STRUCTURAL GENERAL NOTES:	
1. General:	
A. Workmanship and materials shall conform to the AASHTO LRFD Bridge Design Specification, 4th Edition, 2007 including its subsequent interim specifications, and the Hawaii Standard Specifications for Bridge and Road Construction, 2005 as modified by the State of Hawaii Department of Transportation.	F. The locations of the proof test piles, as shown on Sheet S1.4 are subject to change based upon construction observations of the micropile installation. 4. Concrete:
B. The Contractor shall compare the Civil, Electrical and Structural drawings with each other and report in writing to the Engineer, inconsistencies or omissions.	
C. The Contractor shall take field measurements and verify field conditions and shall	a. Prestressed planks7000 psi
compare such field measurements and conditions with the drawings before	b. Slab topping, beams, abutment walls,
commencing the work. Report in writing to the Engineer all inconsistencies or omissions.	and wing walls4000 psi c. Abutment footings4000 psi
D. The Contractor shall be responsible for methods of construction, workmanship and	d. Traffic railing and end posts4000 psi
job safety. The Contractor shall provide temporary shoring and bracing as required	e. Approach slabs4000 psi
for stability of structural members and systems.	f. All other concreteClass "A"
E. Details noted as typical on structural drawings shall apply in all conditions unless specifically shown or noted otherwise.	All concrete shall have maximum w/c ratio of 0.45 except for Class "A" which sha
F. The Contractor shall be responsible for coordinating the work of all trades.	be as specified in the standard specifications. B. Tetraguard AS20 shrinkage reducing admixture, Eclipse Plus shrinkage reducing
G. The Contractor shall be responsible for protection of the adjacent properties,	admixture, or an approved equal, shall be included in the concrete mix for the
structures, streets, and utilities during the construction period. Any damage or	concrete topping and bridge railing. The required dosage shall be 128 ounces per
deteriorated property shall be restored to the same or better condition at no cost to	cubic yard of concrete or as recommended by the manufacturer. Addition of
the State.	shrinkage reducing admixture shall be as recommended by the manufacturer. C. A corrosion inhibiting admixture shall be included in the concrete mix for all
2. Design Criteria:	concrete. The corrosion inhibiting admixture shall contain a minimum of 30%
A. Dead Load	calcium nitrate by mass and shall be added at a dosage rate of 4.0 gallons per
Weight of all components of the structures, appurtenances attached thereto, and	cubic yard of concrete or as recommended by the manufacturer. The admixture
earth covers. Future wearing surface (curb to curb)25 psf	shall be Rheocrete CNI Calcium Nitrate-Based corrosion inhibitor, DCI S corrosion
Future wearing surface (curb to curb)25 psf Future utilities on each side of the bridge150 plf	inhibitor or an approved equal. Addition of corrosion inhibiting admixture shall be as recommended by the manufacturer.
B. Live Load	D. All inserts, anchor bolts, plates, etc. embedded in concrete shall be hot-dip
AASHTO HL-93 Loading	galvanized unless otherwise noted.
C. Seismic Seismic design is in accordance with the AASHTO Guide Specifications for LRFD	E. Conduits, pipes, and sleeves passing through a wall not conforming to typical detail
Seismic design is in accordance with the AASINO Guide Specifications for Livi D Seismic Bridge Design, 1st Edition, 2009, as modified by the State of Hawaii	shall be located and submitted to the Engineer for approval. F. Construction joints may be relocated by the Contractor and submitted to the
Department of Transportation.	Engineer for approval. Construction joints shall be made and relocated as not to
0.2-second spectral response acceleration coefficient, Ss = 0.50	impair the strength of the structure and to minimize shrinkage stresses. All
1.0-second spectral response acceleration coefficient, S ₁ = 0.14	construction joints shall be cleaned, laitance removed and wetted. See typical
Horizontal peak ground acceleration coefficient, PGA = 0.25 Site class	details for specific requirements. G. Unless otherwise noted, chamfer all concrete edges 3/4".
Seismic design catagory, SDC = C	H. Concrete delivery tickets shall record all free water in the mix: at batching by
D. Basic Wind Speed = 105 MPH	plant, for consistency by driver, and any additional request by Contractor if
E. Bridge Railingin accordance with AASHTO TL-2 design factors F. Static Lateral Earth Pressure	permitted by the mix design. I. Reinforcing bars, anchor bolts, inserts and other items to be cast in the concrete
Active condition, above groundwater = 40 pcf	shall be secured in position prior to placement of concrete.
Active condition, below groundwater = 80 pcf	J. Non-shrink Grout Shall Conform to Section 712.04 of the Standard Specification.
At-Rest condition, above groundwater = 58 pcf	
At-Rest condition, below groundwater = 88 pcf	5. Reinforcing Steel: A. Reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60 Unles
3. Foundation:	Otherwise Noted.
A. Foundation design is based upon geotechnical investigations by Geolabs Inc. and	B. Pre-stress strands shall be uncoated seven wire low-relaxation strand conforming
dated July 30, 2010.	ASTM A416 Grade 270, or AASHTO M203 Grade 270.
B. Contractor shall provide for de-watering of excavation from either surface water, ground water or seepage. NPDES permit required for discharging into State waters.	C. Clear concrete coverage for reinforcing bars shall be as follows, unless otherwise noted:
C. Contractor shall provide for design and installation of all cribbing, sheeting, and	a. Footing, Grade Beams, ETC.
shoring necessary for personnel safety and to preserve excavations and earth banks,	Cast against earth3"
and adjacent structures and property for damage.	b. Footing, Grade Beams, ETC.
D. Excavation boundaries and grade elevations for footing shall be approved by the Engineer prior to placing the concrete and reinforcing.	Formed and exposed to earth2"
E. Backfill behind the retaining structures (above the groundwater level) may consist	c. Wall faces exposed to earth or weather
of the on-site soils or select granular fills (Type A Structure Backfill). Backfill shall	d. Deck slabs
be placed in uniform lifts of no more than 8 inches in loose thickness and uniformly	i. Top bars
compacted to at least 95 percent relative compaction. Because shallow groundwater	ii. Bottom bars1 1/2"
conditions are anticipated, backfill materials below the groundwater level should consist of free-draining granular materials, such as AASHTO M43, No 67 gradation	
(ASTM C33, No 67 gradation), wrapped on all sides with non-woven filter fabric	

(Mirafi 180N or equivalent). The free-draining granular materials should be used up to

a level of about 12 inches above the groundwater level to facilitate compaction of the

backfill materials.

FED. ROAD
DIST. NO.STATEFED. AID
PROJ. NO.FISCAL
YEARSHEET
NO.TOTAL
SHEETSHAWAIIHAW.BR-0450(8)20115893

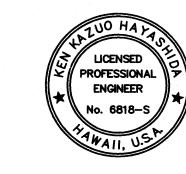
- D. Stainless steel reinforcing bars shall be Type 316 grade conforming to ASTM A955 grade 60.
- E. Fiber Reinforced plastic (FRP) rebars shall have a minimum ultimate tensile strength of 115 ksi, and a modulus of elasticity of 6.5x10 psi. Use glass fibers and vinyl ester binder. Field cutting of FRP rebars shall be with a high speed grinder cutter or saw. Do not shear bars.
- F. Thru bolts for guardrail connection shall conform to AASHTO M164 (ASTM A325), unless otherwise noted.
- G. Splices:
- a. Reinforcing steel shall be spliced only where indicated on plans. Provide lap splice length per typical details and schedule, Sht S0.3, unless otherwise noted.
- H. Bar bends and hook shall be "standard hooks" in accordance with Typical Details Sht S0.3.

6. Structural Steel:

- A. Fabrication and erection of structural steel shall conform to the AASHTO LRFD Bridge Construction Specifications, Third Edition, including its subsequent interim specifications.
- B. Structural steel shall conform to ASTM A36, unless otherwise noted.
- a. Steel wide flange sections shall conform to ASTM A992.
- b. Steel pipes shall conform to ASTM A53, Grade B.
- c. Steel tubes shall conform to ASTM A500, Grade B.
- d. Bolts shall conform to ASTM A307, Grade A, unless otherwise noted.
- e. Welds and welding procedures shall conform to the structural welding code AWS D1.1 of the American Welding Society.
- f. Welding shall be performed by welders that are prequalified for applicable welding procedures.
- g. The Contractor shall hire an independent special inspector to conduct ultrasonic NDT testing of all field welds. The cost for all testing shall be paid for by the contractor under various contract pay items. The inspection test results shall be provided to the DOT.
- h. Welding electrodes shall be E70XX.
- i. All steel shall be hot-dip galvanized after fabrication.

7. Metal Pedestrian Railing:

- A. The metal pedestrian railings shall be not dip galvanized after fabrication and shop painted according to Paint Schedule, Sht. SO.2.
- B. The metal railing posts shall be field welded to embedded plate in the top of concrete railing as shown on Sht. S4.1.
- a. After welding is complete, remove all weld slag, splatter, damaged galvanizing material, and all other welding by-products from the surface of the railing posts and embedded plates.
- b. Prepare surface of steel per SSPC-SP1.
- c. Rust scale shall be cleaned per SSPC-SP3.
- d. Apply (2) coats of cold applied, galvanizing compound containing 95% metallic zinc content by weight in dry film and 52% solids content by volume.
- e. The coating shall be well stirred before use so that it is completely homogeneous during application.
- f. Minimum dry film build is 3 mils, using manufacturer's recoat time directions.
- g. The coating shall be applied at sufficient wet film thickness to achieve a minimum dry film.
- h. Apply paint system according to paint schedule Sht. So.2.



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HIGHWAYS DIVISION

<u>STRUCTURAL</u> <u>GENERAL NOTES</u> KAMEHAMEHA V HIGHWAY

<u>Kawela Bridge Replacement</u> <u>Federal Aid Project No. BR-0450(8)</u>

Scale:AS NOTED

Date: June, 2010

SHEET No. S0.1 OF 93 SHEETS

Painting Notes:

1. Contractor shall shop coat all members. Field coating shall consist of touch up only.

2. The touch up paint shall consist of the following:

a. prepare surface per SSPC-SP1, solvent cleaning.

b. apply first and second coat according to paint schedule

3. Color for top coat shall be an earthtone light brown or green, as proposed by the contractor and approved by the Engineer. Intermediate coat shall have contrasting light color. Finish for top coat shall not be gloss or high gloss.

4. Multiple coats may be required to obtain minimum dry film thickness (DFT).

5. All hot-dip galvanized coating that is damaged shall be repaired. The repairs shall consist of the following:

a. Prepare surface per SSPC-SP1, solvent cleaning.

b. Apply (2) coats of cold applied, galvanizing compound containing 95% metallic zinc content by weight in dry film and 52% solids content by volume.

c. Application rate shall be 1.5 mils dry film thickness per coat.

d. Rust scale shall be cleaned per SSPC-SP3.

e. The coating shall be applied at sufficient wet film thickness to achieve a minimum dry film.

f. The coating shall be well stirred before use so that it is completely homogeneous during application.

g. Minimum dry film build is 3 mils, using manufacturer's recoat time directions.

h. Apply paint system according to paint schedule.

6. Painting of galvanized steel shall be in accordance with ASTM D6386, "Standard Practice for Preparation of Zinc (Hot-Dipped Galvanized) Coated Iron and Steel Product and Hardware Surfaces for Painting."

7. Cost of painting is incidental to metal railings.

		PAINT SCHEL	DULE	
	SPECIAL COAT	ING SCHEDULE FOR ZING	C COATED METAL BRIDGE R.	AILING
	<u>OPTION 1</u>	OPTION 2	<u>OPTION 3</u>	OPTION 4
PREPARATION:	Carboline thinner #2 or surface cleaner #3, per SSPC-SP1, Apply Rustbond Penetrating Sealer.	Solvent clean per SSPC-1, and as recommended by the manufacturer	Solvent clean per SSPC-SP1, and as recommended by the manufacturer	Solvent clean per SSPC- SP1. Apply Galvanized Zinc Treatment (Acid Etching)
IST COAT:	Carboline Carboguard 890 epoxy DFT 5 mil (min) WFT 7 mil (min)	Tnemec High-Build Epoxoline II Series N69 DFT 5 mil (min) WFT 7 mil (min)	Sherwin Williams Tile Clad High Solids B62 Series DFT 4 mil (min) WFT 7 mil (min)	Ameron Amercoat 385 epoxy DFT 5 mil (min) WFT 8 mil (min)
RECOATING TIME:	8 HRS (min) 2 Days (max)	10 HRS (min) 2 Days (max)	8 HRS (min) 14 Days (max)	8 HRS (min) 2 Days (max)
TOP COAT:	Carboline Carbothane 133HB Alyphatic polyurethane DFT 5 mil (min) WFT 7 mil (max)	Tnemec Endura-Shield Series 75 DFT 4 mil (min) WFT 7 mil (min)	Sherwin Williams Corothane II B65 W200 Series/B60V2 DFT 4 mil (min) WFT 7 mil (min)	Ameron Amercoat 450 SA Polyurethane DFT 4 mil WFT 7 mil
TOP COAT COLOR:	Medium Shade Green, submit color chips for approval.			

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	BR-0450(8)	2011	59	93

ABBREVIATIONS				
B	Baseline	Galv	Galvanized	
Bot	Bottom	Horiz	Horizontal	
Btwn	Between	HSB	High Strength Bolt	
C	Centerline	I.D.	Inside Diameter	
CJ	Construction Joint	Max	Maximum	
Clr	Clear	Mech	Mechanical	
Conc	Concrete	Min	Minimum	
Cont	Continuous	Misc	Miscellaneous	
Dbl	Double	No. or #	Number	
Dia	Diameter	N.T.S.	Not to Scale	
Dwg	Drawing	O.C.	On center	
Ea	Each	0.D.	Outside Diameter	
E.F.	Each Face	Opng	Opening	
Elev	Elevation	Орр	Opposite	
Eq	Equal	PL or P	Plate	
E.W.	Each Way	Prefab	Prefabricated	
Exist or (E)	Existing	Ref	Reference	
Ext	Exterior	Reinf	Reinforced or Reinforcing	
Fin	Finish	Sim	Similar	
F†	Foot or Feet	Sp	Spaces or Spacing	
Ftg	Footing	SS	Stainless Steel	



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STATE OF HAWAII **DEPARTMENT OF TRANSPORTATION** HIGHWAYS DIVISION

KAMEHAMEHA V HIGHWAY

Kawela Bridge Replacement Federal Aid Project No. BR-0450(8)

Date: June, 2010 SHEET No. S0.2 OF 93 SHEETS

		·				
MI	MINIMUM SPLICE & EMBEDMENT LENGTHS					
	CONCRETE STRENGTH = 4,000 PSI					
	LAP SPL	ICE	EMBEDMENT			
			STRAIGH	STRAIGHT		
BAR SIZE	OTHER BARS	TOP BAR	OTHER BARS	TOP BAR	WITH STANDARD 90° HOOK	
#3, #4	21"	29"	12"	17"	7"	
#5	26"	36"	15"	21"	9"	
#6	31"	43"	18"	26"	10"	
#7	39"	54"	23"	32"	12"	
#8	51"	71''	30"	42"	14"	
#9	64"	90"	38"	53"	15"	
#10	81"	114"	48"	67"	17"	
#11	100"	140"	59"	82"	19"	

Notes:

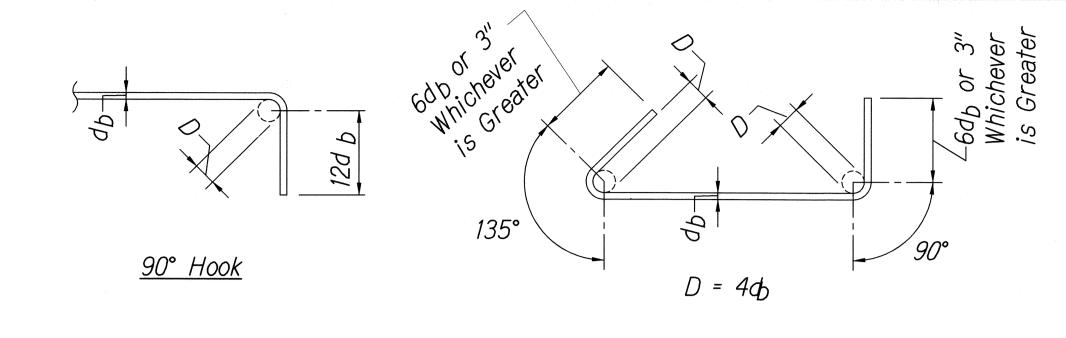
- 1. "Top Bars" are horizontal bars with 12" or more of concrete cast below.

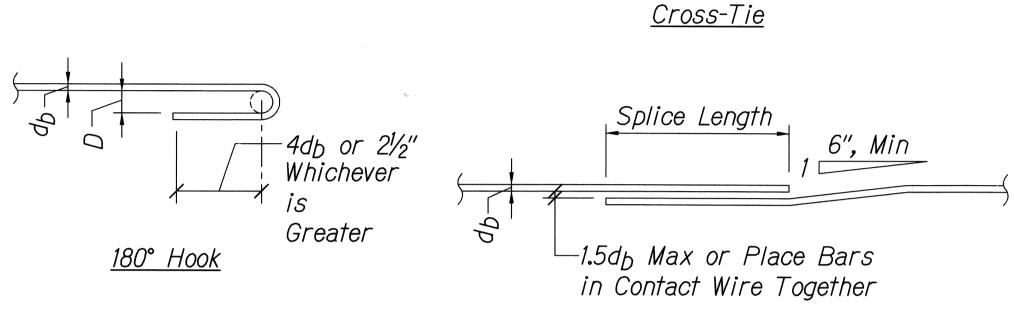
 2. Splice lengths may be reduced by multiplying the tabulated values by
- 2. Splice lengths may be reduced by multiplying the tabulated values by 0.765 if the centerline of splice of adjacent bars are staggered 6'-0" o.c. for #9 bar and smaller and 9'-0" o.c. for #10 bar and larger.
- 3. Embedment lengths for straight bars may be reduced by multiplying the tabulated values by 0.80 if the bars are spaced laterally not less than 6" center-to-center, with not less than 3" clear cover measured in the direction of the spacing.
- 4. Embedment lengths for bars with 90° hook are bars with side cover, normal to plane of hook, of not less than 2½" and cover on bar extension beyond hook not less than 2". Increase embedment length by 43% for bars not meeting these requirements.

TYPICAL REBAR SPLICE AND EMBEDMENT LENGTH SCHEDULE

Not to Scale



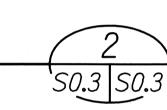




D = 6d_b for #8 and Smaller D = 8d_b for #9 to #11

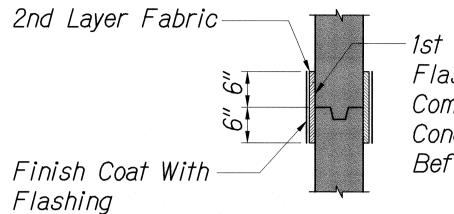
STANDARD HOOKS AND CROSS-TIE DETAIL

Not to Scale



<u>Bar Lap</u>

2000 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
	HAWAII	HAW.	BR-0450(8)	2011	60	93



-1st Layer Coat of Flashing Compound, Clean Concrete Surface Before Application

<u>Wall Joint</u>

——2nd Layer Fabric ——1st layer coat of Flashing Compound, clean concrete surface before application

1/2" Built-up Flashing — Construction Joint Compound to 1/2" x 1/2"

Cont Wall Footing Joint

Note: Typical each face unless noted otherwise

TYPICAL WATERPROOFING DETAIL

Not to Scale

Compound

Flashing

Compound

Finish Coat With





STATE OF HAWAII
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HIGHWAYS DIVISION

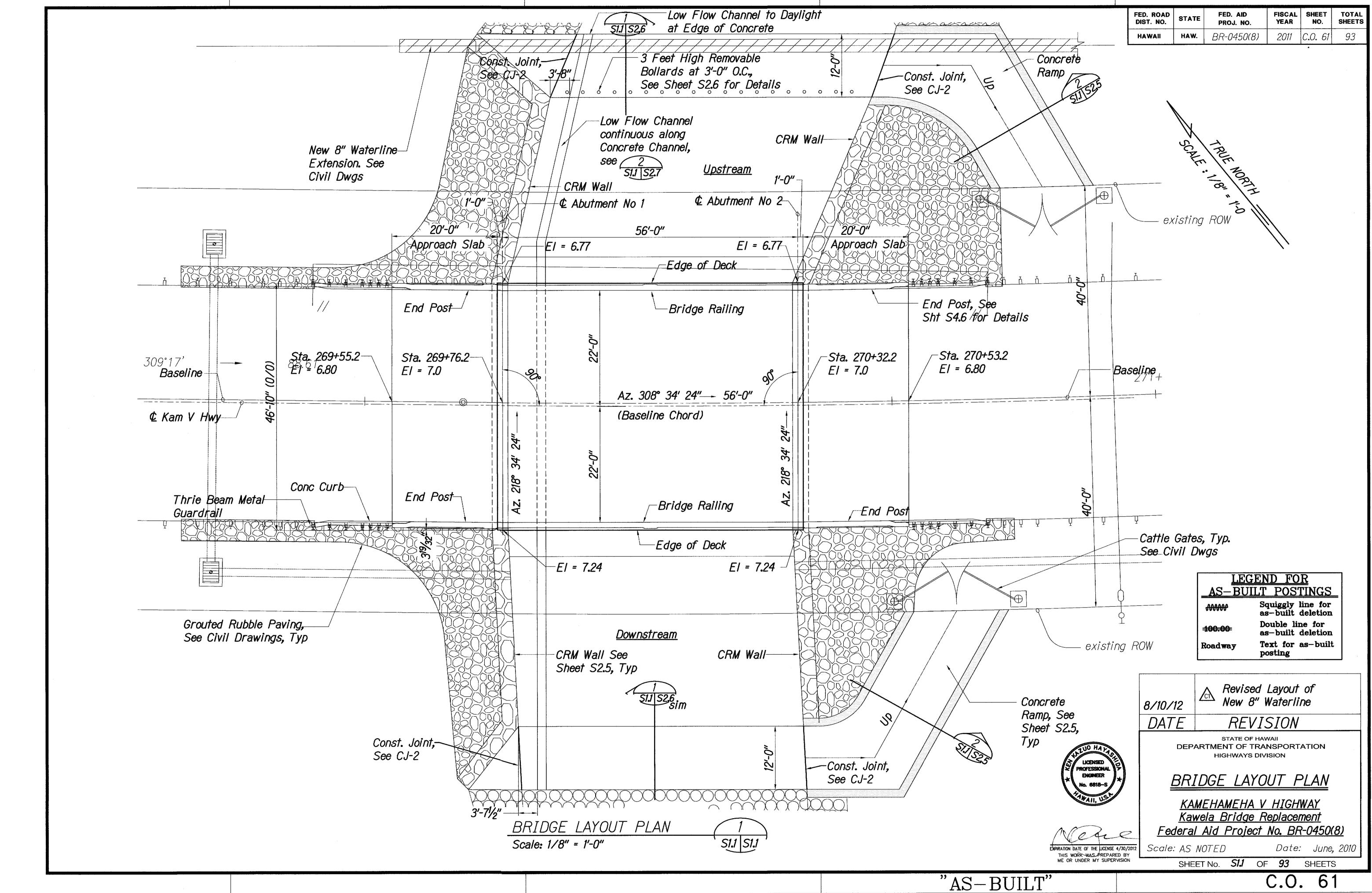
TYPICAL DETAILS

<u>KAMEHAMEHA V HIGHWAY</u> <u>Kawela Bridge Replacement</u> <u>Federal Aid Project No. BR-0450(8)</u>

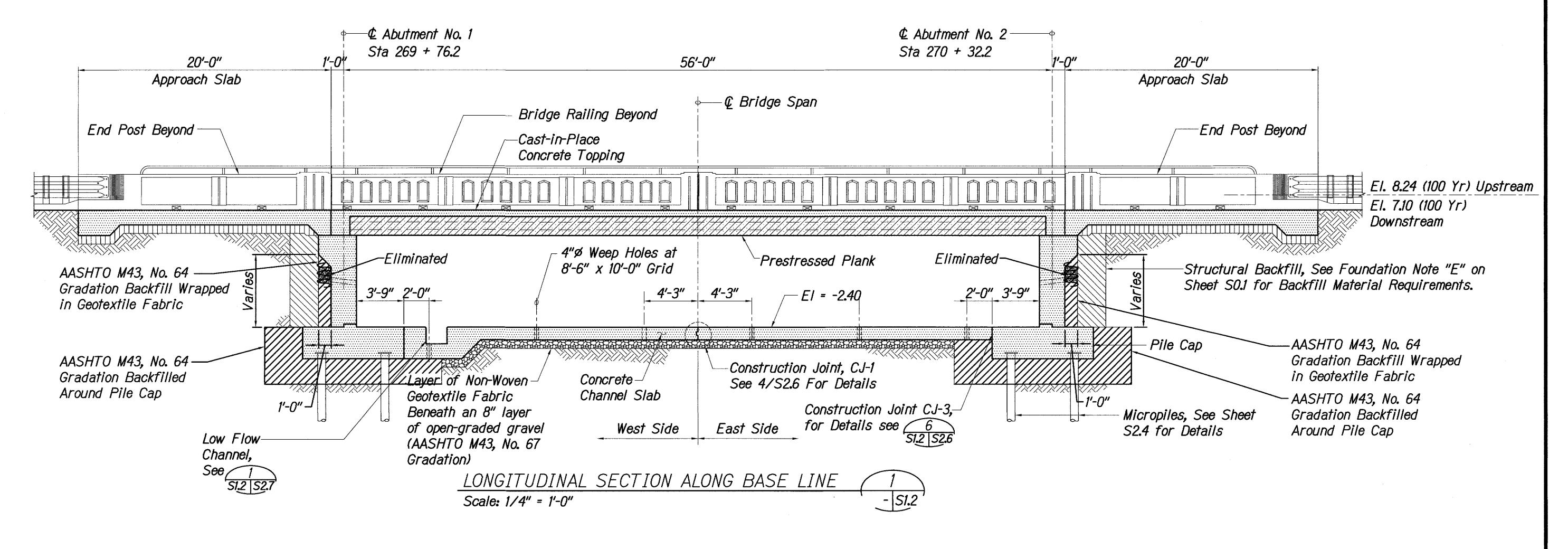
EXPIRATION DATE OF THE LICENSE 4/30/2012
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SHEET No. SO.3 OF 93 SHEETS

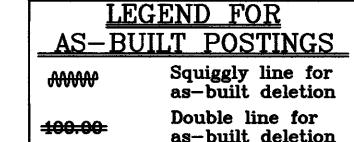


C.O. 61



Notes:

- A. The sequence of construction is the responsibility of the contractor. The contractor shall conduct their construction activities such that the micropiles are not subjected to lateral loads, such as backfilling behind abutments, until after the channel slab beneath the bridge is in place and has sufficiently cured. The following construction sequence shall be followed to avoid imposing lateral loads on the micropiles during construction: 1. Excavate for and construct abutments and channel slab according to Phasing Plan, Sheets C2.01 and C2.02. 2. The abutments shall not be backfilled until after the channel slab is in place and has cured for at least 7 days.
- B. Placement and repositioning of BMP's that will be used for channelizing stream flow during the various phases of the work shall be the responsibility of the contractor. Refer to BMP Notes, Sheets C2.01 and C2.02.
- C. Channel slab subgrade shall be prepared by proof-rolling the soil subgrade to a firm condition.



Text for as-built posting



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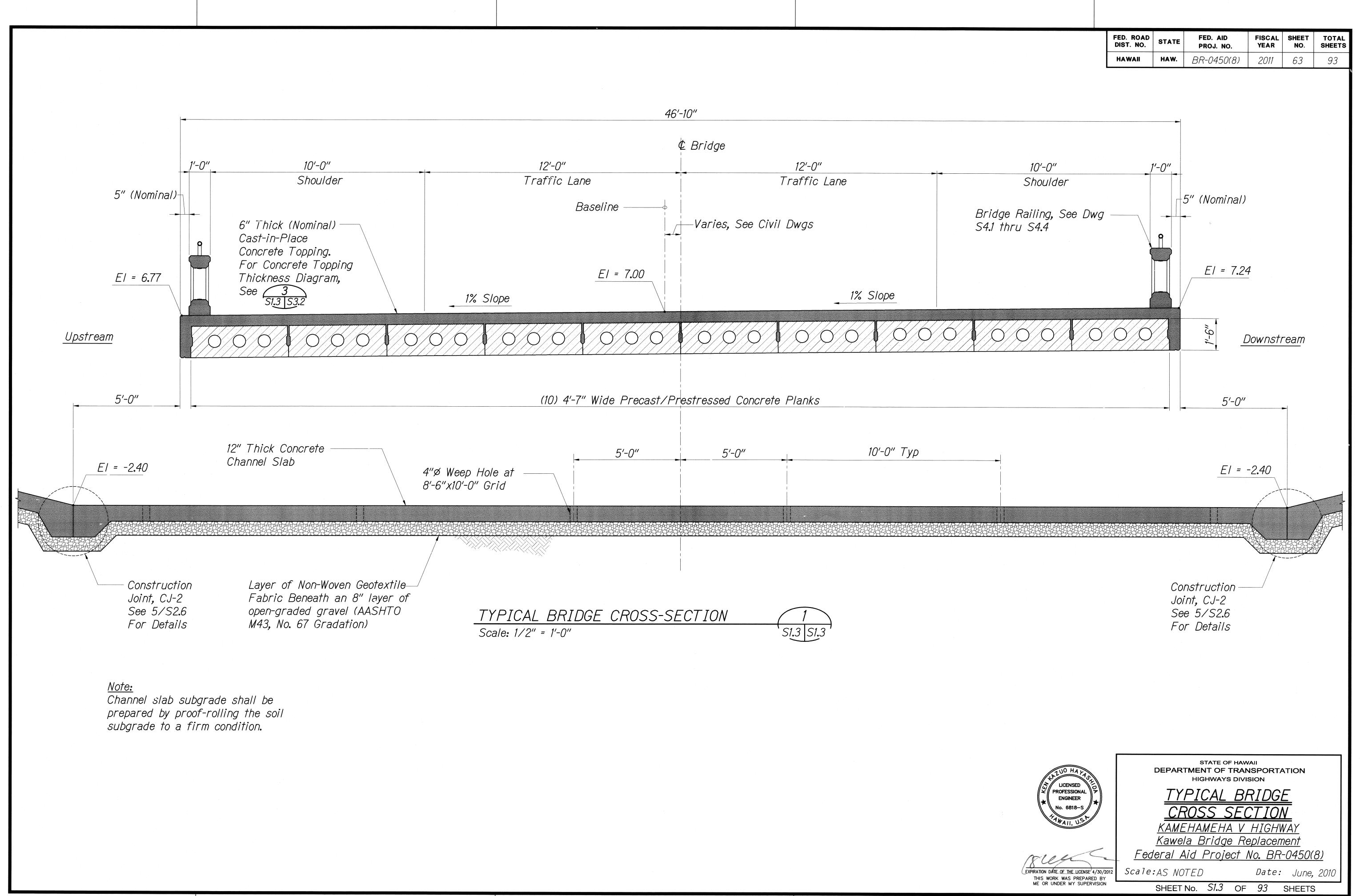
STATE OF HAWAII **DEPARTMENT OF TRANSPORTATION** HIGHWAYS DIVISION

BRIDGE LONGITUDINAL <u>SECTION</u>

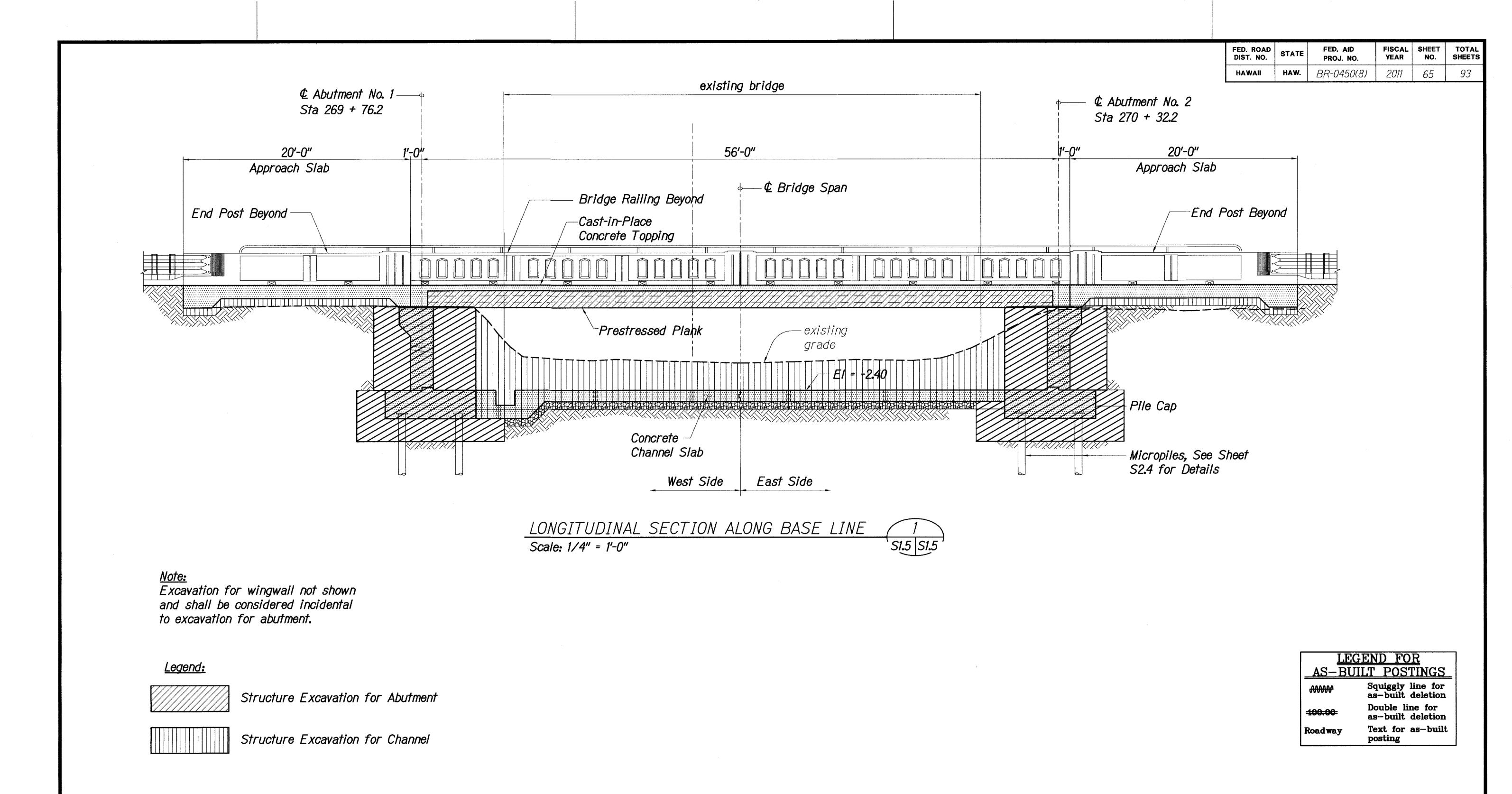
KAMEHAMEHA V HIGHWAY Kawela Bridge Replacement Federal Aid Project No. BR-0450(8)

Scale: AS NOTED

Date: June, 2010 SHEET No. 51.2 OF 93 SHEETS



SHEET No. **51.4** OF **93** SHEETS





EXPRATION DATE OF THE LICENSE 4/30/2012
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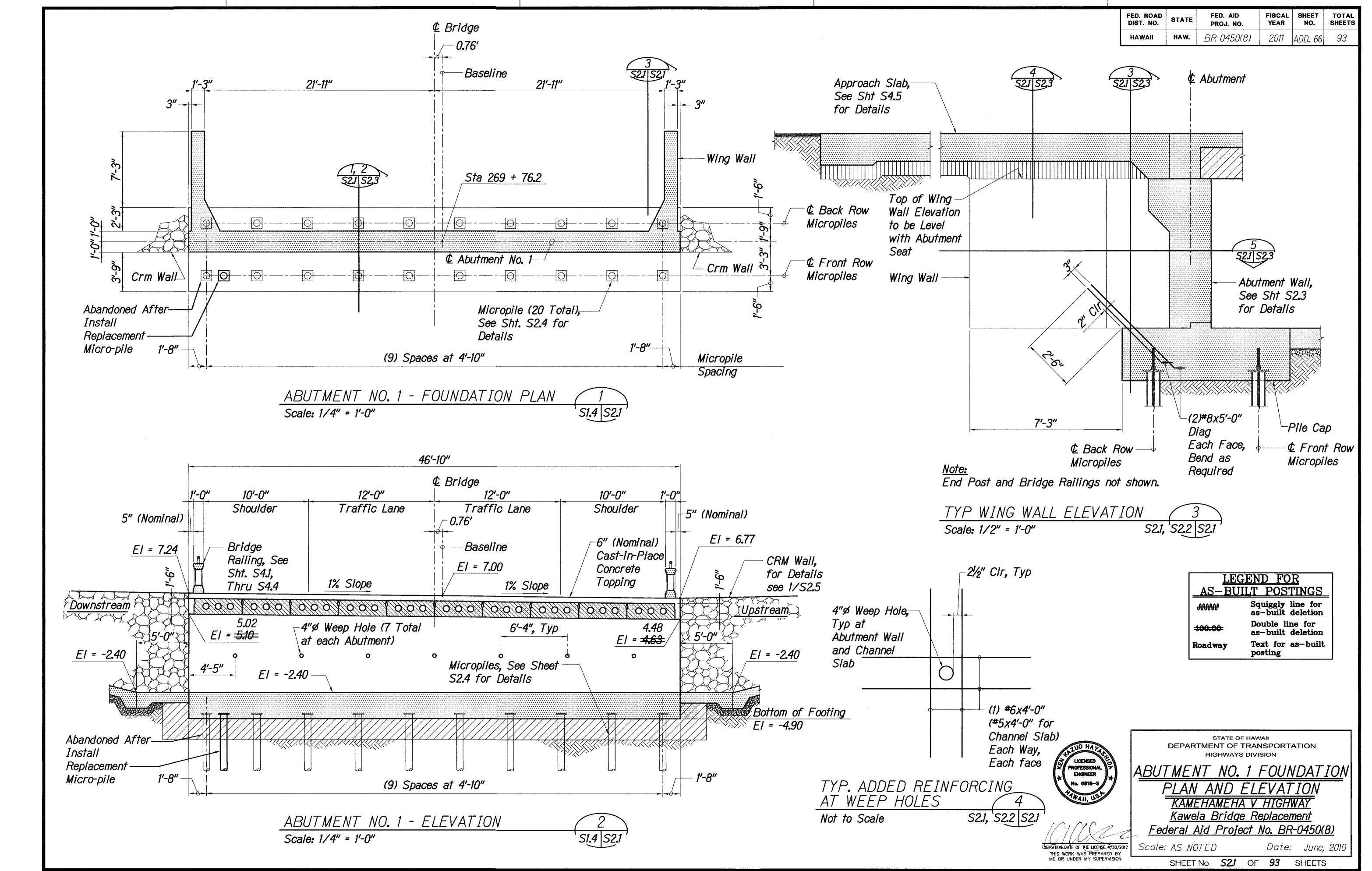
PAY LIMITS

KAMEHAMEHA V HIGHWAY Kawela Bridge Replacement Federal Aid Project No. BR-0450(8)

Scale: AS NOTED

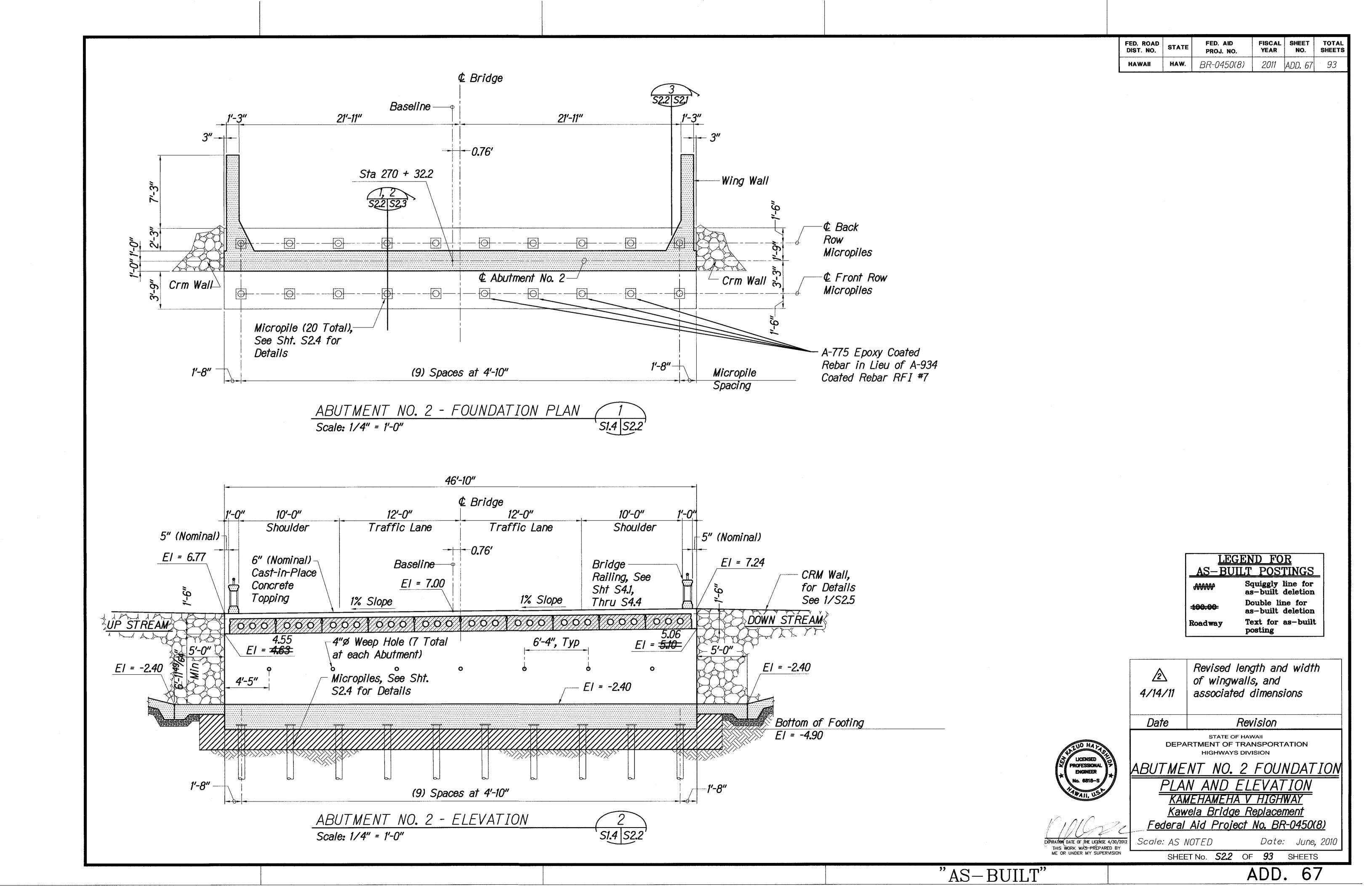
Date: June, 2010 SHEET No. **S1.5** OF **93** SHEETS

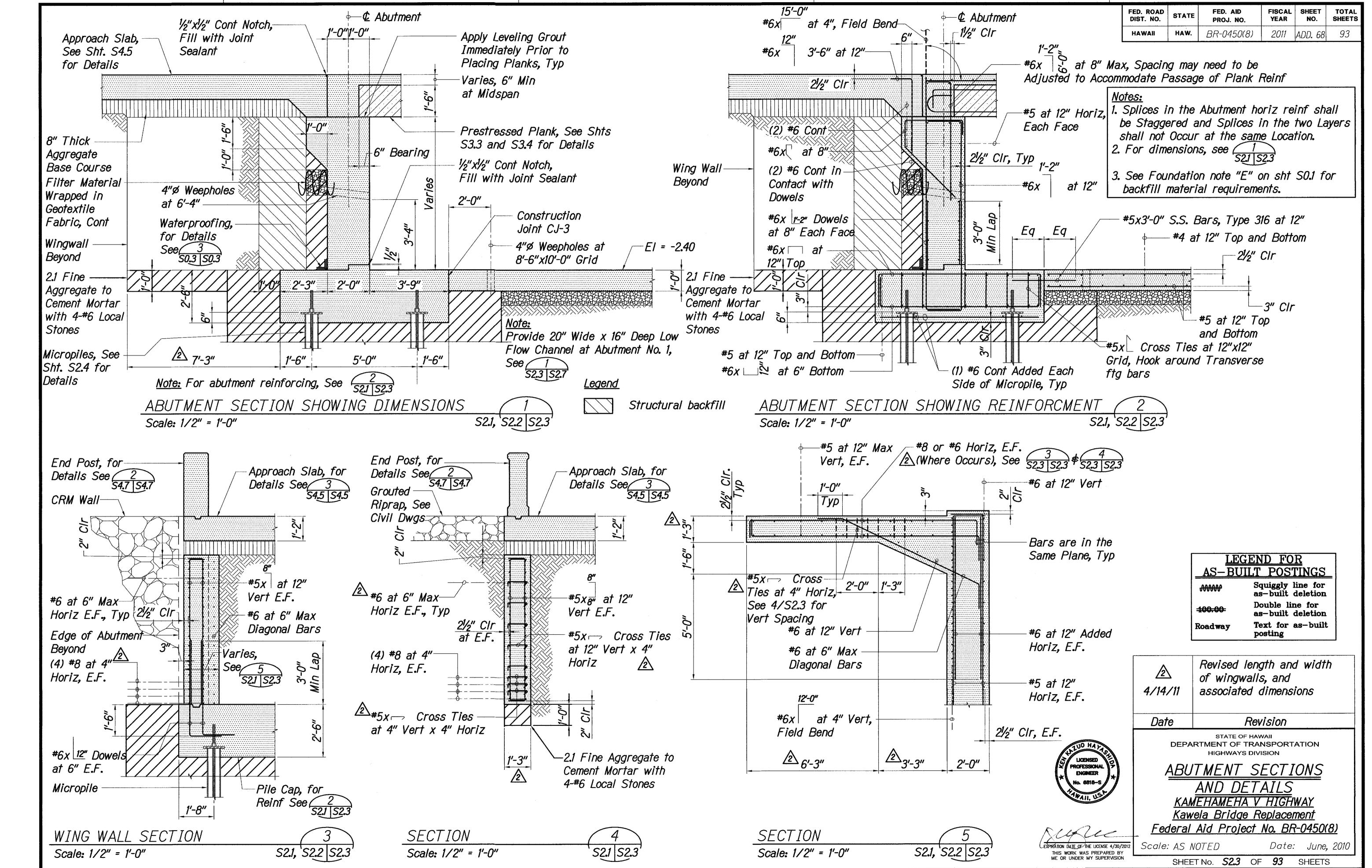
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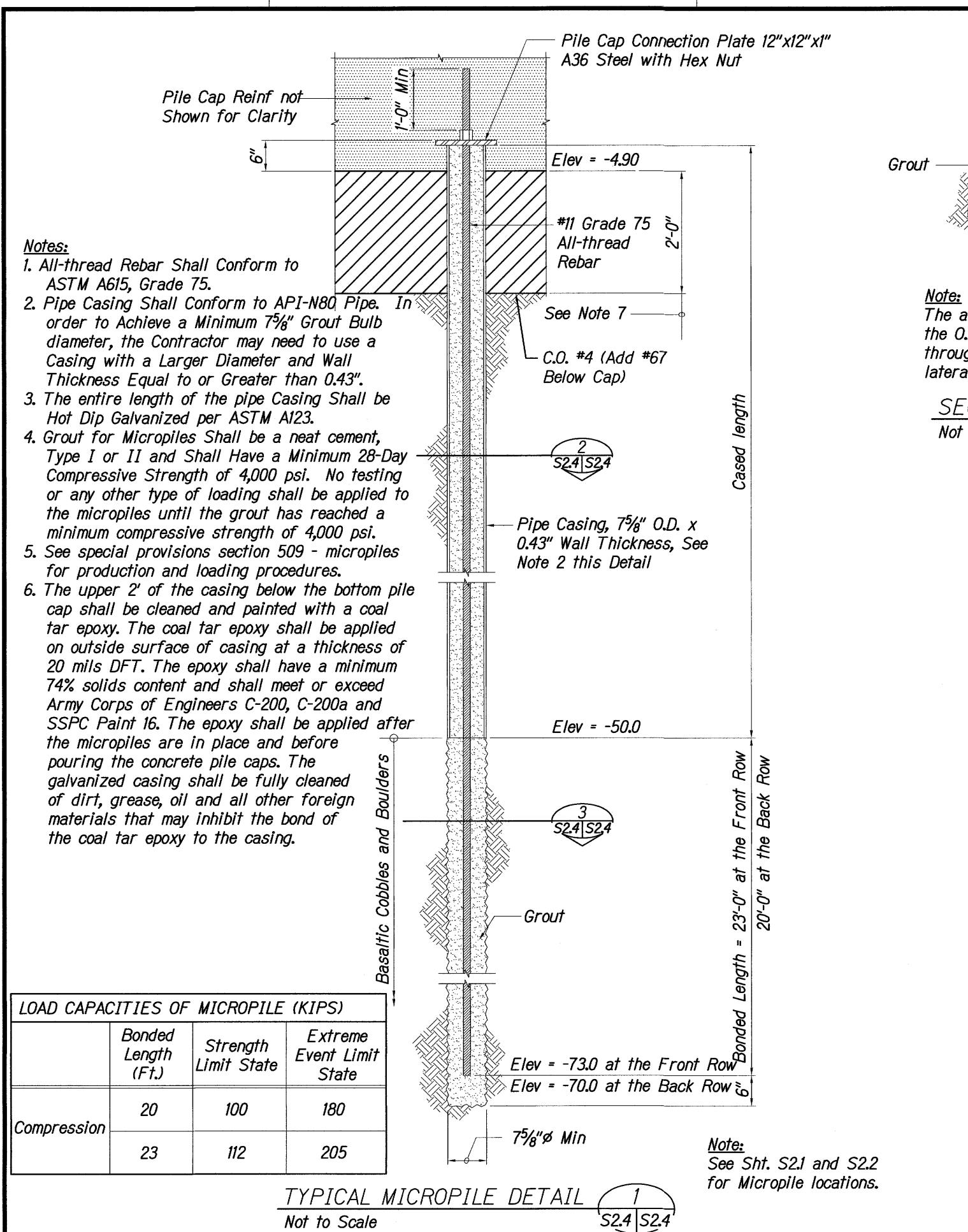
"AS-BUILT" ADD. 66



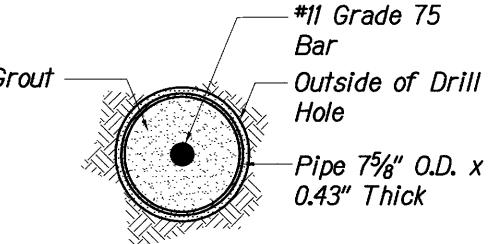


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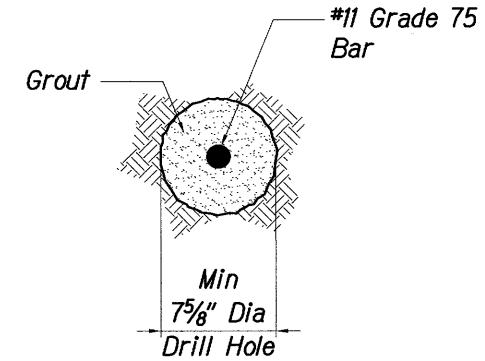


FED. ROAD DIST. NO. FISCAL SHEET TOTAL YEAR NO. SHEETS PROJ. NO. BR-0450(8) 2011 ADD. 69 93 HAW.



The annular space between the drill hole and the O.D. of the pipe casing shall be grouted throughout the cased length in order to provide lateral support of the micropile.

SECTION 52.4 52.4 Not to Scale



SECTION 52.4 52.4 Not to Scale

PILE DESIGN LOAD IN COMPRESSION (KIPS)		MAXIMUM PILE TEST LOAD (KIPS)		
Service Limit	Strength Limit	Verification Test = 2.0 x Strength Limit	1.0 x	of Test = < Strength Limit
86	110	220	105	#0

See RFI #3—

Testing Notes:

- 1. One (1) Pile Verification Load Test shall be performed, this pile shall be sacrificial. Bonded length for this pile shall be 23'-0".
- 2. Four (4) Piles shall be Proof Tested, two on each Abutment.

3. See Sheet S1.4 for locations of Test Piles.

LEGEND FOR AS-BUILT POSTINGS Squiggly line for as-built deletion Double line for as-built deletion Text for as-built Roadway posting

4/14/11

Deleted Lateral Load Table and Previous Note 4.

Date Revision

DEPARTMENT OF TRANSPORTATION LICENSED PROFESSIONAL ENGINEER TYPICAL MICROPILE

DETAILS KAMEHAMEHA V HIGHWAY Kawela Bridge Replacement

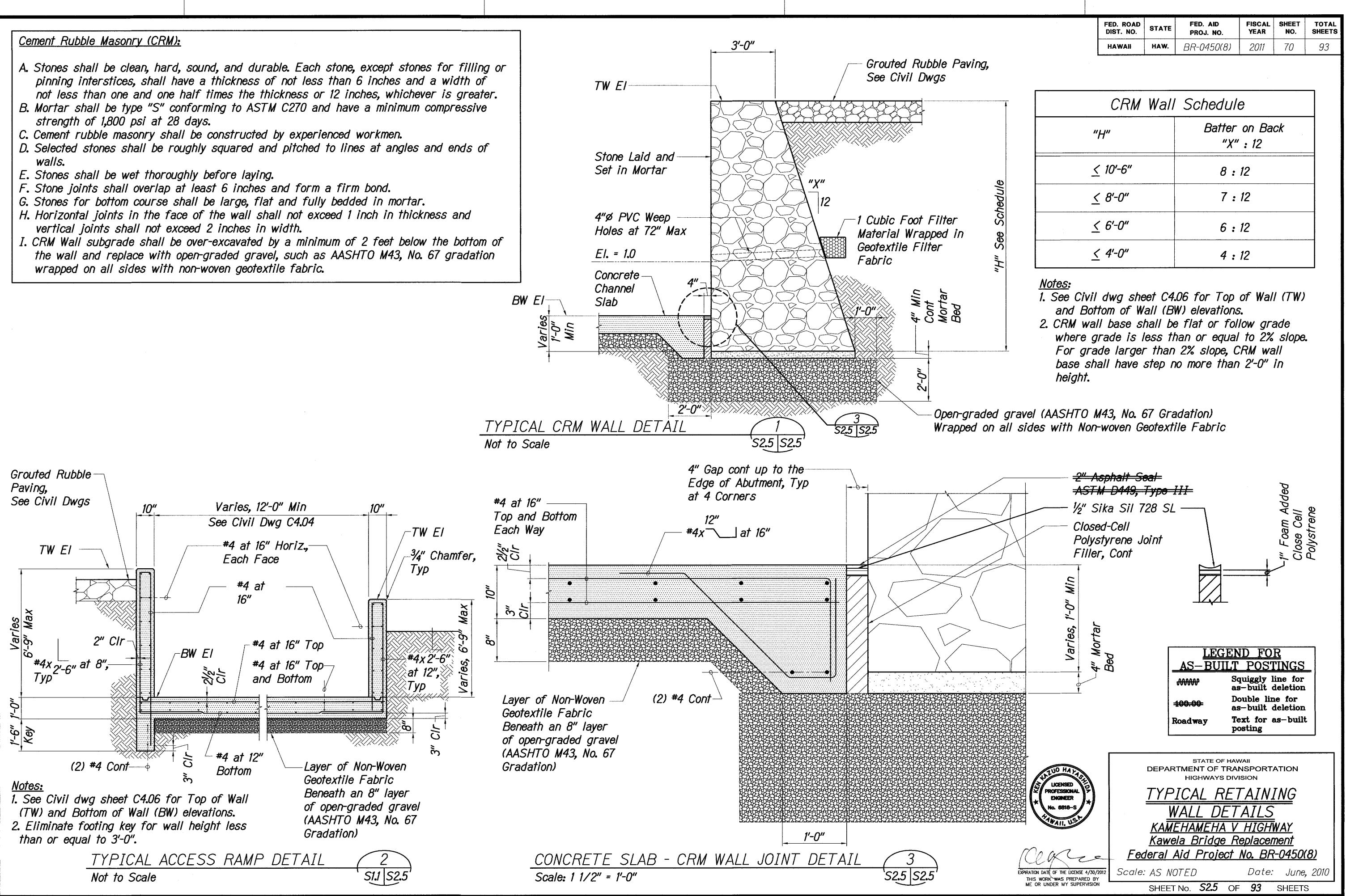
STATE OF HAWAII

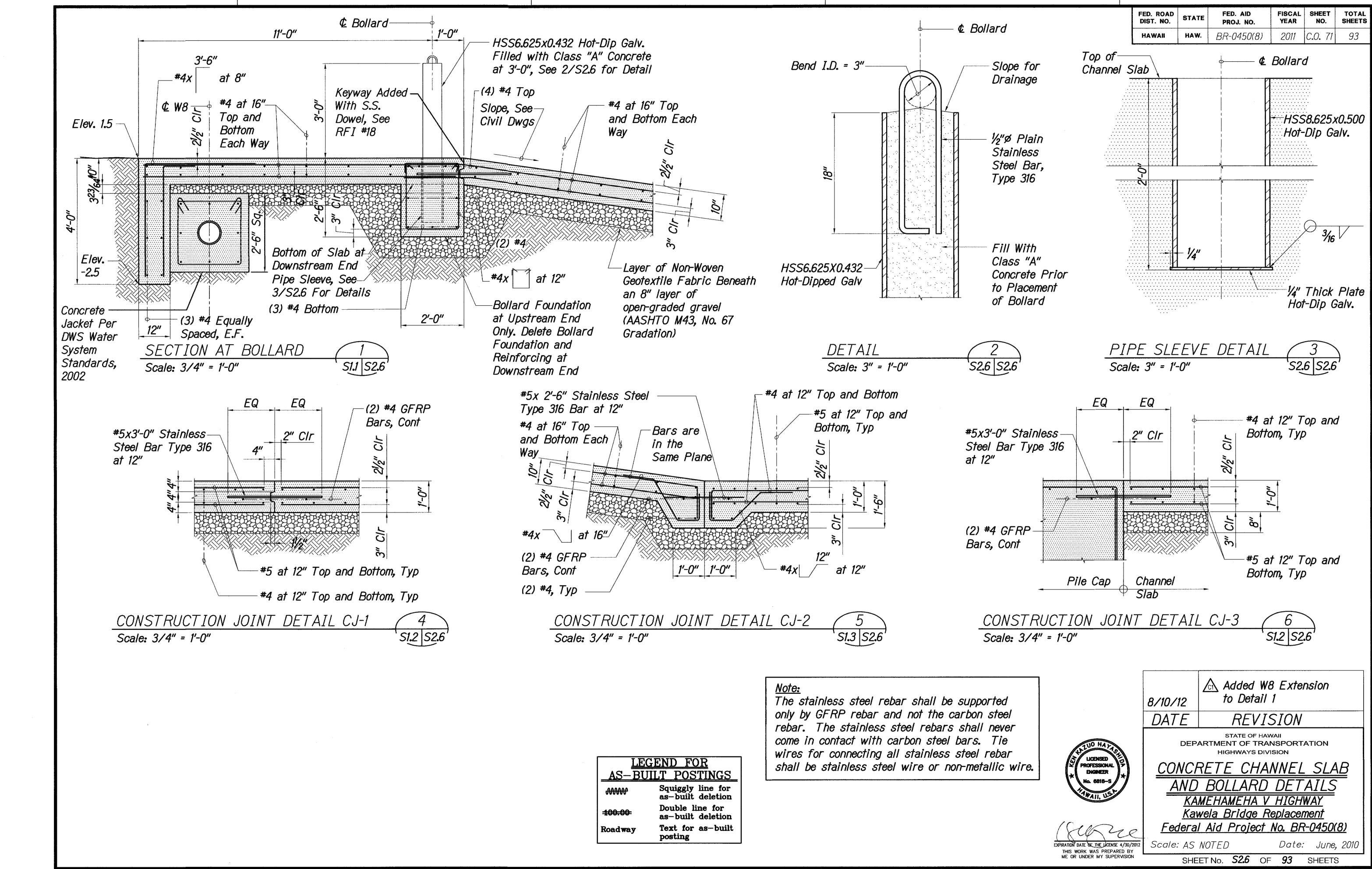
HIGHWAYS DIVISION

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Federal Aid Project No. BR-0450(8) Scale: AS NOTED Date: June, 2010

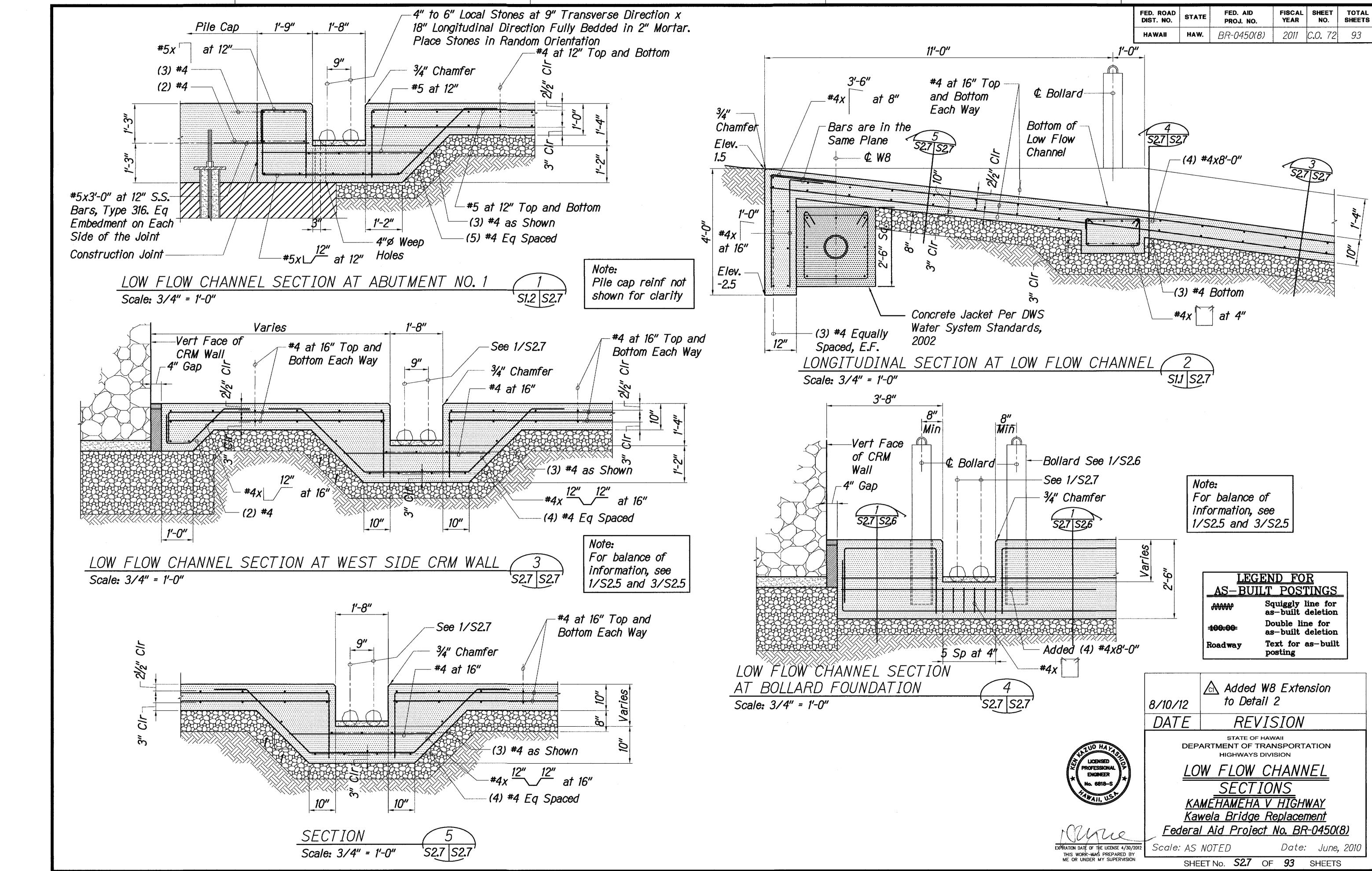
SHEET No. **52.4** OF **93** SHEETS





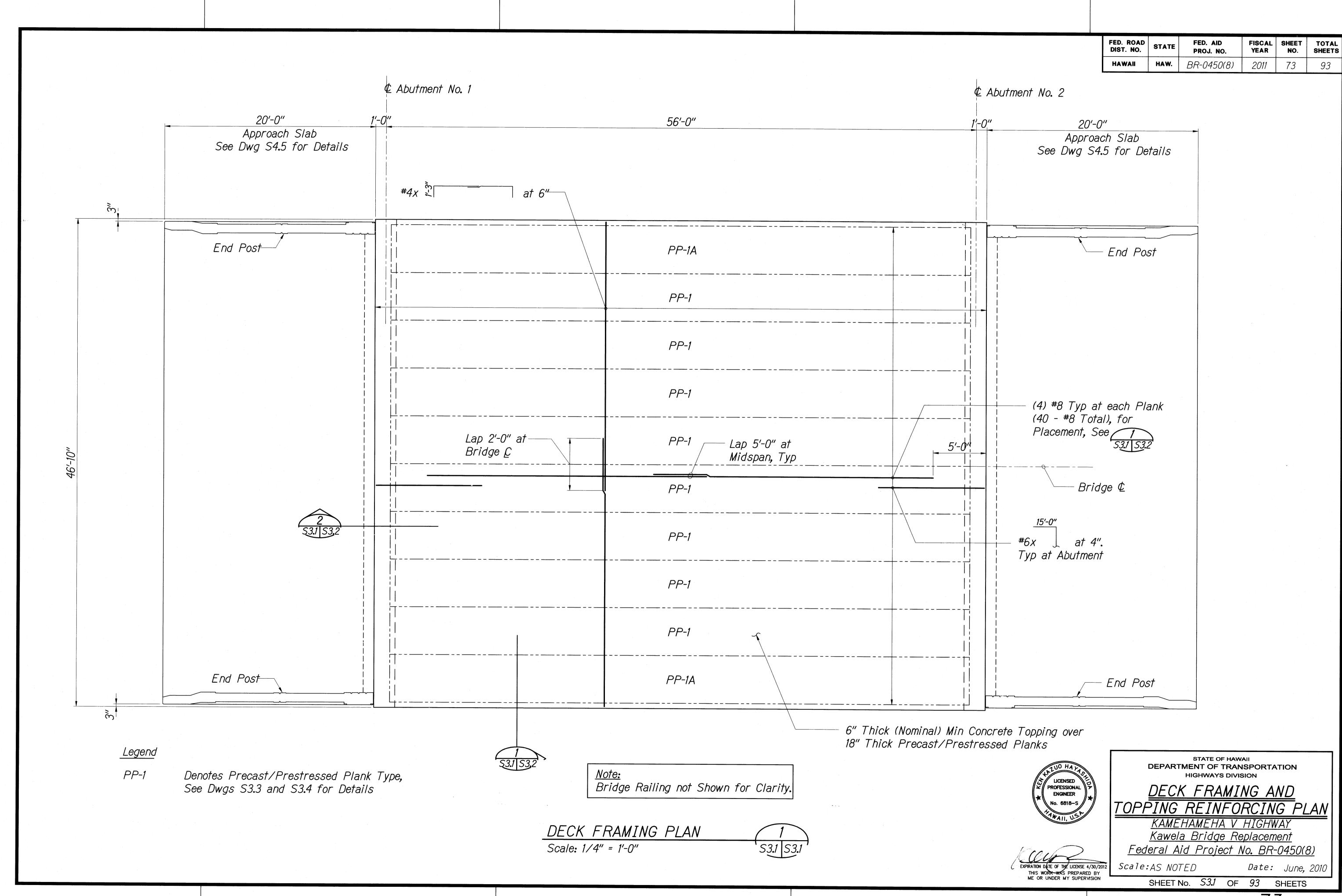
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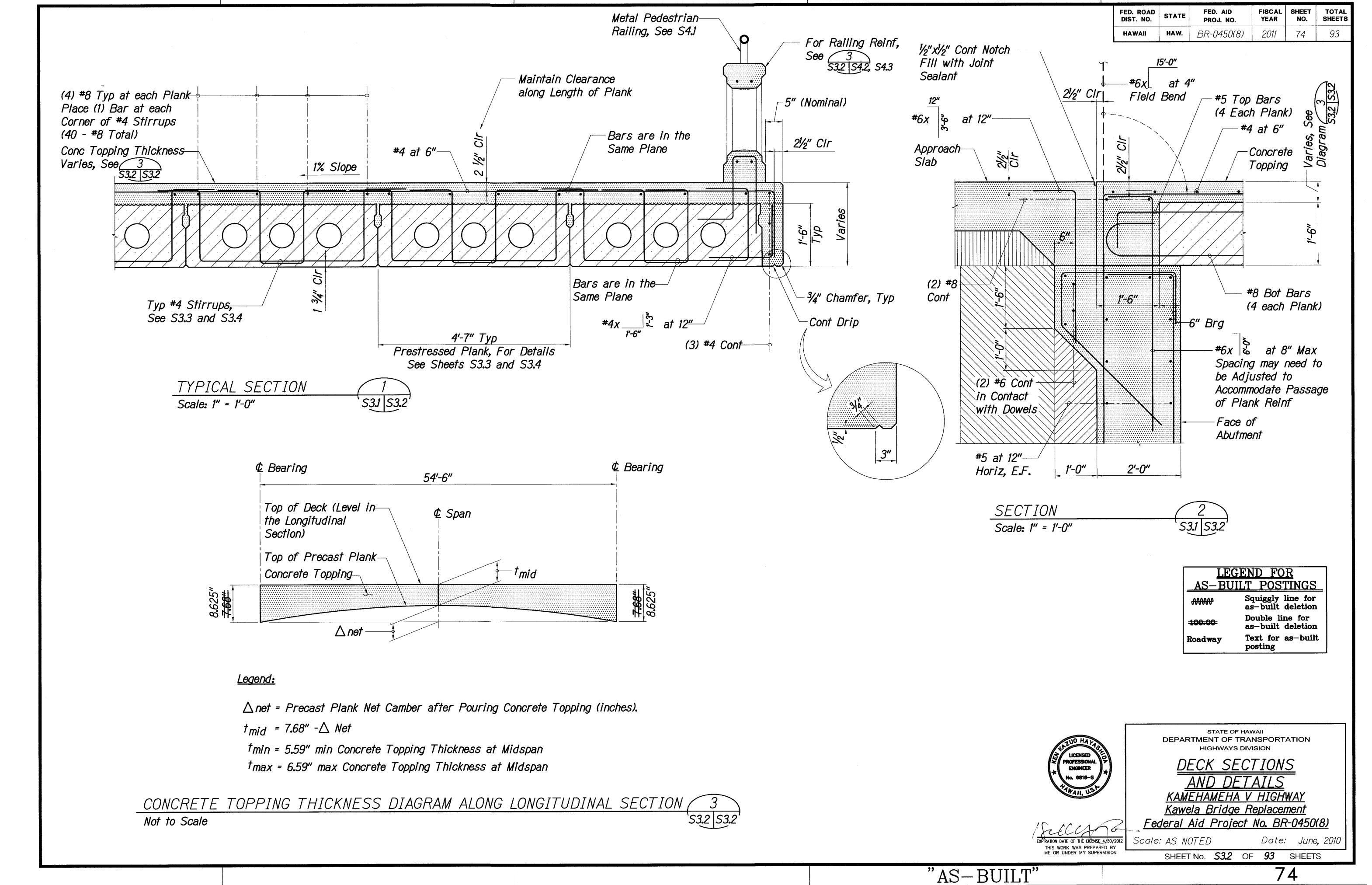
C.O. 71



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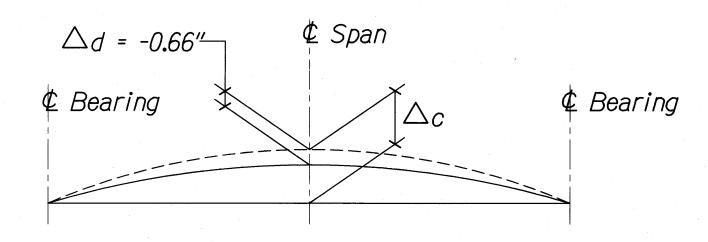
C.O. 72





PRESTRESSED PLANK NOTES:

- 1. Prestressed Concrete 28 Day Strength f'c = 7,000 psi. Prestressed Concrete Strength at time of release fci = 5,500 psi.
- 2. Prestressing Strands shall be (7) Wire 1/2" Low Relaxation Steel Strands (Area = 0.153 in2) conforming to ASTM A416 with an Ultimate Tensile Strength of 270 ksi. Initial Strand Stress (before any Losses) = 202.5 ksi.
- 3. Non-Prestressed reinforcing Steel shall be deformed bars conforming to ASTM A615, Grade 60, unless noted otherwise.
- 4. Strand Pattern shall be symmetrical about the Longitudinal & of the Plank.
- 5. Strand release sequence shall not induce any Lateral Deflection of the Plank.
- 6. Contractor shall submit shop drawings indicating Proposed Strand Pattern, releasing sequence, Reinforcing details and Hold Down Device details to the Engineer prior to fabrication.
- 7. During curing, care shall be taken to avoid any Lateral Deflection to the Plank due to improper orientation. Steam curing may be used to accelerate Strength Gain.
- 8. Lifting Devices shall be placed as close as possible to the Centerline of Bearings of the Plank. Details and locations of Lifting Devices shall be submitted to the Engineer for approval. Such approval does not Relieve the Contractor of his responsibilities if Plank is damaged due to failure of the Lifting Device.
- 9. P_(e) Effective Prestress Force after all Losses (KIPS)
- 10. L = 55'-0" (Prestressed Plank length)
- 11. The stirrup and "U" bar spacing for Plank PP-1A allows for installation of 8" wide x 4" high scupper openings and window openings as shown on Sheets S4.1 and S4.4. Contractor shall take special care in locating these rebars.

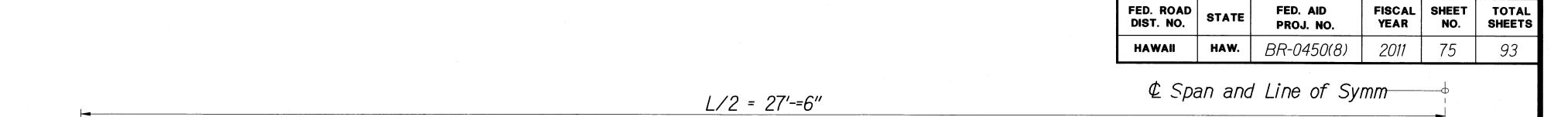


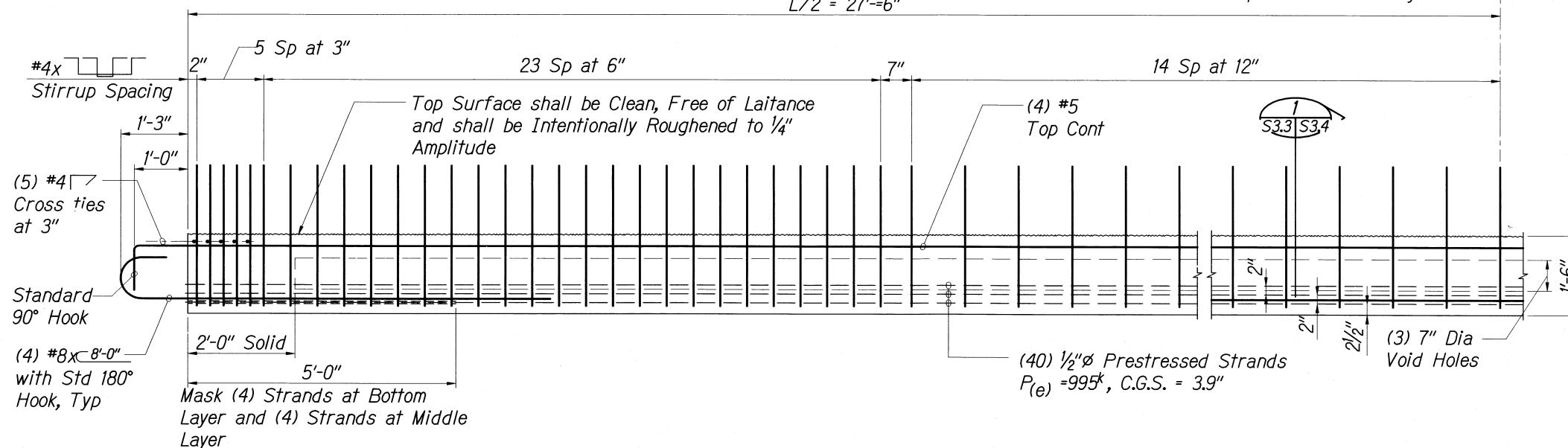
PRESTRESSED PLANK CAMBER DIAGRAM

	Calculated Camber△c
2 Days after Release of Prestress	1.30"
at Erection	2.25"

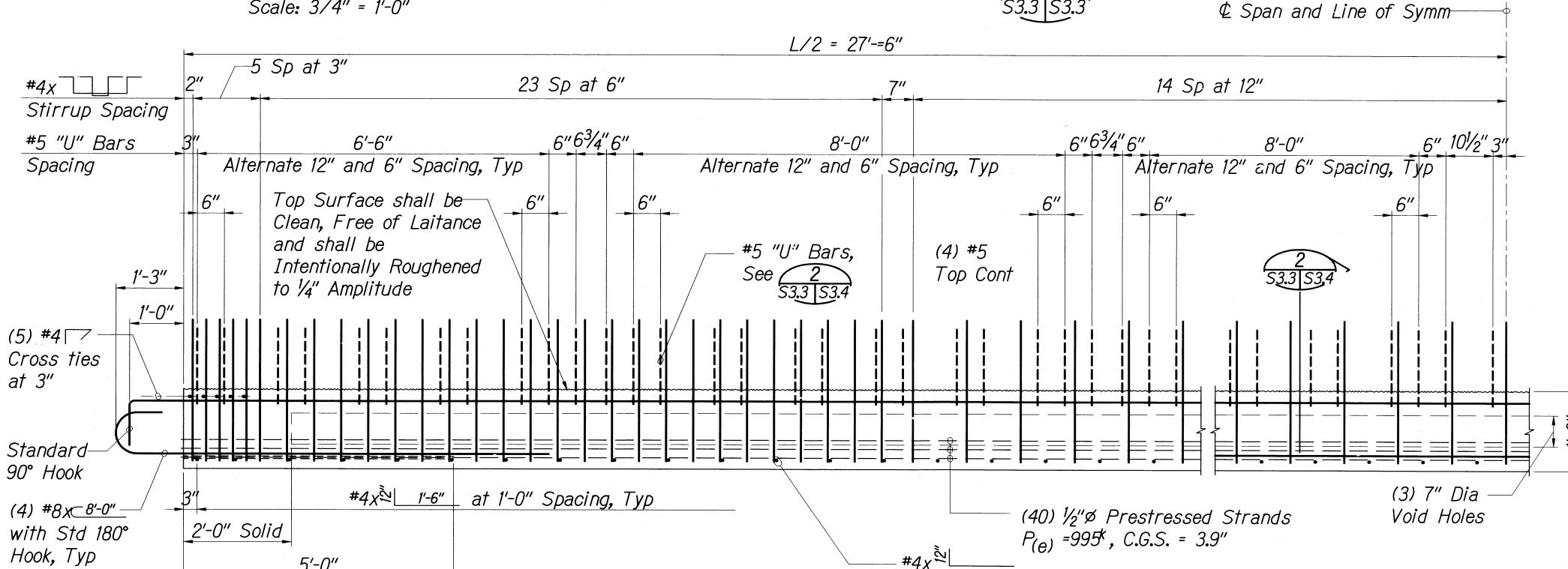
CAMBER DIAGRAM NOTES:

- 1. The Calculated Camber, $\triangle_{\mathcal{C}}$, includes the Effects of the initial Prestress Force and the Weight of the Precast Plank after removal from the Bed. The Calculated Camber value has been Multiplied by Creep factors to approximate the Effect of Camber growth and concrete Creep. Positive values indicate a net Upward Deflection.
- 2. Camber values were obtained using Modulus of Elasticity, E = 3,867 ksi for concrete strength of 5,500 psi (k = 0.86 in AASHTO LRFD equation 5.4.2.4-1). Type of aggregate to be used in all the Girders shall be obtained from a Single supplier and Quarry site.





TYPICAL INTERIOR PRESTRESSED PLANK PP-1 `S3.3 S3.3' Scale: 3/4" = 1'-0"



Layer and (4) Strands at Middle Layer TYPICAL EXTERIOR PRESTRESSED PLANK PP-1A

3. The Contractor shall measure the Actual Camber of each Girder no more than 2 days after the Release of Prestress and again just before Erection in the field. Actual Camber shall not vary from

4. Dead Load deflection, \triangle_d , is due to the Weight of the Concrete Topping.

Mask (4) Strands at Bottom

the Calculated values by +1/2".

Scale: 3/4" = 1'-0"



THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION

53.3 53.3

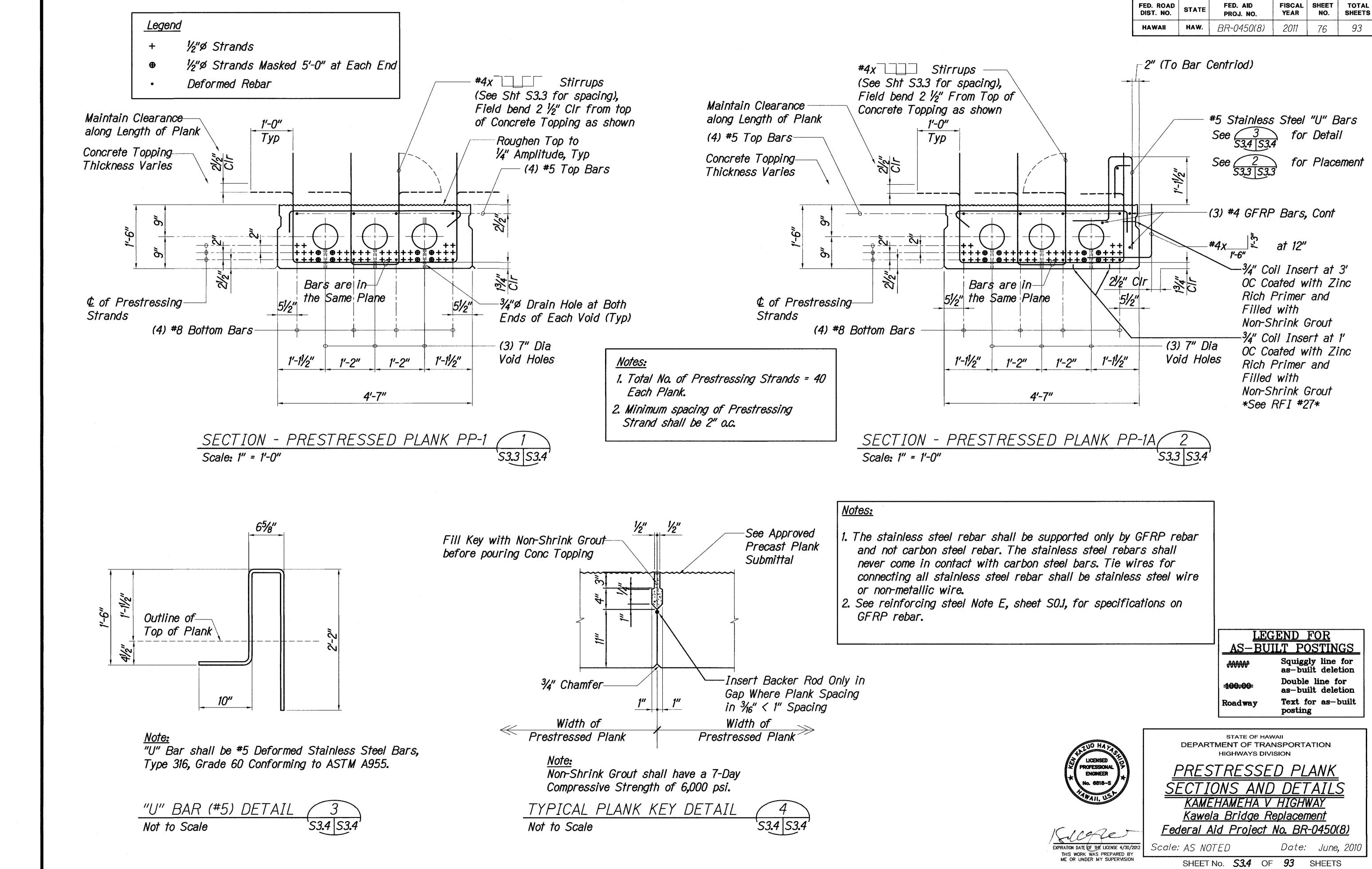
DEPARTMENT OF TRANSPORTATION

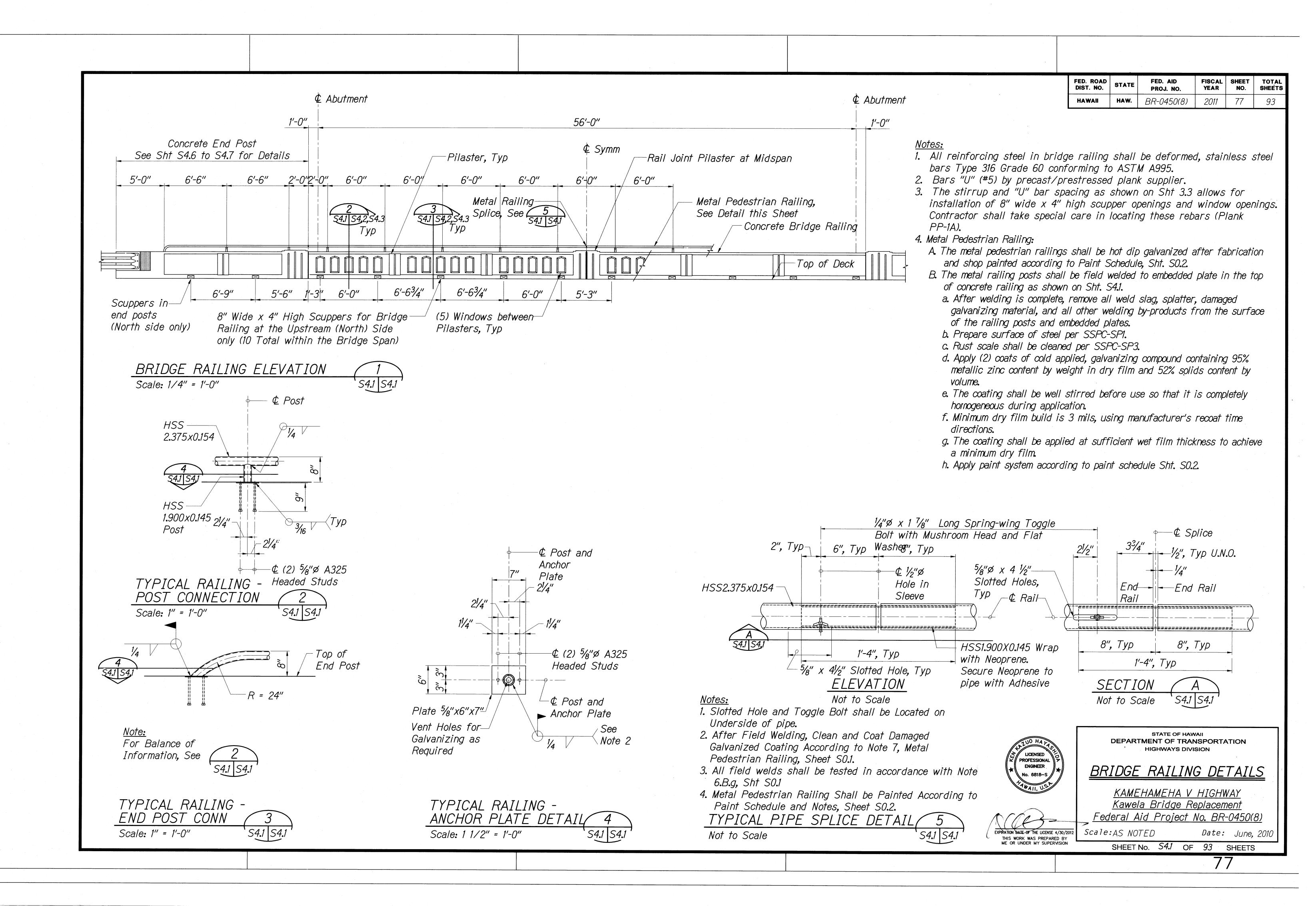
TYPICAL PRESTRESSED PLANK LONGITUDINAL SECTIONS

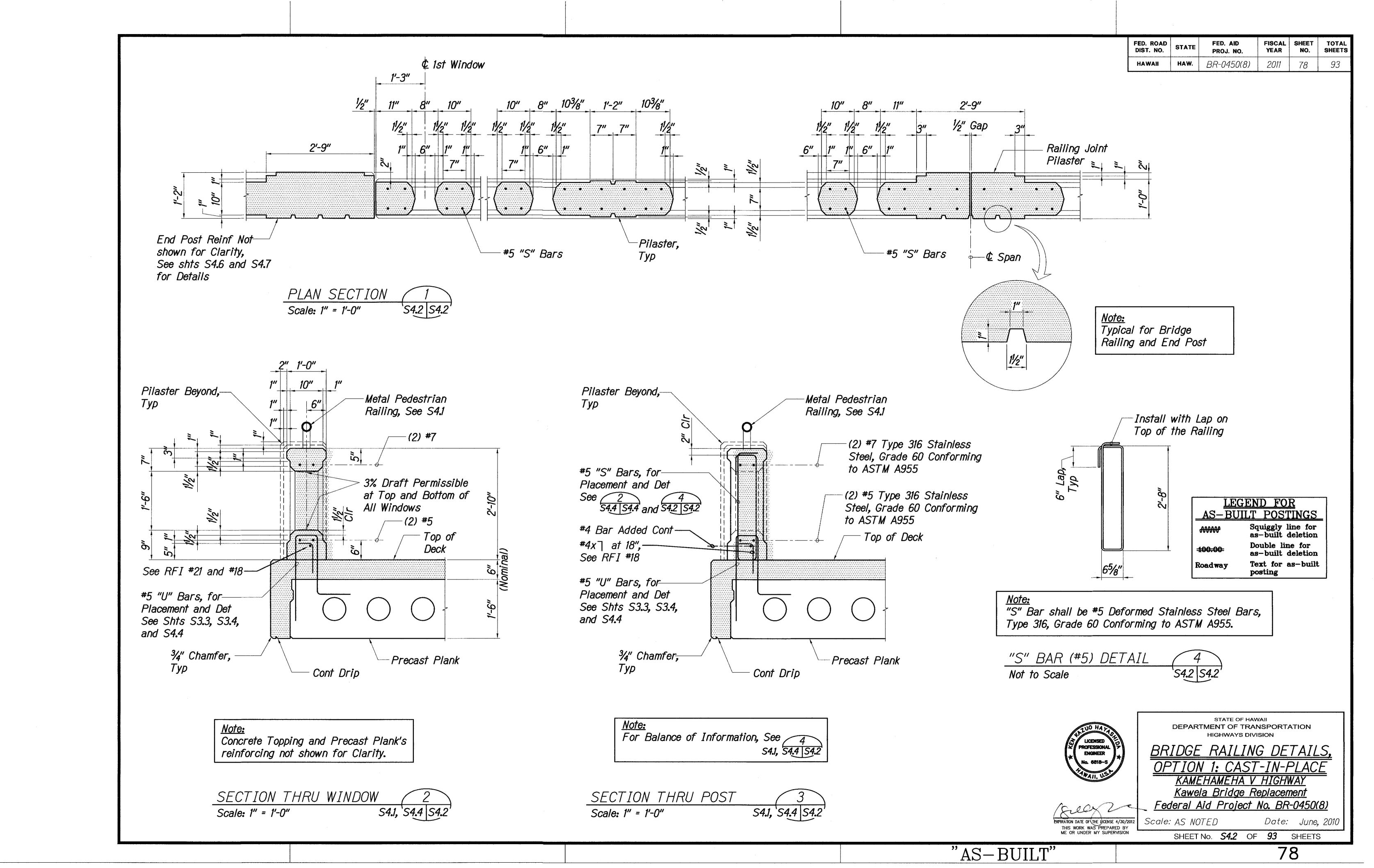
KAMEHAMEHA V HIGHWAY Kawela Bridge Replacement Federal Aid Project No. BR-0450(8)

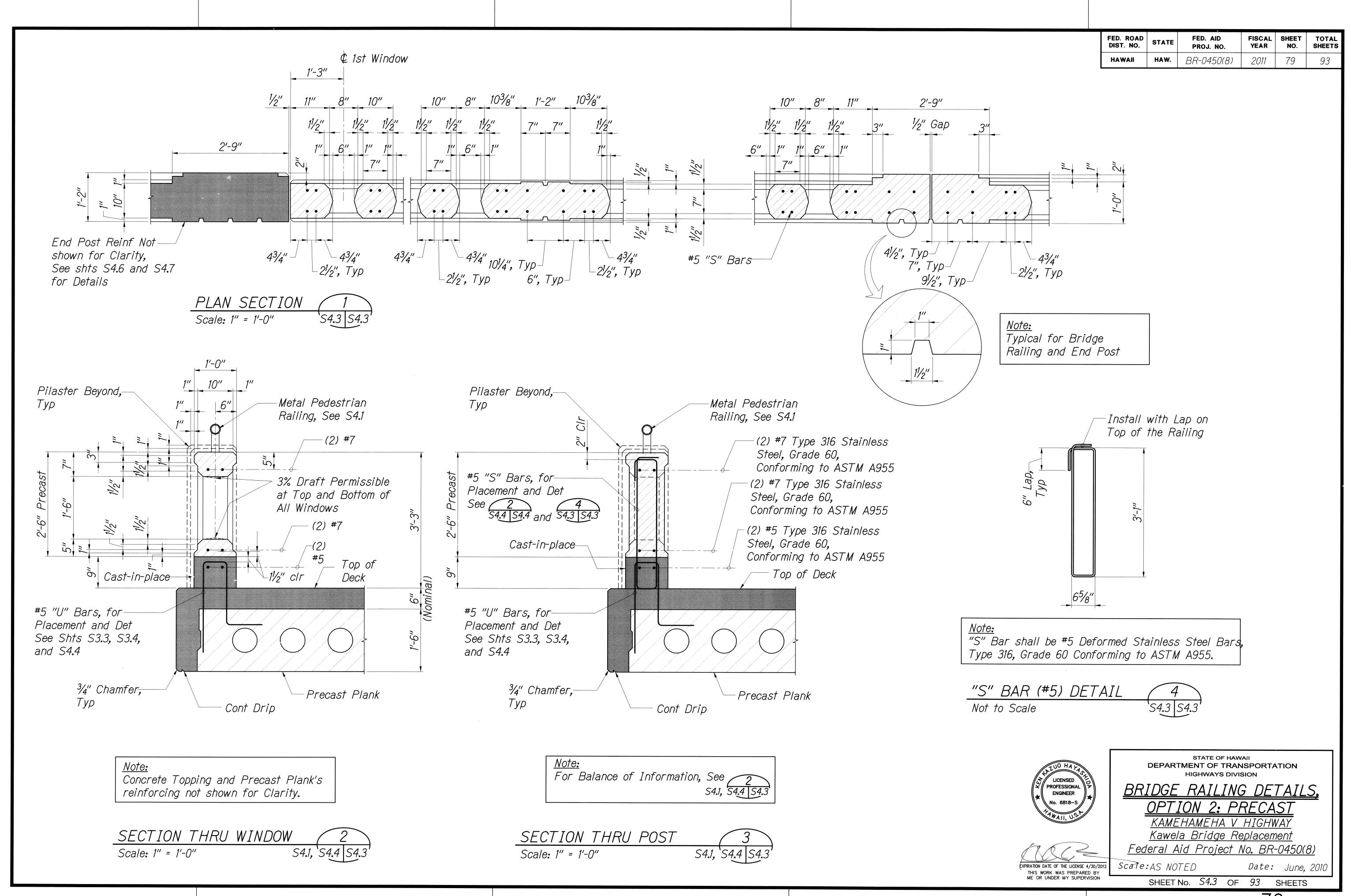
Scale: AS NOTED

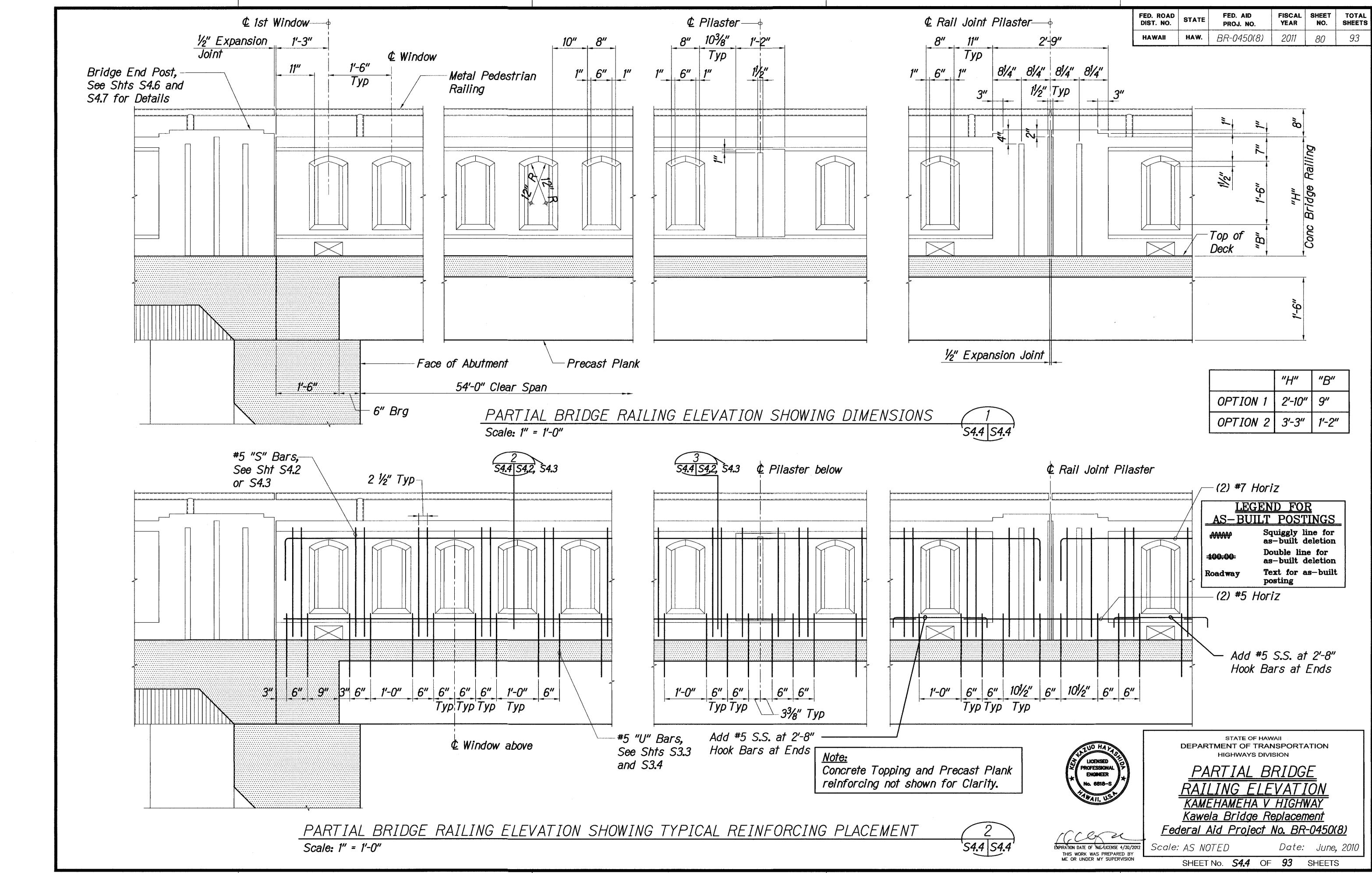
Date: June, 2010 SHEET No. S3.3 OF 93 SHEETS



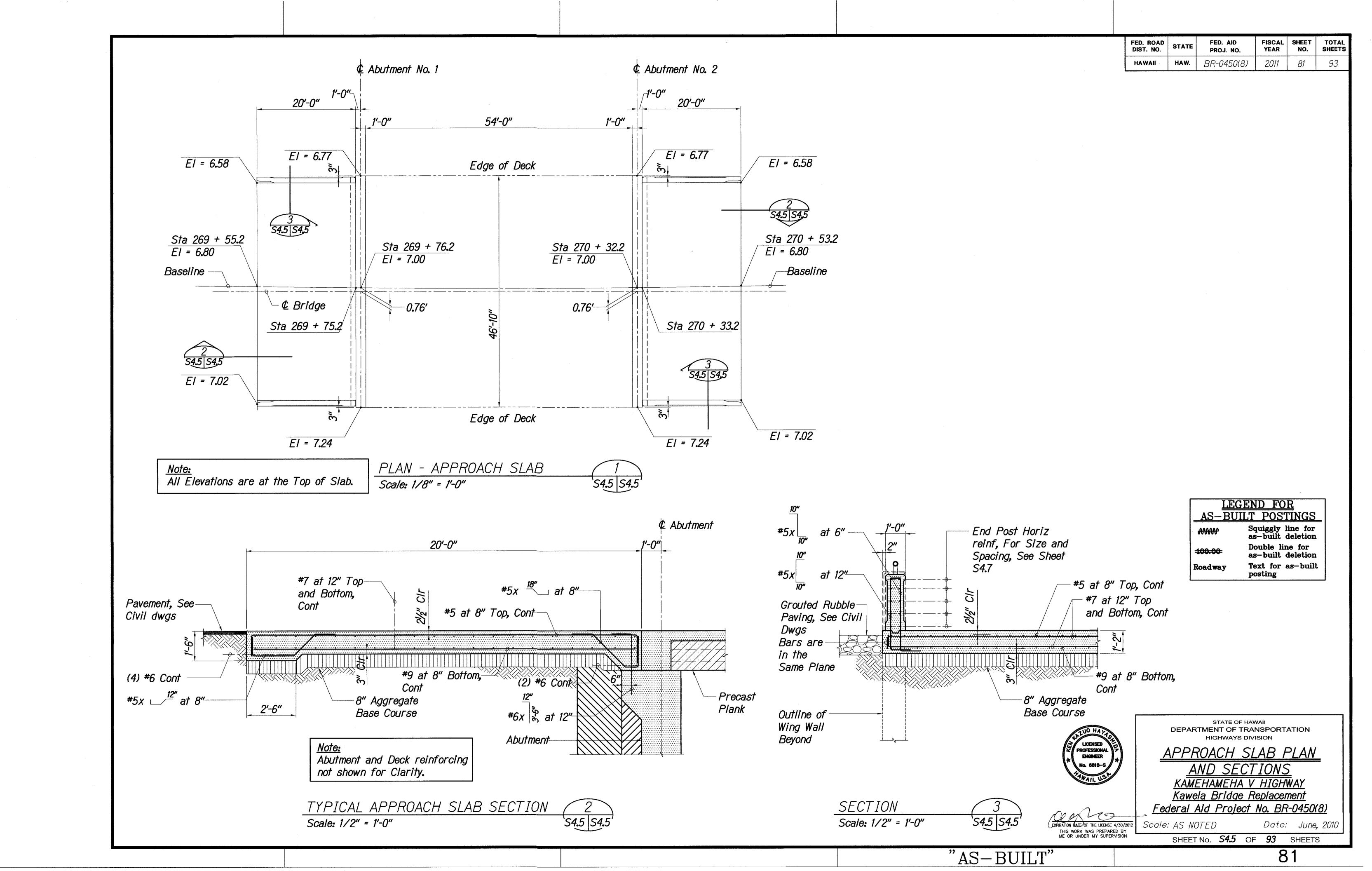


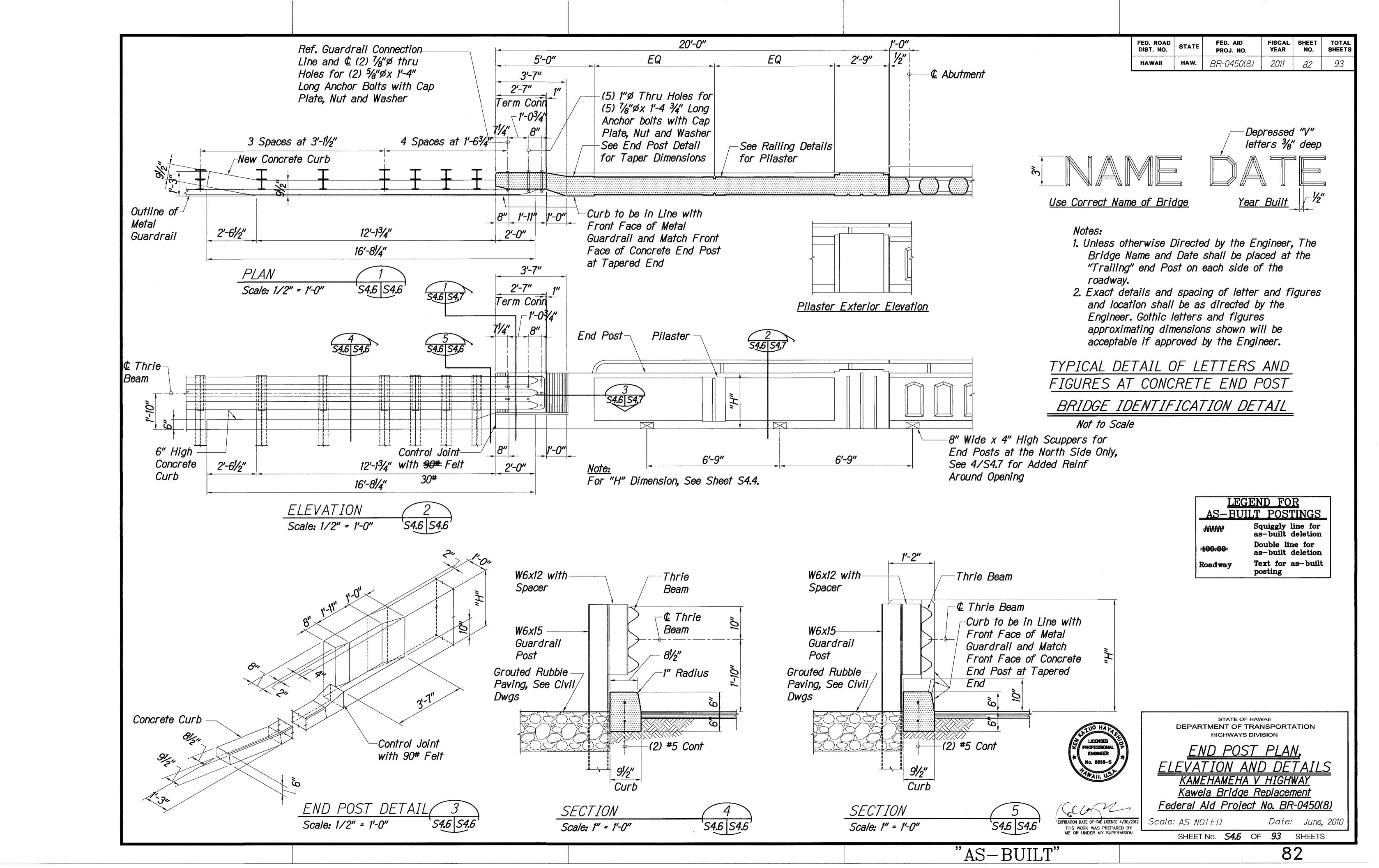


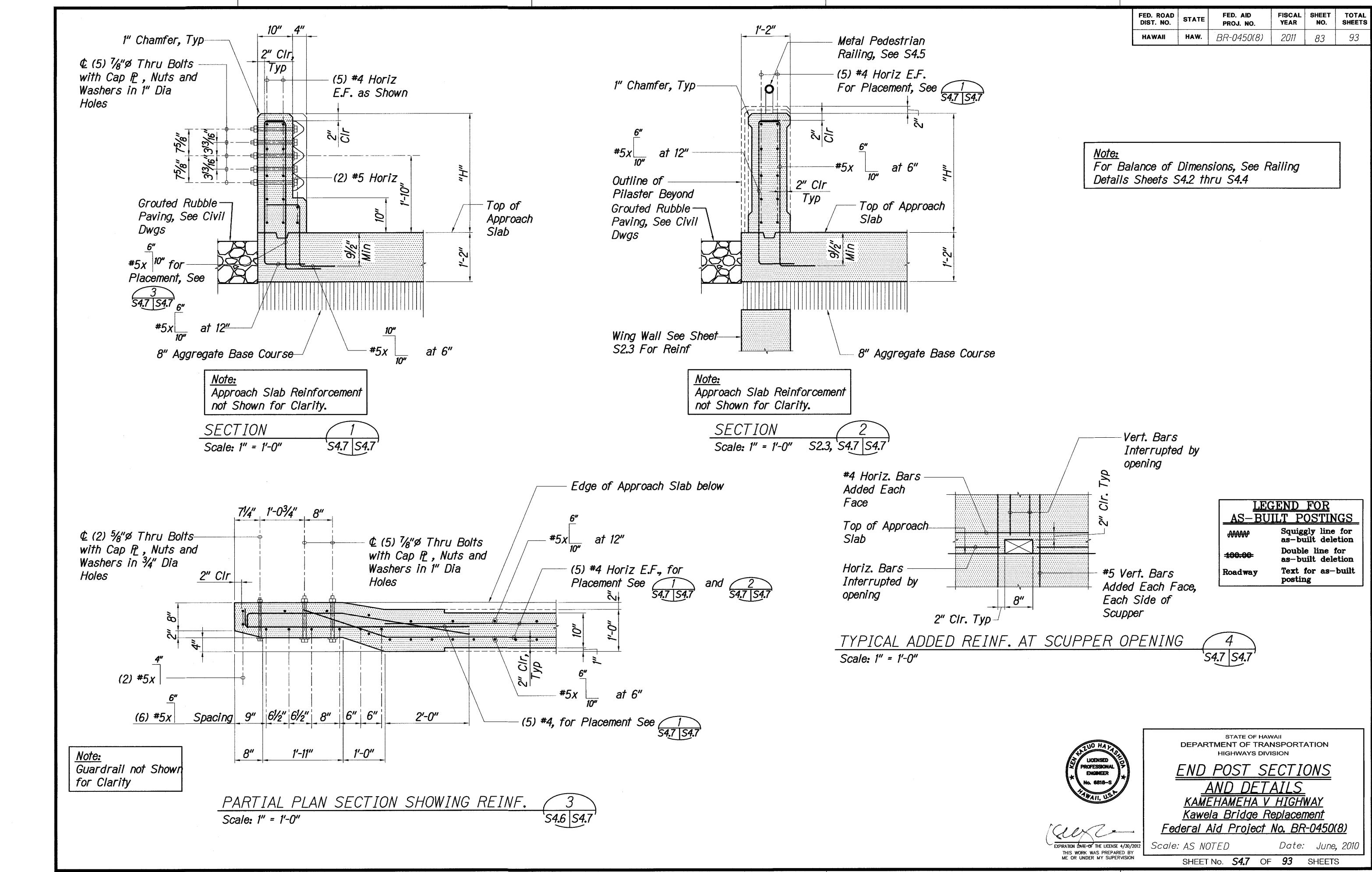




"AS-BUILT"







"AS-BUILT"