INDEX TO BRIDGE DRAWINGS

SHEET NO.	<u>DESCRIPTION</u>				
SG0.1	INDEX TO BRIDGE DRAWINGS				
SG0.2	BRIDGE GENERAL NOTES				
SG0.3	BRIDGE GENERAL NOTES				
SG1.1	LAHAINALUNA ROAD GRADE SEPARATION STRUCTURE — LAYOUT PLAN				
SG1.2	LAHAINALUNA ROAD GRADE SEPARATION STRUCTURE — FOUNDATION PLAN				
SG1.3	LAHAINALUNA ROAD GRADE SEPARATION STRUCTURE — DECK FRAMING AND REINFORCING PLAN				
SG1.4	LAHAINALUNA ROAD GRADE SEPARATION STRUCTURE — ELEVATION				
SG2.1	TYPICAL NORMAL DECK SECTION & SIDEWALK				
SG2.2	DECK SECTIONS				
SG2.3	SEWERLINE SUPPORT DETAILS				
SG2.4	CURB RAMP SECTIONS				
SG2.5	4 INCH DIA. DUCT SUPPORT DETAILS				
SG3.1	BEAM POST—TENSIONING				
SG3.2	BEAM PLAN AND ELEVATION				
SG3.3	BEAM ELEVATION				
SG3.4	BEAM SECTIONS				
SG3.5	BEAM SECTION				
SG3.6	BEAM SECTIONS				
SG3.7	BEAM SECTIONS				
SG3.8	BEAM CLOSURE DETAILS				
SG4.1	TYPICAL MIDSPAN DIAPHRAGM				
SG5.1	ABUTMENT 1 ELEVATION				
SG5.2	ABUTMENT 1 SECTIONS				
SG5.3	ABUTMENT 1 SECTION AND DETAIL				
SG5.4	ABUTMENT 1 SECTION AND DETAIL				
	,				

SHEET NO.	<u>DESCRIPTION</u>
SG6.1	ABUTMENT 2 ELEVATION
SG6.2	ABUTMENT 2 SECTIONS
SG6.3	ABUTMENT SECTIONS
SG7.1	APPROACH SLAB PLANS
SG7.2	APPROACH SLAB SECTIONS
SG7.3	APPROACH SLAB SECTIONS
SG8.1	CONCRETE RAILING ELEVATIONS AND SECTIONS
SG8.2	GUARDRAIL AND END POST — ELEVATION AND SECTIONS
SG8.3	GUARDRAIL AND END POST — PLAN, ELEVATION, SECTIONS AND DETAILS
SG8.4	CONCRETE RAIL TRANSITION PLAN AND SECTION
SG8.5	CONCRETE RAIL TRANSITION ELEVATIONS
SG9.1	CONSTRUCTION SEQUENCE



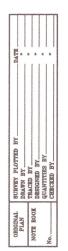


STATE OF HAWARI
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

INDEX TO BRIDGE DRAWINGS

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A
Future Keawe St Extension to Lahainaluna Rd
Part A: Off Ramp Mass Grading
Fed. Aid Proj. No. NH-030-1(35)R Scale: None Date: April, 2009

SHEET No. SGO.1 OF 3 SHEETS



3. <u>Loads:</u>

- (A) Dead Load: An allowance of 25 PSF for future wearing surface of asphalt concrete has been provided for in the design.
- (B) Live Load: AASHTO HL-93 Truck Loading

(C)	Seismic	Loads:	Acceleration coefficient	
			Seismic Performance ZoneImportance Category	
			Soil Profile	Type I

(D) Utility Load: An allowance of 150 PLF on each side of the bridge for Utility Loads has been provided for in the design.

Specified

Compressive

Strongth

4. Materials:

(A) All concrete strengths for Lahainaluna Road Grade Separation Structure shall be as noted below:

	No.	Parts	<u>Concrete</u>	f'c (28 Days)	W/C Ratio
	(1)	Abutment walls (See Note 4.(B))	-	8,000 psi	.4
′8/1C	(2)	Beams (Girders), including closure j and midspan diaphragm. (See Notes 4.(B) and 4.(H))	ioint –	8,000 psi	.4
	(3)	Deck, Sidewalk, End Beams, and Appr Slabs (See Note 4.(I))	oach –	7,000 psi	.38
	(4)	Railings (See Notes 4.(B) and 4.(H,)) -	4,000 psi	.45
	(5)	End posts (See Note 4.(H))	-	4,000 psi	.45
	(6)	Except as noted otherwise, all othe	rs A	3,000 psi	0.55

- (B) A shrinkage reducing admixture (Tetraguard AS20 by BASF) shall be added to the concrete mix for Item No.'s (1), (2), (3), and (4) under Note 4.(A). The minimum dosage requirement shall be 128 ounces per cubic yard of concrete.
- (C) All reinforcing steel shall be ASTM A615 Grade 60 unless otherwise noted.
- (D) All structural steel shall be ASTM A36 hot—dip galvanized after fabrication, unless otherwise noted.
- (E) All anchor bolts, washers and nuts shall be ASTM A325 hot—dip aglvanized after fabrication, unless otherwise specified.

BRIDGE GENERAL NOTES

4. Materials:(Cont.)

- (F) For materials of post-tensioned concrete see applicable post-tensioned concrete notes.
- (G) Steel tubes shall conform to ASTM A500, Grade B.
- (H) A migrating corrosion inhibitor amine carboxylate water—based admixture (CORTEC MCI—2005 NS) shall be added to the concrete mix for Item No.'s (2), (3), (4), and (5) under Note 4.(A). The minimum dosage shall be 1.5 pints per cubic yards of concrete.
- (I) The concrete for Item No. (3) shall be Type SBD as described in Special Provisions Section 601.04.

- (J) The Contractor shall use curing compound
 SINAK LITHIUM for bridge structures and approach slabs.
 Six copies of the manufacturer's brochure and certificates
 of test result shall be submitted.
 All work shall conform with the manufacturer's recommendations.
 Coverage rate shall be 500 square feet per gallon.
 After the curing compound is applied the deck surface shall be
 covered by burlene for seven days.
- (K) Stainless steel bolts, rods, nuts and washers shall be ASTM A193, UNS S31600 Class I. Stainless steel and dissimilar metals, such as reinforcing steel, shall be separated with teflon tape at contact area.
- (L) Asphalt roll roofing shall be ASTM D6380 Class S, Type III.

5. Reinforcement:

- (A) The minimum covering measured from the surface of the concrete to the face of any reinforcing bars shall be as follows, except as otherwise shown:
 - (1) Deck topping
 - A. Top bars = 2 1/2"
 - B Bottom bars = $1 \frac{1}{2}$ except as otherwise noted
 - (2) Abutment Walls = 2" to outermost reinforcing except as otherwise noted
 - = 3" when concrete cast against and permanently exposed to earth
 - (3) Approach slab top bars = 2 1/2"

Approach slab bottom bars = 3"

- (4) Concrete cast against and permanently exposed to earth = 3"
- (5) All others unless otherwise noted = 2"
- (B) Reinforcing bars shall be detailed in accordance with the latest edition of the design specification in note 2 unless otherwise noted.
- (C) Minimum clear spacing between parallel bars shall be 1 1/2 times the diameter of bars (for non bundled bars). In no case shall the clear distance between the bars be less than 1 1/2 times the maximum size of the coarse aggregate or 1 1/2" (except at approach slab dowel).

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	NH-030-1(35)R	2009	134	379

5. Reinforcement (Cont.):

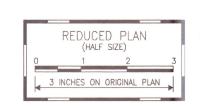
- (D) All dimensions relating to reinforcing bars are to centers of bars unless otherwise noted.
- (E) Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of intersections is less than one foot in each direction, in which case alternate intersections shall be tied.
- (F) Reinforcing steel bar supports for the prestressed beams shall be precast concrete having 8,000 psi compressive strength. Wire bar supports and all plastic supports shall not be allowed in precast concrete beams.

6. Glass Fiber Reinforced Polymer Rebar:

- (A) Glass Fiber Reinforced Polymer (GFRP) rebar shall have a tensile strength of 110 ksi for #3 bars, 100 ksi for #4 bars, 95 ksi for #5 bars, and 90 ksi for #6 bars. The allowable tensile stress is equal to 20% of the minimum ultimate tensile strength.
- (B) The modulus of elasticity of the GFRP bar shall be 5,900,000 psi.
- (C) Minimum concrete cover for the GFRP bars shall be 3/4" unless otherwise noted.
- (D) Minimum lap splice lengths for the GFRP bars shall be 42 bar diameters unless otherwise noted.
- (E) All GFRP bars shall be securely tied in place.

REVISION

- (F) The GFRP bars may be cut in the field with a masonry or diamond blade.
- (G) All work including materials and bends shall follow manufacturer's recommendations.





DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIMSION

BRIDGE GENERAL NOTES

4-8-10 Revised Notes 4(A)(3)

and 4(I)

BRIDGE GENERAL NOTES

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A

Future Keawe St Extension to Lahainaluna Rd, Part B

12-14-09 Revised GFRP Note

Fed. Aid Proj. No. NH-030-1(35)R
Scale: None

SHEET No. SGO.2 OF 3 SHEETS

100% DESIGN 134

- 1. <u>General Specifications:</u> Hawaii Department of Transportation, Standard Specifications for Road and Bridge Construction, 1994, together with Special Provisions prepared for this contract.
- 2. <u>Design Specifications:</u>
 (A) AASHTO 2004 LRFD Bridge Design Specifications
 (Third Edition) and its subsequent interim specifications with interim supplements and modifications by the Highways Division, Department of Transportation, State of Hawaii.
 - (B) HDOT "Design Criteria for Bridges and Structures" dated February 14, 2005.

3. Loads:

- (A) Dead Load: An allowance of 25 PSF for future wearing surface of asphalt concrete has been provided for in the design.
- (B) Live Load: AASHTO HL-93 Truck Loading

(C)	Seismic	Loads:	Acceleration	coefficie	nt	0.24
. ,					Zone	
			Importance	Category		Critical
			Soil Profile			Type I

(D) Utility Load: An allowance of 150 PLF on each side of the bridge for Utility Loads has been provided for in the design.

4. Materials:

All concrete strengths for Lahainaluna Road Grade Separation Structure shall be as noted below:

Item No.	Structural Parts	Classes of Concrete	Specified Compressive Strength, f'c (28 Days)	Maximum W/C Ratio
(1)	Abutment walls (See Note 4.(B))	_	8,000 psi	.4
(2)	Beams (Girders), including closure jo and midspan diaphragm. (See Notes 4.(B) and 4.(H))	oint –	8,000 psi	.4
(3)	Deck, Sidewalk, End Beams, and Appro Slabs (See Note 4.(I))	oach –	8,000 psi	.35
(4)	Railings (See Notes 4.(B) and 4.(H)) –	4,000 psi	.45
(5)	End posts (See Note 4.(H))	-	4,000 psi	.45
(6)	Except as noted otherwise, all other	s A	3,000 psi	0.55

- (B) A shrinkage reducing admixture (Tetraguard AS20 by BASF) shall be added to the concrete mix for Item No.'s (1), (2), (3), and (4) under Note 4.(A). The minimum dosage requirement shall be 128 ounces per cubic yard of concrete.
- (C) All reinforcing steel shall be ASTM A615 Grade 60 unless otherwise noted.
- All structural steel shall be ASTM A36 hot-dip galvanized after fabrication, unless otherwise noted.
- All anchor bolts, washers and nuts shall be ASTM A325 hot—dip galvanized after fabrication, unless otherwise specified.

4. Materials:(Cont.)

- (F) For materials of post-tensioned concrete see applicable post-tensioned concrete notes.
- (G) Steel tubes shall conform to ASTM A500. Grade B.
- (H) A migrating corrosion inhibitor amine carboxylate water-based admixture (CORTEC MCI-2005 NS) shall be added to the concrete mix for Item No.'s (2), (3), (4), and (5) under Note 4.(A). The minimum dosage shall be 1.5 pints per cubic yards of concrete.
- (I) The concrete mix for Item No. (3) shall have a maximum shrinkage strain of .00006 at 28 days and .000145 at 56 days according to
- (J) The Contractor shall use curing compound SINAK LITHIUM for bridge structures and approach slabs. Six copies of the manufacturer's brochure and certificates of test result shall be submitted. All work shall conform with the manufacturer's recommendations. Coverage rate shall be 500 square feet per gallon. After the curing compound is applied the deck surface shall be covered by burlene for seven days.
- (K) Stainless steel bolts, rods, nuts and washers shall be ASTM A193, UNS S31600 Class I. Stainless steel and dissimilar metals, such as reinforcing steel, shall be separated with teflon tape at contact area.
- (L) Asphalt roll roofing shall be ASTM D6380 Class 5, Type III.

5. Reinforcement:

- (A) The minimum covering measured from the surface of the concrete to the face of any reinforcing bars shall be as follows, except as otherwise shown:
 - (1) Deck topping
 - A. Top bars = $2 \frac{1}{2}$ "
 - B Bottom bars = $1 \frac{1}{2}$ except as otherwise noted
 - (2) Abutment Walls = 2" to outermost reinforcing except as otherwise noted
 - = 3" when concrete cast against and permanently exposed to earth
 - (3) Approach slab top bars = $2 \frac{1}{2}$ "

Approach slab bottom bars = 3"

- (4) Concrete cast against and permanently exposed to earth = 3"
- (5) All others unless otherwise noted = 2"
- (B) Reinforcing bars shall be detailed in accordance with the latest edition of the design specification in note 2 unless otherwise noted.
- (C) Minimum clear spacing between parallel bars shall be 1 1/2 times the diameter of bars (for non bundled bars). In no case shall the clear distance between the bars be less than 1 1/2 times the maximum size of the coarse aggregate or 1 1/2" (except at approach slab dowel).

5. Reinforcement (Cont.):

- (D) All dimensions relating to reinforcing bars are to centers of bars unless otherwise noted.
- (E) Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of intersections is less than one foot in each direction, in which case alternate intersections shall
- (F) Reinforcing steel bar supports for the prestressed beams shall be precast concrete having 8,000 psi compressive strength. Wire bar supports and all plastic supports shall not be allowed in precast concrete beams.

Glass Fiber Reinforced Polymer Rebar:

- (A) Glass Fiber Reinforced Polymer (GFRP) rebar shall have a tensile strength of 110 ksi for #3 bars, 100 ksi for #4 bars, 95 ksi for #5 bars, and 90 ksi for #6 bars. The allowable tensile stress is equal to 20% of the minimum ultimate tensile
- (B) The modulus of elasticity of the GFRP bar shall be 5,900,000 psi.
- (C) Minimum concrete cover for the GFRP bars shall be 3/4" unless otherwise noted.
- (D) Minimum lap splice lengths for the GFRP bars shall be 42 bar diameters unless otherwise noted.
- (E) All GFRP bars shall be securely tied in place.
- (F) The GFRP bars may be cut in the field with a masonry or diamond blade.
- (G) All work including materials and bends shall follow manufacturer's recommendations.





STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION BRIDGE GENERAL NOTES

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd, Part B Fed. Aid Proj. No. NH-030-1(35)R

SHEET No. SGO.2 OF 3 SHEETS

12-14-09 Revised GFRP Note

100% DESIGN 134

2. <u>Design Specifications:</u> (A) AASHTO 2004 LRFD Bridge Design Specifications (Third Edition) and its subsequent interim specifications with interim supplements and modifications by the Highways Division, Department of Transportation, State of Hawaii.

(B) HDOT "Design Criteria for Bridges and Structures" dated February 14, 2005.

3. Loads:

(A) Dead Load: An allowance of 25 PSF for future wearing surface of asphalt concrete has been provided for in the design.

(B) Live Load: AASHTO HL-93 Truck Loading

(C) Seismic Loads: Acceleration coefficient. 0.24 Seismic Performance Zone... Importance Category...... Critical Soil Profile Type 1

An allowance of 150 PLF on each side of the bridge (D) Utility Load: for Utility Loads has been provided for in the design.

4. Materials:

All concrete strengths for Lahainaluna Road Grade Separation Structure shall be as noted below:

Item No.	Structural Parts	Classes of Concrete	Specified Compressive Strength, f'c (28 Days)	Maximum W/C Ratio
(1)	Abutment walls (See Note 4.(B))		8,000 psi	.4
(2)	Beams (Girders), including closure jand midspan diaphragm. (See Notes 4.(B) and 4.(H))	oint –	8,000 psi	.4
(3)	Deck, Sidewalk, End Beams, and Appro Slabs (See Note 4.(I))	pach –	8,000 psi	.35
(4)	Railings (See Notes 4.(B) and 4.(H)) –	4,000 psi	.45
(5)	End posts (See Note 4.(H))	-	4,000 psi	.45
(6)	Except as noted otherwise, all other	rs A	3,000 psi	0.55

(B) A shrinkage reducing admixture (Tetraguard AS20 by BASF) shall be added to the concrete mix for Item No.'s (1), (2), (3), and (4) under Note 4.(A). The minimum dosage requirement shall be 128 ounces per cubic yard of concrete.

(C) All reinforcing steel shall be ASTM A615 Grade 60 unless otherwise noted.

All structural steel shall be ASTM A36 hot-dip galvanized after fabrication, unless otherwise noted.

All anchor bolts, washers and nuts shall be ASTM A325 hot-dip galvanized after fabrication, unless otherwise specified.

4. Materials:(Cont.)

- (F) For materials of post-tensioned concrete see applicable post-tensioned concrete notes.
- (G) Steel tubes shall conform to ASTM A500, Grade B.
- (H) A migrating corrosion inhibitor amine carboxylate water-based admixture (CORTEC MCI-2005 NS) shall be added to the concrete mix for Item No.'s (2), (3), (4), and (5) under Note 4.(A). The minimum dosage shall be 1.5 pints per cubic yards of concrete.
- (1) The concrete mix for Item No. (3) shall have a maximum shrinkage strain of .00006 at 28 days and .000145 at 56 days according to ASTM C512.
- (J) The Contractor shall use curing compound SINAK LITHIUM for bridge structures and approach slabs. Six copies of the manufacturer's brochure and certificates of test result shall be submitted. All work shall conform with the manufacturer's recommendations. Coverage rate shall be 500 square feet per gallon. After the curing compound is applied the deck surface shall be covered by burlene for seven days.
- (K) Stainless steel bolts, rods, nuts and washers shall be ASTM A193, UNS S31600 Class I. Stainless steel and dissimilar metals, such as reinforcing steel, shall be separated with teflon tape at contact area.
- (L) Asphalt roll roofing shall be ASTM D6380 Class 5, Type III.

5. Reinforcement:

- (A) The minimum covering measured from the surface of the concrete to the face of any reinforcing bars shall be as follows, except as otherwise shown:
 - (1) Deck topping

A. Top bars = 2 1/2"

B Bottom bars = $1 \frac{1}{2}$ " except as otherwise noted

(2) Abutment Walls = 2" to outermost reinforcing except as otherwise noted

> = 3" when concrete cast against and permanently exposed to earth

(3) Approach slab top bars = $2 \frac{1}{2}$ "

Approach slab bottom bars = 3"

- (4) Concrete cast against and permanently exposed to earth = 3"
- (5) All others unless otherwise noted = 2"
- (B) Reinforcing bars shall be detailed in accordance with the latest edition of the design specification in note 2 unless otherwise noted.
- (C) Minimum clear spacing between parallel bars shall be 1 1/2 times the diameter of bars (for non bundled bars). In no case shall the clear distance between the bars be less than 1 1/2 times the maximum size of the coarse aggregate or 1 1/2" (except at approach slab dowel).

5. Reinforcement (Cont.):

(D) All dimensions relating to reinforcing bars are to centers of bars unless otherwise noted.

HAWAII

- (E) Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of intersections is less than one foot in each direction, in which case alternate intersections shall
- (F) Reinforcing steel bar supports for the prestressed beams shall be precast concrete having 8,000 psi compressive strength. Wire bar supports and all plastic supports shall not be allowed in precast concrete beams.

6. Glass Fiber Reinforced Polymer Rebar:

- (A) Glass Fiber Reinforced Polymer (GFRP) rebar shall have a tensile strength of 100 ksi for #4 bars, 95 ksi for #5 bars and 90 ksi for #6 bars. The allowable tensile stress is equal to 20% of the minimum ultimate tensile strength.
- (B) The modulus of elasticity of the GFRP bar shall be 5,900,000 psi.
- (C) Minimum concrete cover for the GFRP bars shall be 3/4" unless otherwise noted.
- (D) Minimum lap splice lengths for the GFRP bars shall be 42 bar diameters unless otherwise noted.
- (E) All GFRP bars shall be securely tied in place.
- (F) The GFRP bars may be cut in the field with a masonry or diamond blade.
- (G) All work including materials and bends shall follow manufacturer's recommendations.



Wind K. Frynsia APRIL 30, 2010 LIC. EXP. DATE

DEPARTMENT OF TRANSPORTATION

BRIDGE GENERAL NOTES

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R

Scale: None

Date: April. 2009 SHEET No. SGO.2 OF 3 SHEETS

BRIDGE GENERAL NOTES

7. Foundation:

- (A) Lahainaluna Road Grade Separation Structure
 - (1) Bearing Value (service) = 20,000 psf
 - (2) Bearing Value (strength) = 36,000 psf
 - (3) Bearing Value (extreme event) = 60,000 psf
 - (4) At-rest pressure (Top 8 feet of abutment) = 56 pcf
 - (5) At-rest pressure (Abutment wall retaining basalt) = 5 pcf
 - (6) Earthquake load (abutment walls) = 14.5^2 H (Restraint) (H = height of retained soil or backfill in feet)
 - Passive pressure (Extreme Event) = 1,000 psi. Passive pressure (Strength) = 500 psi. Horizontal spring (in contact with rock) = 5,000 psi. Horizontal spring (in contact with soil) = 4 ksf/in.
 - (8) Friction Factor: Extreme Event : $\mathcal{U} = 0.70$ Strength : $\mathcal{U} = 0.60$

Other Notes:

- (A) Reference Drawings:
 - (1) For Construction Sequence of Lahainaluna Road Grade Separation Structure, See sheet SG9.1.
 - (B) Except as otherwise noted, all vertical dimensions are measured plumb.
 - (C) Unless otherwise noted, all exposed concrete edges shall be chamfered 3/4"X3/4".



STATE OF HAWAN
DEPARTMENT OF TRANSPORTATION

BRIDGE GENERAL NOTES

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R

FED. ROAD STATE

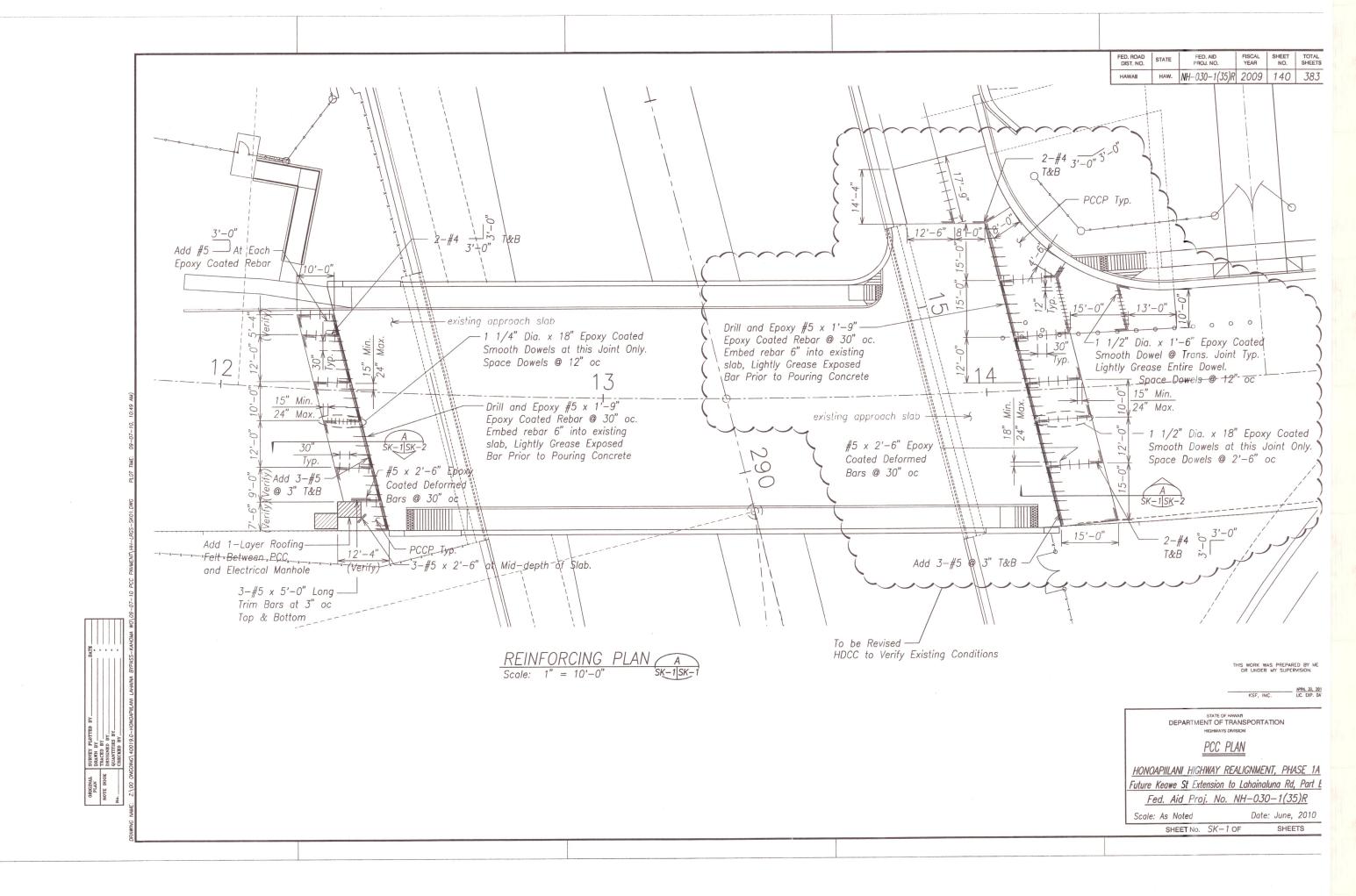
FISCAL YEAR

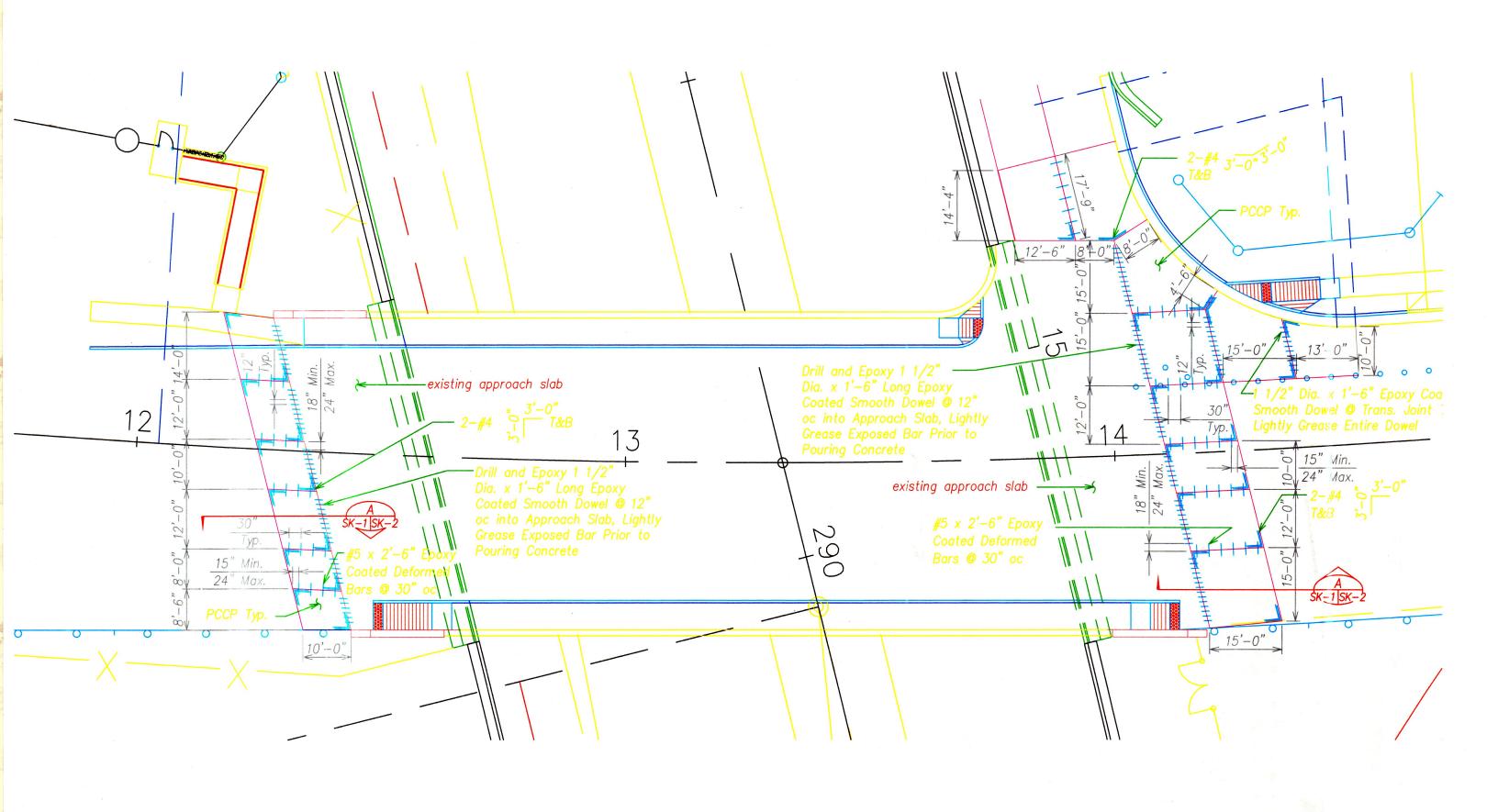
HAW. NH-030-1(35)R 2009 135 286

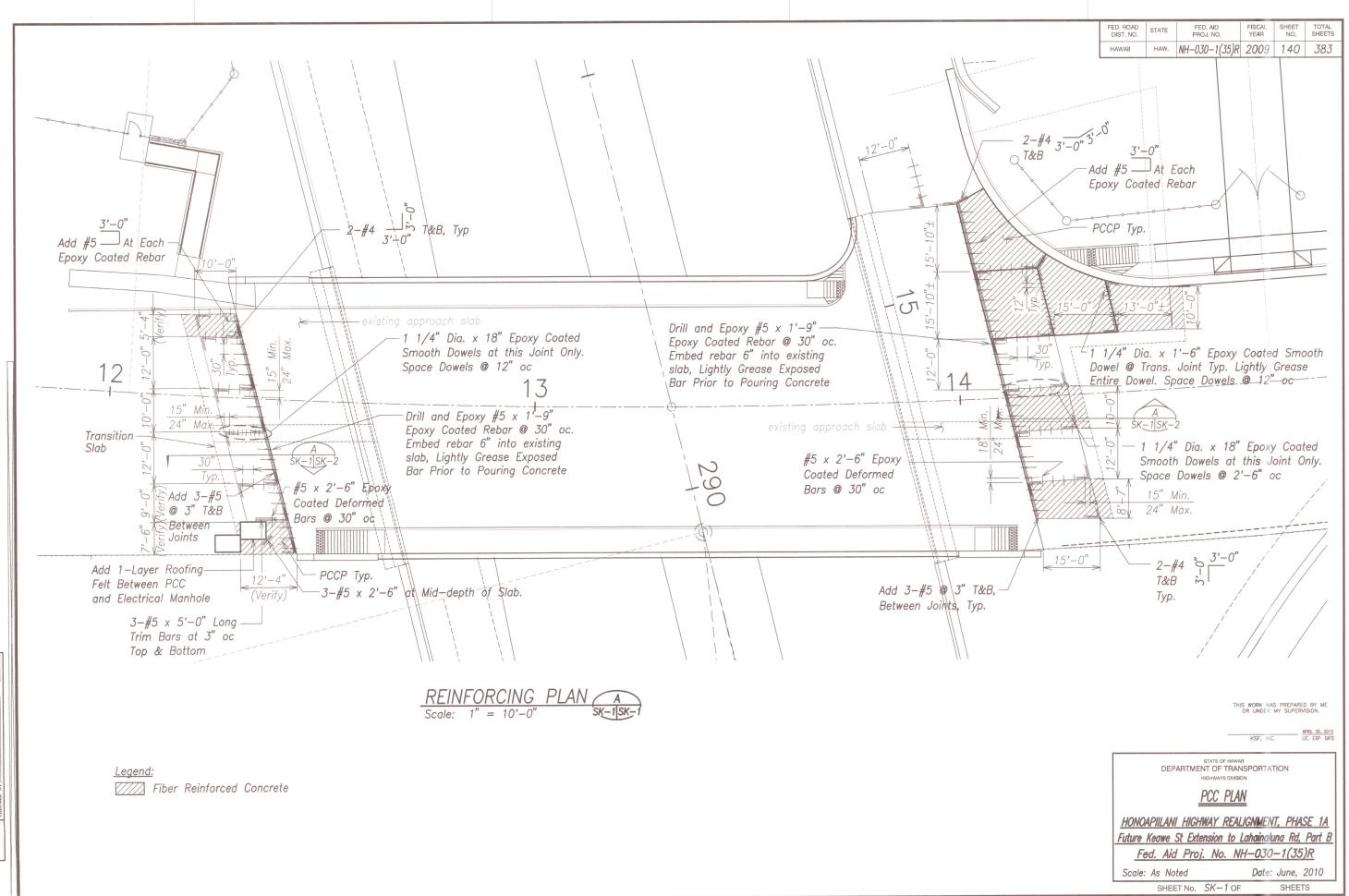
SHEET NO.

SHEET No. SGO.3 OF 3 SHEETS

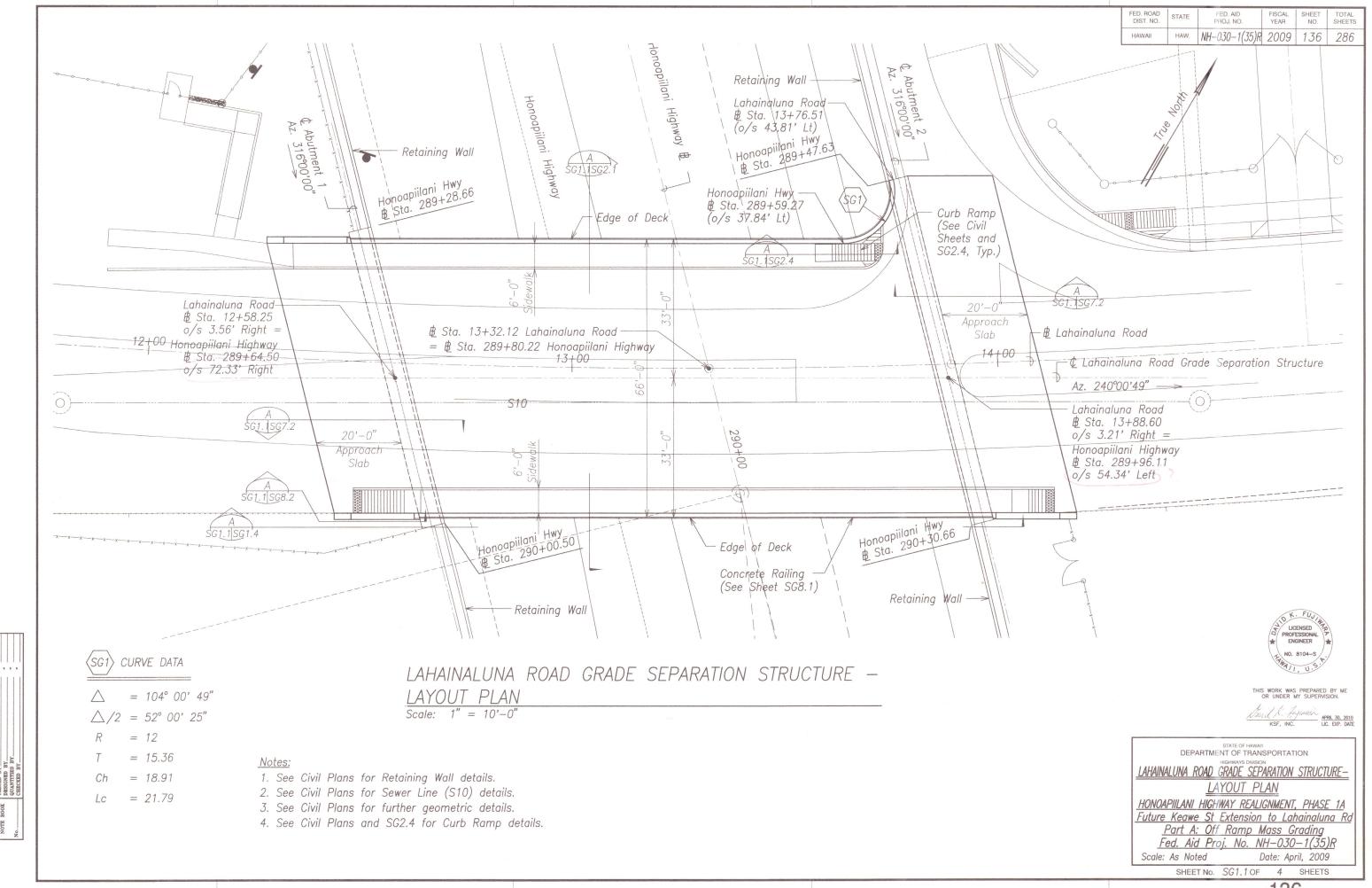
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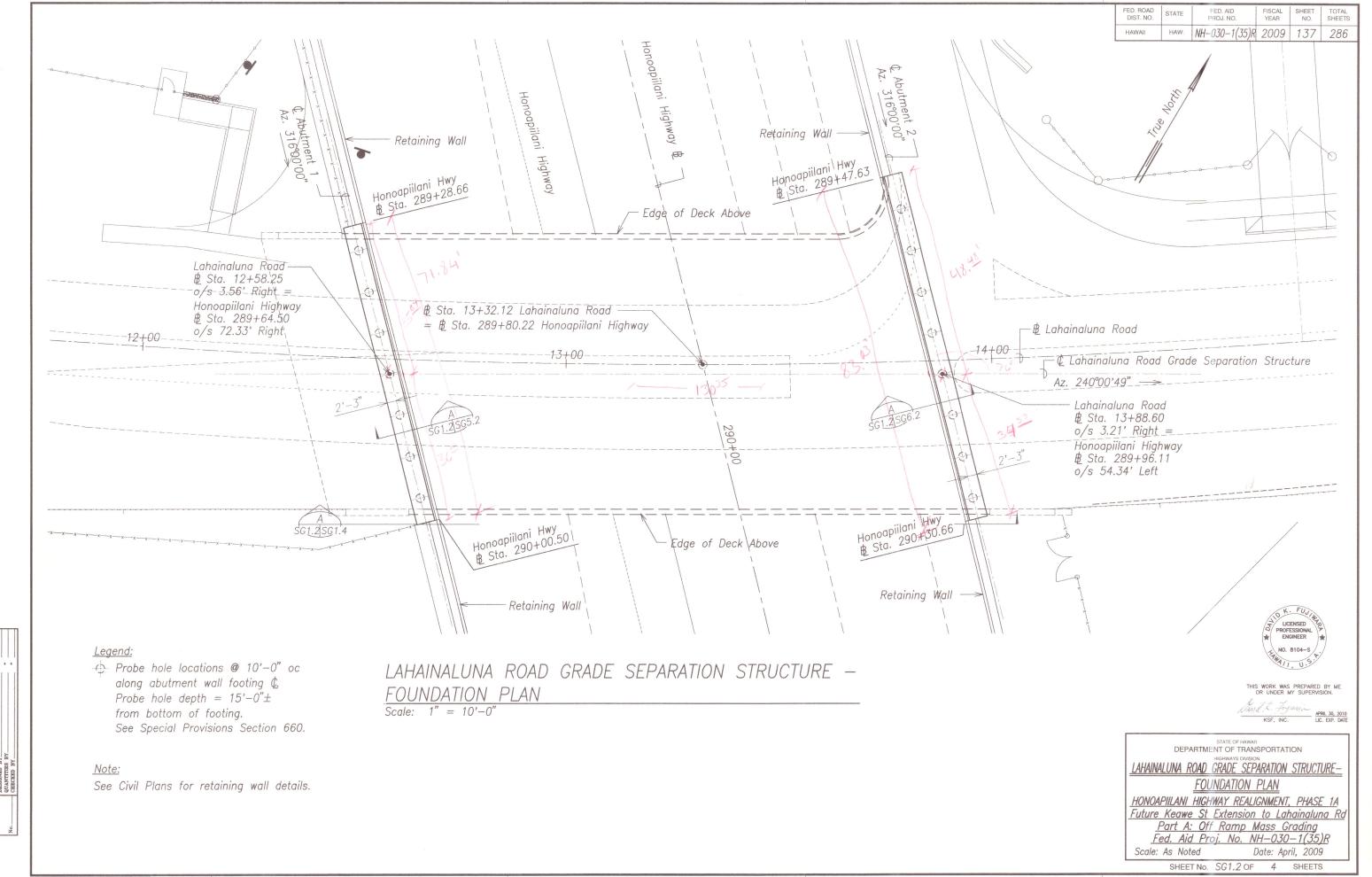


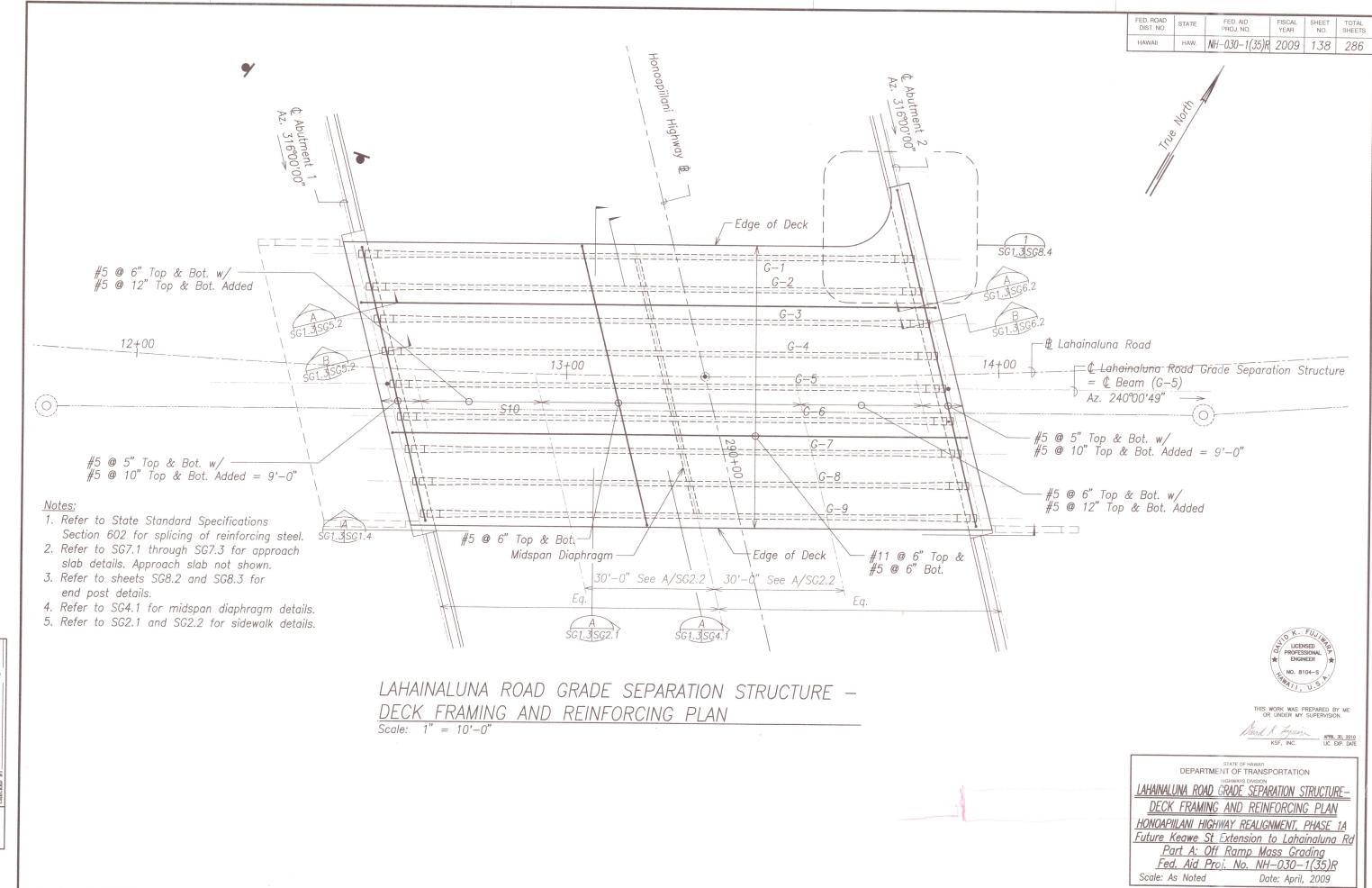




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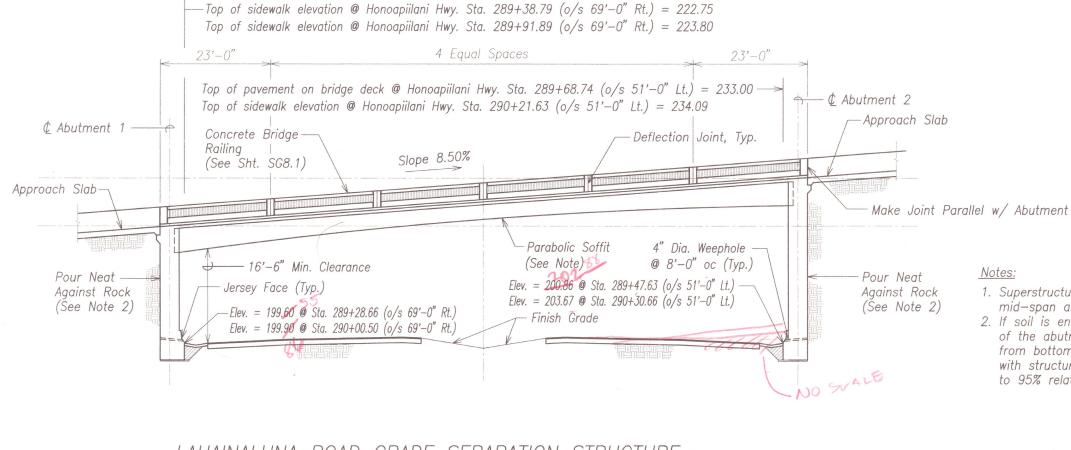






SHEET No. SG1.3 OF 4 SHEETS

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	NH-030-1(35)R	2009	139	286



Notes:

1. Superstructure is 3'-8" deep at mid-span and 5'-8" deep at abutments.

2. If soil is encountered behind the top of the abutment, excavate 1H: 1V from bottom of soil layer and backfill with structural backfill type "A" compacted to 95% relative compaction.

LAHAINALUNA ROAD GRADE SEPARATION STRUCTURE -

ELEVATION

Scale: 1'' = 10'-0''

SG1.2, SG1.3



STATE OF HAWARI
DEPARTMENT OF TRANSPORTATION

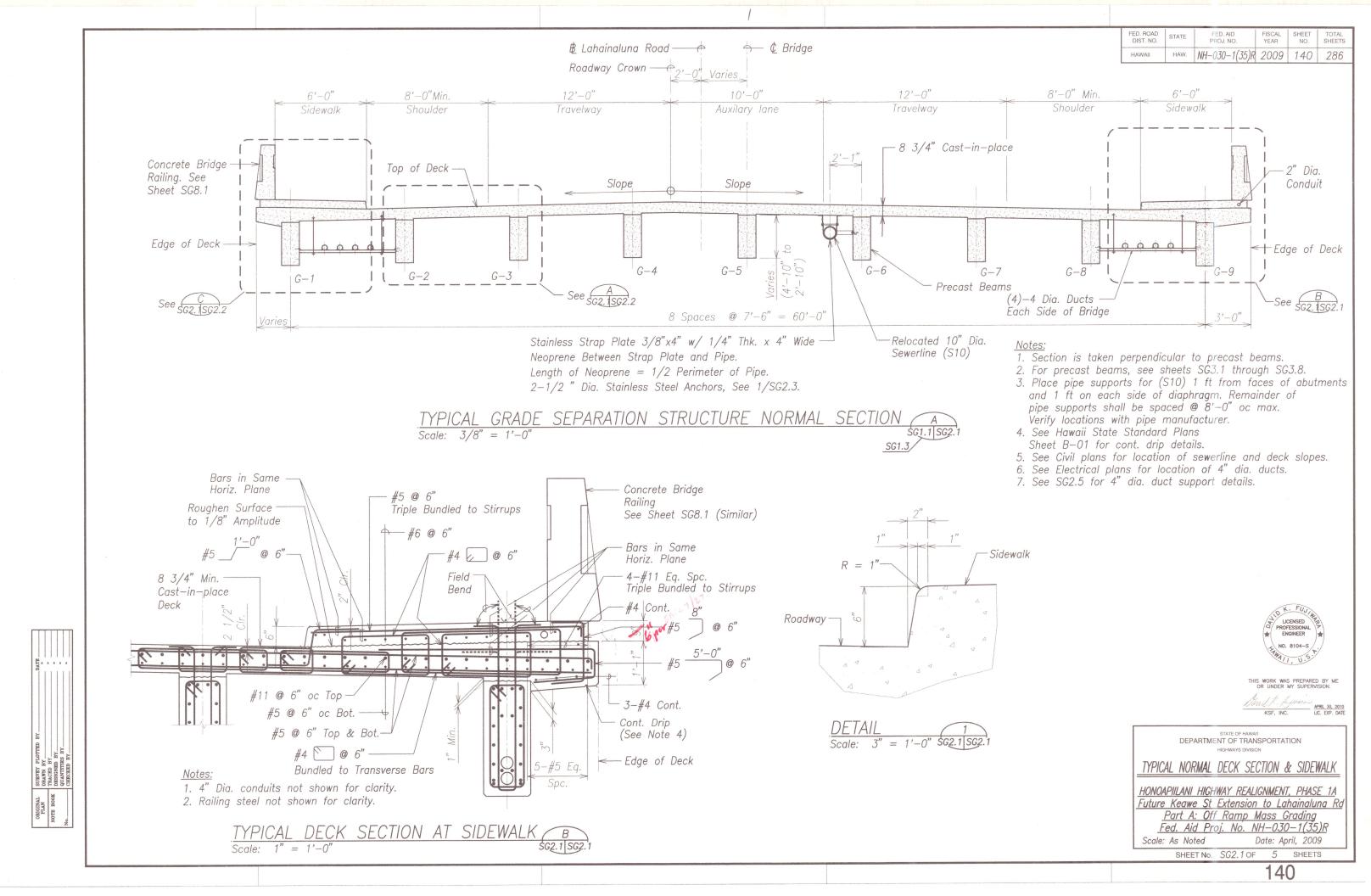
LAHAINALUNA ROAD GRADE SEPARATION STRUCTURE-

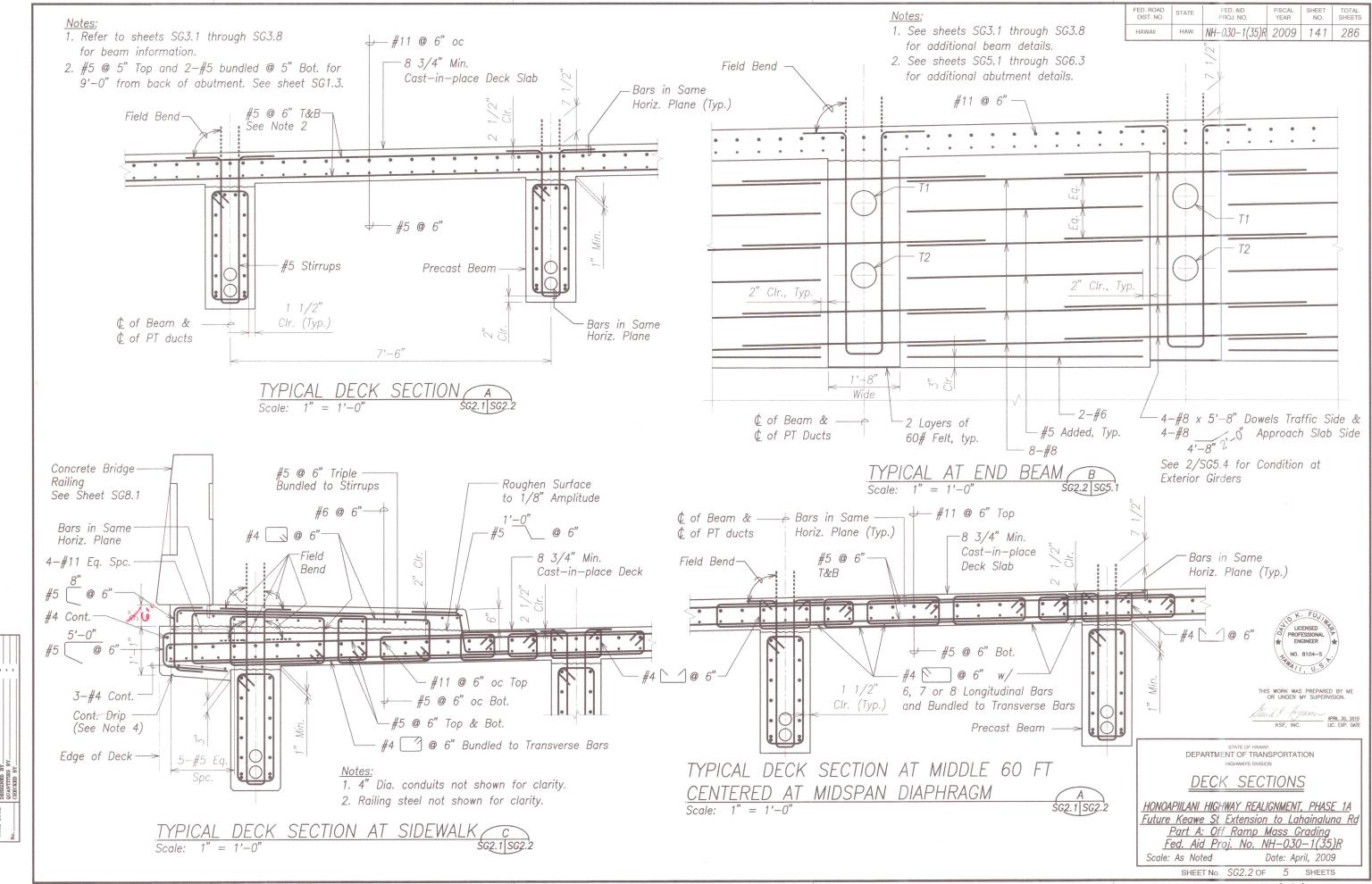
ELEVATION

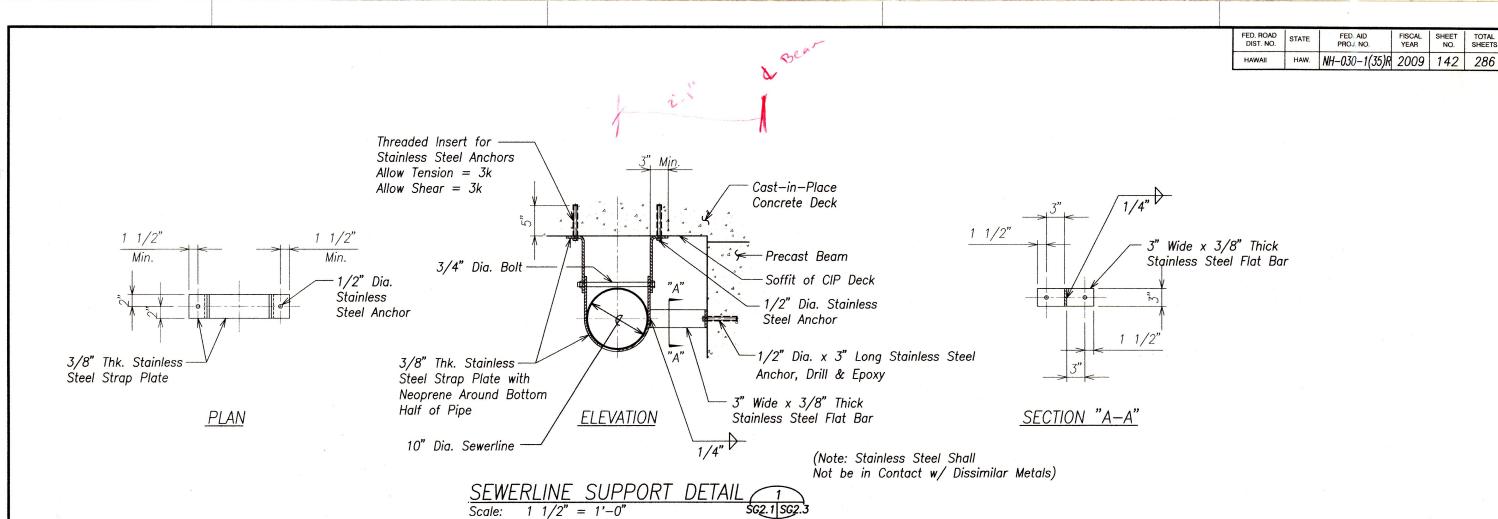
HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Ra
Part A: Off Ramp Mass Grading
Fed. Aid Proj. No. NH-030-1(35)R

Scale: As Noted

Date: April, 2009 SHEET No. SG1.4 OF 4 SHEETS









FISCAL YEAR

SHEET NO.



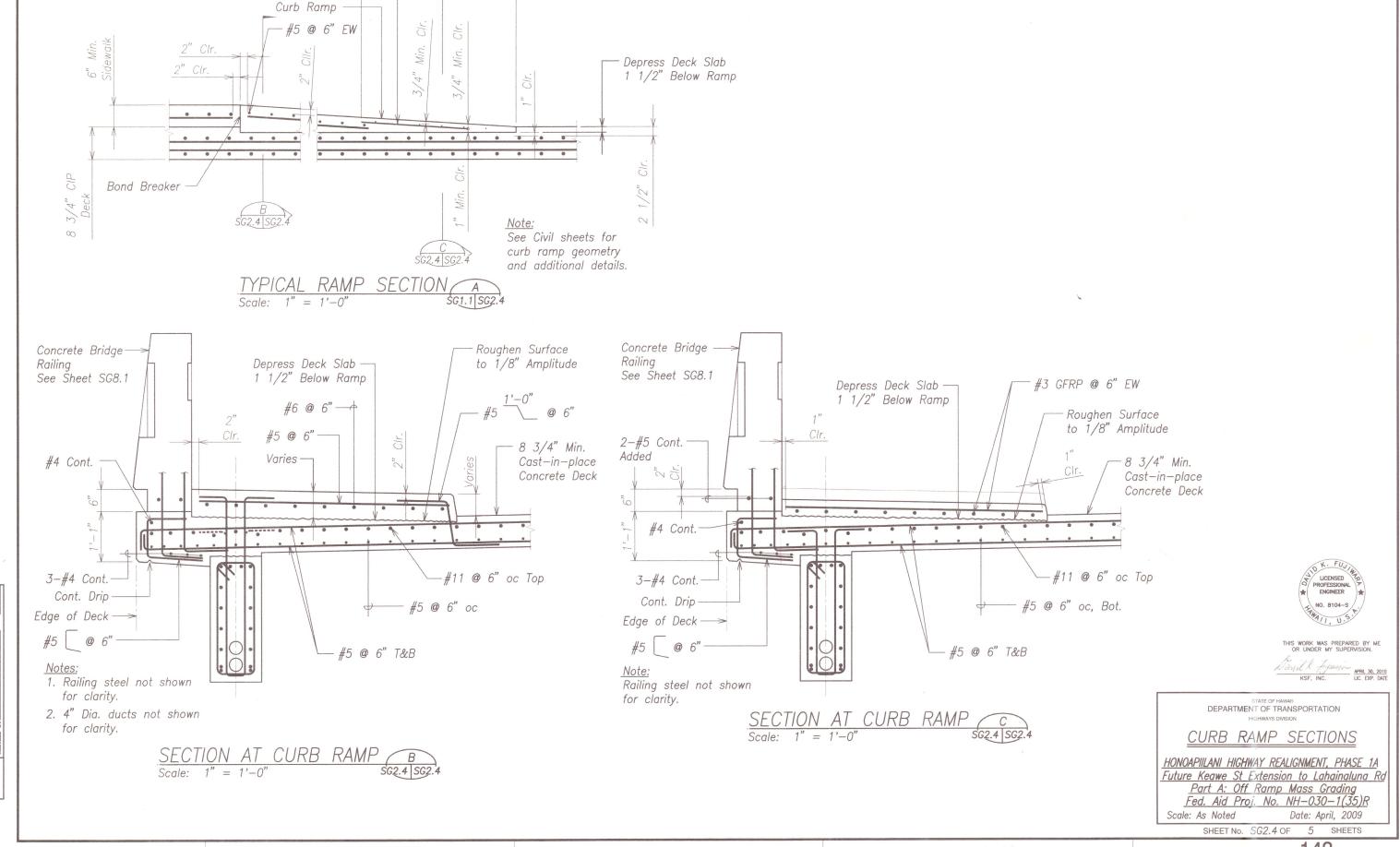
STATE OF HAWAIT
DEPARTMENT OF TRANSPORTATION

SEWERLINE SUPPORT DETAILS

<u>HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A</u> Future Keawe St Extension to Lahainaluna Rd
Part A: Off Ramp Mass Grading
Fed. Aid Proj. No. NH-030-1(35)R Scale: As Noted Date: April, 2009

SHEET No. SG2.3 OF 5 SHEETS





3'-6"

#3 GFRP @ 6" EW-

143

FED. ROAD DIST. NO.

HAWAII

FED. AID PROJ. NO. FISCAL YEAR

HAW. NH-030-1(35)R 2009 143 286

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	NH-030-1(35)R	2009	144	286

Insert for SS Rod

(Min. Allowable shear = 3^k)

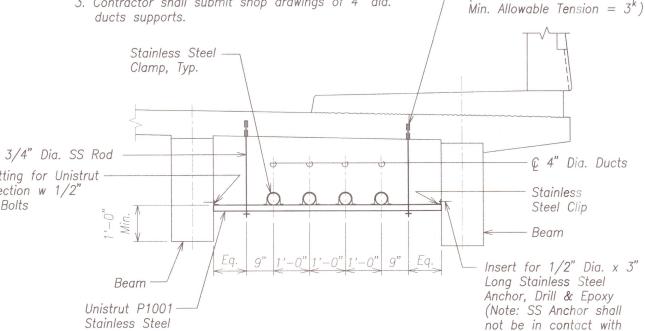
dissimilar metals.)



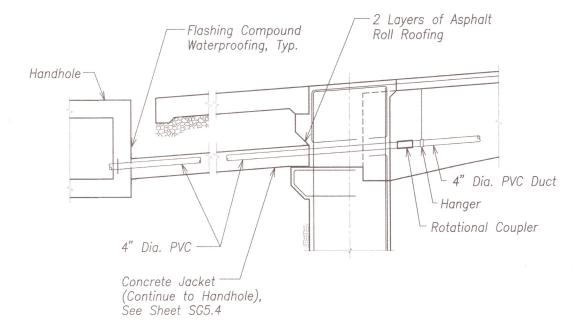
12 Ga. 90° Fitting for Unistrut -Channel Connection w 1/2" Dia. x 3" SS Bolts

- 1. Stainless Steel shall not be in contact with dissimilar metals.
- 2. Place 4" dia. duct supports 1'-0" from faces of abutments and 1'-0" each side of diaphragm. Remainder of supports shall be placed at 8'-0" max. verify locations w/ electrical.

3. Contractor shall submit shop drawings of 4" dia. ducts supports.



SECTION B Scale: 3/4" = 1'-0" SG2.5 SG2.5



<u>Note:</u> See Electrical Plans for couplers.

4" DIA. DUCTS THROUGH ABUTMENT A
Scale: 3/8" = 1'-0"
SG2.5 SG2.5



STATE OF HAWARI
DEPARTMENT OF TRANSPORTATION

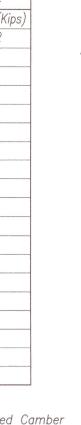
4 INCH DIA. DUCT SUPPORT DETAILS

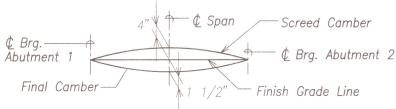
HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Ra Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R Date: April, 2009

Scale: As Noted

SHEET No. SG2.5 OF 5 SHEETS

POST-TENSIONING SEQUENCE					
Cindon No	Tandon	Tendon Design Jacking Force (Kip			
Girder No.	renaon	Stage 1	Stage 2		
G-5	T1	483	966		
G-4	T1	483	966		
G-6	T1	483	966		
G-3	T1	483	966		
G-7	T1	483	966		
G-2	T1	483	966		
G-8	T1	483	966		
G-1	T1	483	966		
G-9	T1	483	966		
G-5	T2	483	966		
G-4	T2	483	966		
G-6	T2	483	966		
G-3	T2	483	966		
G-7	T2	483	966		
G-2	T2	483	966		
G-8	T2	483	966		
G-1	T2	483	966		
G-9	T2	483	966		





POST TENSIONING NOTES:

- 1. Post-tensioning ducts shall be 4 3/8" O.D. Spiro-Type Semi-Rigid galvanized.
- 2. After post-tensioning, all post-tensioning ducts tendons shall be pressure grouted w/ pre-packaged grout as specified in Specification Section 504.03 (H) (5) Grouting. Ducts shall have grouting vents at anchorages and at each high point along the tendon profile.
- 3. Duct shall be secured to prevent misalignment during concreting.
- 4. A minimum compressive strength of 8000 PSI shall be obtained in the beam and deck before the application of post tensioning.
- 5. Post tensioned strands shall be 7 wire 0.6 dia. low relaxation steel strands (Area = 0.217 in.^2) with an ultimate tensile strength of 270 ksi conforming to ASTM A416.
- 6. The post tensioning design assumptions are as follows: Curve Friction Coefficient 0.18 Wobble Friction Coefficient 0.0002/ft. 3/8" Anchor Set
- 7. The design jacking force at end of each tendon = 966 k. The maximum stress in the strand before anchor set in
- 8. Post tensioning anchor shall be AVAR ACS 22 x 6 or equal.
- 9. Jacking symbols are as follows:

> Jacking end ≥ Dead end

- is 75% of the guaranteed ultimate strength.

Abutment 1 ---Parabolic Soffit Note: See sheet SG3.2 for tendon geometry. $\frac{ELEVATION}{Scale: 1" = 10'-0"}$ Edge of Deck G-3G-414+00 13+00 ♠ Bridge Az. 240°00'49" G-6G-7G-8G-9

₡ Beam--

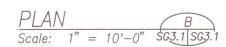
10. Apply corrosion inhibitor amine carboxylate powder (Cortec MCI-309) in all post-tension ducts.

11. The minimum force in prestressing steel shall not be less than shown in table above.

The force in the prestressing steel shall be considered as the smaller of the two values as determined by the measured elongation and the gage pressure. If the difference in stress as obtained by the measured elongation and the measured gage pressure exceeds 5 percent of the required prestressing force, the stressing process shall be terminated and shall not resume until the Contractor submits data indicating the cause of such difference and makes corrections, approved by the Engineer, to rectify such difference.

12. Prevent ducts, at all times, from getting plugged or damaged. Ducts shall be checked to show that ducts are clear and contain no obstructions prior to installing prestressing steel and stressing the member.

13. Extend 8 strands in each T2 tendon at each end 3'-0".



Edge of Deck



STATE OF HAWAIT
DEPARTMENT OF TRANSPORTATION

BEAM POST-TENSIONING

· ₿ Lahainaluna

Road

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R

Scale: As Noted

FED. ROAD DIST. NO.

Abutment 2

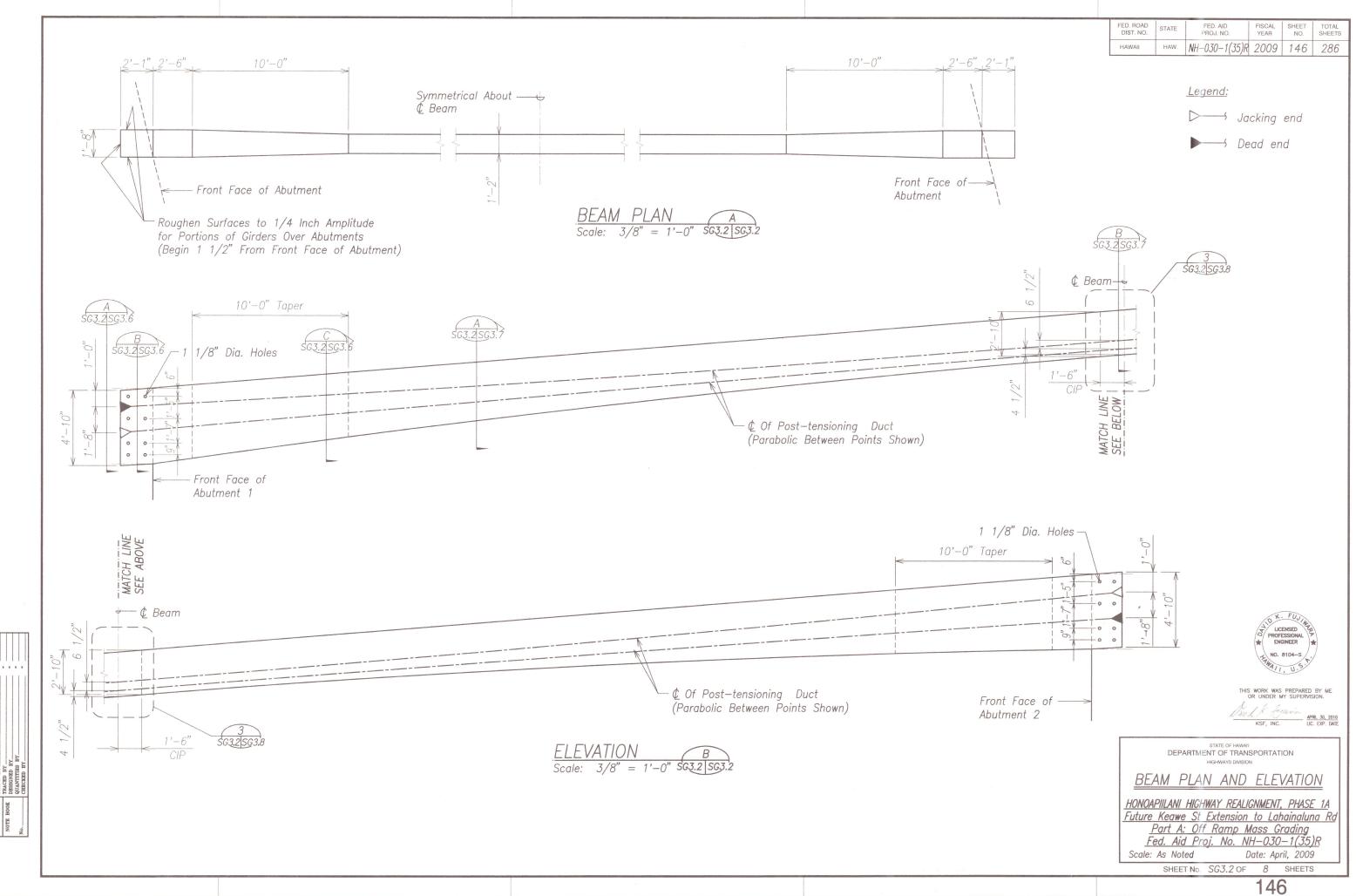
FISCAL YEAR

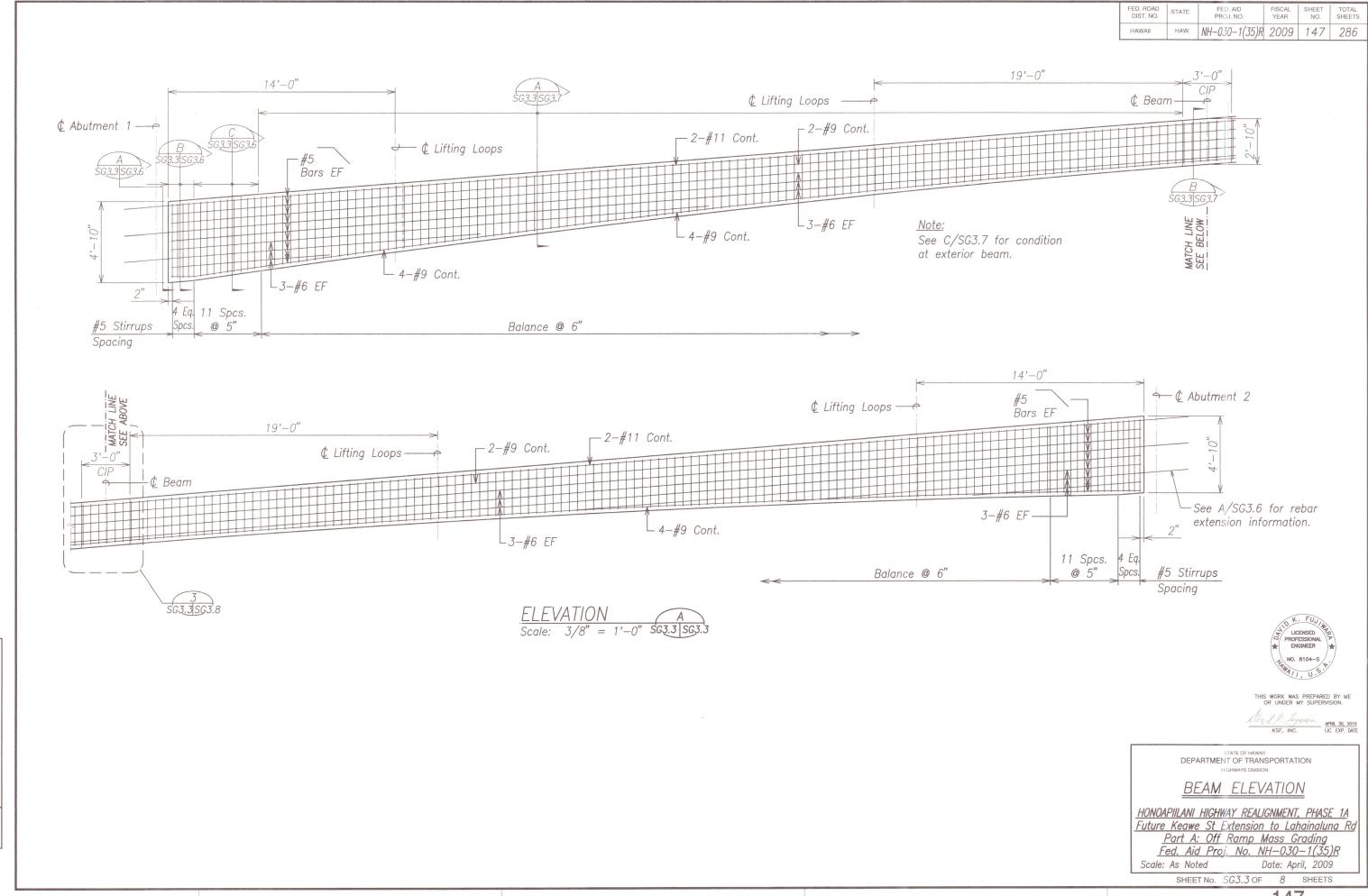
NH-030-1(35)R 2009 145 286

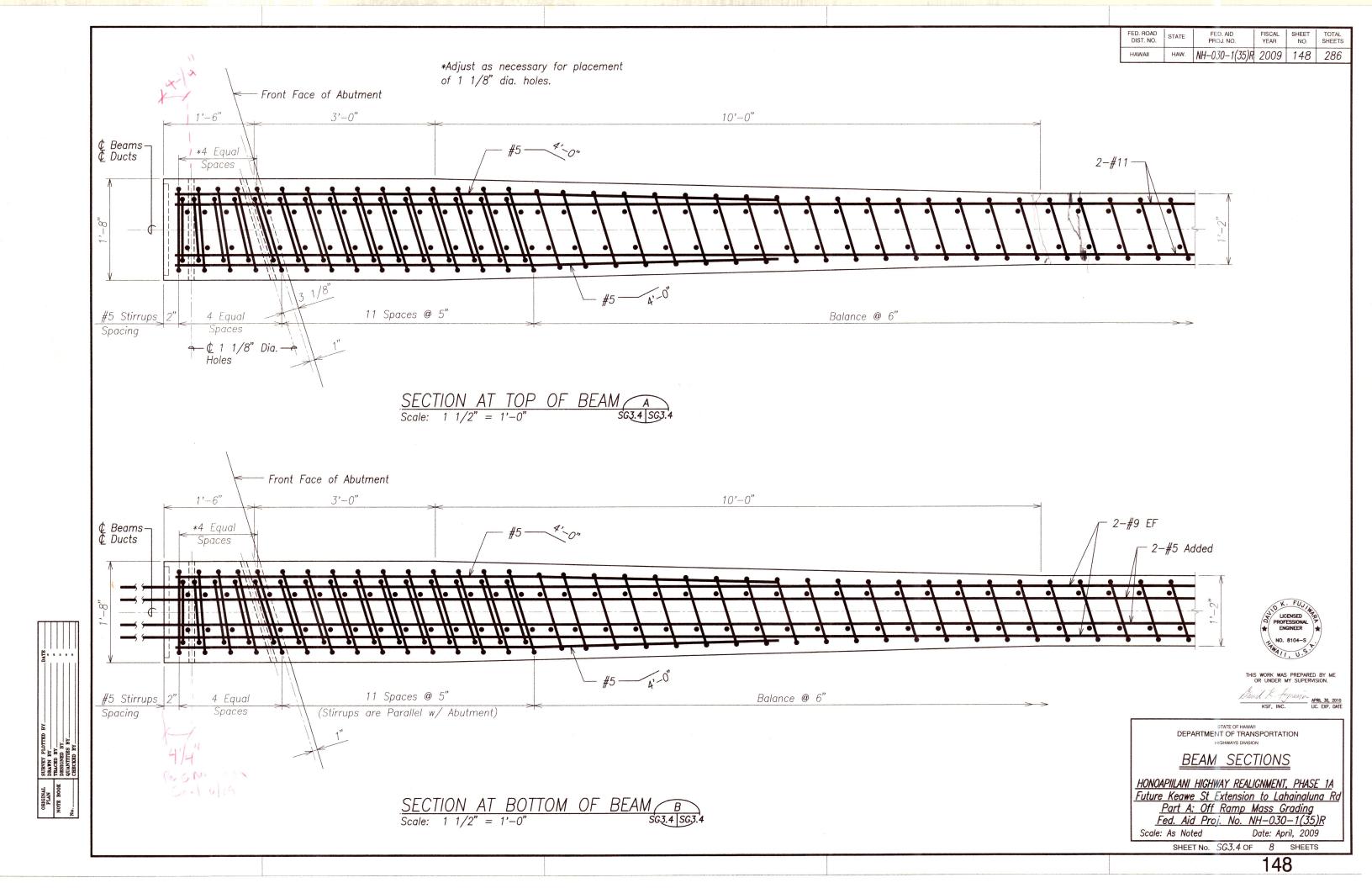
SHEET No. SG3.1 OF 8 SHEETS

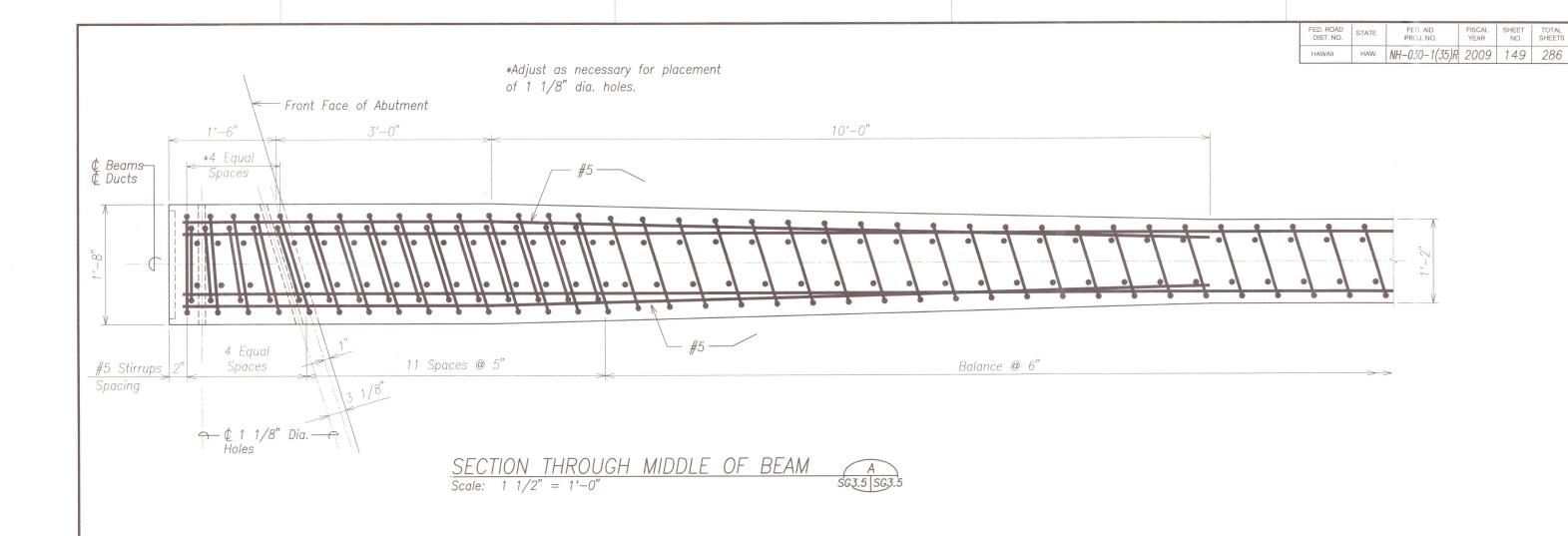


Date: April, 2009











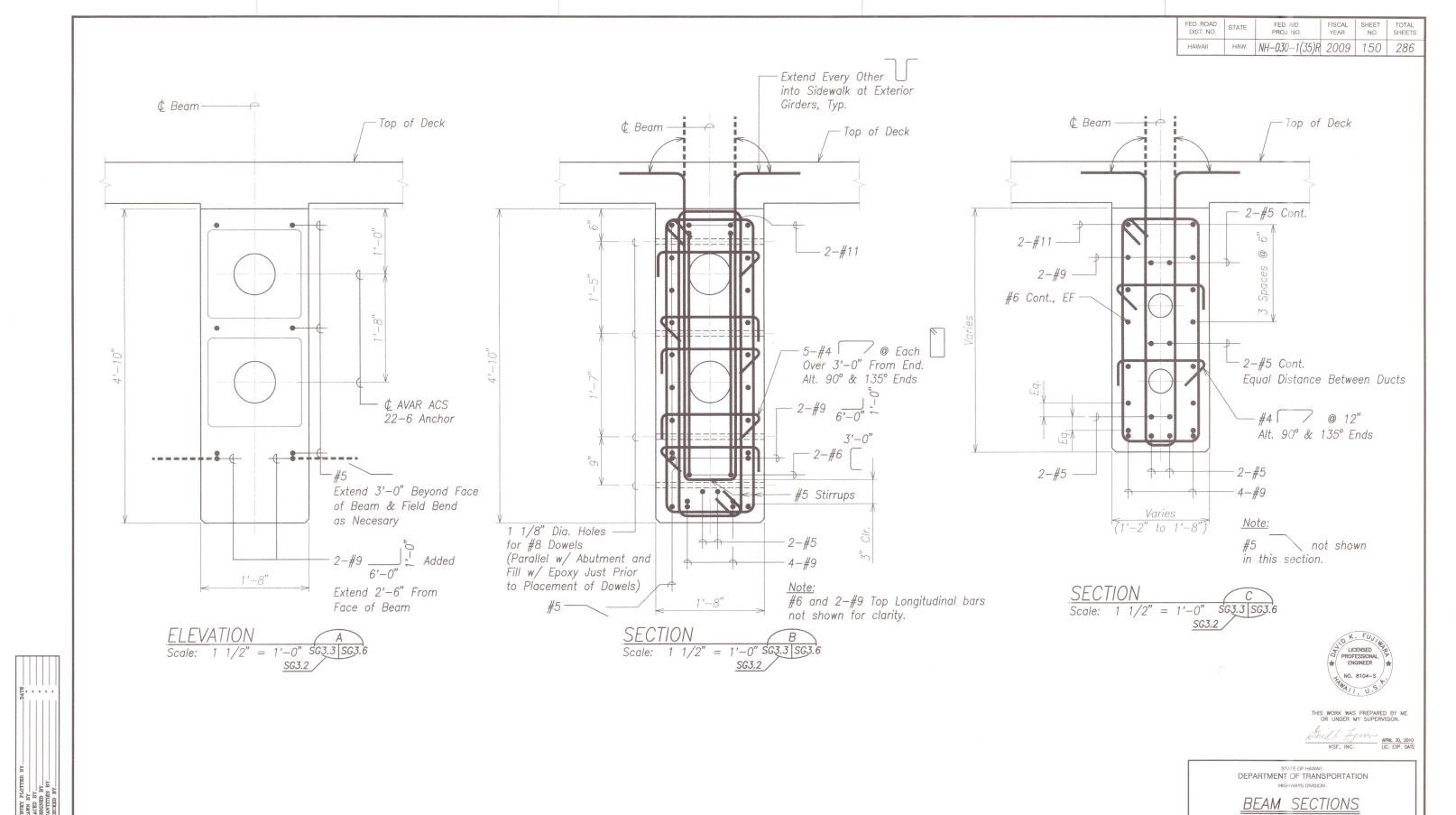
STATE OF HAWARI
DEPARTMENT OF TRANSPORTATION

BEAM SECTION

HONOAPILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd
Part A: Off Ramp Mass Grading
Fed. Aid Proj. No. NH-030-1(35)R

Scale: As Noted

Date: April, 2009 SHEET No. SG3.5 OF 8 SHEETS

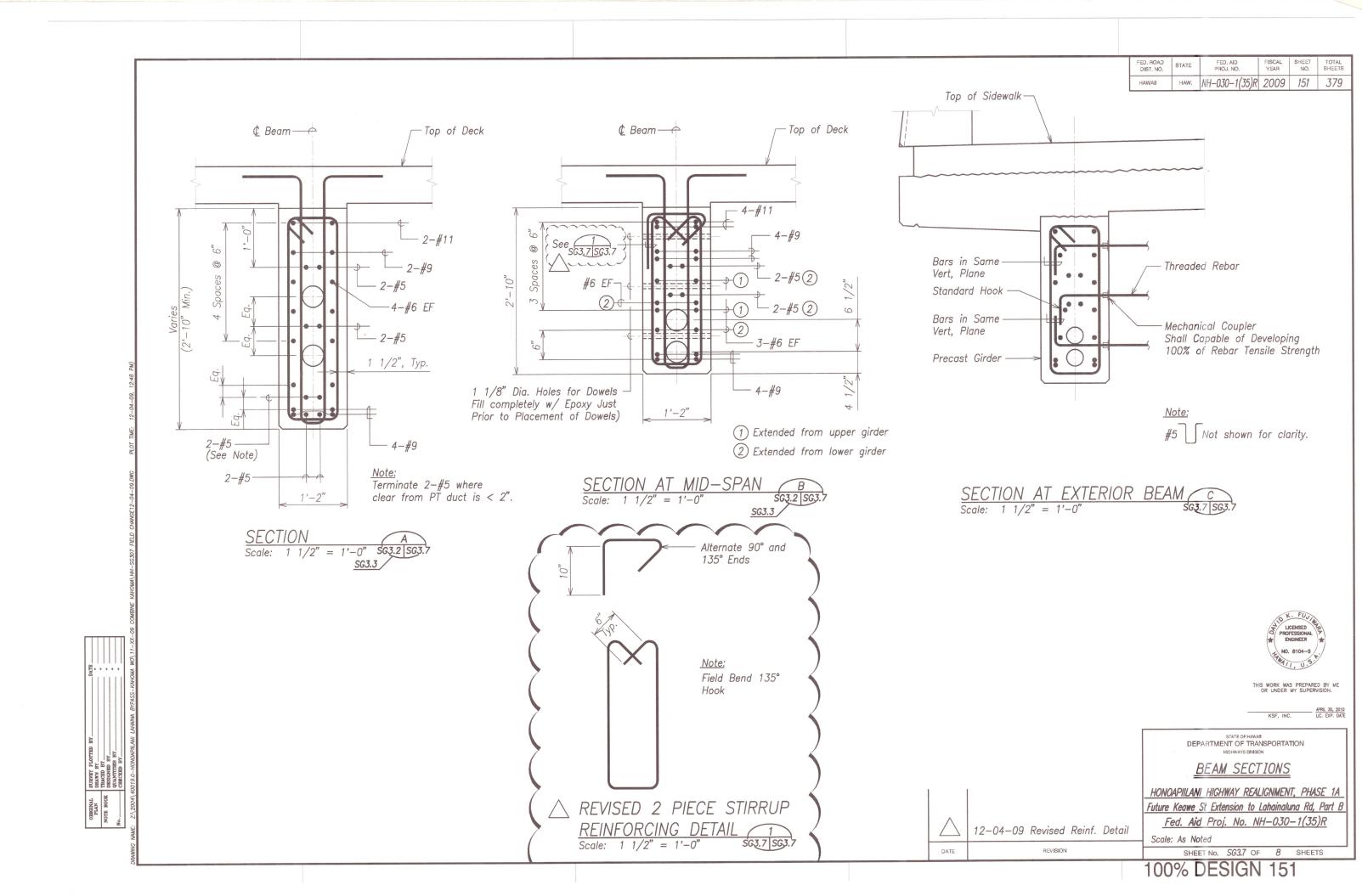


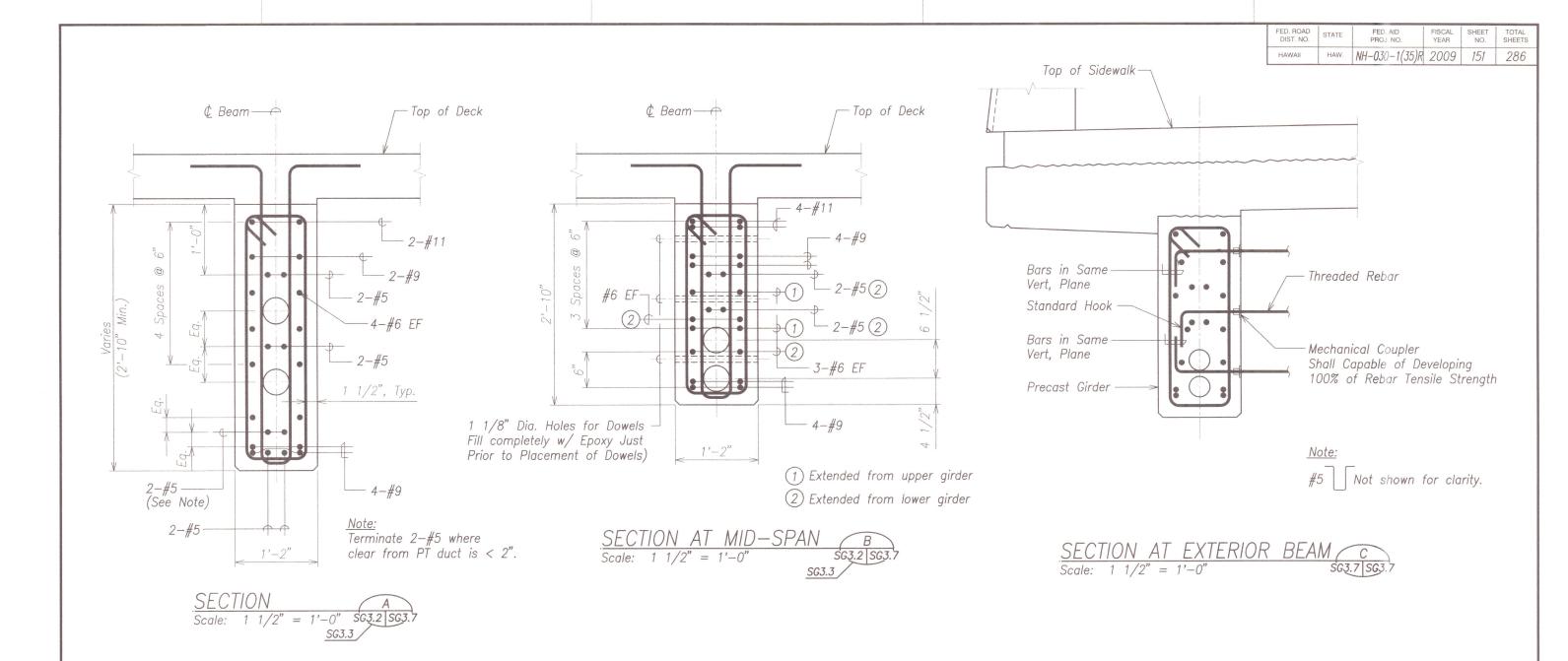
Future Keawe St Extension to Lahainaluna Rd Scale: As Noted

Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R Date: April, 2009

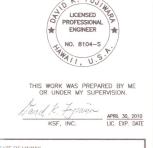
HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A

SHEET No. SG3.6 OF 8 SHEETS









DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

BEAM SECTIONS

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A

Future Keawe St Extension to Lahainaluna Rd

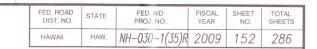
Part A: Off Ramp Mass Grading

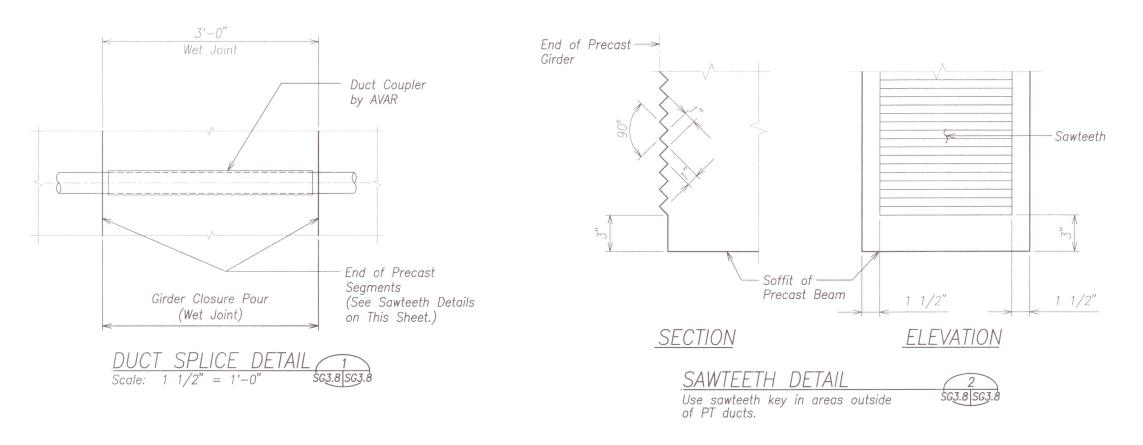
Fed. Aid Proj. No. NH-030-1(35)R

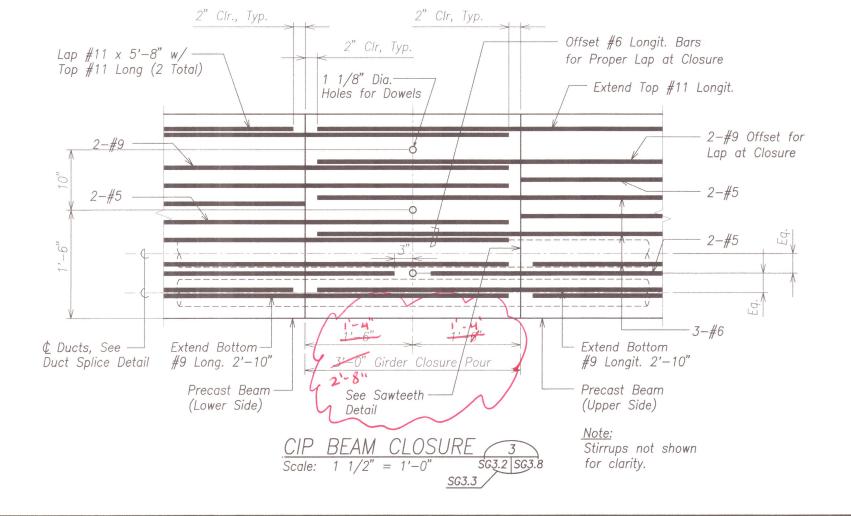
Scale: As Noted Date: April, 2009

SHEET No. SG3.7 OF 8 SHEETS

Per. 151









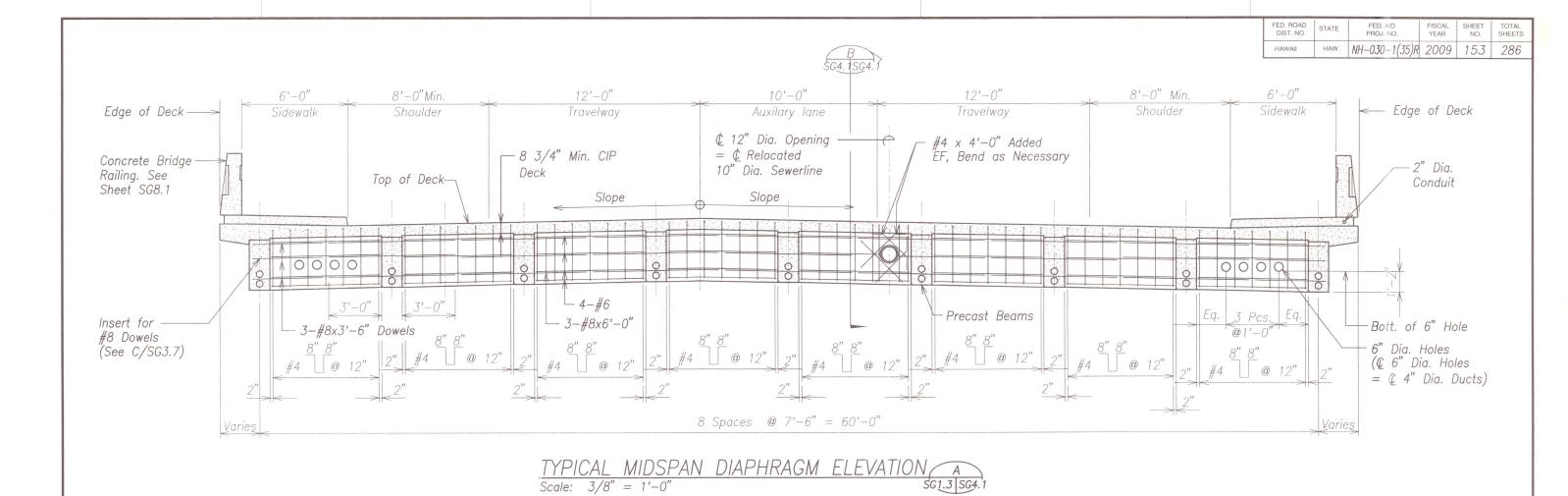
STATE OF HAWARI
DEPARTMENT OF TRANSPORTATION

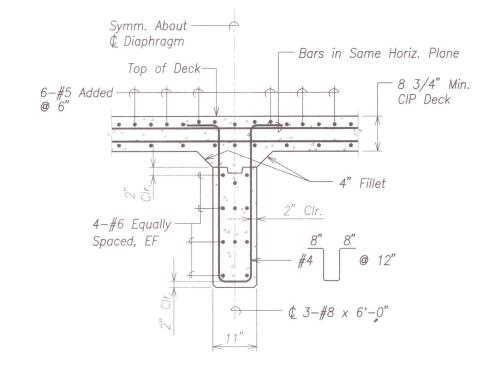
BEAM CLOSURE DETAILS

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R

Scale: As Noted Date: April, 2009

SHEET No. SG3.8 OF 8 SHEETS





- 1. Section is taken perpendicular to Precast beams.
- 2. For Precast beams, see sheets SG3.1 through SG3.8.
- 3. Pipe supports (S10) shall be spaced @ 8'-0" oc max. and as noted on SG2.1. Verify location with pipe manufacturer.
- 4. See Hawaii State Standard Plans Sheet B-01 for drip details.



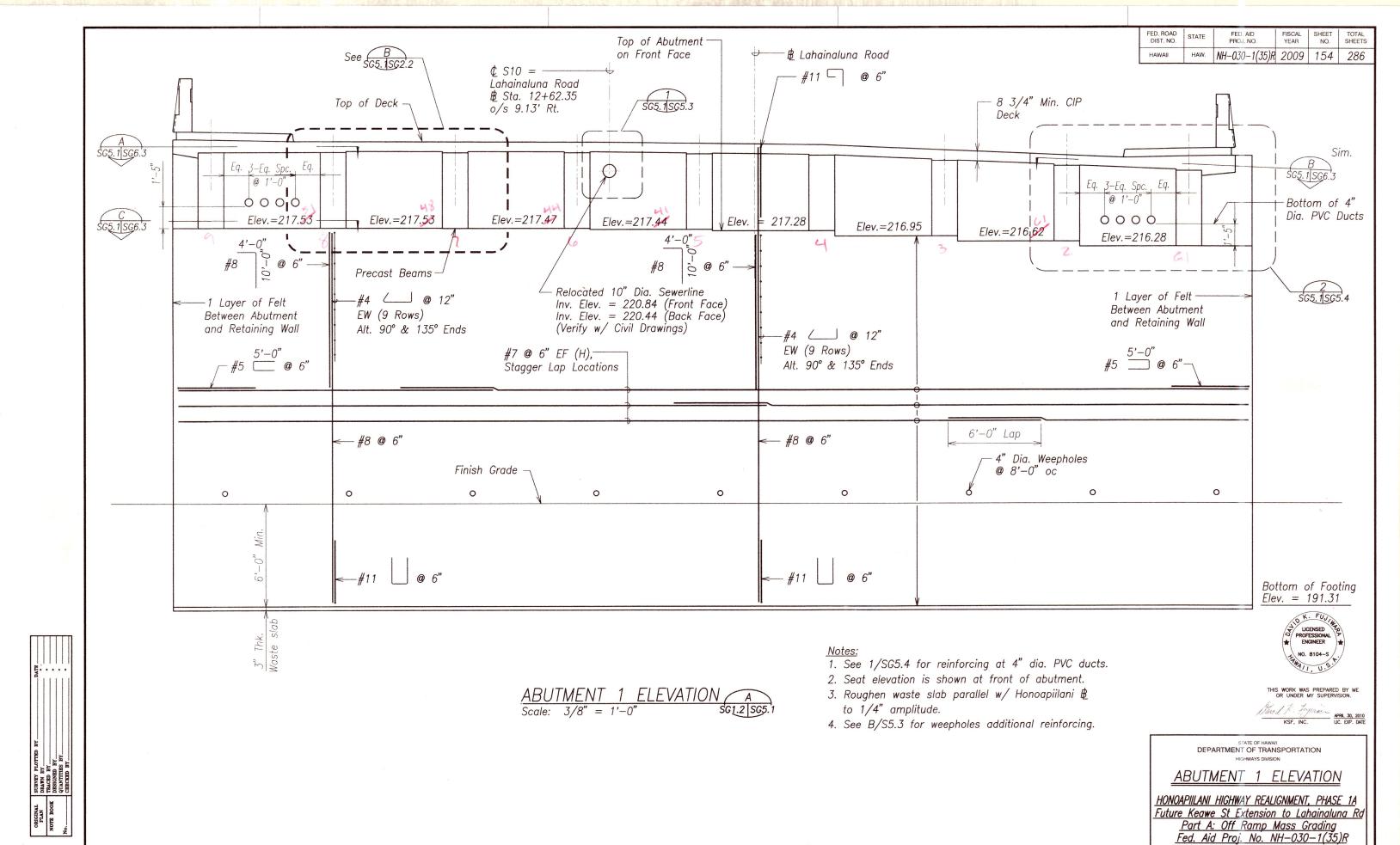
STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION

TYPICAL MIDSPAN DIAPHRAGM

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R Scale: As Noted Date: April, 2009

SHEET No. SG4.1 OF

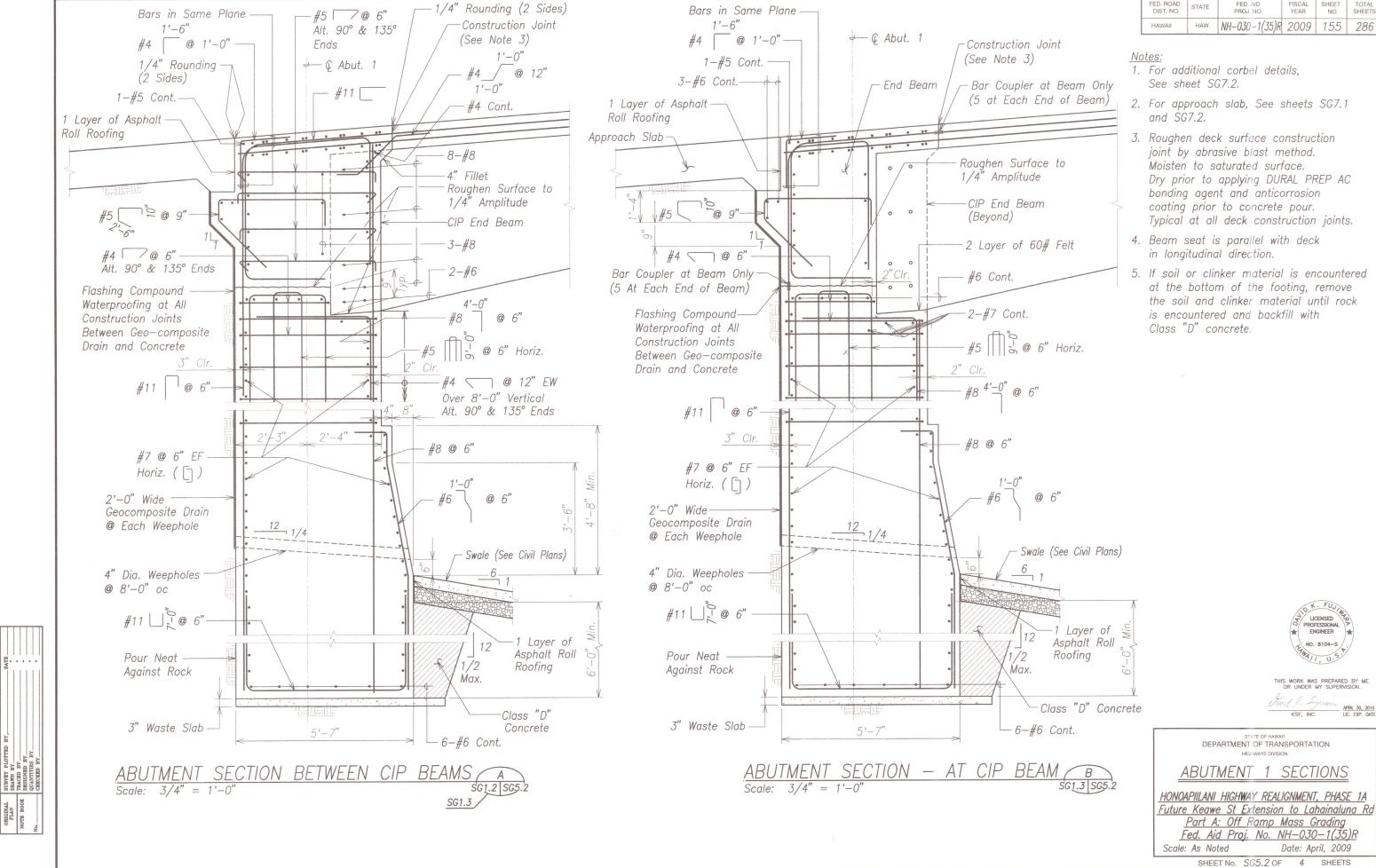
1 SHEETS 153



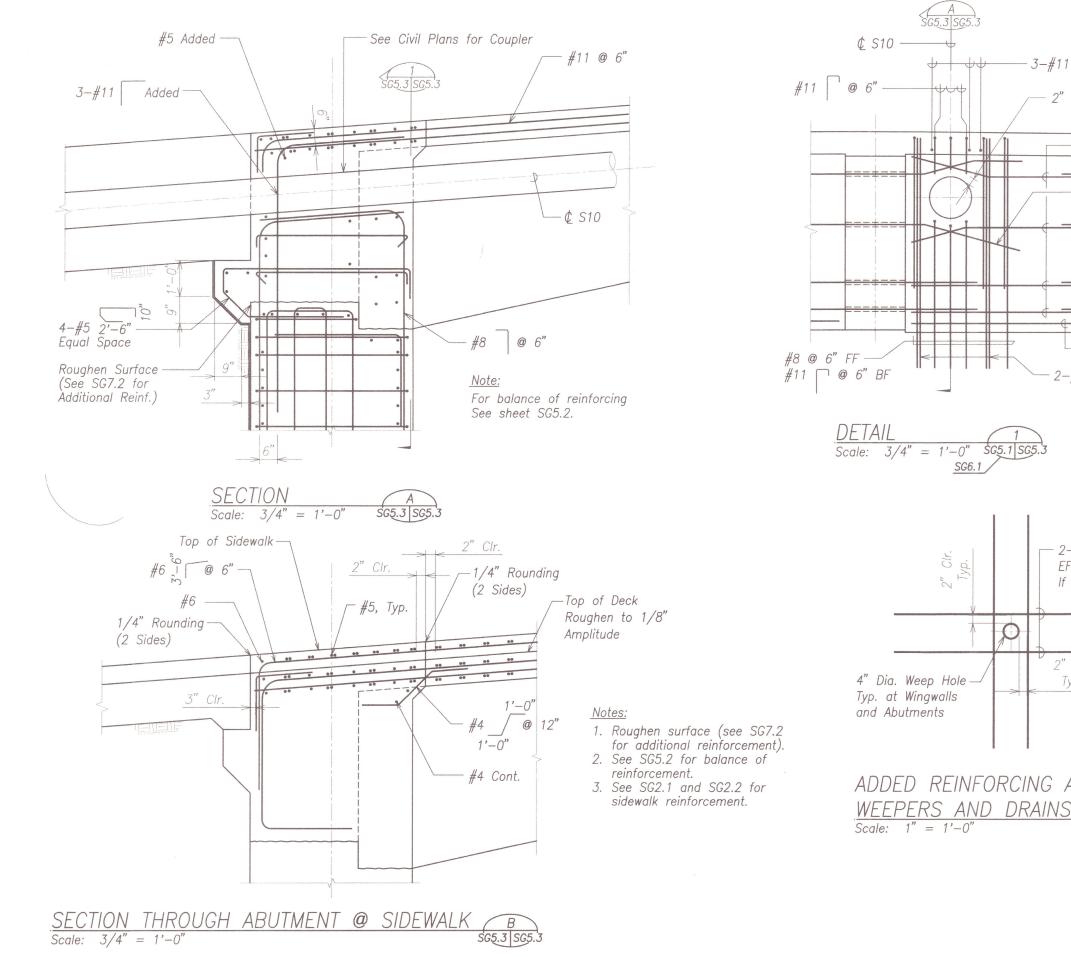
SHEET No. SG5.1 OF 4 SHEETS

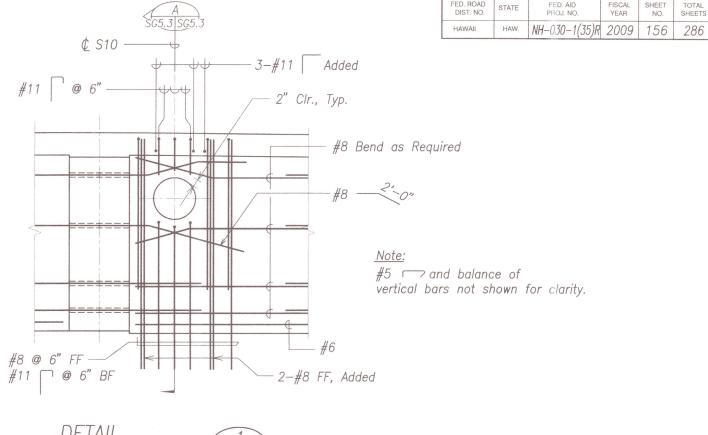
Date: April, 2009

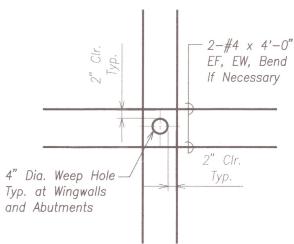
Scale: As Noted



FISCAL YEAR







ADDED REINFORCING AT WEEPERS AND DRAINS
Scale: 1" = 1'-0"



FISCAL YEAR

SHEET TOTAL SHEETS

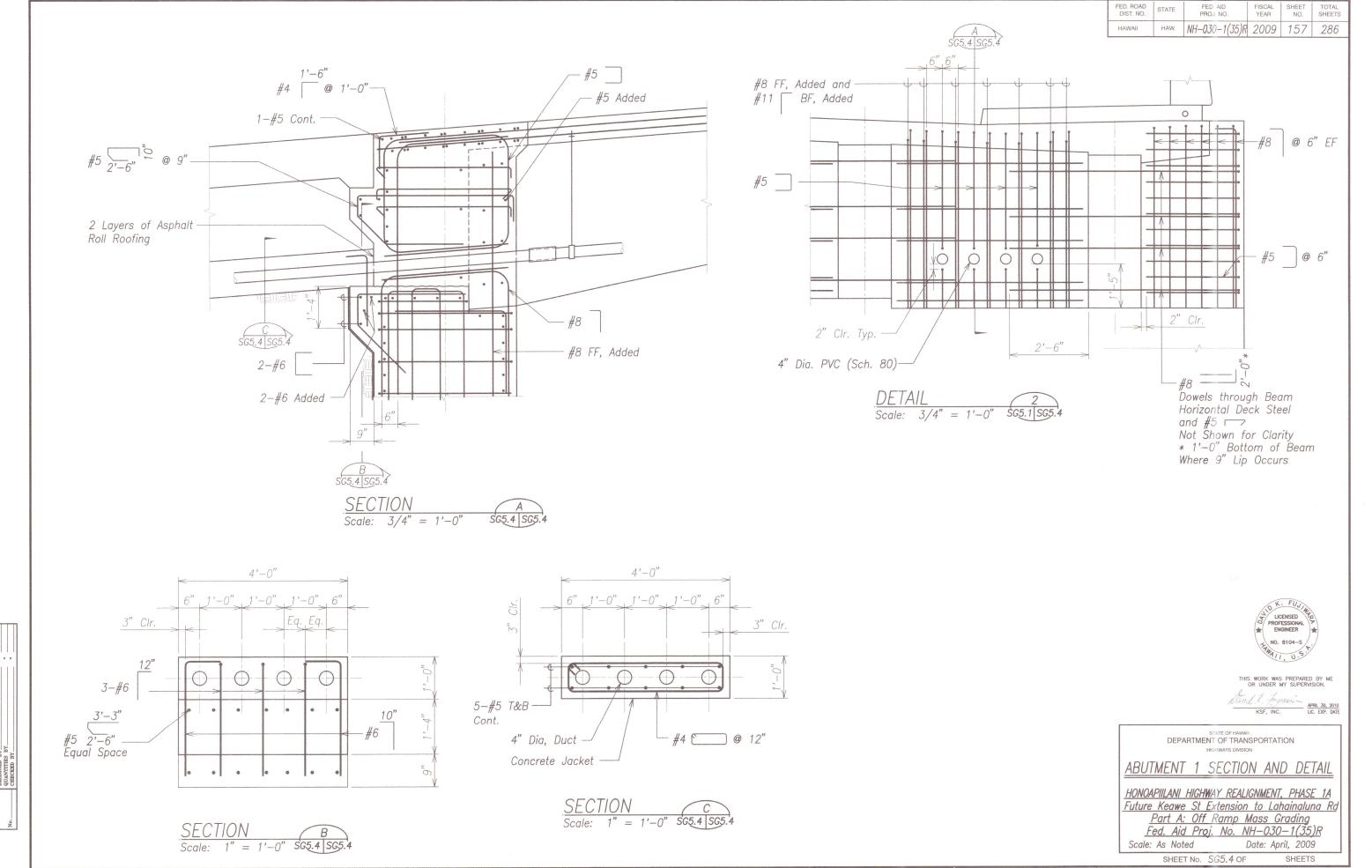
DEPARTMENT OF TRANSPORTATION

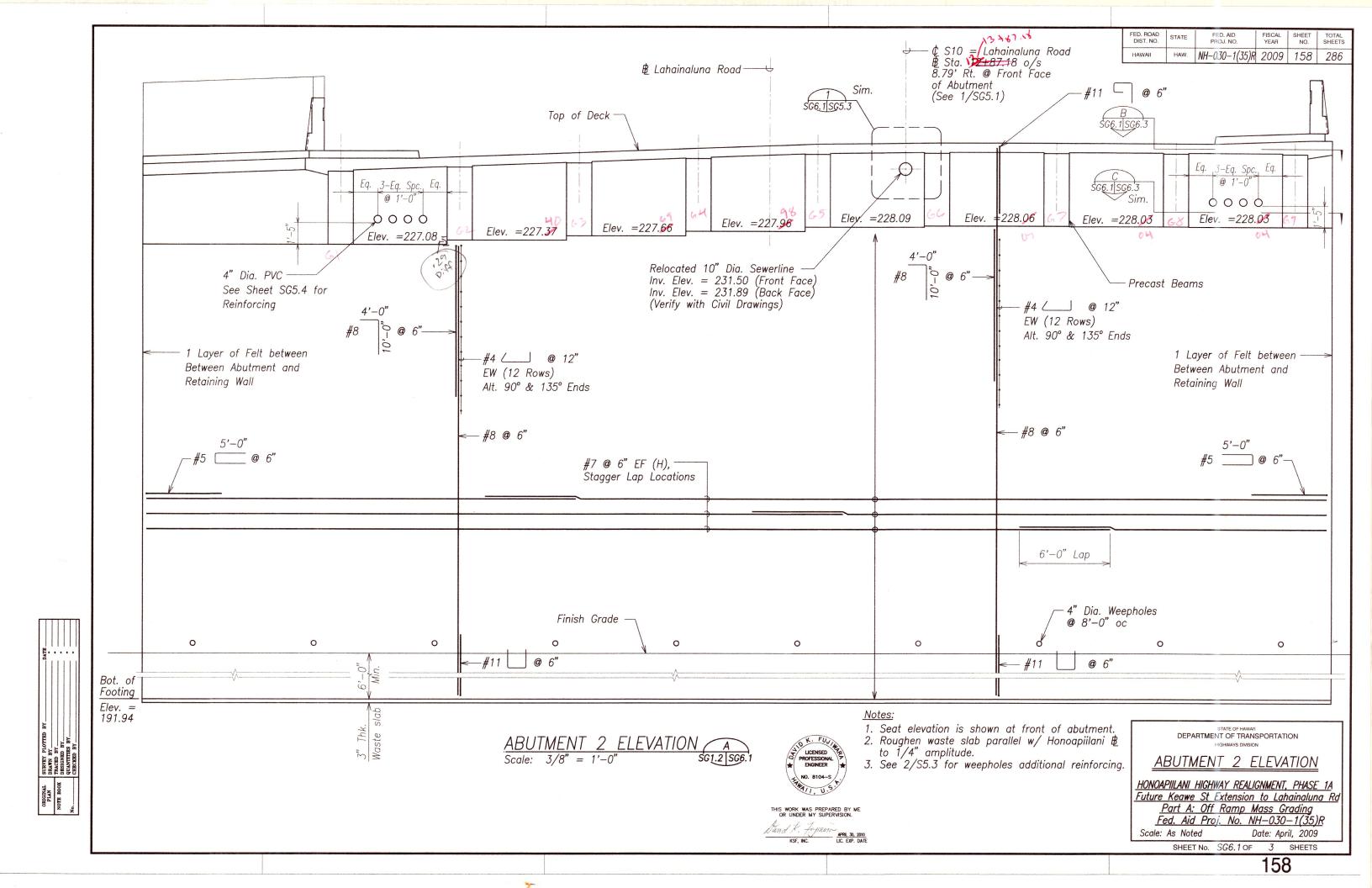
ABUTMENT 1 SECTION AND DETAIL

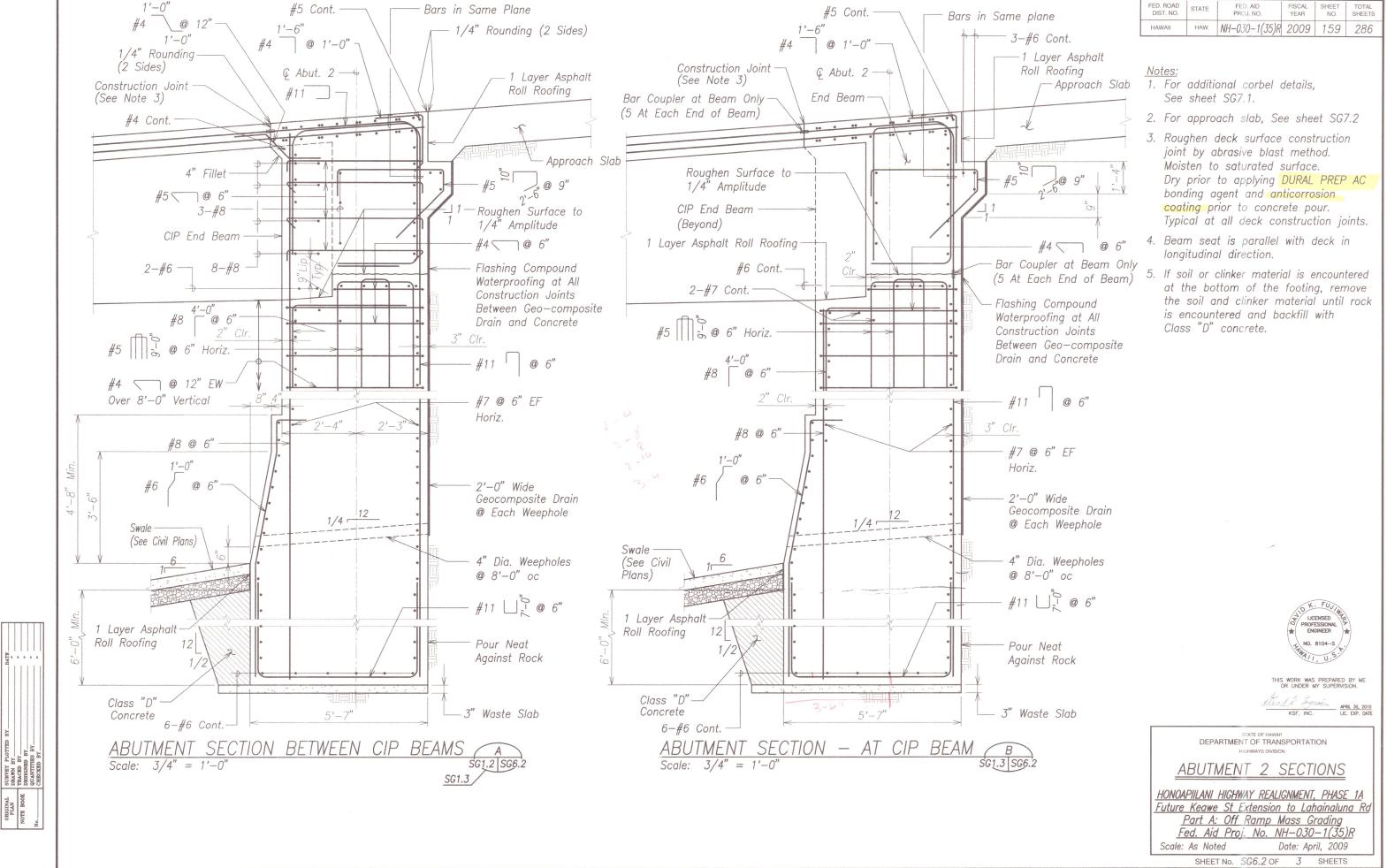
HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd
Part A: Off Ramp Mass Grading
Fed. Aid Proj. No. NH-030-1(35)R

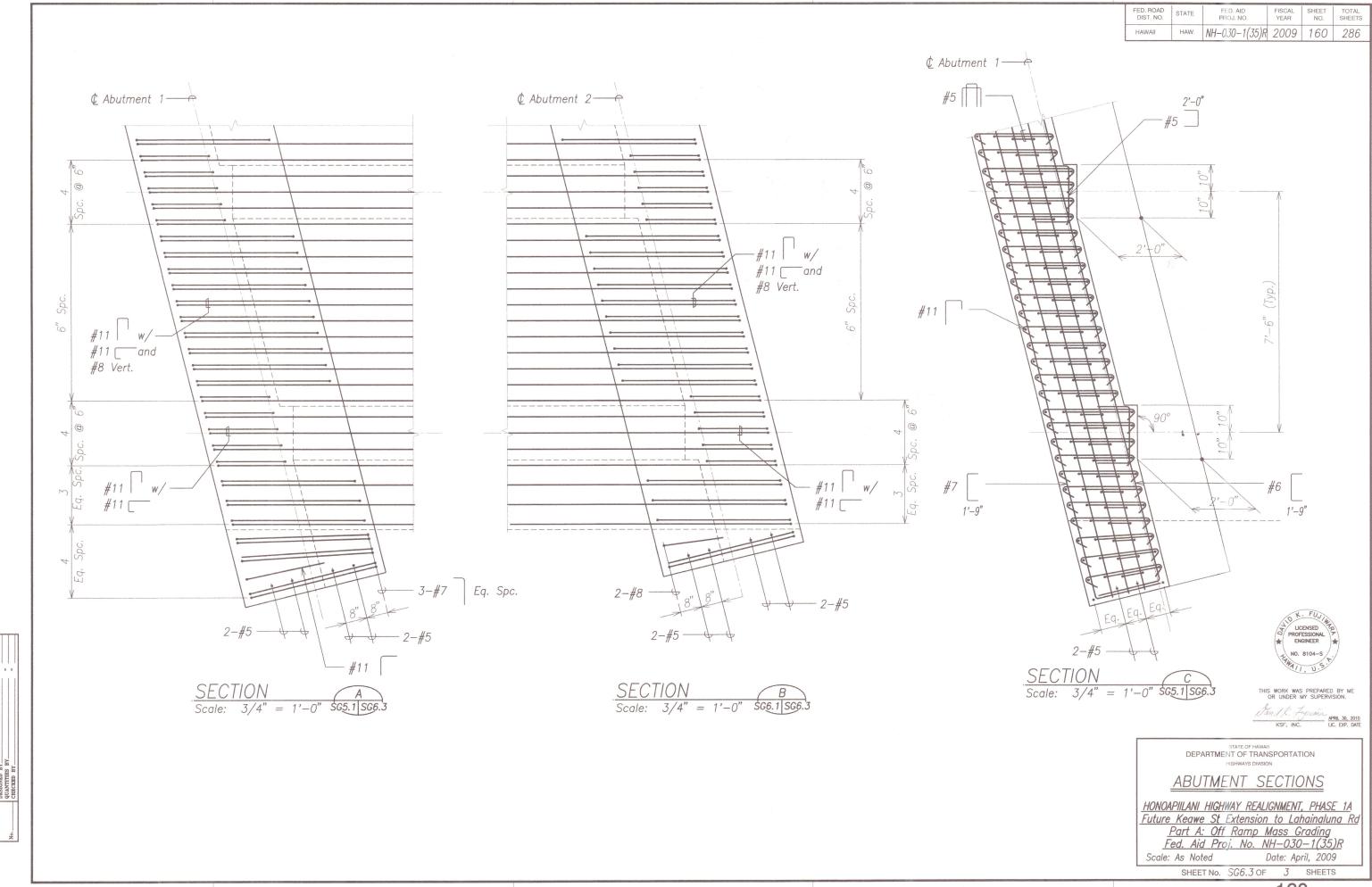
Scale: As Noted Date: April, 2009

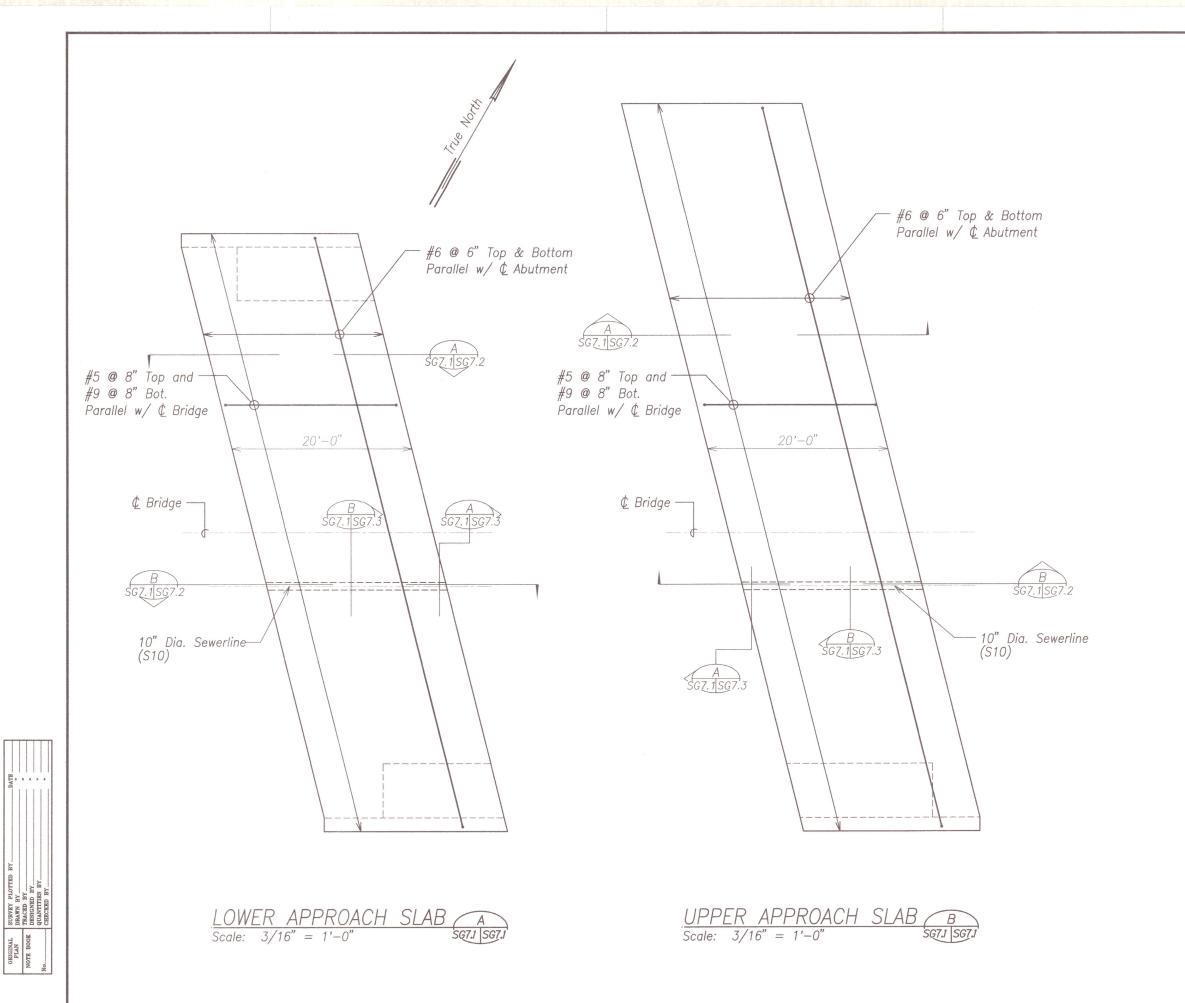
SHEET No. SG5.3 OF 4 SHEETS











FISCAL YEAR SHEET TOTAL SHEETS STATE HAW. NH-030-1(35)R 2009 161 286



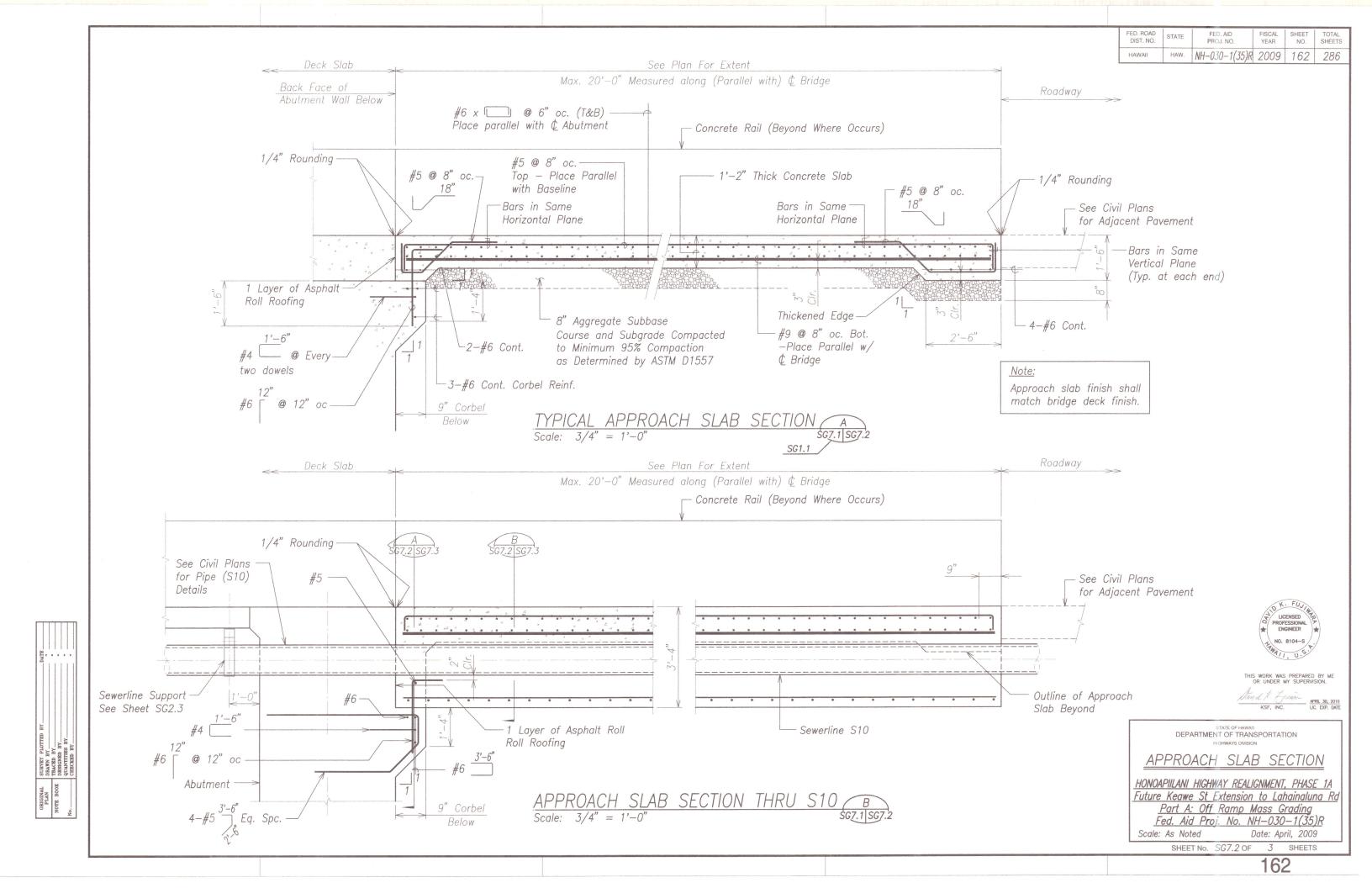
STATE OF HAWARI
DEPARTMENT OF TRANSPORTATION

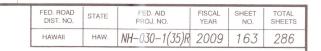
APPROACH SLAB PLANS

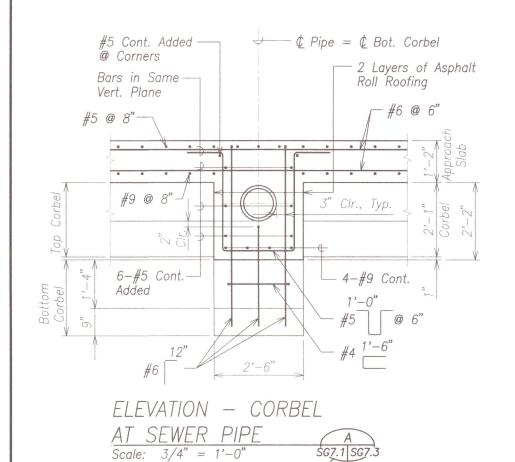
HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd
Part A: Off Ramp Mass Grading
Fed. Aid Proj. No. NH-030-1(35)R Date: April, 2009

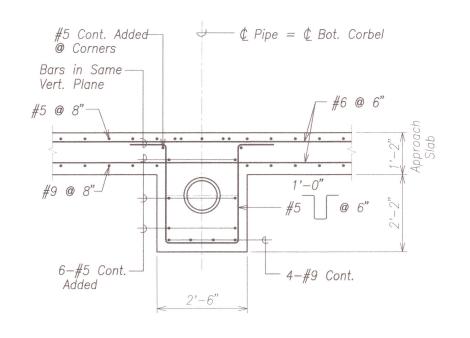
Scale: As Noted

SHEET No. SG7.1 OF 3 SHEETS













STATE OF HAWARI
DEPARTMENT OF TRANSPORTATION

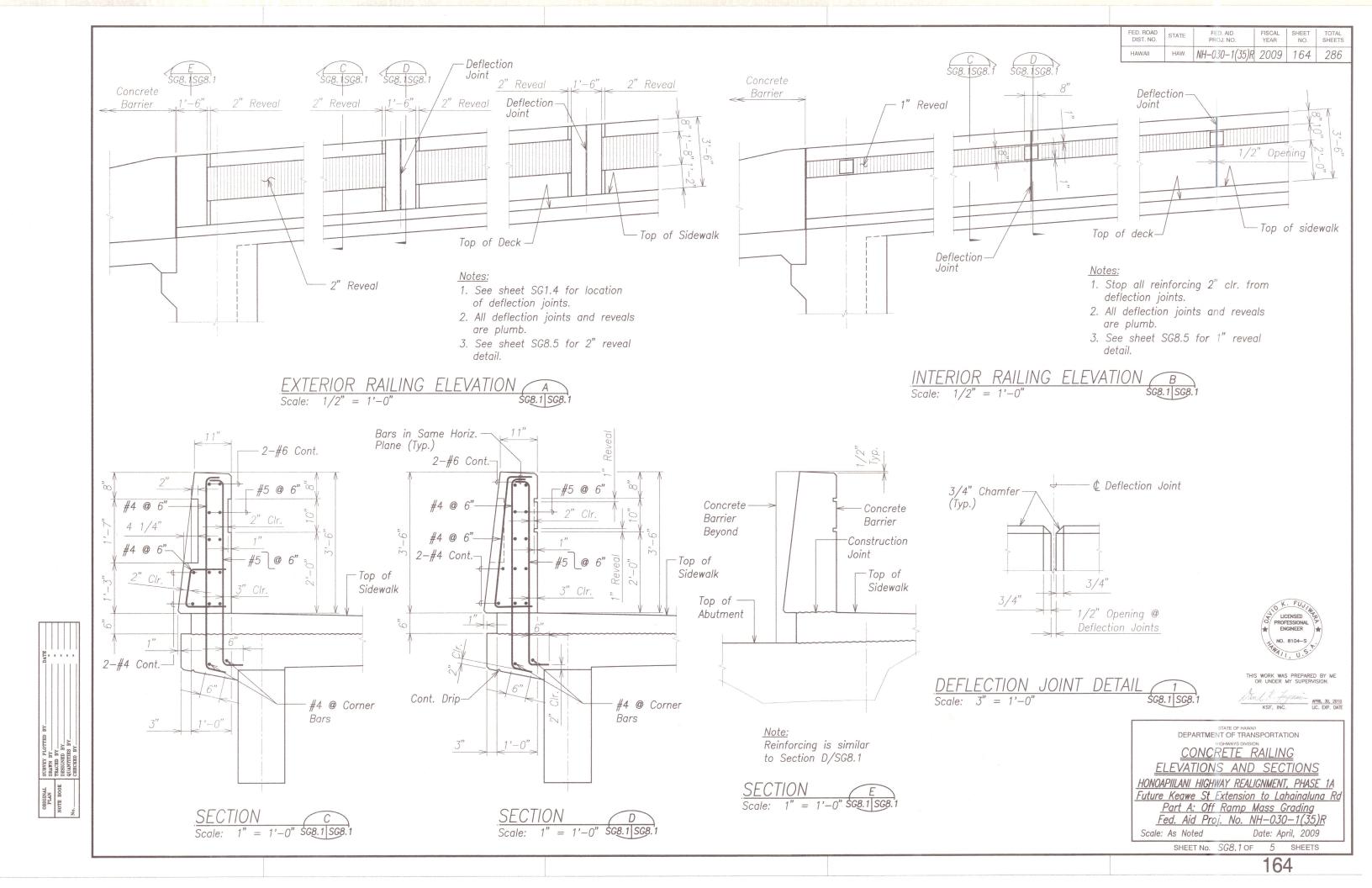
APPROACH SLAB SECTIONS

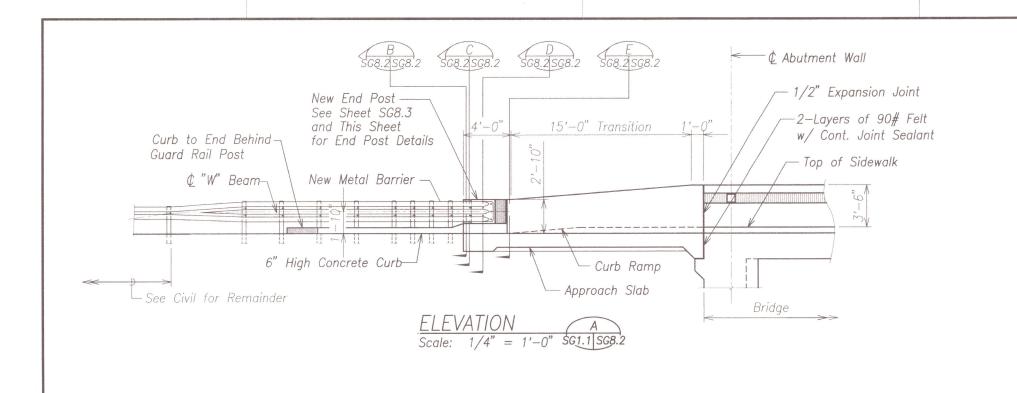
HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd
Part A: Off Ramp Mass Grading
Fed. Aid Proj. No. NH-030-1(35)R Date: April, 2009

Scale: As Noted

SHEET No. SG7.3 OF 3 SHEETS







FISCAL YEAR STATE HAWAII HAW. NH-030-1(35)R 2009 165 286

SG8.2 SG8.2

-Depressed "V" Letters 3/8" Deep

Name of Bridge

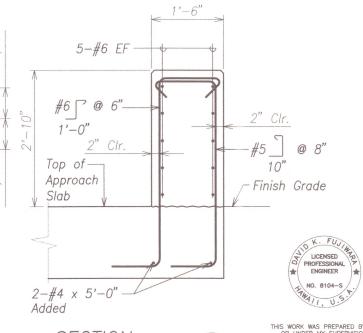
Date of Year Built

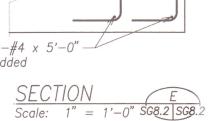
- 1. Exact details and spacing of letter and figures and location shall be as directed by the Engineer. Gothic letters and figures approximating the dimensions shown will be acceptable if approved by the Engineer.
- 2. Name & date shall be place on the trailing (exit) end post on each side of bridge.
- 3. Unless otherwise directed by the Engineer, The name of the bridge shall be "LAHAINALUNA ROAD GRADE SEPARATION". The date shall be 2009.

TYPICAL DETAIL OF LETTERS AND FIGURES

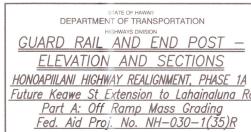
BRIDGE IDENTIFICATION DETAIL

Scale: 3'' = 1'-0''

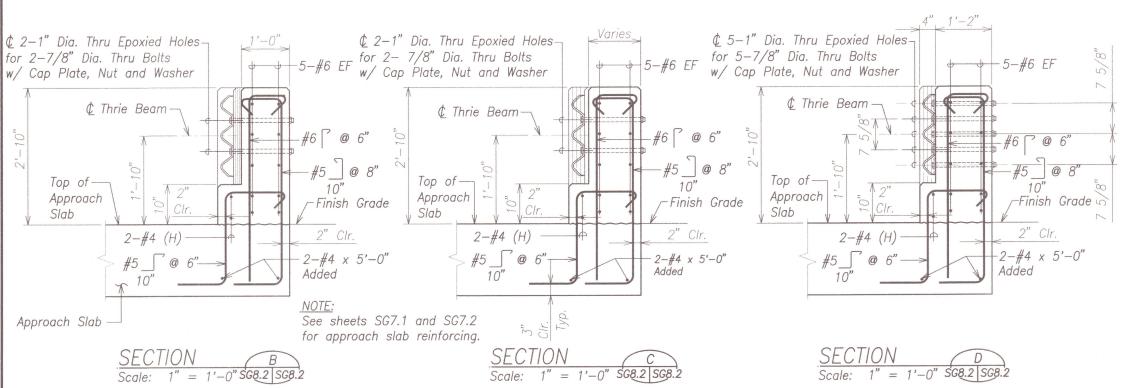


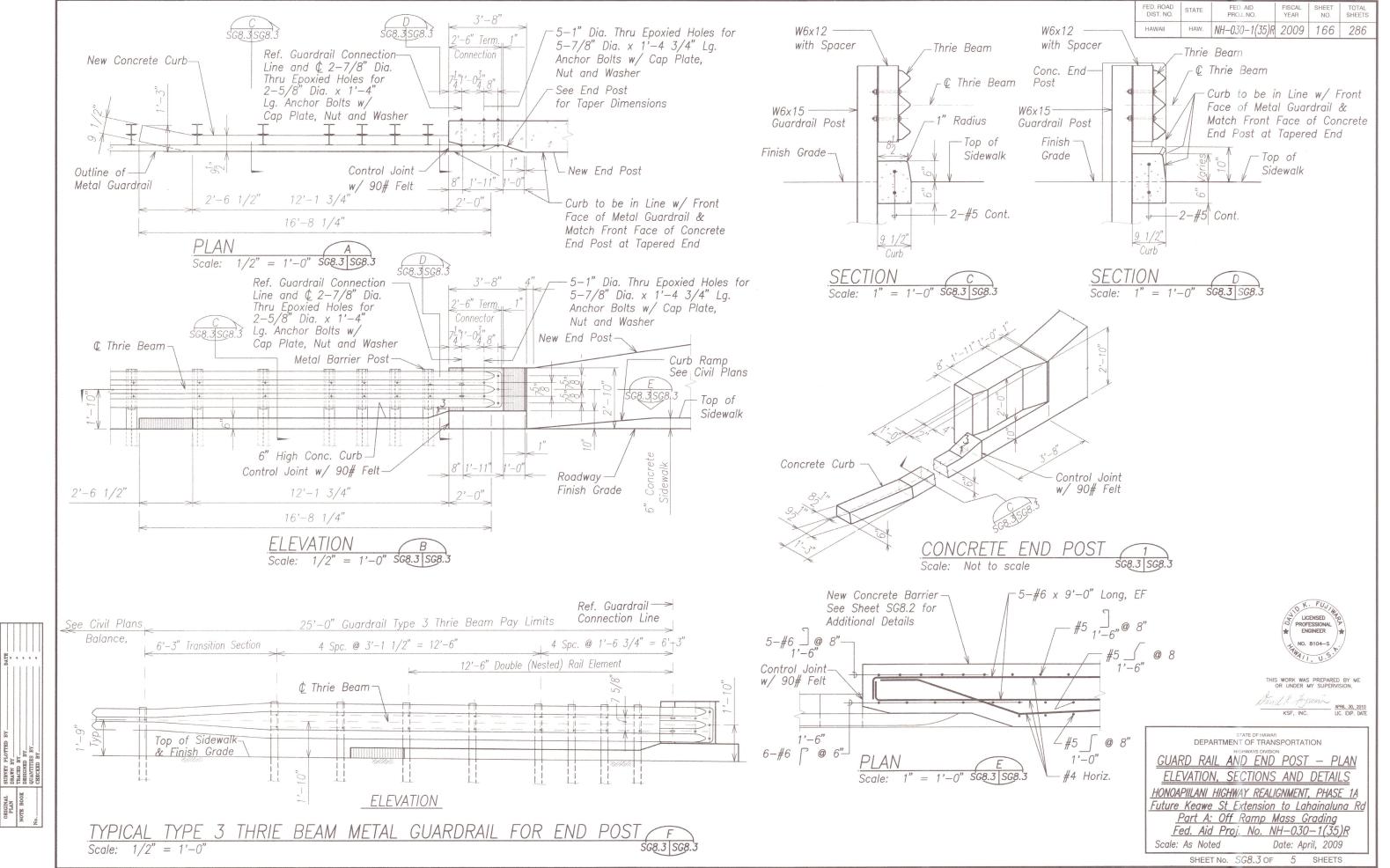


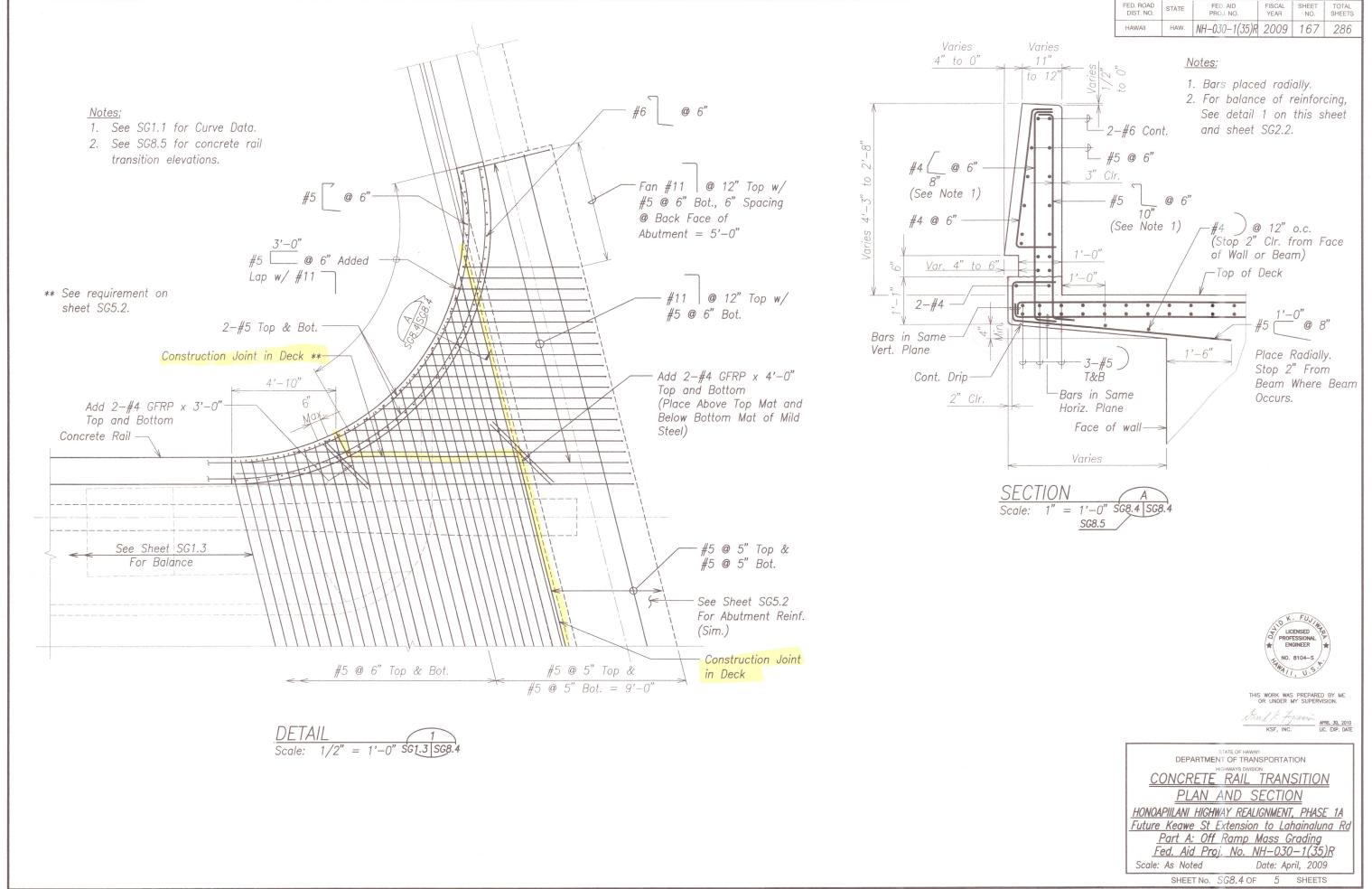


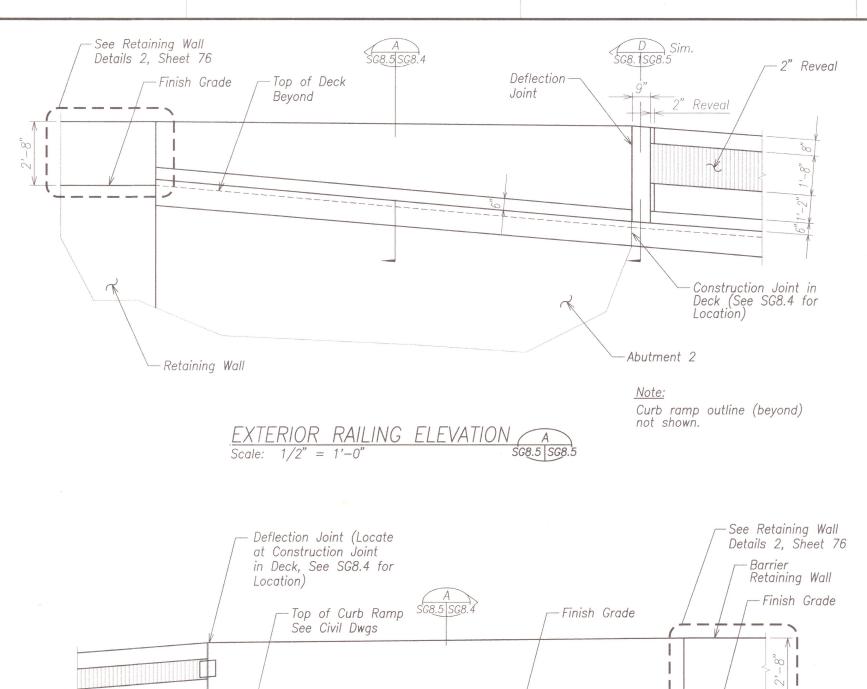


Scale: As Noted Date: April, 2009 SHEET No. SG8.2 OF 5 SHEETS

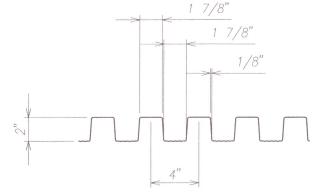








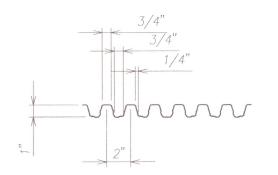
INTERIOR RAILING ELEVATION (Scale: 1/2" = 1'-0" \$6



2" FLORIDA FIN REVEAL

FORMLINER DETAIL

Scale: 3" = 1'-0" \$68.5 SG8.5



1" SAN DIEGO FIN REVEAL

FORMLINER DETAIL

Scale: 3" = 1'-0"

\$68.5 | SG8.5|



FISCAL YEAR

HAW. NH-030-1(35)R 2009 168 286

STATE

THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

MARIL 30, 2011
KSF, INC. LIC. EXP. DAT

STATE OF HAWAI'I
DEPARTMENT OF TRANSPORTATION

CONCRETE RAIL
TRANSITION ELEVATIONS

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R

Scale: As Noted

SHEET No. SG8.5 OF 5 SHEETS

| PIAN | PRAW BY | PROPER BY | PRAW BY | PRAW

168

Date: April, 2009

HAW. NH-030-1(35)R 2009 169 379

Pour end beams, and corbels at least 7 days after the concrete pour in Stage 16. The concrete pour shall occur between mid-

[Coordinate installation and removal of grout tubes with AVAR.]

Remove falsework under on-ramp at least 14 days after the

Grout post tension ducts. Grout shall be prepackaged.

Backfill remainder of abutment and construct approach slab and curb ramps once grout in Stage |19| has aged for 72 hours and has attained a compressive strength of 5,000 psi.

Construct railings and end post after approach slab has aged 7 days and has attained a compressive strength of 4,000 psi.

Grind and mechanical groove deck and approach slab 7 days after the concrete pour in Stage 21 but not before the concrete in Stage | 21 has attained a compressive strength of 4,000 psi. Re-apply SINAK LITHIUM over entire bridge deck and approach slab.

Bridge may be opened for traffic.

4/20/10

Revised Construction Sequence

REVISION

- 1. Order of construction sequence shall not be changed.
- 2. Each sequence stage shall be completely finished before proceeding to the next stage unless otherwise noted. The Engineer of record will be the sole judge of whether the sequence stage is complete, and may direct the Contractor to stop work on a sequence stage to complete work on the preceeding sequence stage.



KSF, INC. LIC. EXP. DATE

STATE OF HAWAIN
DEPARTMENT OF TRANSPORTATION

CONSTRUCTION SEQUENCE

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd, Part B

Fed. Aid Proj. No. NH-030-1(35)R

Scale: As Noted SHEET No. SG9.1 OF 1 SHEETS

100% DESIGN 169

CONSTRUCTION SEQUENCE - LAHAINALUNA ROAD GRADE SEPARATION STRUCTURE STAGE STAGE Construct detour road. See Civil Plans. Pour deck. Apply corrosion inhibitor amine carboxylate powder 2 Cast beams. (Cortec MCI-309) in all post tension ducts. Post tension longitudinal tendons in beams 28 days after concrete pour in Stages 8 and 9, and the concrete in Probe and grout at abutments. Stages 8 and 9 have attained a compressive strength Pour waste slab. of 8,000 psi. Construct abutments after concrete in stage $\boxed{3}$ has attained a compressive strength of at least 3,000 psi. Grout post tension ducts. Install diagonal (tilt-up) bracing prior to removing Grout shall be prepackaged. horizontal bracing for abutment formwork/falsework. Note: Stages 5 and 6 maybe done concurrently. Pour sidewalks once grout in Stage $\lfloor 11 \rfloor$ has aged for 72 hours and has attained a compressive strength of 5,000 psi. 5 Backfill in front of abutment footing w/ Class "D" concrete. Backfill in back of abutment up to an elevation that is 1 ft. Remove wedge between beams end of and 9" abutment lip. below horiz, joint after concrete in stage | 4 | has attained Remove falsework supporting vertical loads 21 days after concrete a compressive strength of at least 8,000 psi. pour in stage | 12 | and the concrete in stage | 12 | has attained 6 Install falsework. a compressive strength of 8,000 psi. Place wedge between bottom 9" at end of beams and 9" Do not remove diagonal (Tilt-up) bracing. vertical abutment lip. Fill gaps behind the girders at the abutments with the same Erect beams on falsework. prepackage post tensioning grout as used in the girders. Inject epoxy into gaps under girders with Sikadur 55 SLV. Pour closure joint and diaphragm. Abutment 2 -Approach slab Exterior concrete-♠ Abutment 1— ♠ Slope 8.50% bridge railing Approach slab Falsework 4 Diagonal (Tilt-up)-Bracing Pour neat Pour neat 4" Dia. weephole against rock – Diagonal (Tilt–up) against rock Finish grade @ 8'-0" oc (Typ.) Bracing 4 (Note: Remove tilt bracing in Stage [16])

 FED. ROAD DIST. NO.
 STATE
 FED. AID PROJ. NO.
 FISCAL YEAR
 SHEET NO.
 TOTAL SHEETS

 HAWAII
 HAW.
 NH-030-1(35)R
 2009
 169
 286

STAGE

- Pour end beams, remainder of deck, sidewqlk, and corbel at least 72 hours after the grouting operation in Stage 14, whichever occurs later. The concrete pour shall occur between midnight and 3:00 AM (3 hours).
- Remove diagonal (Tilt—up) bracing at least 14 days after the concrete pour in Stage 15 and until the concrete in Stage 15 has attained a compressive strength of 7,000 psi.
- Backfill remainder of abutment and construct approach slab.
- Construct railings and end post after approach slab has aged 14 days and has attained a compressive strength of 4,000 psi.
- Grind and mechanical groove deck and approach slab
 14 days after the concrete pour in Stage 17 but not before
 the concrete in Stage 17 has attained a compressive
 strength of 4,000 psi. Re-apply SINAK LITHIUM over
 entire bridge deck and approach slab.
- Bridge may be opened for traffic.

CONSTRUCTION SEQUENCE NOTES:

- 1. Order of construction sequence shall not be changed.
- 2. Each sequence stage shall be completely finished before proceeding to the next stage unless otherwise noted. The Engineer of record will be the sole judge of whether the sequence stage is complete, and may direct the Contractor to stop work on a sequence stage to complete work on the preceeding sequence stage.



THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

WAS APRIL 30, 2010

KSF, INC. U.C. EXP. DATE

DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

CONSTRUCTION SEQUENCE

HONOAPIILANI HIGHWAY REALIGNMENT, PHASE 1A Future Keawe St Extension to Lahainaluna Rd Part A: Off Ramp Mass Grading Fed. Aid Proj. No. NH-030-1(35)R

Scale: As Noted Date: April, 2009

SHEET No. SG9.1 OF 1 SHEETS

Reve -

169



CONSTRUCTION SEQUENCE A
Scale: 1" = 10'-0" SG9.1 SG9.1