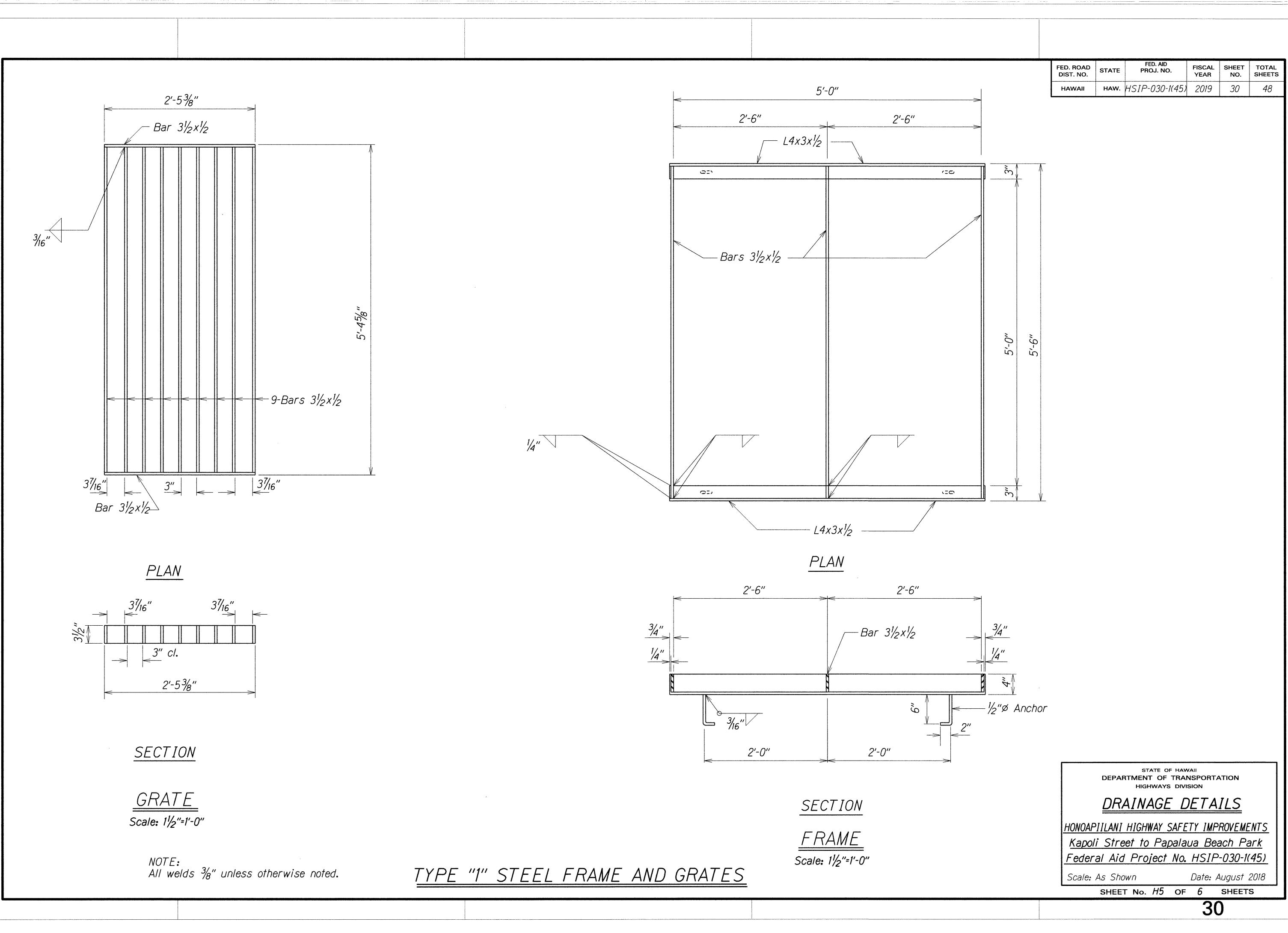
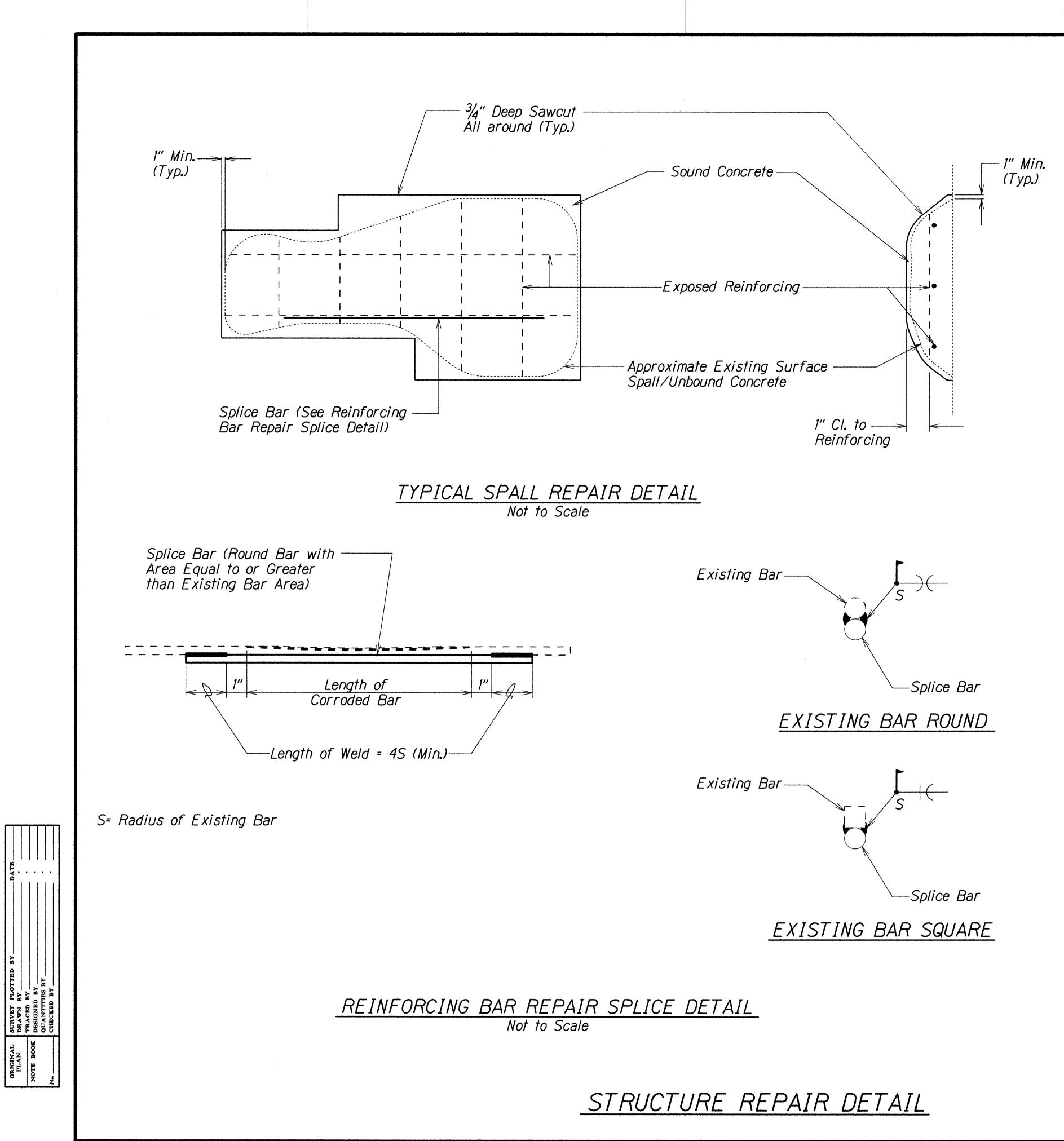


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ORIGINAL PLAN	NOTE BOOK ddx.× N. X.dgn



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ORIGINAL	PLAN	NOTE BOOK	dt02.dgn	



RECOMMENDATIONS.

A RUST PRIMER.

ED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
 HAWAII	HAW.	HSIP-030-1(45)	2019	31	48

GENERAL SPALL REPAIR NOTES:

1. CONTRACTOR SHALL PROTECT EXISTING SURFACES AND OBJECTS TO REMAIN FROM DAMAGE. ANY ITEM TO REMAIN THAT IS DAMAGED BY THE CONTRACTOR SHALL BE REPLACED OR REPAIRED TO MATCH THE EXISTING ADJACENT SURFACES AT NO ADDITIONAL COST TO THE STATE. 2. EDGES OF SPALL REPAIRS SHALL NOT BE FEATHERED. PROVIDE A^{3}_{4} "

DEEP SQUARE CUT ALONG PERIMETER OF SPALL REPAIRS. 3. SURFACE PREPARATION OF REPAIR AREA, AND MIXING AND APPLICATION OF REPAIR MATERIAL SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S

4. SPALLS AND DELAMINATIONS ARE CALLED OUT AS "SPALLS". NO SEPARATE DISTINCTION IS MADE BETWEEN THEM SINCE THE REPAIRS ARE THE SAME.

SPALL REPAIR MATERIALS:

1. SPLICE BARS TO BE WELDED TO EXISTING STEEL REINFORCING BARS SHALL BE DEFORMED REINFORCING BARS CONFORMING TO ASTM A706, GRADE 60. WELDING OF STEEL REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE LATEST ADOPTED AWS DI.4 - STRUCTURAL WELDING CODE - REINFORCING STEEL. 2. ALL EXPOSED STEEL REINFORCING BARS (INCLUDING WELDED SPLICE BARS) SHALL BE COATED WITH "VOCI CORRVERTER" RUST PRIMER AS SUPPLIED BY CORTEC CORPORATION. SURFACE PREPARATION AND APPLICATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. 3. REPAIR MORTAR FOR PATCHING SPALL REPAIRS SHALL BE "MCI-2702" POLYMER-MODIFIED REPAIR MORTAR BY CORTEC CORPORATION. SURFACE PREPARATION, MIXING, AND APPLICATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

4. MIGRATING CORROSION INHIBITOR TO BE APPLIED TO THE CONCRETE SURFACE AFTER SPALL REPAIR SHALL BE "MCI-2020 V/O" SURFACE APPLIED CORROSION INHIBITOR FOR VERTICAL AND OVERHEAD APPLICATIONS BY CORTEC CORPORATION. SURFACE PREPARATION AND APPLICATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

PROCEDURES FOR SPALL REPAIR:

1. PRIOR TO START OF REPAIR WORK, CONTRACTOR SHALL SOUND THE EXISTING CONCRETE SURFACES WITH A HAMMER OR OTHER SUITABLE DEVICE. THE SURFACE SHALL BE MARKED TO IDENTIFY THE PERIMETER OF

THE REPAIR AREA. 2. PROVIDE 3/4" DEEP SQUARE CUT EDGES AROUND THE PERIMETER OF THE REPAIR AREA (SEE "SPALL REPAIR DETAIL") AND CHIP TO SOUND CONCRETE. 3. ALL EXPOSED STEEL REINFORCING BARS THAT HAVE CORRODED MORE THAN 25% OF THE ORIGINAL CROSS-SECTIONAL AREA SHALL BE STRENGTHENED AS SHOWN ON "REINFORCING BAR REPAIR SPLICE DETAIL".

4. SPLICE BARS SHALL BE ROUND DEFORMED REINFORCING BARS WITH A CROSS-SECTIONAL AREA THAT IS EQUAL TO OR GREATER THAN THE CROSS-SECTIONAL AREA OF THE EXISTING BAR.

5. ALL EXISTING STEEL REINFORCING BARS THAT ARE EXPOSED AFTER CHIPPING AND WELDED SPLICE BARS SHALL BE CLEANED AND COATED WITH

6. CLEAN THE SPALL REPAIR AREA AND PATCH WITH REPAIR MORTAR. 7. APPLY MIGRATING CORROSION INHIBITOR TO THE ENTIRE EXPOSED CONCRETE SURFACE. COVERAGE RATE DURING APPLICATION SHALL BE AS RECOMMENDED BY THE MANUFACTURER.

STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION
TYPICAL SECTION
HONOAPIILANI HIGHWAY SAFETY IMPROVEMENTS
<u>Kapoli Street to Papalaua Beach Park</u>
Federal Aid Project No. HSIP-030-1(45)
Scale:1/4"=1'-0" Date: August 2018
SHEET No. H6 OF 6 SHEETS

31

STRUCTURAL GENERAL NOTES:
<u>General:</u> A. Workmanship and materials shall conform to the AASHTO LRF.
Design Specification, 8th Edition, Hawaii Standard Specification and Bridge Construction (2005 Edition), and HDOT Design Criter Bridges and Structures, August 8, 2014.
B. The contractor shall compare all the contract documents with and report in writing to the engineer all inconsistencies and c C. The contractor shall take field measurements and verify field
and shall compare such field measurements and conditions with drawings before commencing work. Report in writing to the e inconsistencies and omissions.
D. The contractor shall be responsible for coordinating the work trades.
E. The contractor shall be responsible for means and methods of construction, workmanship and job safety.
F. The contractor shall provide temporary shoring and bracing as for stability of structural members and systems. G. Construction loading shall not exceed design live load unless s
shoring is provided. Permitted construction loads shall be provided in areas where the structure has not attained full de strength.
H. The contractor shall be responsible for protection of the adjace properties, structures, streets and utilities during the constru- Any damaged or deteriorated property shall be restored to the
prior to the beginning of work or better at no cost to the Sta I. Details noted as typical on the structural drawings shall apply conditions unless specifically shown or noted otherwise.
J. The contractor shall be responsible for verifying all existing elevation existing structure details and shall notify the engineer in writing of discrepancies for further action.
<u>Concrete:</u> A. Concrete shall be regular weight with a maximum water to cement ra
 A. Concrete shall be regular weight with a maximum water to centen relation and shall have aminimum 28-day compressive strength of 4,000 psi. B. The use of any calcium chloride in any concrete is prohibited. C. Concrete delivery tickets shall record all free water in the mi plant, added for consistency by driver, and any additional required.
<i>Contractor up to the maximum amount allowed by the mix designable.</i> D. Construction joints may be relocated by the contractor and submitted
structural engineer for approval. Construction joints shall be made as not to impair the strength of the structure and to minimize shrin stresses. All construction joints shall be cleaned, laitance removed a See typical details for specific requirements.
E. Unless otherwise noted, chamfer all exposed concrete edges 3/4". F. Reinforcing bars, anchor bolts, inserts and other items to be cast in
shall be secured in position prior to placement of concrete. G. All inserts, anchor bolts, plates, and other structural items to the concrete shall be hot-dipped galvanized according to ASTM
otherwise noted. H. Non-shrink grout shall be a premixed non-metallic formula, cap developing a minimum compressive strength of 4,000 psi in 1 d
psi in 28 days. I. Stay-in-place forms shall not be allowed.
<u>Reinforcing Steel:</u> A Poinforcing steel shall be deformed bars conforming to ASTM A615
A. Reinforcing steel shall be deformed bars conforming to ASTM A615, (unless otherwise noted. Low-Alloy reinforcing steel for welding shall ASTM A706, Grade 60 unless otherwise noted.
B. Clear concrete coverage for reinforcing bars shall be as follows, unle otherwise noted:
1. Footings, etc. cast against earth

2. Footings, walls, etc. formed and exposed to earth or weather ------ 2" C. Reinforcing steel shall be spliced only where indicated on plans. Provide lap

splice length per typical details and schedule, Sheet S0.2, unless otherwise noted. Bar bends and hook shall be "standard hooks" in accordance with typical details Sheet SO.2.

E. SURVEY PLOTT DRAWN BY TRACED BY DESIGNED BY QUANTITIES BY CHECKED BY ORIGINAL PLAN NOTE BOOK No.__ Existing Concrete:

A. Contractor shall not damage, cut or drill through existing reinforcing that is to remain and as noted on plans. If reinforcing is damaged, the contractor shall inform the engineer immediately and shall be responsible for repairing the damage at contractor's sole expense and to the satisfaction of the engineer.

- B. All holes which need to be abandoned due to the presence of reinforcing, shall be filled with epoxy or non-shrink grout.
- C. The contractor will not be paid for the holes which need to be filled and abandoned. the engineer shall review and approve all relocated holes prior to installing dowels.
- D. All drilled holes for anchors shall be brushed to remove loose material then cleaned with compressed air, prior to injecting the epoxy.
- E. Anchoring adhesive shall be a two-component 100% solids epoxy based system supplied in manufacturer's standard side-by-side cartridge and dispensed through a static-mixing nozzle supplied by the manufacturer. Epoxy shall meet the minimum requirements of ASTM C-881 specification for Type IV, Grade 3, class c and must develop a minimum of 12,650 psi compressive yield strength after 7 day cure. Epoxy shall be formulated for optimum performance in both cracked and uncracked concrete.

Surface Preparation Notes for Spall Repairs:

- A. Deteriorated concrete shall be removed down to sound substrate, or to the specified depth as noted in the spall repair details. Sawcut all edges minimum of 1" deep, no feathering of patching material is allowed. Avoid cutting any reinforcing steel when sawcutting. The exposed concrete shall be roughened to a $\frac{1}{4}$ " amplitude and shall be cleaned and free of laitance, dust and other bond inhibiting materials.
- B. All reinforcing steel damaged due to the contractor's operations shall be repaired by the contractor at his/her expense and to the satisfaction of the engineer.
- C. All loose, soft, honey-combed, disintegrated concrete, plus ³/₄" minimum depth of concrete beyond the back face of the rebar within the spall area shall be removed.
- D. After completion of the removal operation, the engineer will resound the areas to ensure that only sound concrete remains.
- E. Cleaning shall precede application of the patching material by not more than 24 hours.

Bonding Agent:

- A. After the concrete surfaces have been prepared and cleaned, and immediately before placing the concrete patching, a coat of bonding agent shall be applied. The surface shall receive a thorough and even coating, and excess bonding agent shall not be permitted to collect in pockets. The rate of progress in applying the bonding agent shall be limited so that it does not become dry before it is covered with the concrete patching. Should the surface of the bonding agent dry, the dried bonding agent shall be completely removed and fresh bonding agent applied. Removal shall be by sandblasting or by another procedure approved by the engineer. The removal of bonding agent shall be at the expense of the contractor.
- B. The bonding agent shall provide corrosion protection to the reinforcing steel and shall act as a bonding agent for the fresh patching mortar. All exposed reinforcing steel shall receive two (2) coats at 20 mils each, total of 40 mils. the concrete surface shall receive one (1) coat at 20 mils. Follow manufacturer's specifications for recommended time between application of bonding agent and patching mortar. The minimum bond strength provided by the bonding agent shall be 2,000 psi after 14 days (ASTM C-882).

Polymer Modified Patching Mortar:

A. Patching mortar shall be a polymer modified mortar, have high abrasion resistance and shall be suitable for horizontal, vertical and overhead surfaces. The minimum bond strength provided by the patching mortar shall be 2,000 psi after 28 days (ASTM C-882). The minimum compressive strength provided by the patching mortar shall be 4,000 psi after 28 days. Refer to manufacturer's specifications for preparation and application guidance.

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FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAI'I	HAW.	HSIP-030-1(45)	2019	31 S-1	48

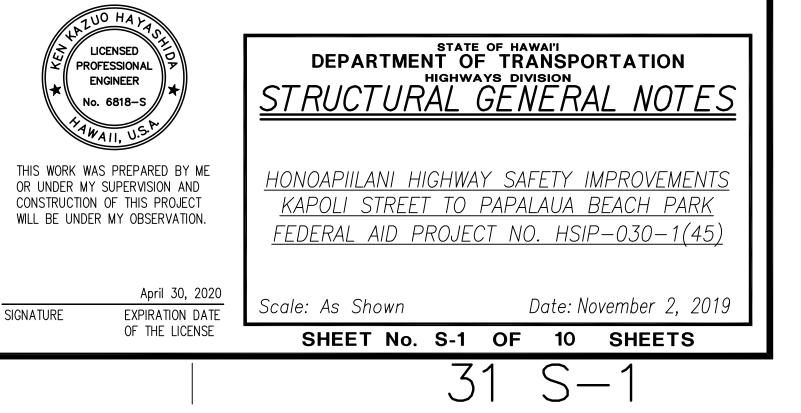
<u>Curing:</u>

A. As per ACI recommendations for portland cement concrete, curing is required. follow the manufacturer's recommendation for curing material and procedure.

Multiple Lifts:

A. Follow the manufacturer's limitations for maximum thickness for application of patching mortar. if the required thickness of a repair is greater than the single application limit, multiple lifts are required. large, unconfined or overhead repairs may also require multiple lifts. if successive lifts are to be applied, roughen the surface of the previous lift and apply subsequent lifts within the time period, both as recommended by the manufacturer.

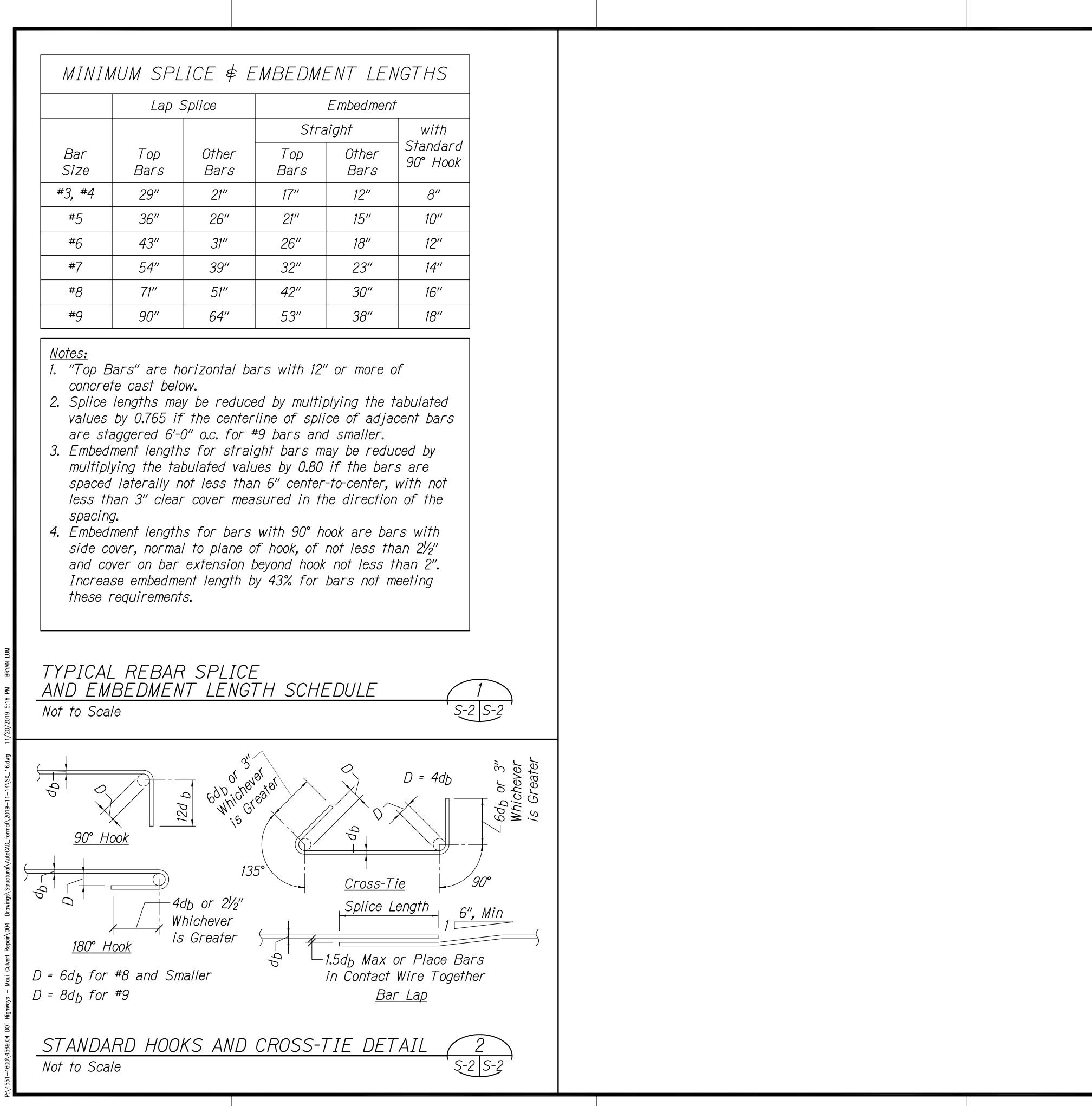


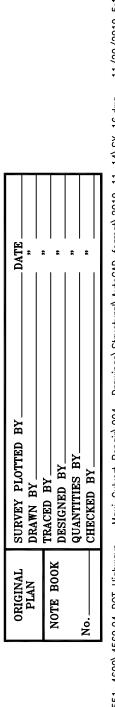


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Bar Size	Top Bars	Other Bars	Top Bars	Other Bars	[−] Standa 90° Ho
#3, #4	29″	21″	17"	12"	8"
#5	36″	26″	21″	15″	10"
#6	43″	31″	26″	18″	12"
#7	54″	39″	32"	23"	14"
#8	71″	51″	42"	30"	16"
#9	90″	64″	53″	38"	18"

- concrete cast below.
- are staggered 6'-0" o.c. for #9 bars and smaller.
- multiplying the tabulated values by 0.80 if the bars are spacing.
- Increase embedment length by 43% for bars not meeting these requirements.

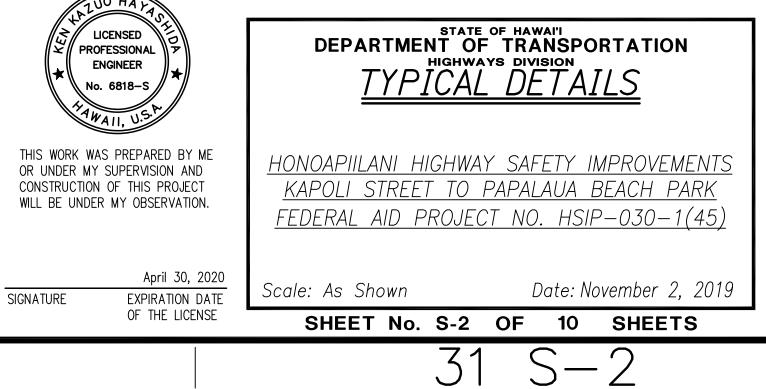




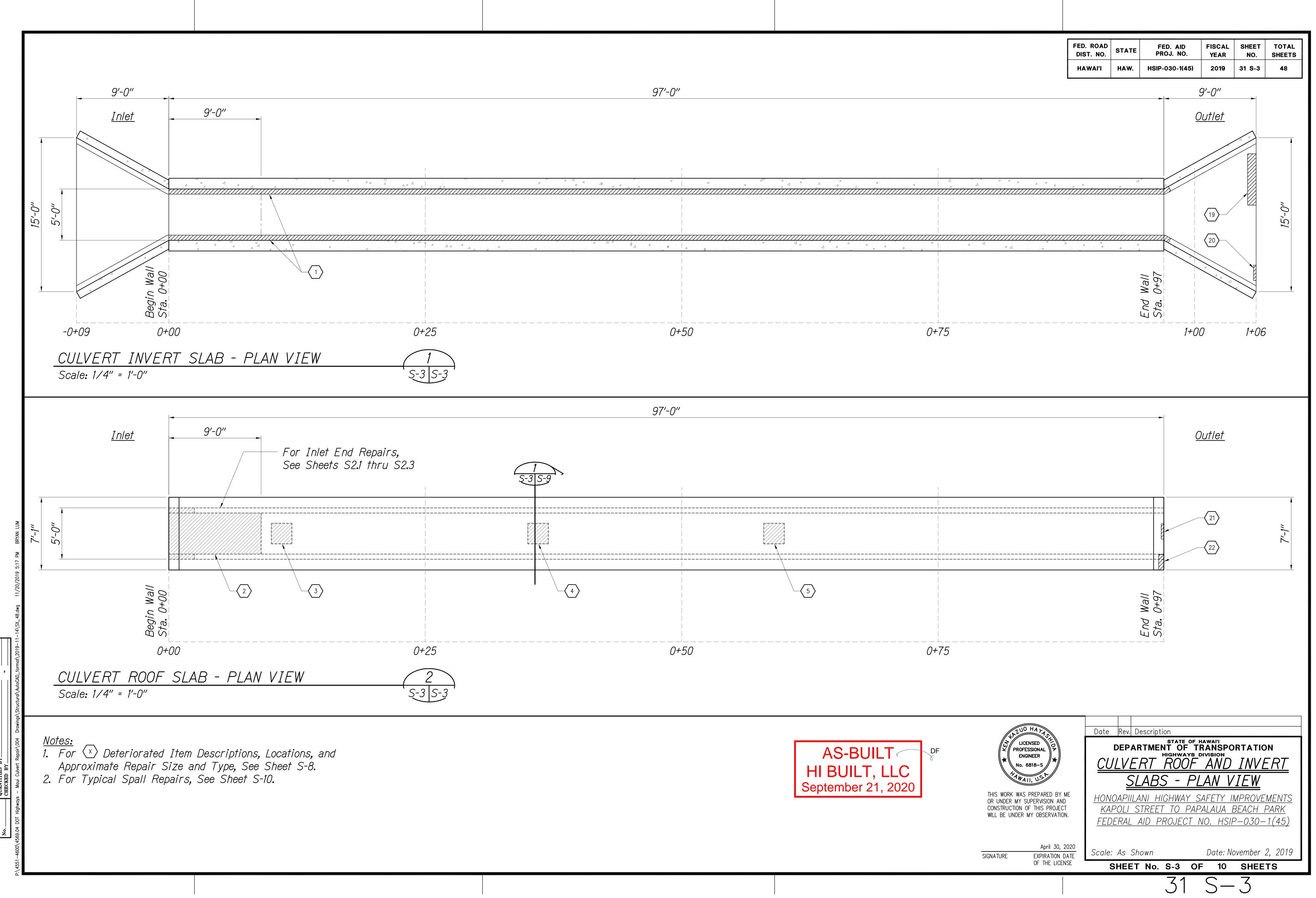


FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAI'I	HAW.	HSIP-030-1(45)	2019	31 S-2	48







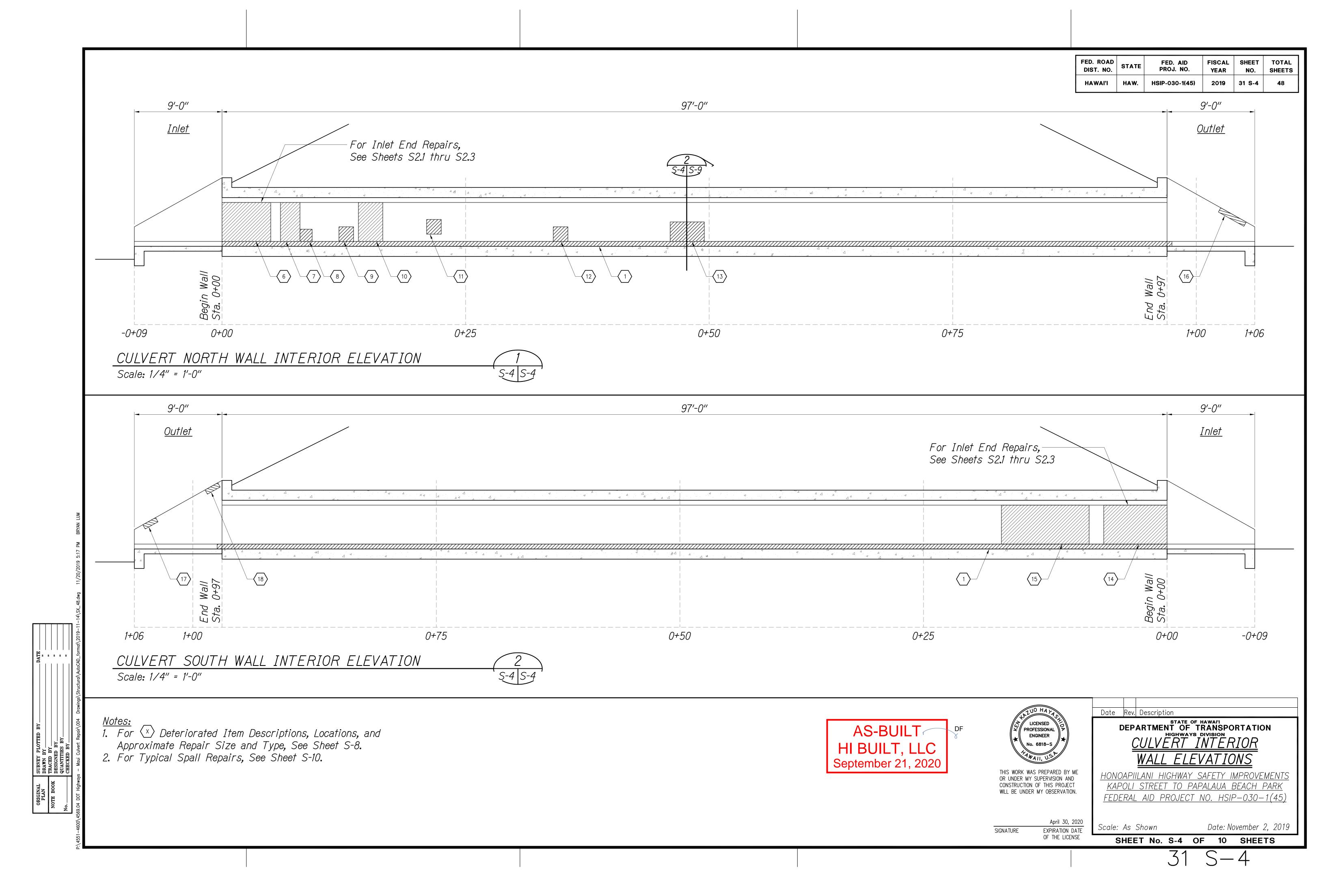


 ORIGINAL
 SURVEY PLOTTED BY
 DATE

 PLAN
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 NOTE BOOK
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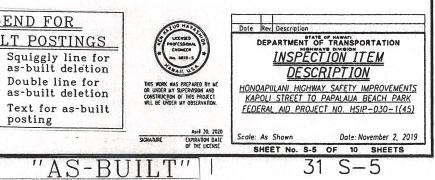


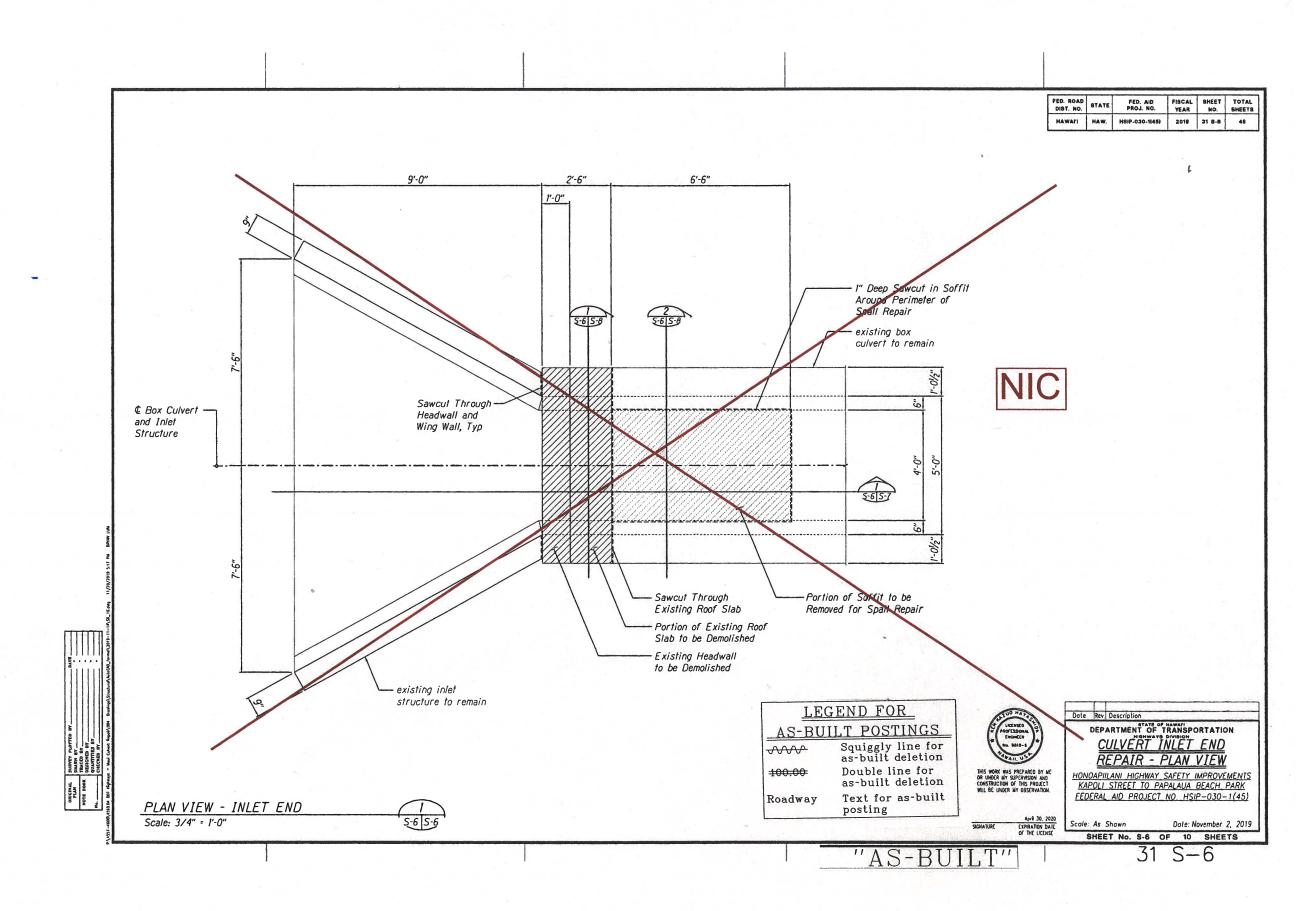
P:\4551-4600\4569.04 DOT Highways - Maui Culvert Repair\004 Drawings\Structural\AutoCAD_format\2019-11-14\SX_48.dwg, 11/20/2019 5:17:01 PM, Kai_Adobe PD

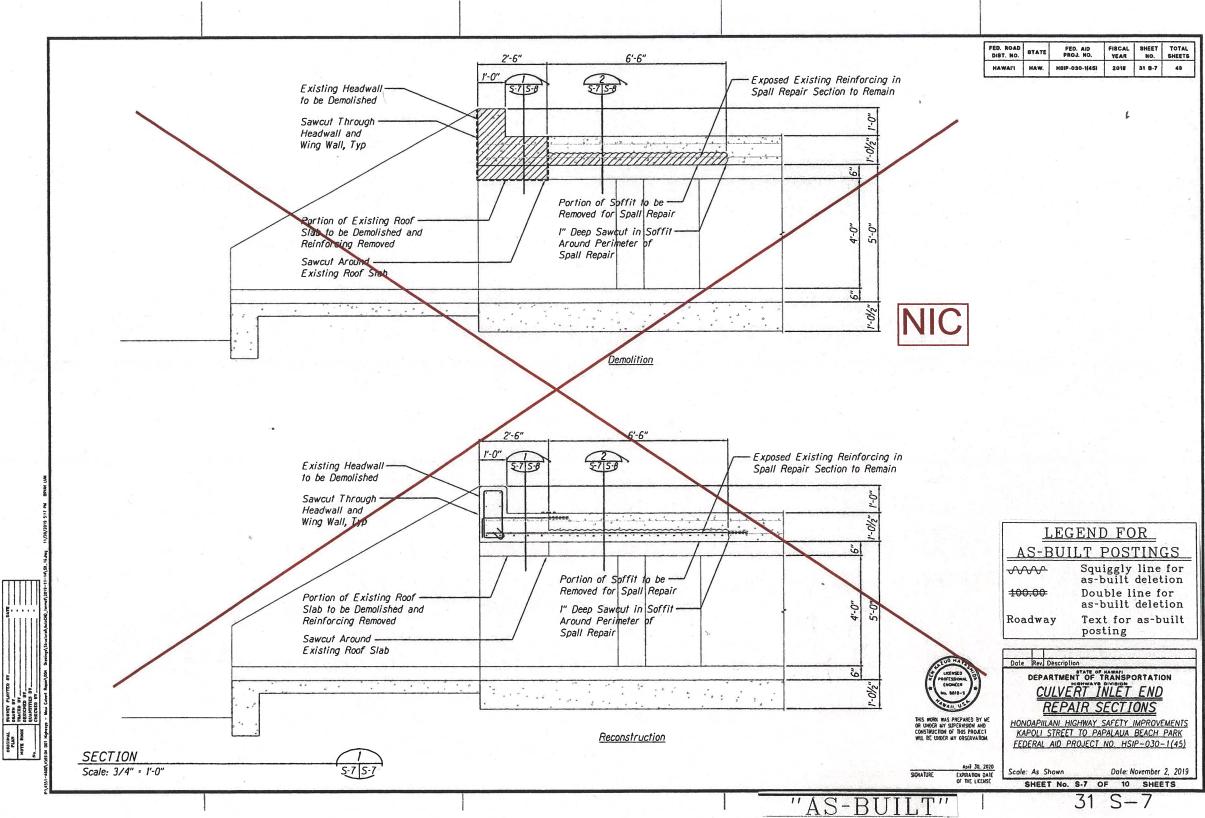
Deteriorated Item Description: (1) 6"x6" Fillet on Both Sides Exhibit Spalling (195 LF Total) (2) 9'x5' Area on Soffil and Headwall with Spalling and Delaminating Concrete (3) 2'x2' Delaminating Concrete on Soffit at Sta. 0+10 () 2'x2' Delaminating Concrete on Soffit at Sta. 0+35 (5) 2'x2' Delaminating Concrete on Soffit at Sta. 0+58 6 4'Hx5'W Spall on Wall at Sta. 0+00 () 4'Hx2'W Spall on Wall at Sta. 0+06 (1.25'Hx1.25'W Spall on Wall at Sta. 0+08 (1) 1.5'Hx1.5'W Spall on Wall at Sta. 0+12 (10) 4'Hx2.5'W Spall on Wall at Sta. 0+14 (1) 1.5'Hx1.5'W Spall on Wall at Sta. 0+21 (12) 1.5'Hx1.5'W Spall on Wall at Sta. 0+34 (3) 2'Hx3.5'W Spall on Wall at Sta. 0+46 (14) 4'Hx6.5'W Spall on Wall at Sta. 0+00 (15) 4'Hx9'W Spall on Wall at Sta. 0+08 (16) 3'x6"x6" Spall on Wing Wall 1.5'x6"x6" Spall on Wing Wall (18) 1.5'x6"x6" Spall on Wing Wall (19) 5'x10"x10" Spall on Slab Edge (20) 1.5'x10" Spall on Slab Edge (2) 1.5'x3"x6" Spall on Headwall (22) 1.5'x6"x6" Spall on Headwall

LEGEND FOR AS-BUILT POSTINGS m 100.00 Roadway posting

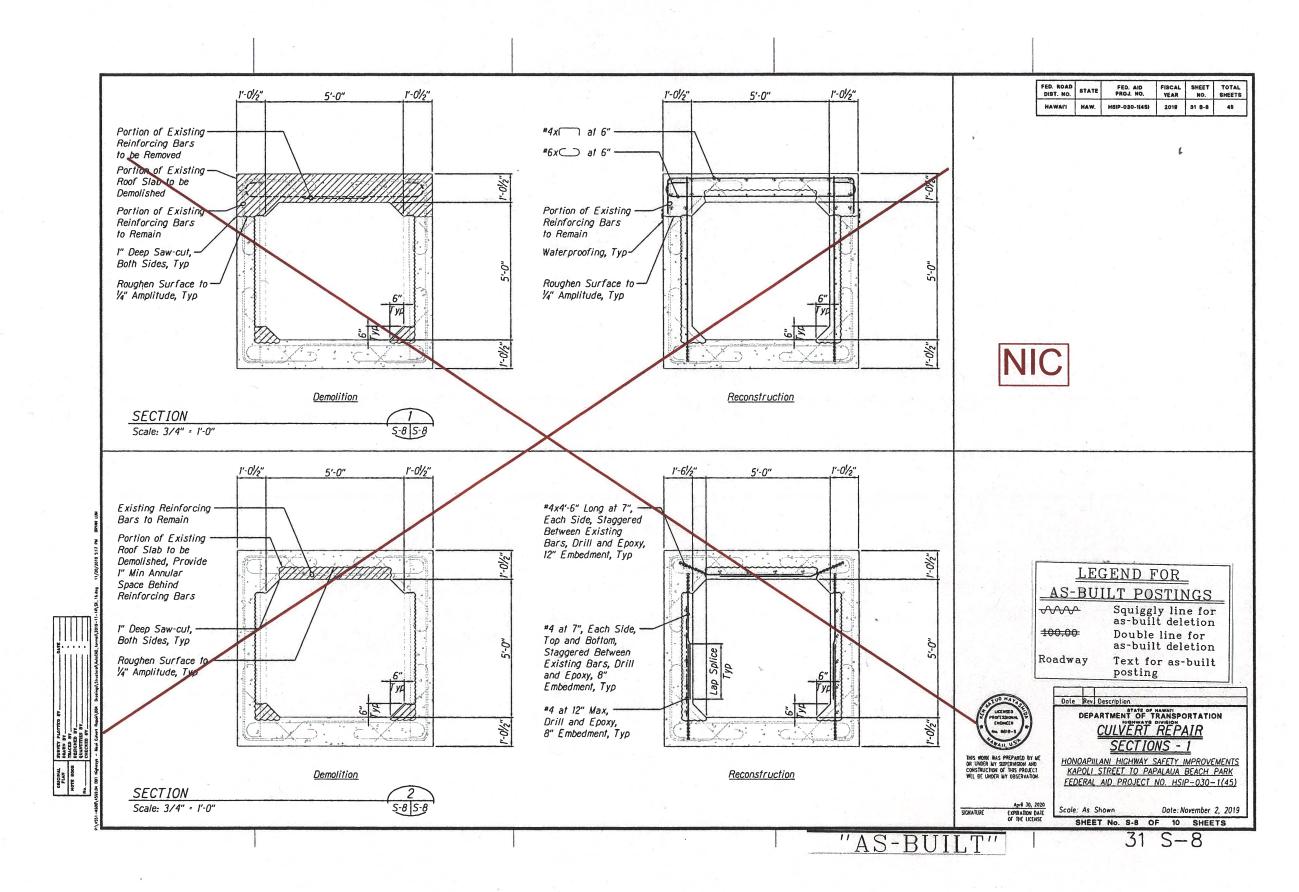
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FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL	SHEET NO.	TOTAL
HAWAN	HAW.	HBIP-030-1(45)	2019	31 8-5	48

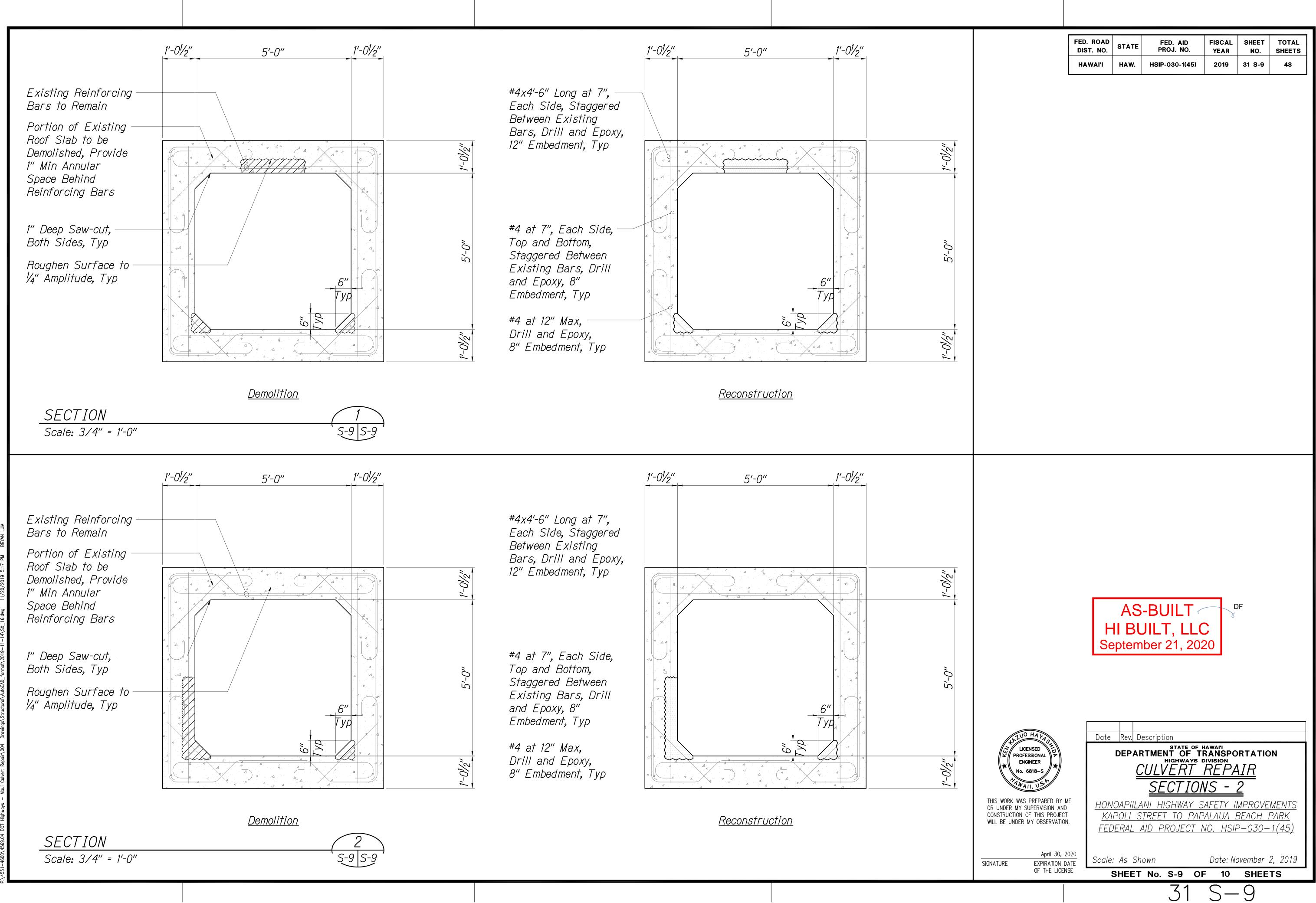






FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL	SHEET NO.	SHEET
HAWAI'I	HAW.	HBIP-030-1(45)	2019	31 8-7	48

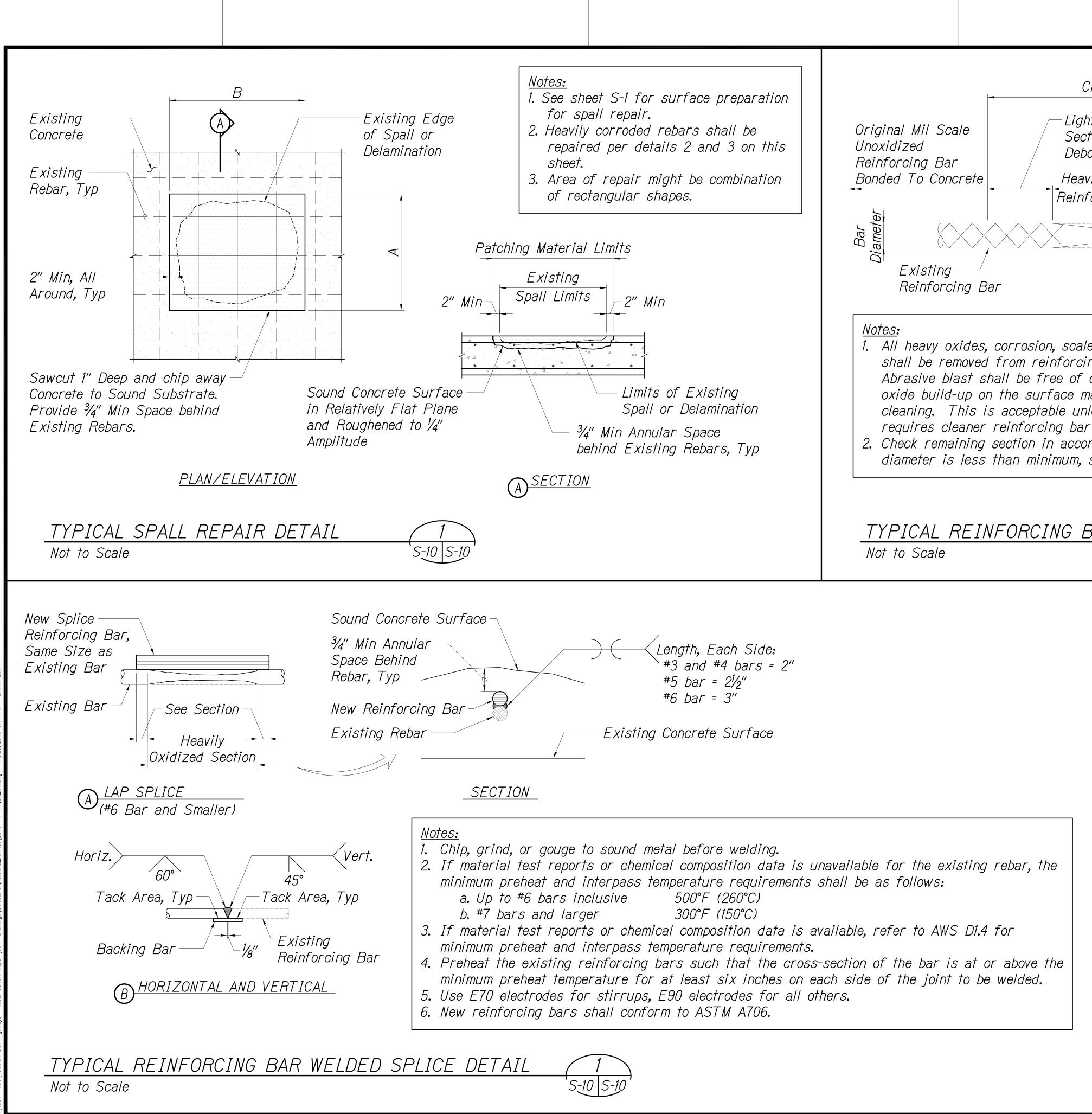








ORIGINAL PLAN NOTE BOOK





Cleaning Required	-	FED. ROAD DIST. NO. HAWAI'I	STATE HAW.	FED. AID PROJ. NO. HSIP-030-1(45)	FISCAL YEAR 2019	SHEET NO. 31 S-10	TOTAL SHEETS 48
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