GENERAL STRUCTURAL NOTES:

DESIGN SPECIFICATIONS

AASHTO 1998 LRFD Bridge Design Specifications with Subsequent Interim Specifications.

GENERAL SPECIFICATIONS

Hawaii Standard Specifications for Road, Bridge, and Public Works Construction 1994 and Special Provisions prepared for this project.

DESIGN LIVE LOAD: HL-93

ADDITIONAL LOADS:

25 PSF for Future Asphalt Concrete Wearing Surface. 150 PLF for Future Utilities between Girders G-1 & G-2, G-7 & G-8

SEISMIC ACCELERATION COEFFICIENT:

A = 0.25

Seismic zone 3

Importance Category = Essential Bridge

MATERIALS:

Reinforced Concrete: f'c=4000 PSI, unless noted otherwise.

Drilled Shafts: f'c=4500 PSI

Reinforcing Steel: ASTM A615, Grade 60 unless noted otherwise. Structural Steel: ASTM A36, Hot Dip Galvanized, unless noted

otherwise.

Stainless Steel: Type 316.

Anchor Bolts: ASTM A325 Hot Dip Galvanized, unless noted

otherwise.

Prestressed Concrete: f'c=6500 PSI f'ci=5000 PSI

All expansion and premolded joint filler shall be incidental to concrete and will not be paid for separately.

All welding shall be in accordance with the current edition of Reinforcing Steel Welding Code AWS D 1.4.

REINFORCEMENT:

ATE ...

ORIGINAL SURVEY
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NOTEBOOK DESIGNED
QUANTITII

- 1. The minimum clear cover measured from the surface of the concrete to the face of any reinforcing bar shall be as follows, except as noted otherwise.
 - A. Abutments, retaining walls, channel construction. 2"
 - B. Concrete cast against and permanently exposed to earth. . 3"
- Reinforcing shall be detailed in accordance with the latest editions of CRSI "Placing Reinforcing Bars" and ACI "Manual of Standard Practice" and the "Detailing Manual", unless noted otherwise.
- 3. Dimensions relating to reinforcing (E.G. spacing of bars) are to center of bars, unless noted otherwise.
- Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of intersections is less than one foot in each direction, in which case alternate intersections shall be tied.

GIRDER BEARINGS:

- 1. Girder concrete seats receiving elastomeric pads shall be poured monolithically with supporting structure. Top of concrete seats shall be finished with a steel trowel to a smooth level surface to the elevation shown on the plans.
- 2. Elastomeric pads: Bottom of bridge bearing pads shall be secured to the concrete seats, to prevent displacement with adhesives approved by the Engineer.

CONSTRUCTION NOTES:

- 1. See Standard Specifications and Special Provisions.
- 2. In general, top of concrete deck slab shall be constructed to follow the roadway vertical and horizontal curves.
- 3. For the installation of anchor bolts, the Contractor shall provide rigid templates to maintain the proper locations and shall protect such anchor bolts at all times during the period of construction.

 Methods shall be approved by the Engineer.
- 4. Except as noted otherwise, all vertical dimensions are measured plumb.
- 5. The Contractor shall verify all site conditions and not rely upon these plans for utilities or stream location, etc. Conditions may differ from those shown.
- The Contractor shall verify the location of all utility lines and notify the respective owners before commencing the work of excavation or the installation of drilled shafts, including any temporary piling or sheeting. Any damage to utility lines caused by the Contractor shall be replaced at his expense and at no cost to the State.
- 7. For concrete finish, see Standard Specifications.
- . Construction joints may be relocated or additional ones added, subject to the approval of the Engineer.
- 9. Unless noted otherwise, all exposed concrete edges shall be chamfered 3/4"x3/4".
- O. Concrete for closure pours in bridge deck and diaphragms to have admixtures to reduce setting time and drying shrinkage. See special provisions for requirements.

GENERAL:

- 1. All items noted incidental will not be paid for separately.
- 2. Standard detail drawings refer to all structures in general, except for modifications as may be required for special conditions. For such modifications, refer to the corresponding detailed drawings.
- 3. For electrical conduit location details, see electrical drawings.

FOUNDATION:

- 1. General:
 - A. The pay limits for excavation shown on the standard plans should not be considered as indicative of actual excavation requirements.
 - B. The Contractor shall assume sole responsibility for the construction and safety of all footing excavations. The Contractor shall submit his excavation plan to the Engineer for review, although such review will not absolve the Contractor from sole responsibility, as described.
- 2. A geotechnical report titled, "Geotechnical Engineering Exploration, Kamehameha V Highway, Emergency Replacement of Kawaikapu Bridge, Federal Aid Project No. ER-12(4), Island of Molokai, Hawaii" dated March 2001, by Geolabs, Inc., has been prepared and is available for review at the office of the Engineer.

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	ER-12(4)	2003	51	76

	SUMMARY OF ESTIMATED QUANTITIES		
ITEM NO.	CONTRACT ITEM	QUANTITY	UNIT
206.5500	STRUCTURE EXCAVATION FOR BRIDGE ABUTMENTS	475	C.Y.
206.5600	STRUCTURE EXCAVATION FOR CONCRETE CHANNEL	45	C.Y.
206.7500	STRUCTURE BACKFILL FOR BRIDGE ABUTMENTS	215	C.Y.
206.7600	STRUCTURE BACKFILL FOR CONCRETE CHANNEL	50	C.Y.
503.1092	CONCRETE IN BRIDGE ABUTMENTS (EXCEPT FOOTINGS) (33 C.Y.)	L.S.	L.S.
503.1093	CONCRETE IN FOOTINGS FOR BRIDGE ABUTMENTS (101 C.Y.)	L.S.	L.S.
503.1094	CONCRETE IN BRIDGE DECK (63 C.Y.)	L.S.	L.S.
503.1095	CONCRETE IN CONCRETE CHANNEL (145 C.Y.)	L.S.	L.S.
503.1096	CONCRETE IN APPROACH SLAB (67 C.Y.)	L.S.	L.S.
504.4501	AASHTO TYPE II GIRDERS (500 L.F.)	L.S.	L.S.
507.7100	BRIDGE RAILING	123	L.F.
511.1100	FURNISHING DRILLED SHAFT DRILLING EQUIPMENT	L.S.	L.S.
511.2000	UNCLASSIFIED SHAFT EXCAVATIONS	978	L.F.
511.3000	OBSTRUCTIONS	10	HR.
511.4000	DRILLED SHAFTS	978	L.F.
511.5000	TRIAL SHAFT	104	L.F.
511.6000	LOAD TESTS	1	EA.
511.7000	CORING FOR INTEGRITY TESTING	170	L.F.
602.0091	REINFORCING STEEL FOR BRIDGE ABUTMENTS (EXCEPT FOOTINGS) (8,500 LBS.)	L.S.	L.S.
602.0092	REINFORCING STEEL FOR FOOTINGS FOR BRIDGE ABUTMENTS (29,240 LBS.)	L.S.	L.S.
602.0093	REINFORCING STEEL FOR BRIDGE DECK (14,500 LBS.)	L.S.	L.S.
602.0094	REINFORCING STEEL FOR CONCRETE CHANNEL (14,100 LBS.)	L.S.	L.S.
602.0095	REINFORCING STEEL FOR APPROACH SLAB (10,500 LBS.)	L.S.	L.S.
			٠.



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Signature
April 30, 2004

Expiration Date of the License

DATE REVISION

STATE OF HAWAII

DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

GENERAL NOTES AND SUMMARY

OF ESTIMATED QUANTITIES

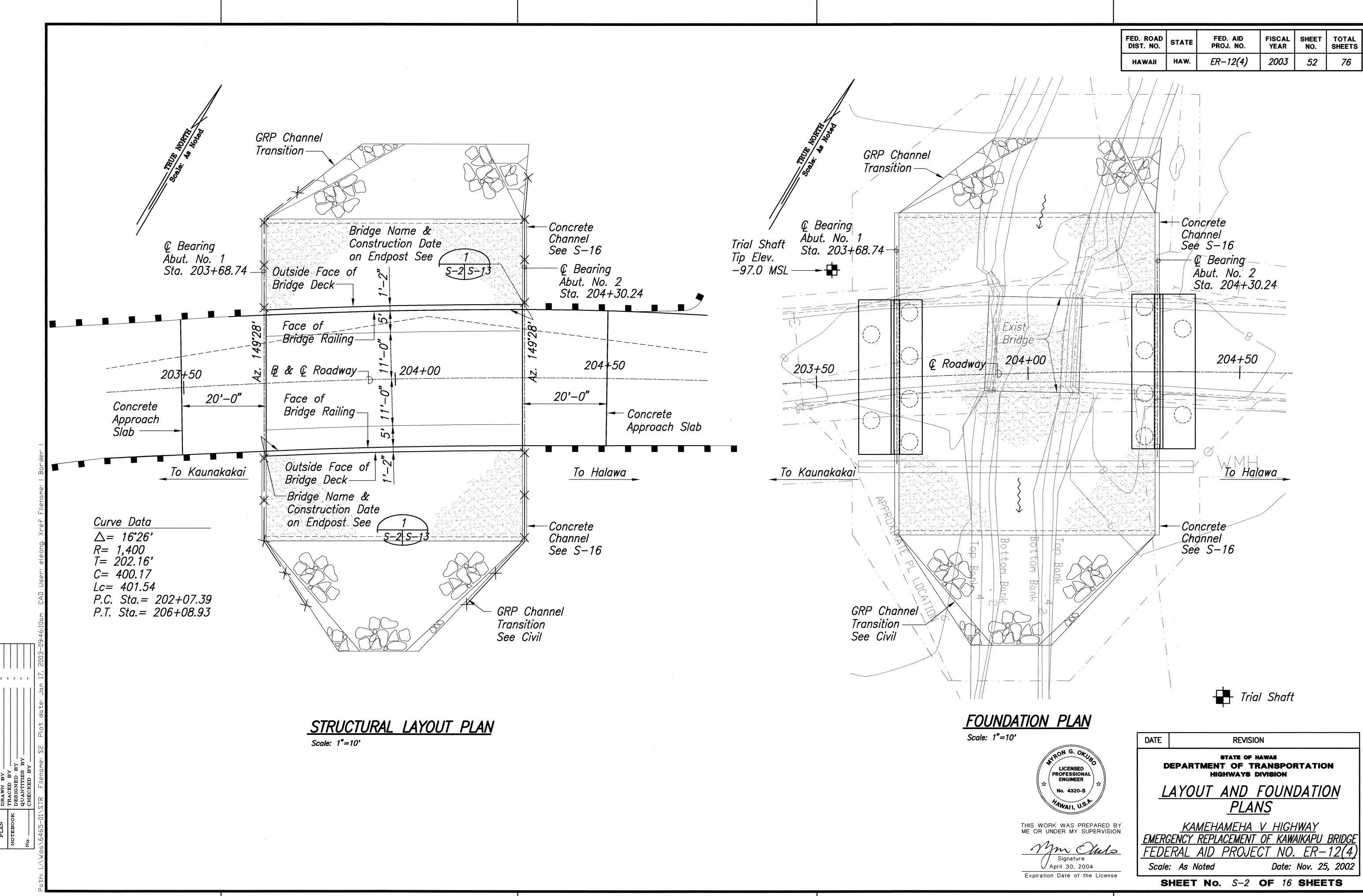
KAMEHAMEHA V HIGHWAY

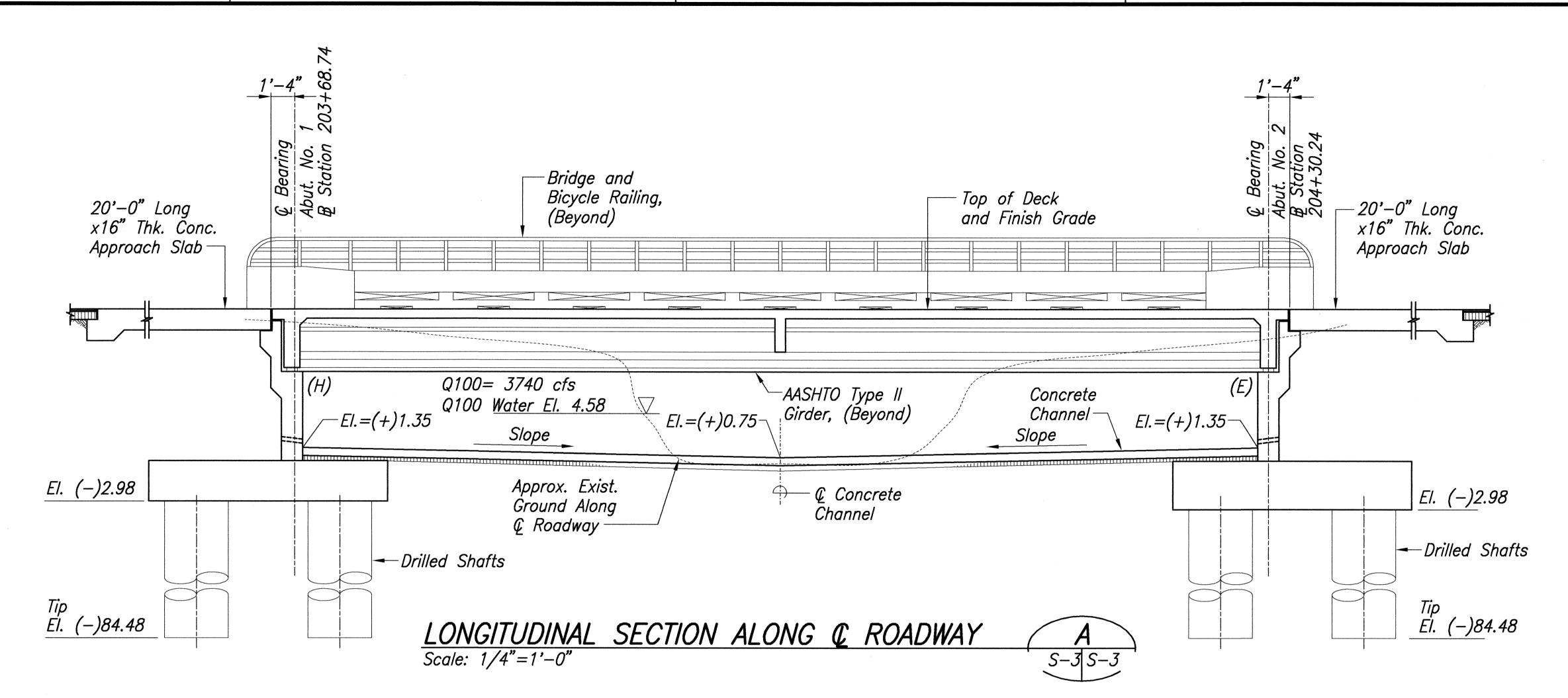
EMERGENCY REPLACEMENT OF KAWAIKAPU BRIDGE

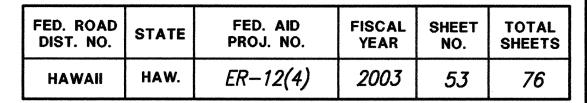
FEDERAL AID PROJECT NO. ER-12(4)

Scale: As Noted Date: Nov. 25, 2002

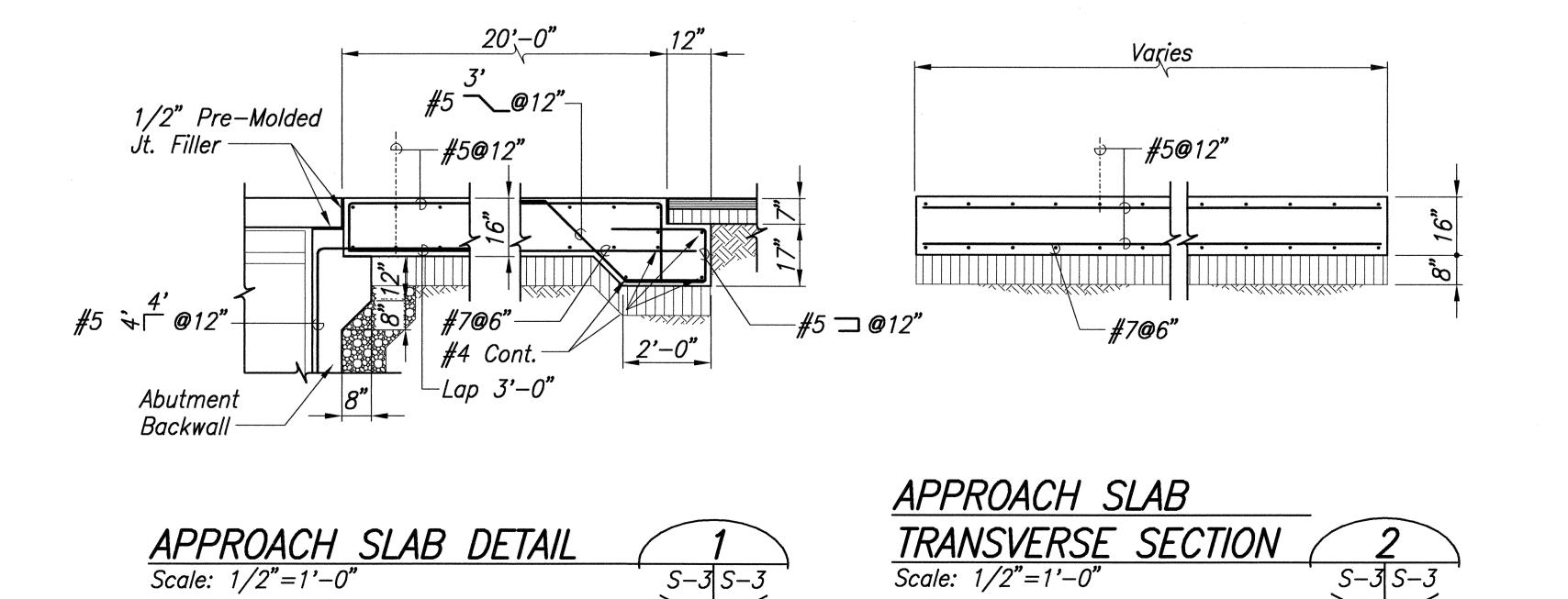
SHEET No. S-1 OF 16 SHEETS



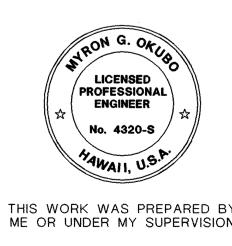




- Hinged Girder End
- Expansion Girder End



S-3 S-3

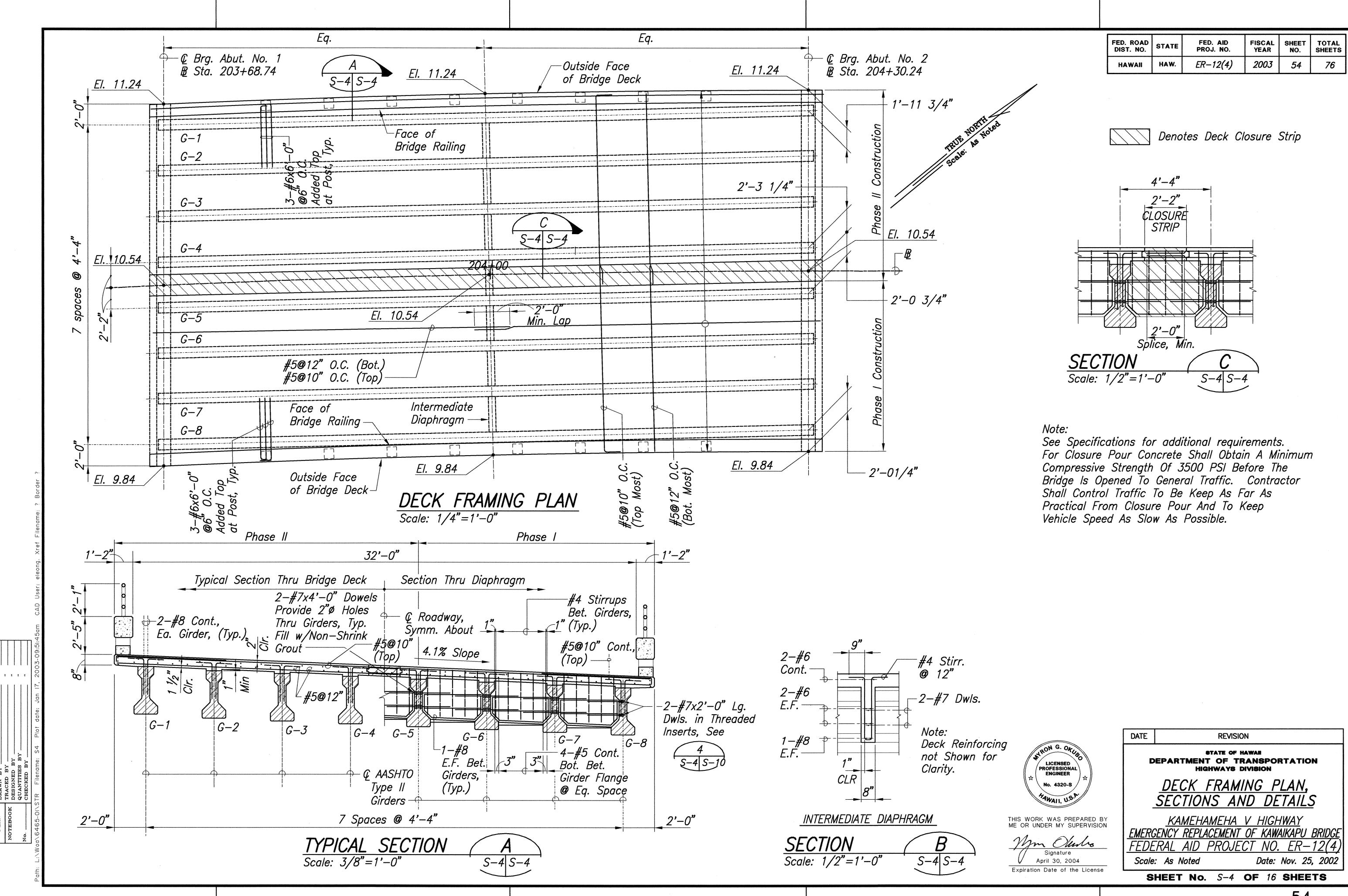


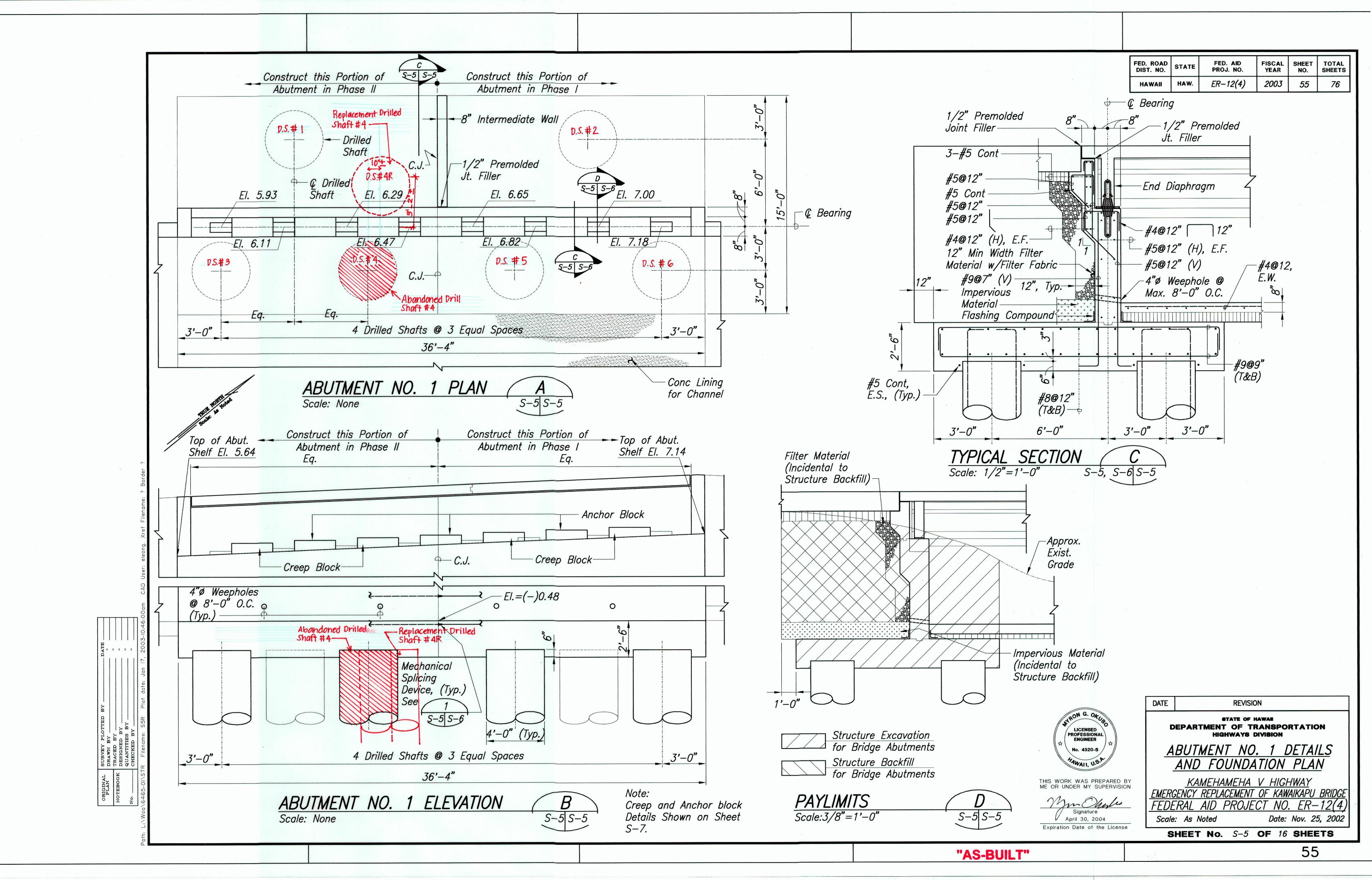
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION Signature
April 30, 2004 Expiration Date of the License

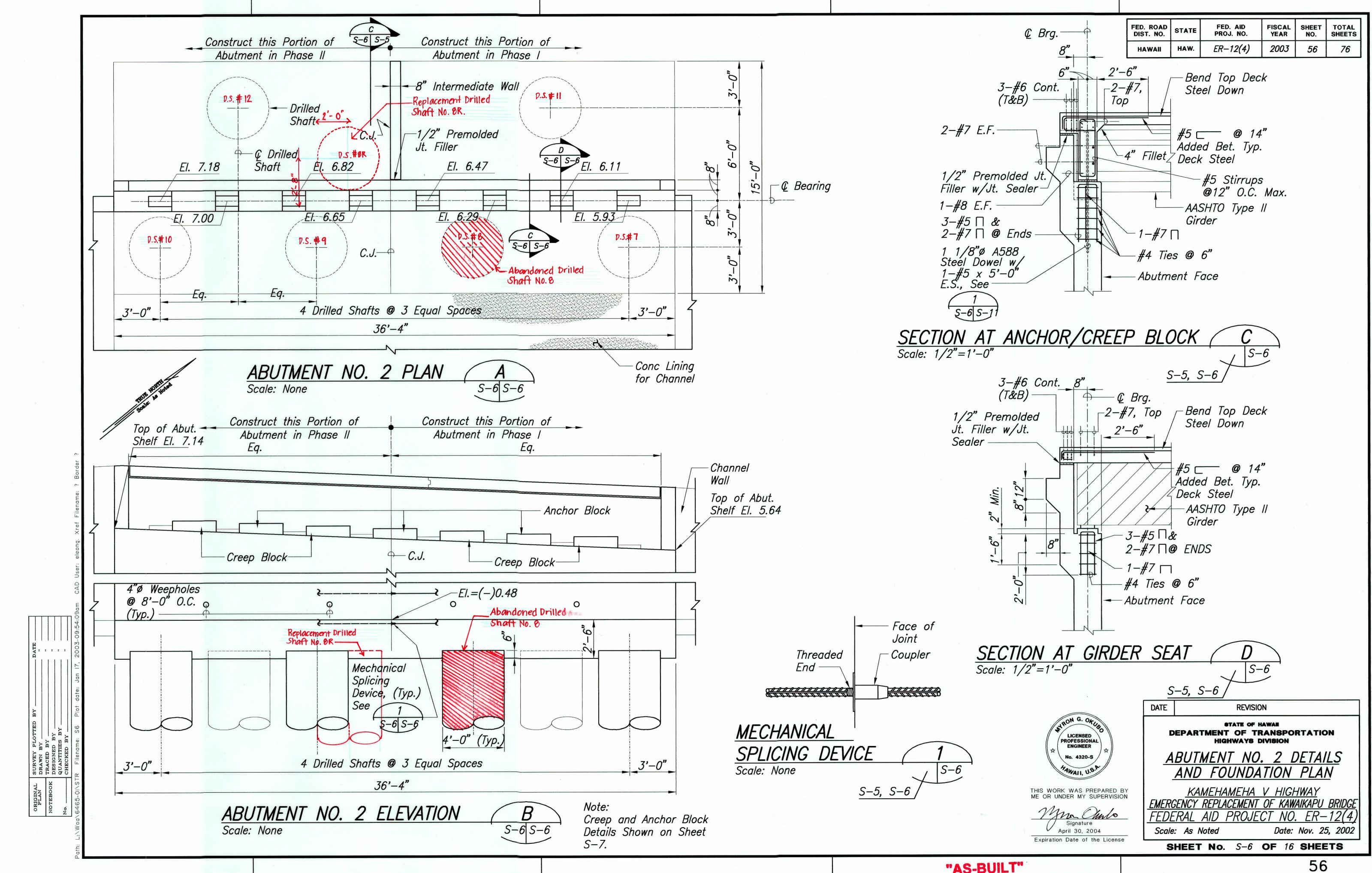
DATE **REVISION** STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION LONGITUDINAL SECTION

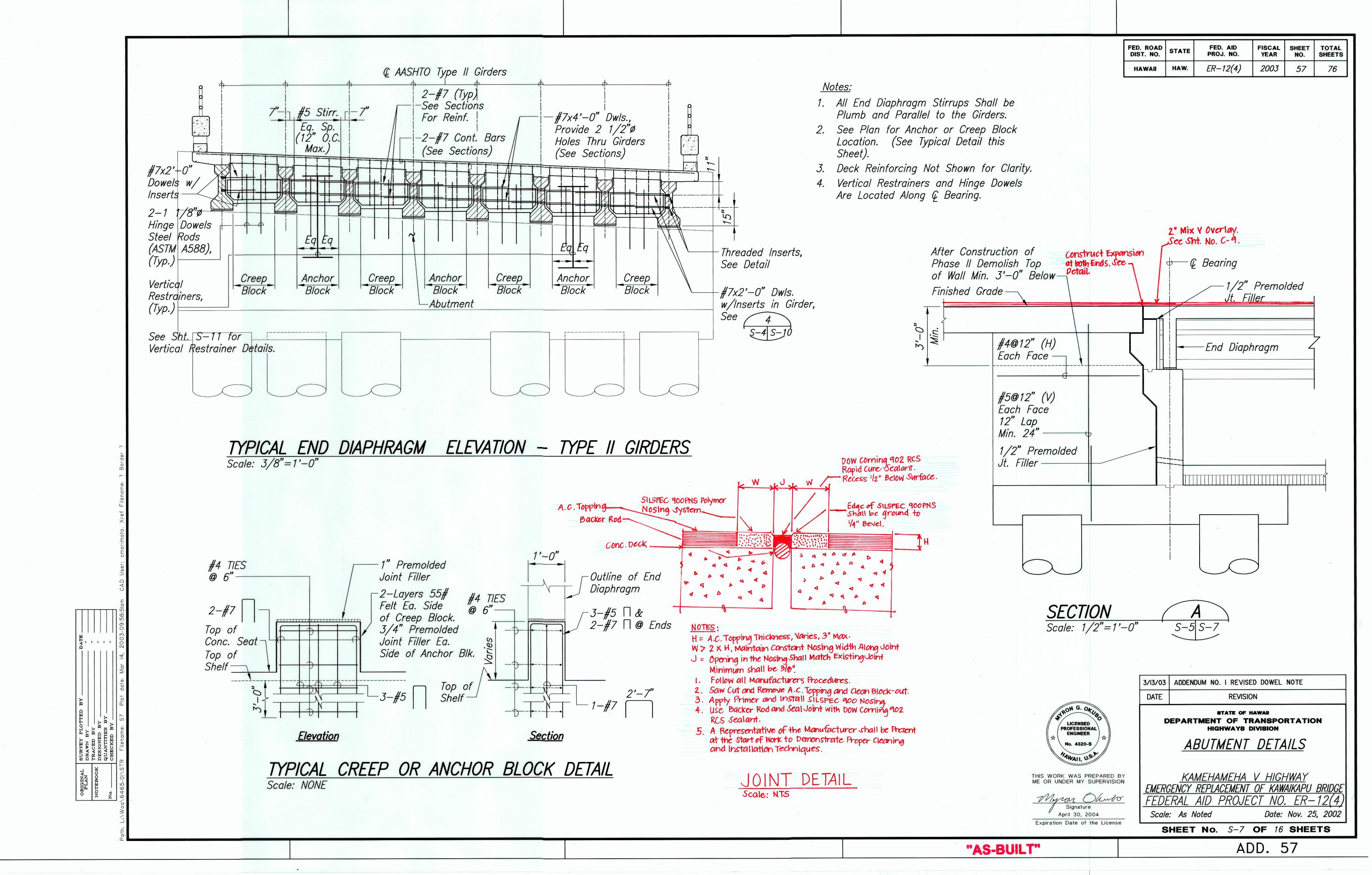
KAMEHAMEHA V HIGHWAY EMERGENCY REPLACEMENT OF KAWAIKAPU BRIDGE FEDERAL AID PROJECT NO. ER-12(4) Scale: As Noted Date: Nov. 25, 2002

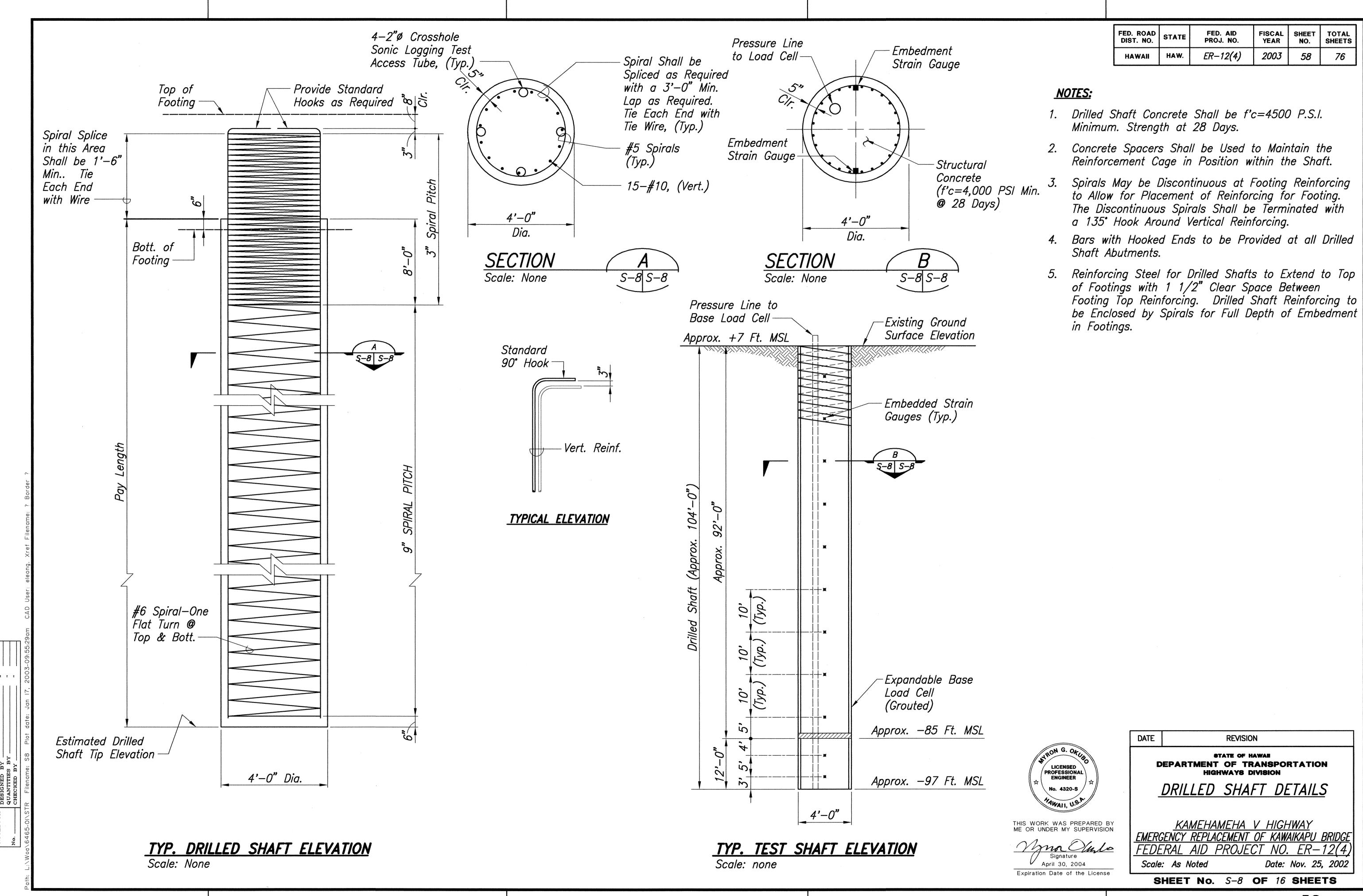
SHEET No. S-3 OF 16 SHEETS











PRESTRESSED GIRDER NOTES:

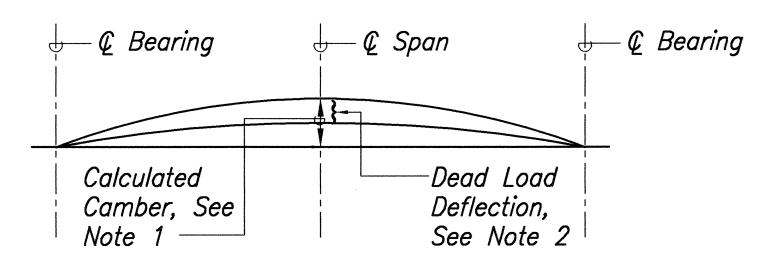
- 1. Concrete Shall Have a Minimum Ultimate 28 Day Compressive Strength of 6500 PSI. Minimum Compressive Strength Before Release of Strands is 5000 PSI.
- 2. Prestressing Stands are One-half Inch Diameter, 7 Wire Low-Relaxation Steel Strands (Area = 0.1531 in 2) with an Ultimate Strength of 270 KSI, and Shall Conform to ASTM A416.
- 3. Total Long Term Losses Due to Creep, Shrinkage, Elastic Shortening and Relaxation of Steel Determined per AASHTO LRFD, Bridge Design Specifications, Second Edition, 1998, Section 5.9.5.
- 4. Strands Shall be Released in Such a Manner as to Minimize Lateral Eccentricity.
- 5. Care Shall be Taken During Curing, Transportation and Erection to Avoid Any Lateral Deflection of the Girder.
- 6. Lifting Devices or Other Embedded Items Used to Lift and Transport the Girders Shall be Located as Close as Possible to the Girder Centerline of Bearing. The Girder Supplier Shall Submit Details Indicating the Adequate Type, and Location of Lifting Devices for Approval. Approval by the Engineer does not Relieve the Contractor of His Responsibilities if the Girder is Damaged Due to Failure of the Lifting Devices.
- 7. Reinforcing to Conform to ASTM A615 Grade 60.
- 8. Strand Pattern Shall be Symmetrical About the Longitudinal Centerline of the Girders.
- 9. The Contractor Shall Submit His Proposed Strand Pattern and Releasing Sequence to the Engineer for Approval.
- 10. The Contractor Shall Incorporate All Holes, Inserts, and Other Embedded Items Required in Girders During Fabrication of the Girders.
- 11. End of Girder Shall be Plumb After Erection.

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- 12. Top of Concrete Seat Shall be Within One—sixteenth of an Inch of the Theoretical Elevation and Slope Indicated.
- 13. For AASHTO Type II Girders, the Contractor Shall Measure and Record Slippage of Strands. Slippage is the Amount of Movement that a Point on the Strand at the End of the Girder Recedes into the Member after Detensioning. For Slippage Monitoring Purposes, Prior to Detensioning, Reference Marks Shall be Made on the Strands Not More than Two Inches from the Surface of the Member End. The Movement of the Reference Mark Shall be Measured to One—sixteenth of an Inch.
- 14. After Detensioning, the Strand Reference Marks Shall be Preserved to Permit Measurements of Strand Movement to the Time the Strands are Made Flush with the Member. The Contractor Shall Submit His Proposed Method of Strand Slippage Measurement with the Shop Drawings. The Engineer will Monitor the Contractor's Method of Measurement and the Slippage Values Obtained.
- 15. Measurement and Recording of Slippage of Strands Shall be Incidental to AASHTO Type II Girders.
- 16. Contractor is Responsible to Provide Adequate Lateral Bracing of Girders During all Phases of Construction Including Transporting, Placing, Erecting and Casting of Other Members.

GIRDER CAMBER NOTES:

- 1. The Calculated Camber Includes the Effect of the Initial Prestress Force and the Weight of the Girder After Removal from the Bed. Negative Values Shown for Calculated Camber Indicate a Net Upward Deflection. The Calculated Camber Value has been Multiplied by Applicable Factors Provided in PCI Handbook Table 4.6.2. to Approximate the Effect of Camber Growth and Concrete Creep. The Actual Camber Shall not Exceed the Calculated Camber by more than One—half inch.
- 2. The Dead Load Deflection Includes the Combined Effects of the Weight of Slab, Haunches, and Diaphragms as Applicable. See Table this Sheet.
- 3. Contractor Shall Camber the Deck Form work as Required to Account for the Calculated Dead Load Deflection in Order to Provide the Specified Finish Deck Elevations.
- 4. All Cambers and Deflections are in Inches.



FED. AID PROJ. NO.

ER-12(4)

#4 Stirrups, _I

for Spacing,

S-9 S-9

See Sect.

FISCAL YEAR

2003

SHEET NO.

59

SHEETS

FED. ROAD DIST. NO.

HAWAII

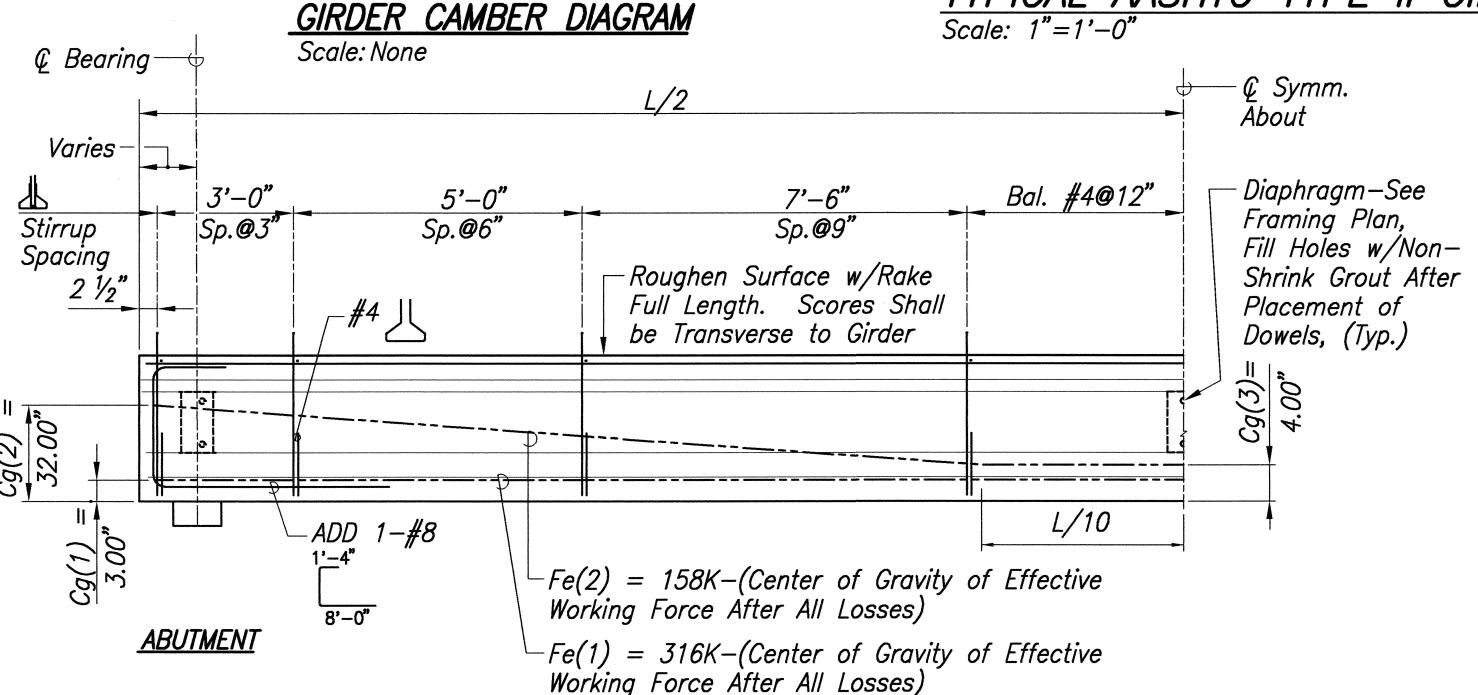
TYPICAL AASHTO TYPE II GIRDER SECTION B

Scale: 1"=1'-0"

S-9 S-9

3/4" Chamfer

Bend 8" (Typ.)



1. Prestressing Strands Not Shown for Clarity.

AASHTO TYPE II GIRDER LONGITUDINAL SECTION

Scale: None

S-9 STON G. OKUBO

LICENSED

PROFESSIONAL

ENGINEER

No. 4320-S

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Vynon Olinlo

April 30, 2004

Expiration Date of the License

DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

PRESTRESSED GIRDER DETAILS

REVISION

KAMEHAMEHA V HIGHWAY

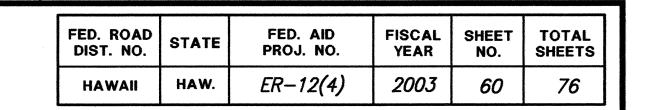
EMERGENCY REPLACEMENT OF KAWAIKAPU BRIDGE

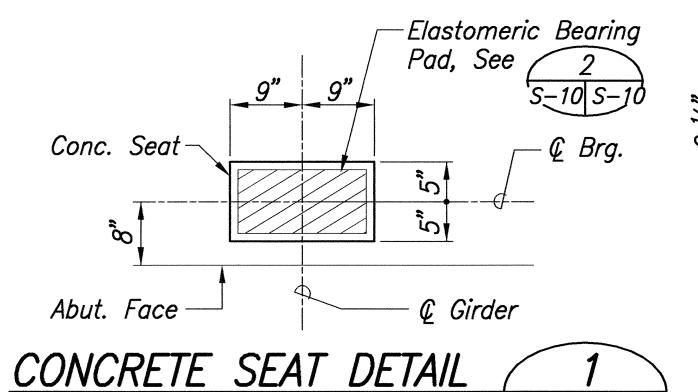
FEDERAL AID PROJECT NO. ER-12(4)

Scale: As Noted Date: Nov. 25, 2002

SHEET No. S-9 OF 16 SHEETS

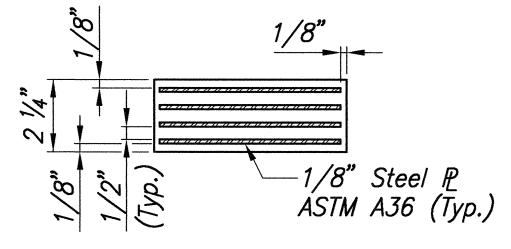
Girder Mark	Length & Brg. to & Brg.	Calculated Camber	Calculated Dead Load Deflection	Conc. Brg. Seat Slope	Remarks
G-1 Thru G-8	61.50'	-1.97"	1.25"	0" / 12"	

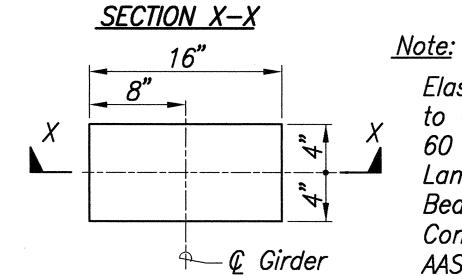




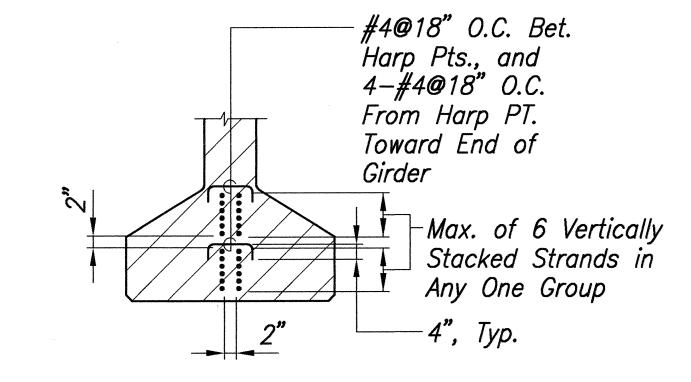
Scale: 1"=1'-0"

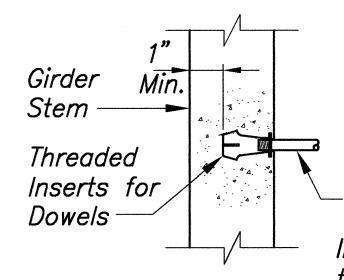
S-10 S-10





Elastomeric Bearing Pad to Consist of Neoprene,
60 Hardness with Steel
Laminations. Elastomeric
Bearing Pads Shall be
Considered Incidental to AASHTO Type II Girders.



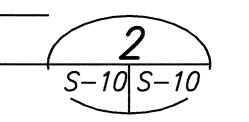


Capacity of Inserts Allow. Allow. Tension Shear Dowel Size 4.8 K 5.0 K

-Threaded Dowels

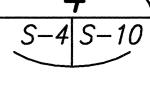
Inserts are Incidental to Reinforcing Steel in Girder.

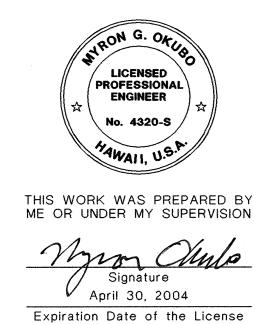










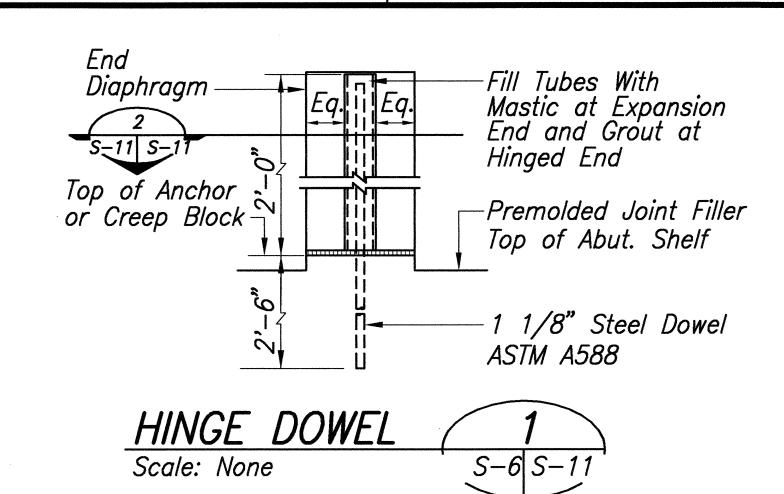


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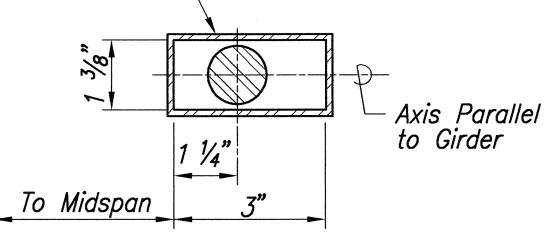
REVISION STATE OF HAWAN DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION SECTIONS AND DETAILS

KAMEHAMEHA V HIGHWAY
EMERGENCY REPLACEMENT OF KAWAIKAPU BRIDGE Scale: As Noted Date: Nov. 25, 2002

SHEET No. S-10 OF 16 SHEETS



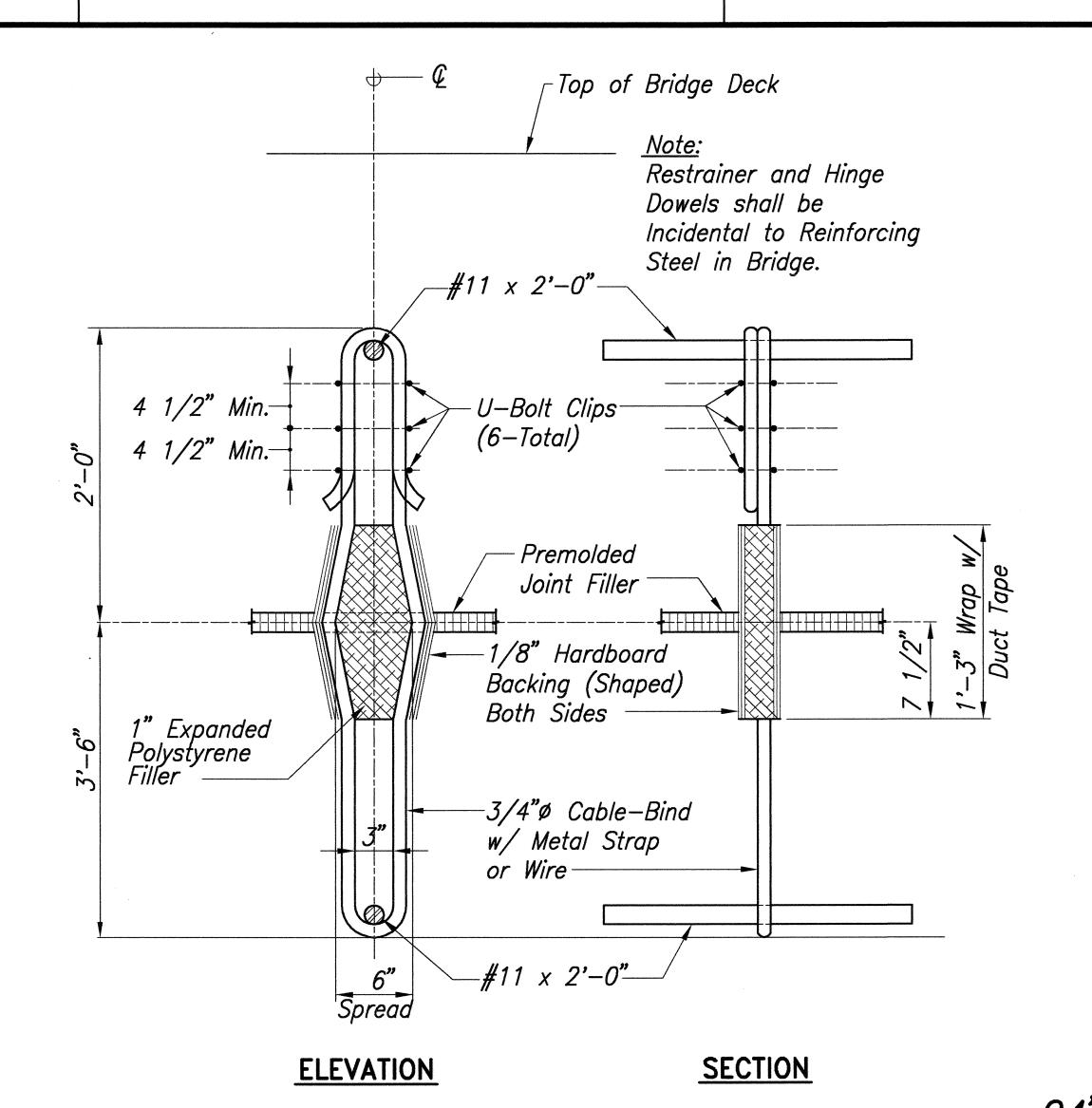
1/8" Galvanized Steel Tube (ASTM A36). Fabricated From Flat Bars Welded Together. Assembly Shall be Incidental Reinforcing Steel in Bridge. Tubes Shall be Capped by Welding a Steel Plate Following Placement of End Diaphragm And After Inspection And Placement of Mastic or Grout. Provide 1/2" Gap Between Top or Dowel And Bottom of Steel Cap Plate.—



TUBE ASSEMBLY SECTION

Scale: None

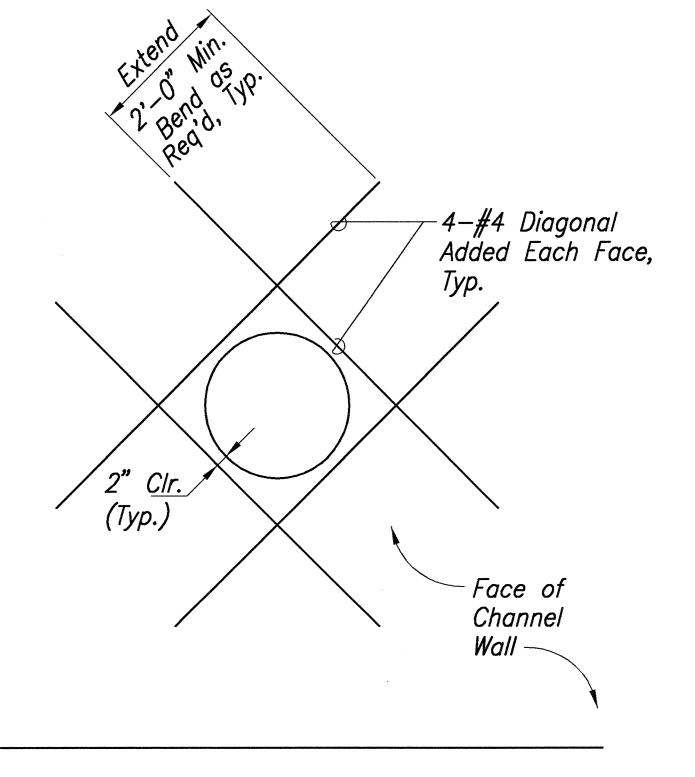
Scale: None



VERTICAL RESTRAINER DETAIL

Scale: None

FED. ROAD DIST. NO. FED. AID PROJ. NO. FISCAL YEAR SHEET TOTAL NO. SHEETS ER-12(4) 2003 61 HAW. HAWAII





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April 30, 2004 Expiration Date of the License DEPARTMENT OF TRANSPORTATION

MISCELLANEOUS SECTIONS AND DETAILS

REVISION

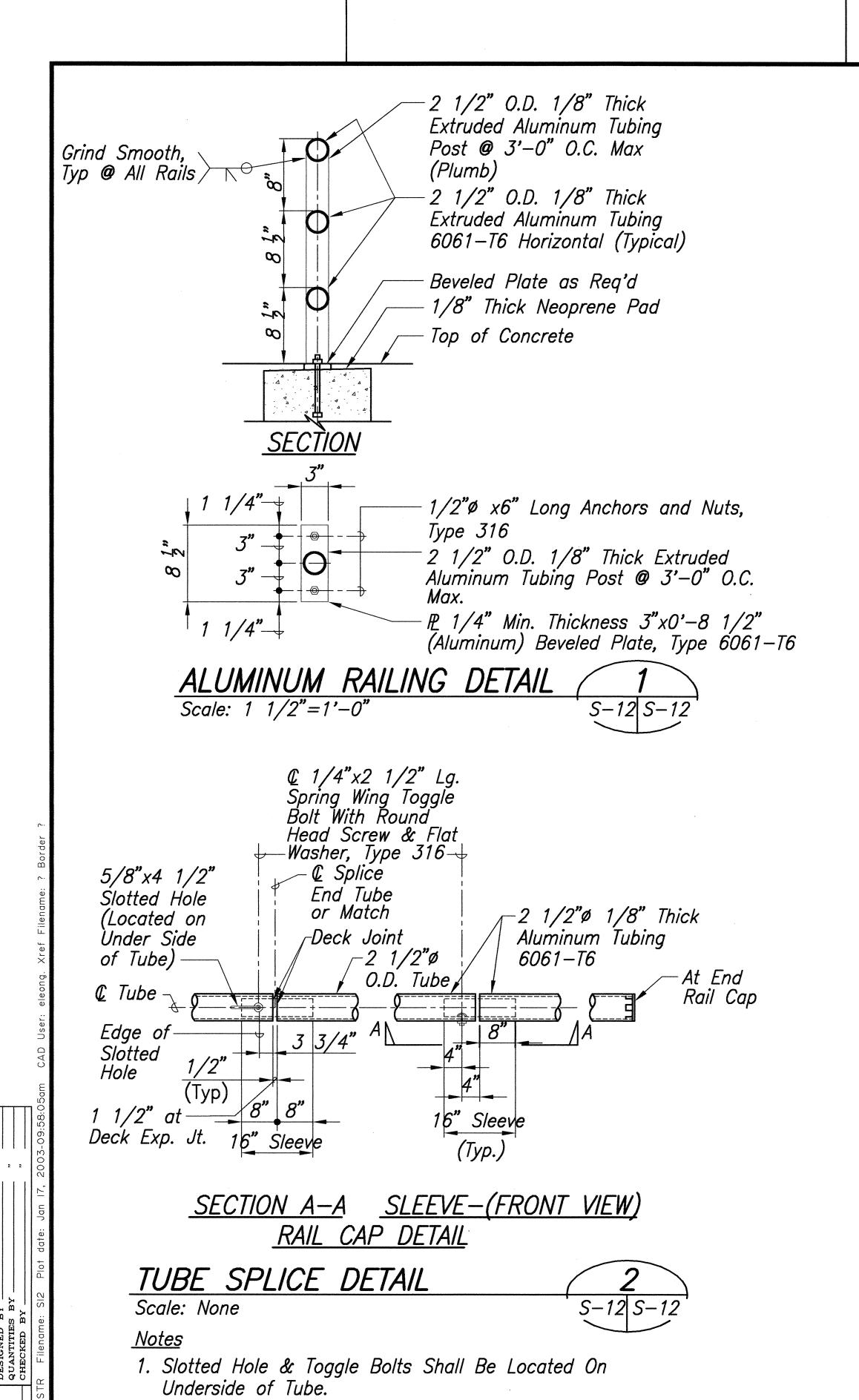
KAMEHAMEHA V HIGHWAY EMERGENCY REPLACEMENT OF KAWAIKAPU BRIDGE Date: Nov. 25, 2002 Scale: As Noted

SHEET No. S-11 OF 16 SHEETS

61

LICENSED PROFESSIONAL

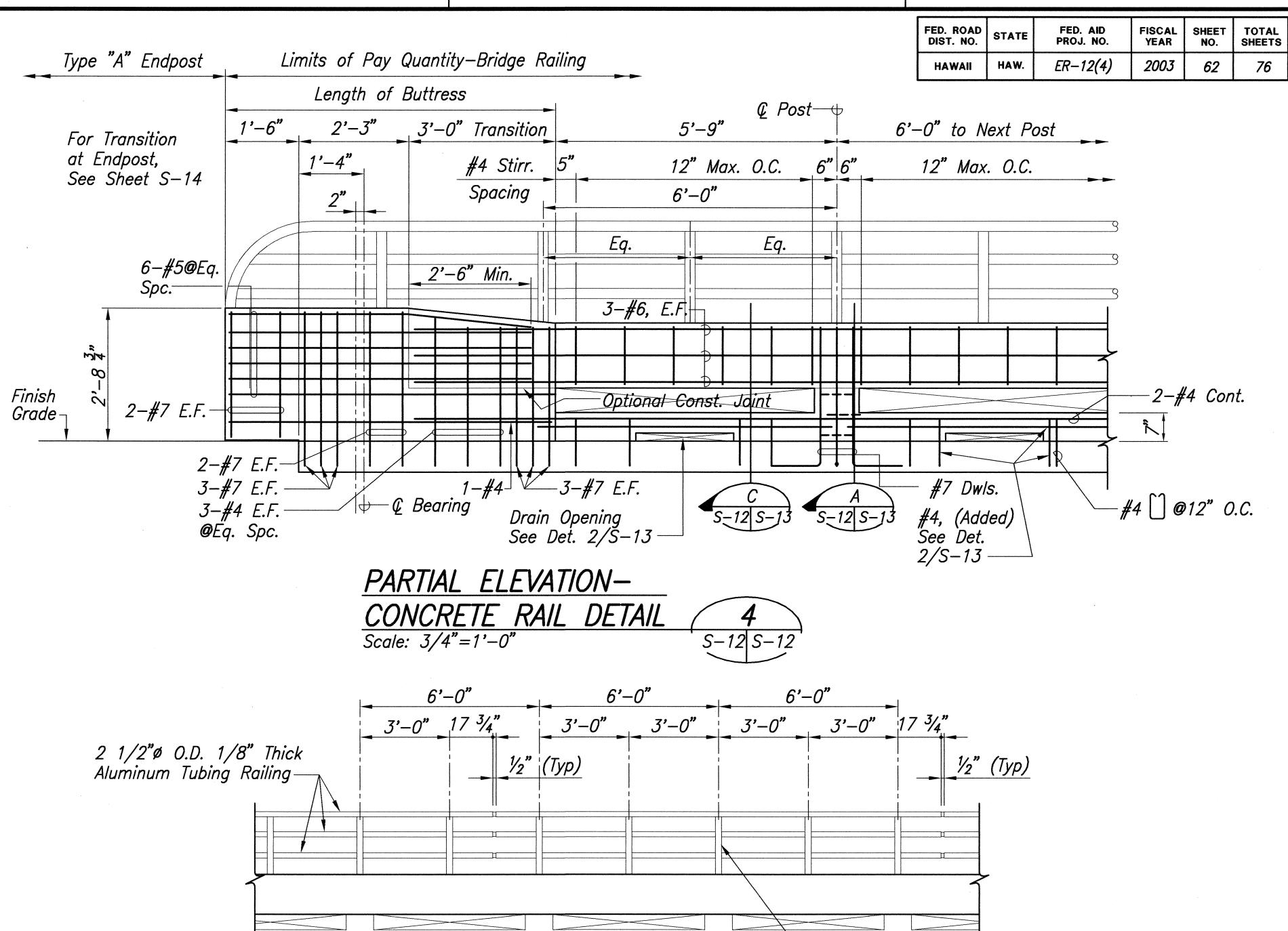
DATE



2. Aluminum Railing To Be Considered Incidental to Bridge Railing.

3. Bolts, Nuts, & Washers for Aluminum Railing to be Type 316

Stainless Steel.





S-12 S-12

LICENSED PROFESSIONAL

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-2 1/2"ø Aluminum Tubing Post (Typ) See Details 1&4/S-12

DATE

Signature
April 30, 2004 Expiration Date of the License

RAILING DETAILS KAMEHAMEHA V HIGHWAY
EMERGENCY REPLACEMENT OF KAWAIKAPU BRIDGE
FEDERAL AID PROJECT NO. ER-12(4) Scale: As Noted Date: Nov. 25, 2002

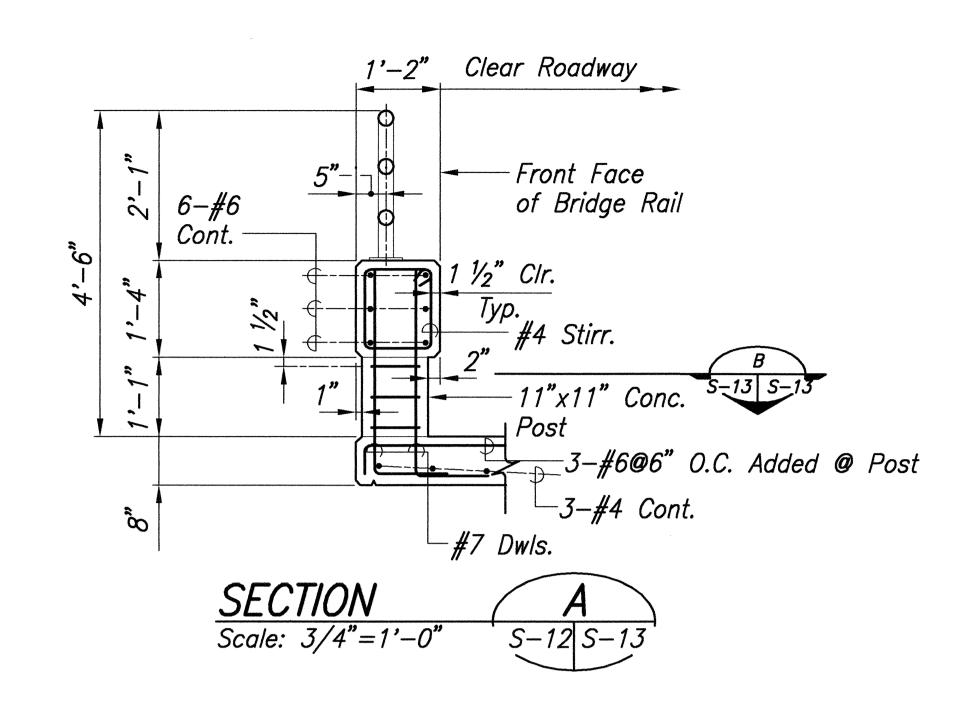
REVISION

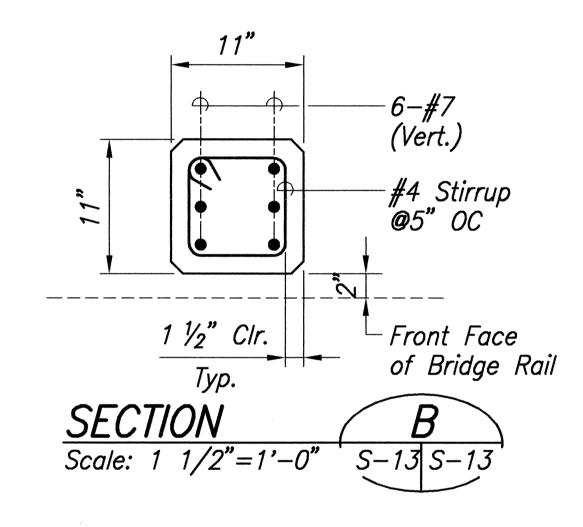
STATE OF HAWAII

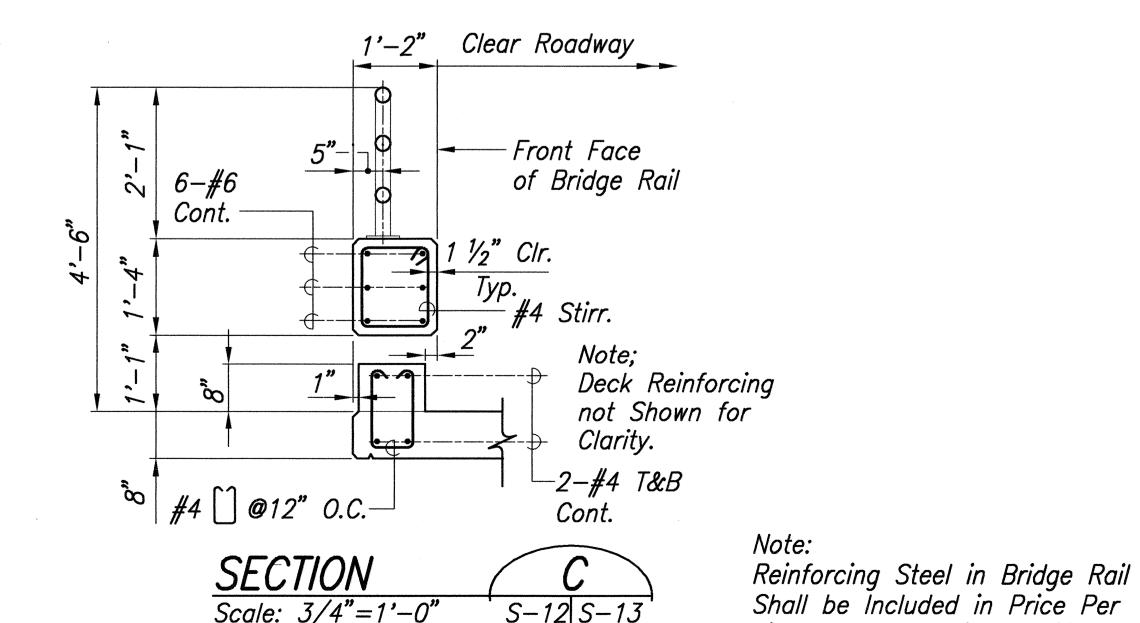
DEPARTMENT OF TRANSPORTATION

HIGHWAYS DIVISION

SHEET No. S-12 OF 16 SHEETS







S-12 S-13

Lineal Foot of Bridge Railing.

Scale: 3/4"=1'-0"

FED. ROAD DIST. NO. FED. AID PROJ. NO. FISCAL YEAR TOTAL SHEETS SHEET NO. 2003 ER-12(4) *63* HAW. HAWAII

Depressed "V" Letters 3/8" Deep —

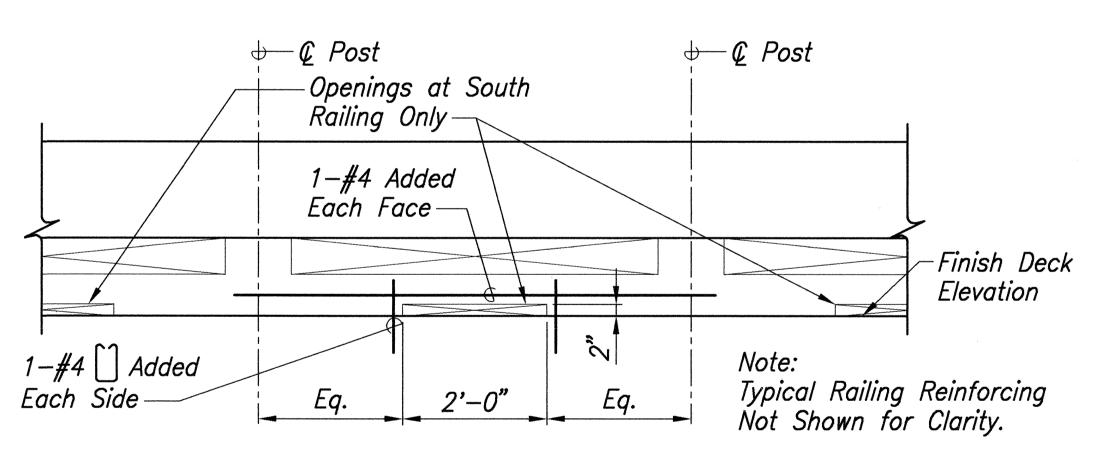
Use Correct Name of Bridge

Date of Year Built

<u>Note:</u> Refer to Corresponding Detail Drawings for Placement Such Names & Dates at End Post. Exact Details & Spacing of Letters & Figures Shall be as Directed by the Engineer. Gothic Letters and Figures Approximating Dimensions Shown will be Acceptable if Approved by the Engineer.

TYPICAL DETAIL OF LETTERS & FIGURES AT CONCRETE END POST Scale: None

S-2 S-13



TYPICAL ELEVATION OF SOUTH RAILING Scale: 3/4"=1'-0" S-13 S-13



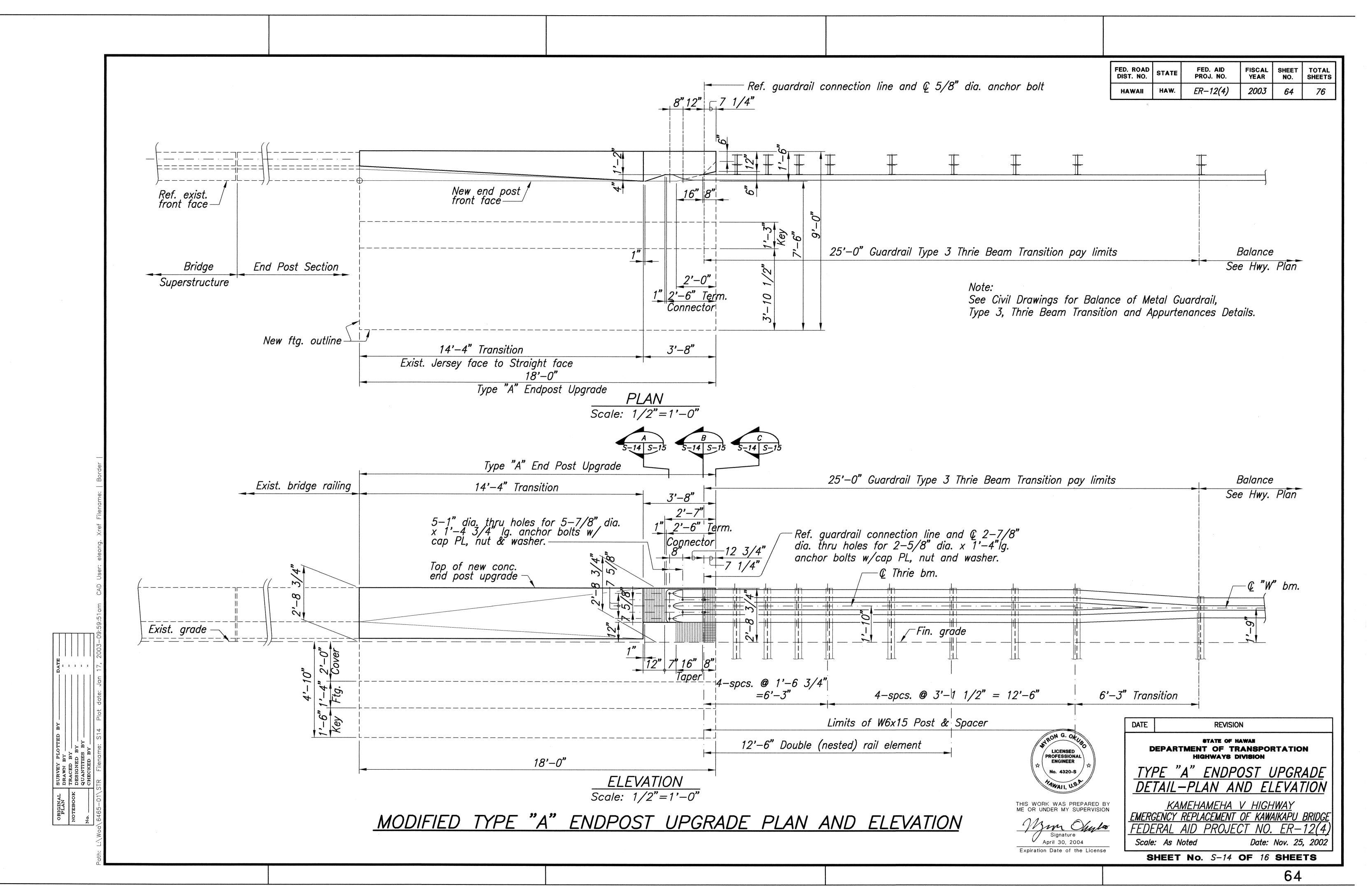
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION

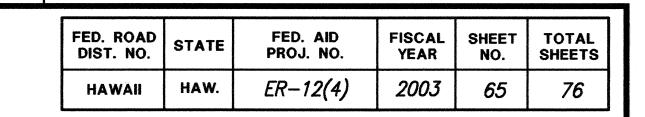
Signature
April 30, 2004 Expiration Date of the License

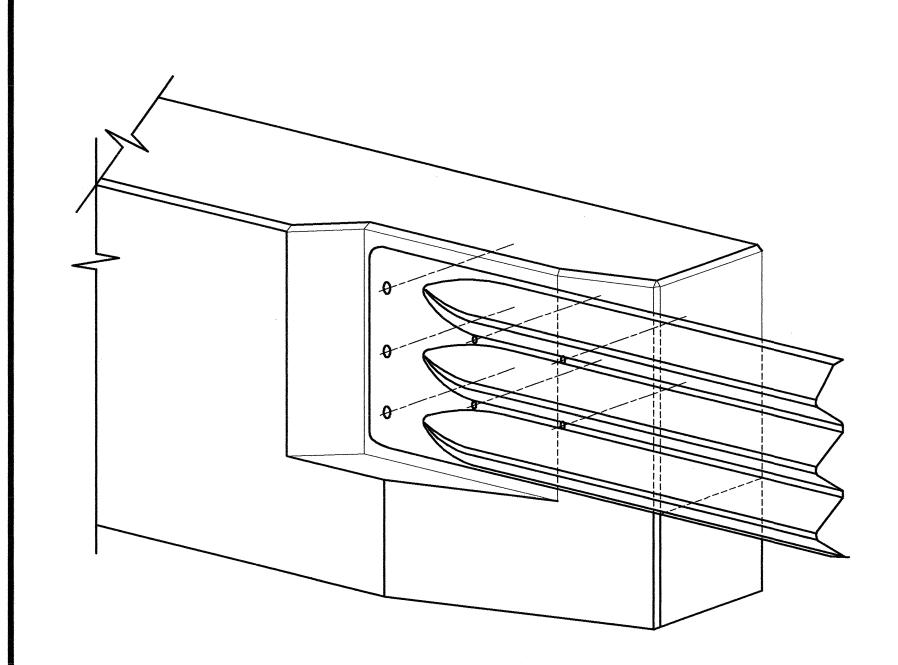
REVISION STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION RAILING SECTIONS, DETAILS AND MISC. DETAILS

KAMEHAMEHA V HIGHWAY
EMERGENCY REPLACEMENT OF KAWAIKAPU BRIDGE
FEDERAL AID PROJECT NO. ER-12(4) Scale: As Noted Date: Nov. 25, 2002

SHEET No. S-13 OF 16 SHEETS



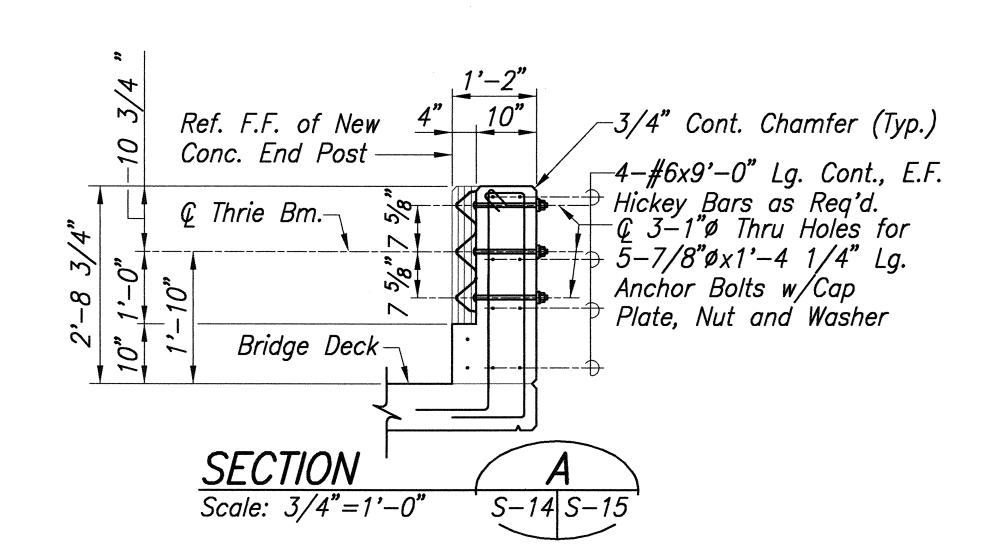


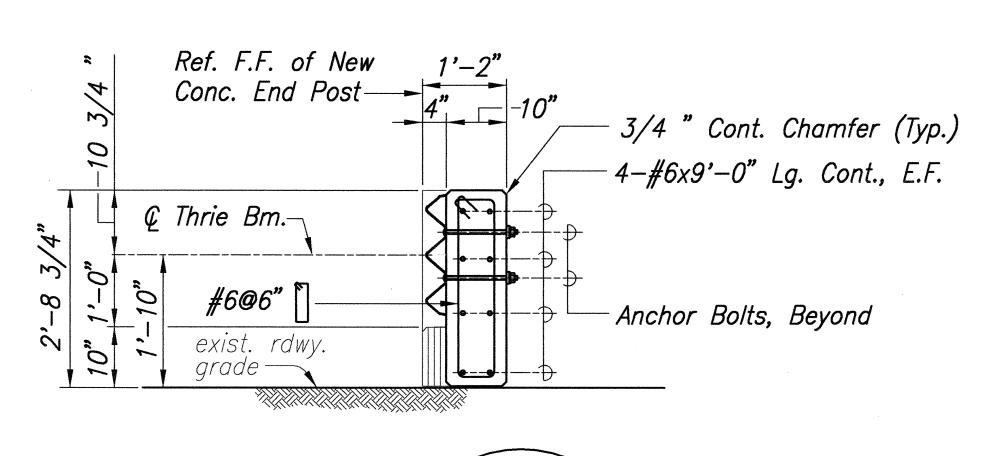


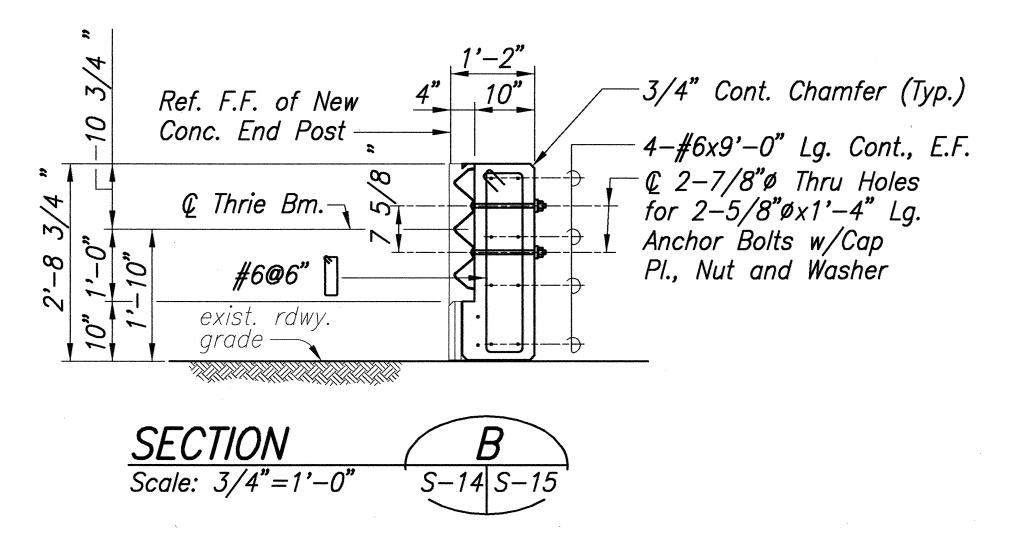
PARTIAL ISOMETRIC VIEW -

TYPE "A" ENDPOST UPGRADE

Not to Scale











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April 30, 2004 Expiration Date of the License

REVISION STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

END POST DETAILS

KAMEHAMEHA V HIGHWAY

EMERGENCY REPLACEMENT OF KAWAIKAPU BRIDGE
FEDERAL AID PROJECT NO. ER-12(4) Date: Nov. 25, 2002 Scale: As Noted

SHEET No. S-15 OF 16 SHEETS

