) elevations shown on plans are approximate. Final T.O.F. elevations shall be determined by actual site conditions and the MSERW manufacturer, except in areas as noted on sheet S-23. Shop drawings indicating actual T.O.F. elevations will be prepared by the Contractor and submitted to the Engineer for review . Final T.O.F. elevations shall be approved by the Engineer prior to placement of the Concrete Leveling Pad.
- 8. Concrete leveling pad and concrete cap size and reinforcing shall be determined by the MSERW manufacturer.
- 9. Expansion joints (30'-0" maximum) shall be provided for C.I.P. concrete cap and concrete parapet. At expansion joints, provide steel sleeve in metal railing as shown in detail C/S-22. Actual spacings of joints, details and reinforcing shall be determined by the MSERW manufacturer and approved by the Engineer.
- 10. Brace all C.I.P. (Cast-In-Place) structures as required during construction.
- 11. MSERW manufacturer to verify the adequacy of the reinforcing steel for the various structures. Additional reinforcing steel for a particular system shall be added, as required by the MSERW manufacturer. No payment shall be made for additional reinforcing steel required for the particular MSERW System used.
- 12. Finish for MSERW shall be:
 - a. Heavy sandblast texture finish using formliners for precast concrete wall panel and drop inlet.
 - b. Light sandblast texture finish using formliners for concrete barrier and cap.
- 13. The minimum length of reinforcing strip or mesh shall be 1.2 times the wall height but not less than 10 feet. The wall height is to be measured from top of leveling pad to upper finish grade.

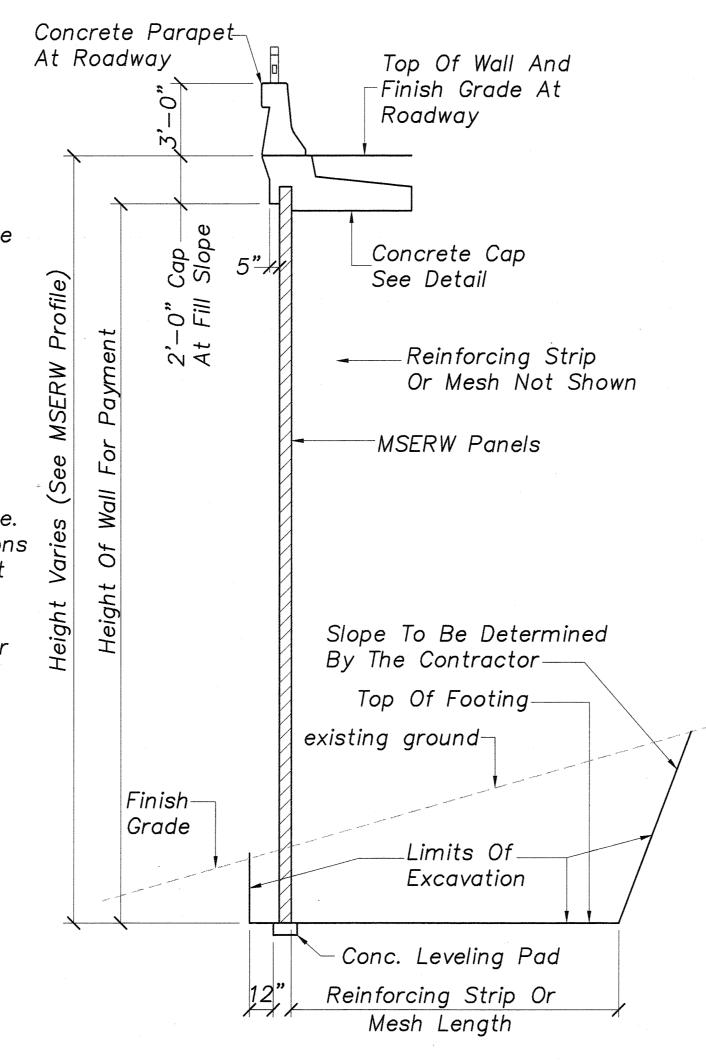
Note "A"

SURVEY DRAWN TRACED DESIGNE

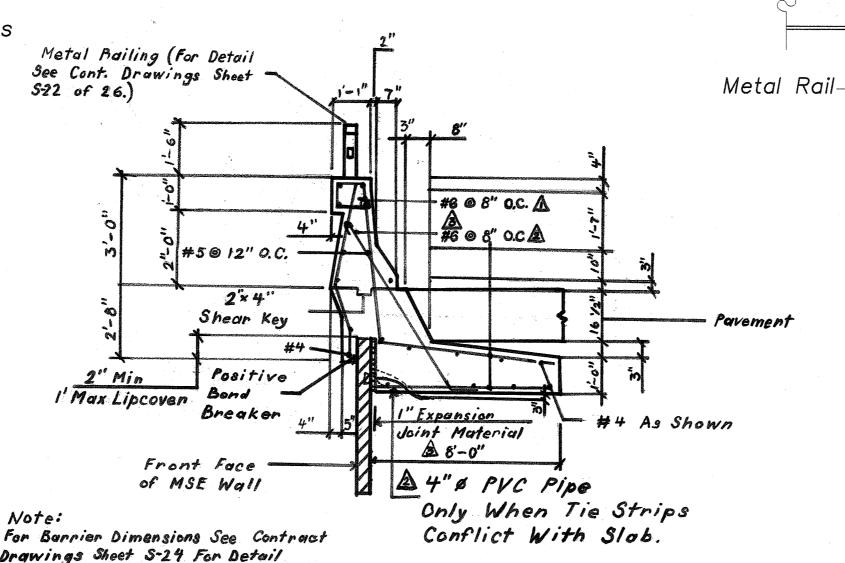
Positive bond breaker shall be provided between cast-in-place concrete and precast concrete wall.

INSPECTION ELEMENT NOTES:

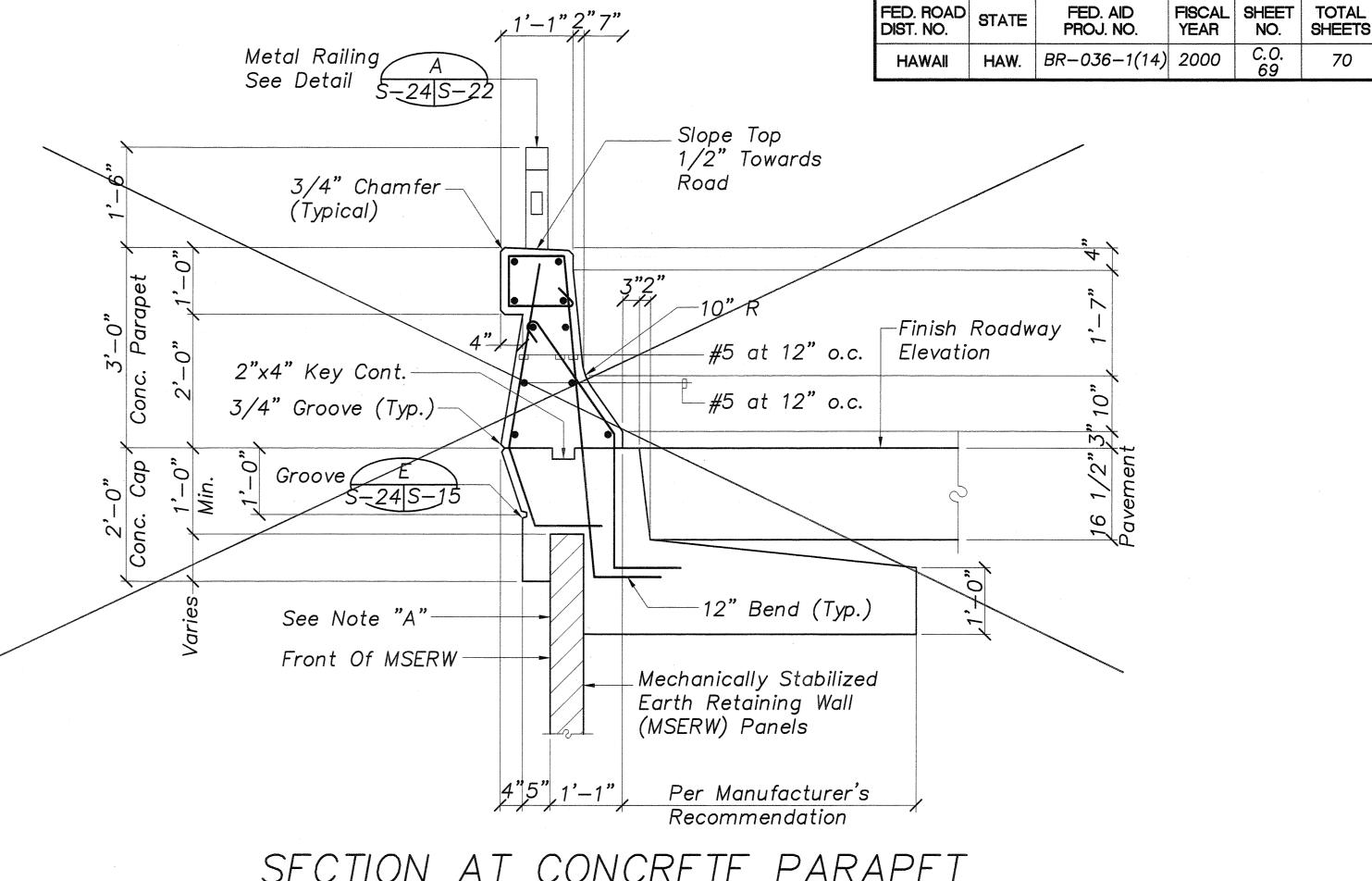
- 1. Inspection elements for inspecting corrosion of metal reinforcing strip or mesh at future date shall be installed on each MSERW at a uniform spacing of 50 feet (maximum) horizontally along the wall. The first exposed panels above final finish grade.
- 2. Inspection elements shall be fabricated from an element representative of the elements used to stabilize the wall.
- Contractor shall submit shop drawings indicating locations of the inspection element on each wall, complete details of the inspection element installation and provisions for removal of these elements.
- Inspection elements will not be measured and shall be considered incidental to MSERW materials.



YPICAL SECTION (MSERW) Not To Scale



REVISED RECO CONCRETE PARAPET DETAIL Scale: NTS



SECTION AT CONCRETE PARAPET

Metal Rail 3/16" _3/16" Thick Steel Plates 3 7/16" ---Metal Rail Steel Sleeve Fabricate

S-23 S-24 Scale: 1-1/2"=1'-0"

Side Rail

-With 3/16" Thick Steel

Plates. Weld To One

Scale: 3/4"=1'-0"

2/28/01 Note 7. Revised DATE REVISION

STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION MSERW RETAINING WALL DETAILS

SHEETS

C.O. 69

HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES DISTRICT OF MAKAWAO Federal—Aid Project No. BR-036-1(14)

Scale: As Noted Date: May, 1999 SHEET No. S-24 OF 26 SHEETS

"AC-BULT"

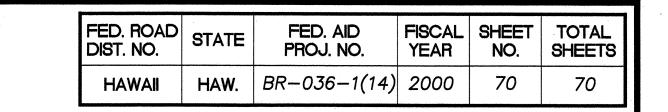
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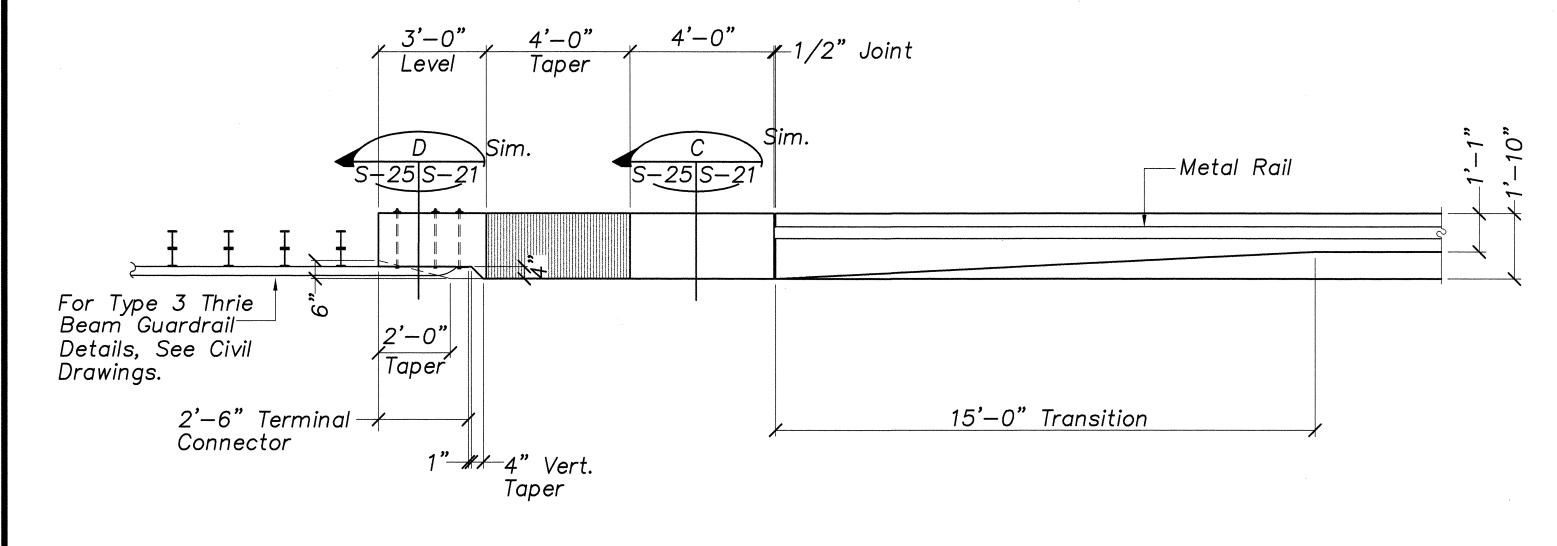
PROFESSIONAL

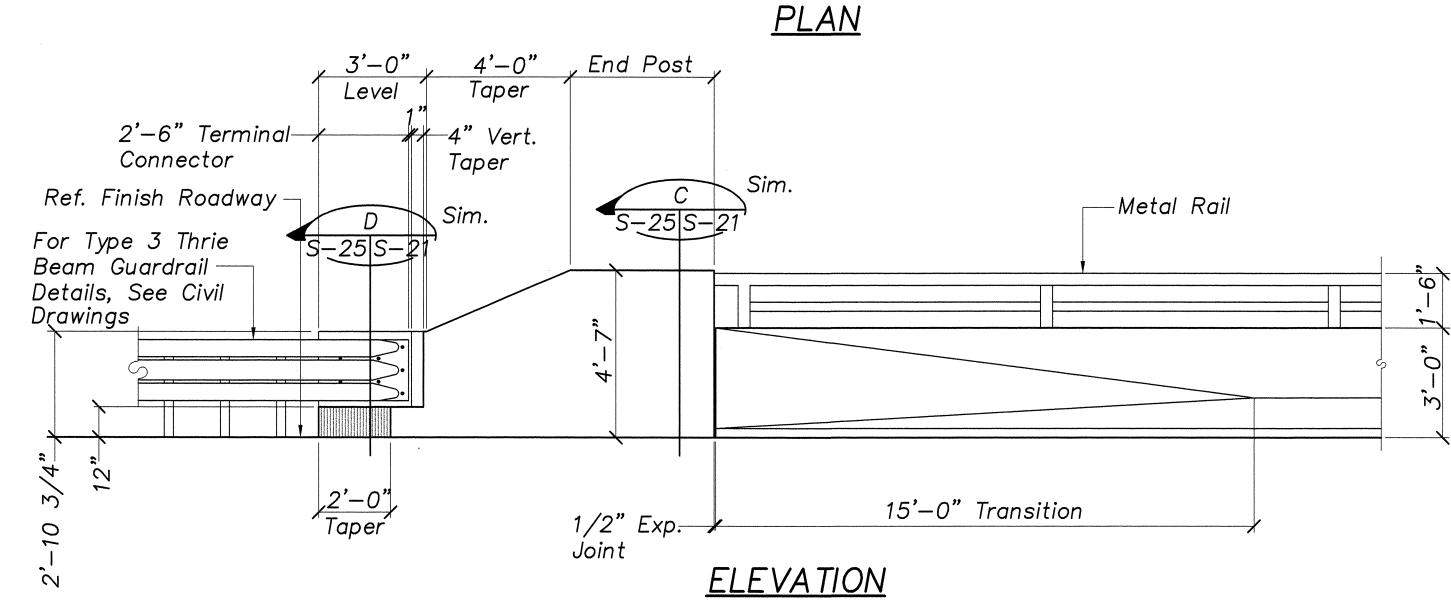
ENGINEER No. 1388-5

THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION

C.O. 69







TYPICAL GUARDRAIL TO END POST NO. 4 CONNECTION DET. S-23 S-25 Scale: 3/8"=1'-0"

LICENSED
PROFESSIONAL
ENGINEER
No. 1388-S
THIS WORK WAS PREPARED BY
ME OR UNDER MY SUPERVISION.

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

TYPICAL GUARDRAIL TO END POST NO. 3 CONNECTION DETAIL

HANA HIGHWAY REPLACEMENT OF UAOA BRIDGE AND APPROACHES DISTRICT OF MAKAWAO Federal—Aid Project No. BR—036—1(14)

Scale: As Noted Date: May, 1999
SHEET No. S-25 OF 26 SHEETS