	INDEX TO STRUCTURAL PLANS			
SHEET NO.	DESCRIPTION			
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S-2	S-2 STRUCTURAL NOTES			
S-3	CONSTRUCTION NOTES & QUANTITIES			
S-4	DEMOLITION PLAN AND SECTIONS			
S-5	GENERAL LAYOUT AND LONGITUDINAL ELEVATION			
S-6	TRANSVERSE DECK SECTION			
S-7	TOP SLAB REINFORCING DECK PLAN			
S-8	LONGITUDINAL DECK SECTION, END BEAM AND JOINT DETAIL			
S-9	PRESTRESSED PLANK PLAN AND LONGITUDINAL SECTION			
S-10	PRESTRESSED PLANK TRANSVERSE SECTIONS AND DETAILS			
S-11	APPROACH SLAB AND DETAILS			
S-12	BRIDGE GUARDRAIL AND DOWEL DETAIL			
S-13	BIKE RAILING SECTIONS AND DETAILS			

STRUCTURAL AND CONSTRUCTION NOTES:

CONSTRUCTION SPECIFICATIONS:

State of Hawaii, Department of Transportation Standard Specifications for Road, Bridge, and Public Works Construction, 1994, and Special Provisions prepared for this Project

DESIGN SPECIFICATIONS:

AASHTO LRFD Bridge Design Specifications, 2nd Edition, 1998, including subsequent interim specifications.

DESIGN LOADS:

SURVEY PLOTTED BY
DRAWN BY A. Coodin
TRACED BY
DESIGNED BY C. Miyasato
QUANTITIES BY
CHECKED BY

ORIGINAL PLAN NOTE BOOK Description of the No.

1. Live Load: HL-93

STRUCTURAL NOTES:

- 1. Record drawings from Project No. 56A-01-91 and original plans from Lihue Plantation dated November 1920 (redrawn October 1993) on file with HWY-K have been used to prepare the plans. Actual conditions could vary from those indicated on the plans. The Contractor shall verify all existing site conditions, elevations, dimensions, structure configurations, member sizes, and connections prior to commencing the work.
- 2. The Contractor shall visit the site to determine and observe actual field conditions that could affect his operations prior to submitting a bid.
- 3. All vertical dimensions are measured plumb, and all horizontal dimensions are measured flat except where otherwise noted.
- 4. In the event certain features of the construction are not fully shown on the drawing, or called for in the notes or specifications, their construction shall be of the same character as similar conditions that are shown or called out. The construction shall be reviewed and approved by the Engineer.
- 5. Refer to Standard Plans for additional details and notes not covered by details and typical drawings.

CONCRETE:

					FED. ROAD DIST. NO.	STATE	
shall	have	the	following	class	HAWAII	HAW.	
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1. The concrete for the following elements sh designations corresponding to 28-day compressive strength (f'c). a. Prestressed Planks ——————— f'c = 6000 psi

b. Deck Topping — f'c = 4000 psi c. Approach Slab — f'c = 4000 psi

- 2. The concrete for the deck topping shall achieve a minimum flexural strength of 500 psi and a minimum compressive strength of 3000 psi in 24 hours and prior to opening the bridge to traffic.
- 3. The concrete for the approach slab shall achieve a minimum flexural strength of 650 psi prior to opening the bridge to traffic.

4. The minimum cover measured from the surface of the concrete to the face of any reinforcing bar shall be as follows, except as noted otherwise:

a. Deck Slab Top Bars -

c. Concrete cast against and permanently exposed to earth —

5. Unless noted otherwise, chamfer all exposed concrete edges $\frac{3}{4}$ inch.

- 6. Repair concrete and nonshrink grout shall have a minimum compressive strength of 5,000 psi at 28 days, and shall be nonmetallic and nonstaining.
- 7. For concrete finishes, see Standard Specifications.

REINFORCING STEEL:

- 1. All reinforcing steel shall conform to the requirements of ASTM A615M and shall be deformed, Grade 60 unless noted otherwise.
- 2. The reinforcing steel shall be clad with Type 316L austenitic stainless steel meeting ASTM A276. The minimum thickness of the cladding shall be 0.007 inches (0.175mm).
- 3. The cladding manufacturer shall seal all ends of bars with a Type 316L weld during material fabrication. When bars are cut in the field, the cut end shall be sealed with a Type 316L weld, following manufacturers specifications, or sealed with a high-performance elastomeric sealant filled stainless steel cap. Any breach in the bar cladding shall be sealed with a Type 316L weld, following manufacturer's specifications.
- 4. All reinforcing steel shall be cold bent with no wrinkling or loss of adhesion of the cladding on the inside radius of the bent portion, and with no cracking of the cladding on the outside radius on the bent portion.
- 5. All reinforcing steel, anchor bolts, dowels and other embedded items shall be securely tied in place with Type 316L stainless steel tie wire prior to pouring of concrete. All chairs and lifts shall be plastic or Type 316L stainless steel.
- 6. Splice length for reinforcing steel shall be 52 bar diameters but shall be a minimum 2'-4" unless otherwise noted.
- 7. All dimensions relating to reinforcing bars are to centers of bars unless noted otherwise.
- 8. All stainless steel clad reinforcing shall be isolated from direct contact with any adjacent non-stainless steel with 1/8" thick neoprene tubing. The neoprene tubing shall be sized appropriately and slit longitudinally to slip over the stainless steel clad reinforcing.

9. Prestressing strands shall be uncoated, $\frac{1}{2}$ diameter 7-wire, low relaxation strands, grade 270 conforming to AASHTO M203.



INDEX AND STRUCTURAL NOTES

KUHIO HIGHWAY Wailua River Plantation Bridge Replace and Reconstruct Pavement and Structural Supports Project No. 56A-02-00M

Scale: As Noted

Date: May 2001 SHEET No. S-1 OF 13 SHEETS

FISCAL

2002

PROJ. NO.

56A-02-00M

SHEET TOTAL

LICENSED PROFESSIONAL

ENGINEER

STRUCTURAL STEEL:

- 1. All structural steel shapes and plates, including bike railing and bridge railing posts, and blocks (with the exception of thrie beam element) shall be Type 316 stainless steel with No. 4 finish meeting ASTM A276 and A479. All bolts, including anchor bolts, nuts, and washers shall be Type 316 stainless steel.
- 2. Thrie beam element shall conform to AASHTO M180 and shall be double-dipped galvanized after fabrication. Galvanized coating shall meet ASTM A53.
- 3. Bolt faying surfaces shall be free from debris, dirt, paint, mill scale, and other material prior to assembly.
- 4. The Contractor shall furnish shop placement and detailed drawings for all structural steel work.

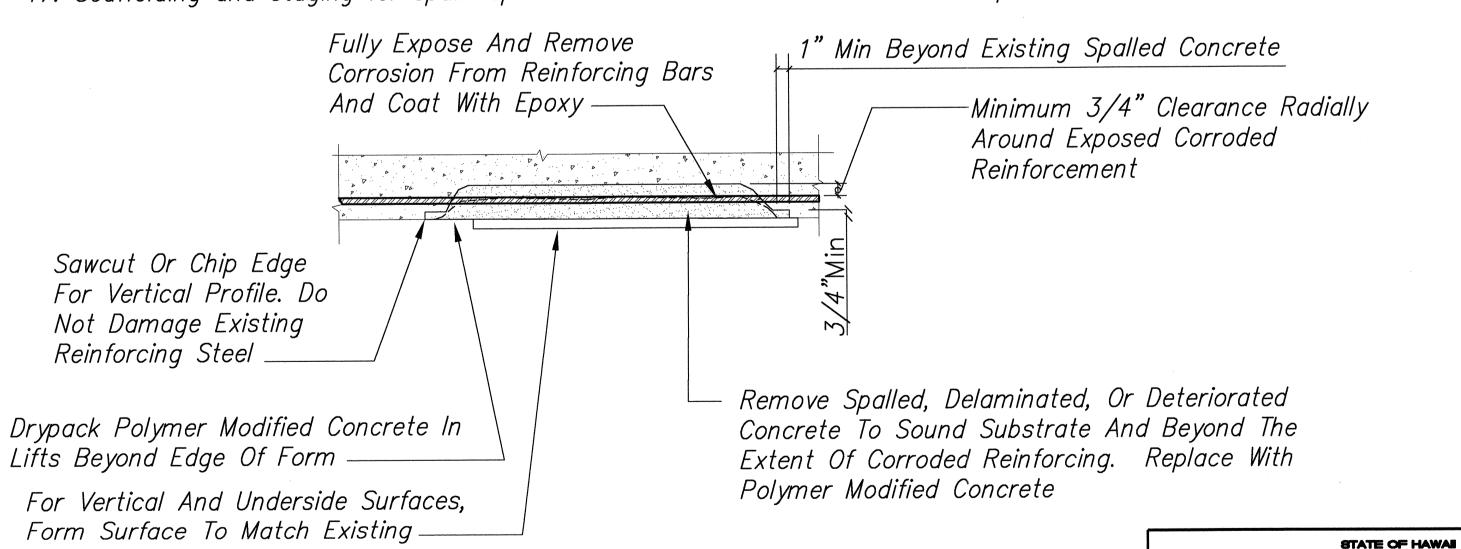
CEMENT RUBBLE MASONRY (CRM):

- 1. Stones shall be clean, hard, sound, and durable. Each stone, except stones for filling or pinning interstices, shall have a thickness of not less than 6 inches and a width of not less than one and one half times the thickness or 12 inches, whichever is greater.
- 2. Mortar shall be type "M" conforming to ASTM C270 and have a minimum compressive strength of 2,500 psi at 28 days.
- 3. Select stones shall be roughly squared and pitched to lines at angles and ends of walls.
- 4. Stones shall be wet thoroughly before laying.
- 5. Stone joints shall overlap at least 6 inches and form a firm bond.
- 6. Horizontal joints in the face of the wall shall not exceed 1 inch in thickness and vertical joints shall not exceed 2 inches in width.

SPALL REPAIR PROCEDURE:

- 1. The Contractor shall sound all exposed concrete to identify spalls and delaminations.
- 2. The spalled and delaminated concrete shall be completely removed to sound substrate and beyond the extent of the corroded reinforcing. The contractor shall take the necessary precautions to avoid damaging the underlying sound concrete.
- 3. The spalled and delaminated edges shall be squared by saw-cutting or chipping to a vertical profile the concrete at the perimeter beyond the removal area to a minimum depth of 3/4 in. Exercise great care to avoid cutting or damaging any existing embedded steel reinforcing in the slab. At areas where concrete cover is less than 3/4 in. over the steel reinforcing, the perimeter shall be carefully saw cut or chipped vertically to a safe depth away from the reinforcing, and the remaining depth chipped away to square the edge. Angles between adjacent saw-cuts around the perimeter shall not be less than 90 degrees and the shape of each patch shall not be irregular.
- 4. Additional concrete shall be removed to the depth of the saw-cut.
- 5. For any exposed reinforcement within the repair area, additional concrete shall be removed for a minimum $\frac{3}{4}$ in. clear space measured radially around the bars.
- 6. All exposed concrete surfaces and reinforcing bars in the repair area shall be sandblasted or needle gunned to remove all scale, loose rust, debris and deteriorated concrete. Any areas not patched more than 48 hours after sandblasting shall be resandblasted.
- 7. Any reinforcement to receive less than 3/4 in. of new concrete cover or which has lost more than 20 percent of its cross-sectional area shall be brought to the attention of the Engineer.

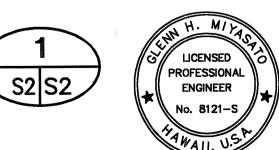
- 8. The patch area shall be cleaned of all dust and debris just prior to patching with high pressure, oil-free compressed air at a minimum pressure of 100 psi.
- 9. Epoxy coating for Spall repairs shall be Sikadur 32, Hi-Mod, manufactured by the Sika Chemical Corporation. All exposed steel shall be liberally coated with two coats of epoxy for a minimum dry mil thickness of 10 mils. The coating shall be complete with no skips, pin holes or holidays around the entire surface of the exposed steel. Exercise care to avoid application of the epoxy to the surrounding concrete substrate. Epoxy spillage on adjacent concrete surfaces within the patch area shall be removed completely. The epoxy shall be allowed to set prior to the placement of the repair concrete.
- 10. Vertical and overhead surfaces shall be formed. Void spaces beyond the edge of the form shall be dry packed in lifts with patch material.
- 11. Prior to placement of patch material, the exposed concrete surface shall be saturated with no water accumulation on the surface.
- 12. Cementitious repair mortar for spall repair shall be SikaTop 122 and SikaTop 123 for horizontal and vertical/overhead surfaces, respectively. A slurry coat of the polymer modified cementitous repair mortar having the consistency of a thick cream shall be used to scrub into and prime the saturated surface dry substrate. While the slurry coat is still wet, apply the polymer modified cementitious repair material to fill the voids.
- 13. For patch depths in excess of 1 in., 3/8 in. aggregates shall be added to the polymer modified cementitious repair mortar used.
- 14. The repair concrete shall be vibrated, rodded or tamped during placement to consolidate the pour and fill all corners of the patch or form and beneath the reinforcing.
- 15. The surface finish shall be textured or ground to match adjacent conditions.
- 16. The repair concrete shall be cured for seven days by covering the surface with a polyethylene sheet over wet burlap.
- 17. Scaffolding and staging for spall repairs shall be considered incidental to repair cost.



SPALL REPAIR DETAIL

Scale: NTS

Note: For Spall Repair Detail at Connection *Plate, See Sheet S−3.*

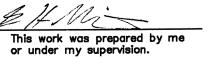


STRUCTURAL NOTES KUHIO HIGHWAY

DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

Wailua River Plantation Bridge Replace and Reconstruct Pavement and Structural Supports

Project No. 56A-02-00M Scale: As Noted Date: May 2001



SHEET No. S-2 OF 13 SHEETS

FISCAL SHEET TOTAL YEAR NO. SHEETS

FED. ROAD DIST. NO.

HAWAII

STATE

HAW.

PROJ. NO.

56A-02-00M

DEMOLITION AND CONSTRUCTION NOTES:

- 1. The Engineer will permit the closure of the bridge to traffic only on Monday, Tuesday, Wednesday, and Indicate the following day, and on weekends between the hours of 10:00 P.M. Friday and 5:00 A.M. Monday. Monday through Friday, Contractor's work shall not interfere with contra-flow operations. Contractor will be responsible for paying all cost for overtime charges incurred by State Highway personnel, including the State's Construction Manager overtime charges.
- 2. The Contractor shall provide permanent guardrails and smooth transitions between existing and new portions of the bridge deck to allow for safe and normal traffic flow during periods when the bridge is open to traffic.
- 3. The Contractor shall verify the locations of all existing utility lines near the bridge and notify their respective owners before commencing the work.
- 4. The Contractor is solely responsible for the safety of workers and general public users of the bridge and waterway below at all times during demolition and construction.
- 5. The Contractor is solely responsible for the protection of new and existing structures, adjacent property and utilities from damage due to demolition and construction, and shall repair any damage at his own expense, to the satisfaction of the Engineer.
- 6. The methods of demolition and construction, adequacy of falsework and bracing and other temporary work items used in the execution of the work are also solely the Contractor's responsibility. Such measures shall include but are not limited to, shoring and support of existing bridge during construction, shoring for excavation, bracing for forms and scaffolding, and shoring for demolition and construction equipment. The Contractor shall submit shop drawings for erection of falsework, cofferdams and/or other temporary construction to the Engineer for review and approval prior to commencing the work.
- 7. The Contractor shall prevent debris from falling into Wailua River. Details for the Debris Control Scheme shall be submitted to the Engineer for review and approval prior to commencing the work.
- 8. The Contractor shall submit actual demolition/construction sequence with detailed descriptions of each work task to the Engineer for review and approval prior to commencing the work.

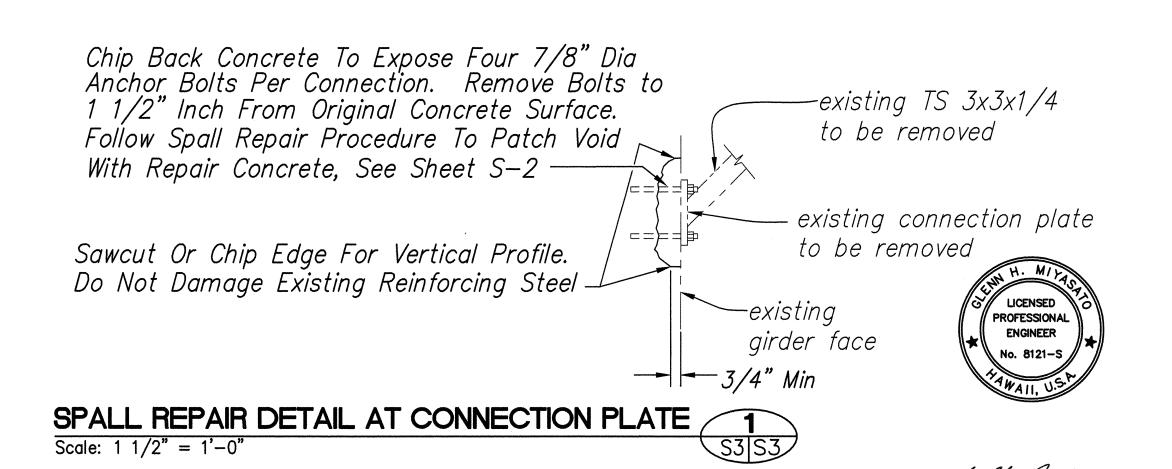
ANTICIPATED CONSTRUCTION SEQUENCE:

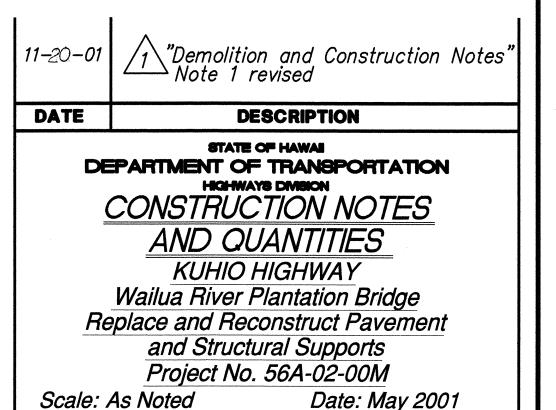
- 1. The anticipated construction sequence listed below is general in nature and shall be modified by the Contractor as necessary and supplemented with detailed descriptions of each work task.
- 2. Submit Traffic Control Plan, Temporary Construction Shop Drawings, Debris Control Scheme, and actual Demolition/Construction Sequence to the Engineer for review and approval.
- 3. Install traffic control and debris control measures.
- 4. Repair unsound concrete on bridge structure except at top surfaces of girders.
- 5. Remove metal guardrails, asphalt topping and metal railroad track decking, connection plates, angles and supports.
- 6. Repair unsound concrete at top surfaces of girders.
- 7. Drill holes at top surfaces of girders for precast plank dowels. Care shall be taken to avoid damaging existing girder reinforcing.
- 8. Install precast planks on grout bed.
- 9. Install dowels and reinforcing steel for concrete topping.

- FED. ROAD DIST. NO. STATE PROJ. NO. FISCAL YEAR NO. SHEETS

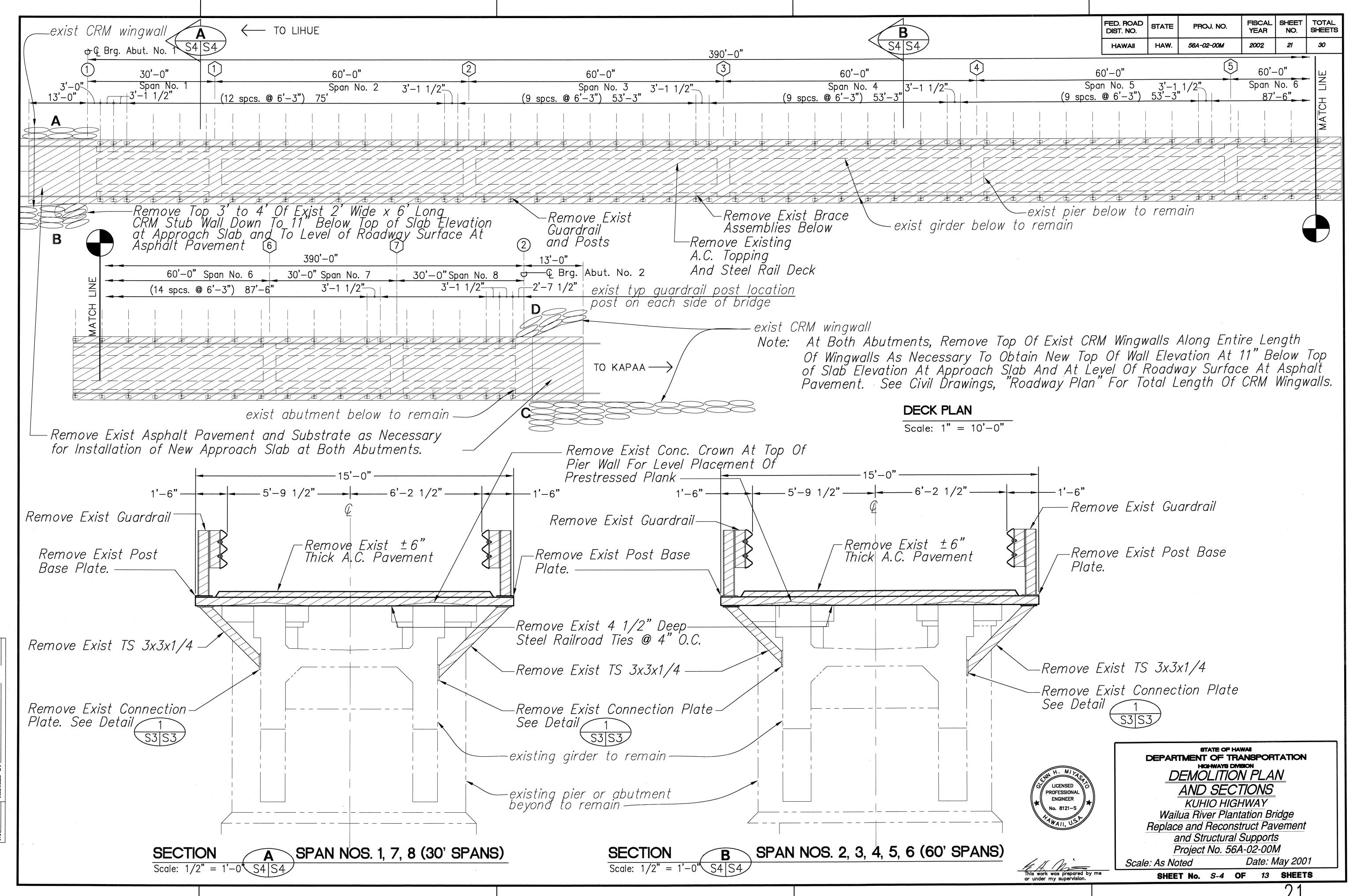
 HAWAII HAW. 56A-02-00M 2001 ADD. 20 30
- 10. Install concrete topping on precast planks. Prior to opening the bridge to traffic, allow concrete topping to attain minimum compressive strength and degree of hardness necessary so that the surface is not damaged by vehicular traffic.
- 11. Provide temporary guardrails and smooth transitions between existing and new portions of the bridge deck to allow for safe and normal traffic flow during periods when the bridge is open to traffic. No section of incomplete guardrail footing and/or excavation shall be left unshelled at the end of each work day.
- 12. Install approach slabs.
- 13. Install thrie beam guardrails.

ESTIMATED QUANTITIES					
ITEM NO.	ITEM	UNIT	TOTAL		
202.0410	Removal of Bridge Deck Including A.C. Pavement	Sq Ft	6000		
202.0421	Removal of Bridge Guardrails and Steel Posts	Lin Ft	780		
411.1117	12—inch Concrete Pavement for Approach Slab	Cu Yd	15		
418.1000	4-inch Ultra Thin White Topping	Cu Yd	85		
503.1090	Concrete Spall Repair	Sq Ft	1,000		
504.7410	Prestressed Concrete Plank	Sq Ft	5,586		
507.0100	Stainless Steel Bike Railing With Post	Lin Ft	937.5		
602.0091	Reinforcing Steel for Bridge (Incidental)	Lb	18,000		
602.0900	Reinforcing Steel for Approach Slab (Incidental)	Lb	3,000		
606.3111	Special Design Thrie Beam Guardrail With Strong Post	Lin Ft	962.5		
655.1000	Drilling Holes and Installing Dowel Reinforcing Bars	Ea	404		

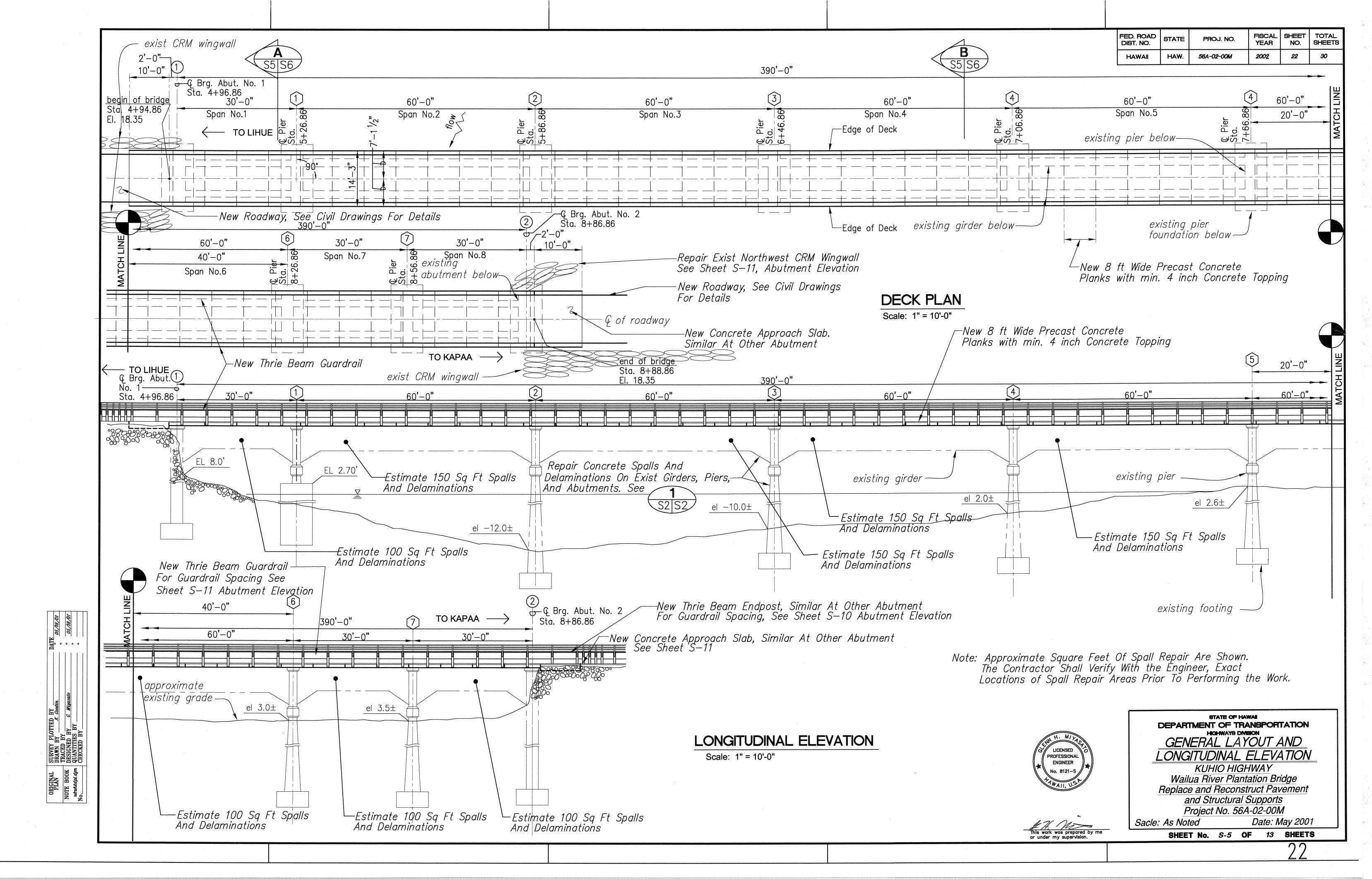




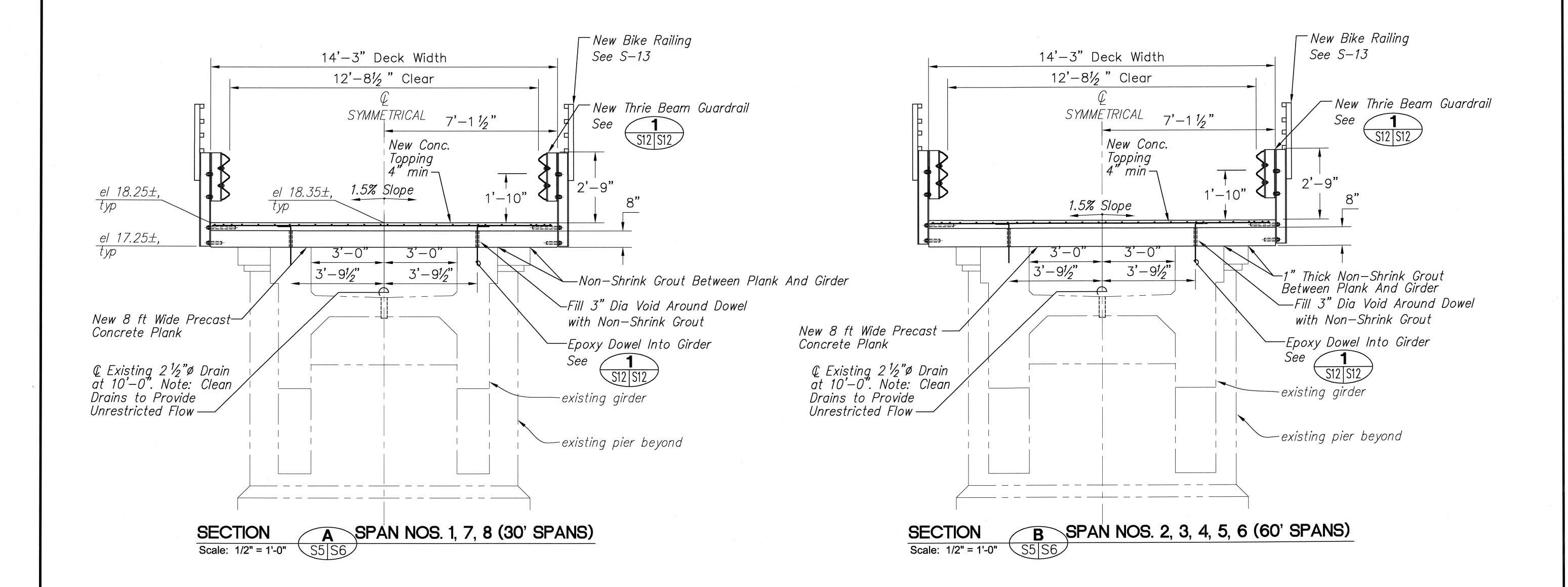
SHEET No. S-3 OF 13 SHEETS

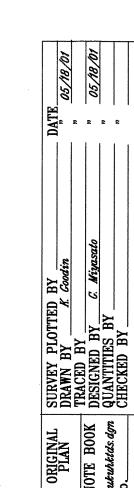


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FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	56A-02-00M	2002	23	30







STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DMISION

TRANSVERSE DECK SECTION

KUHIO HIGHWAY
Wailua River Plantation Bridge
Replace and Reconstruct Pavement
and Structural Supports
Project No. 56A-02-00M

Scale: As Noted

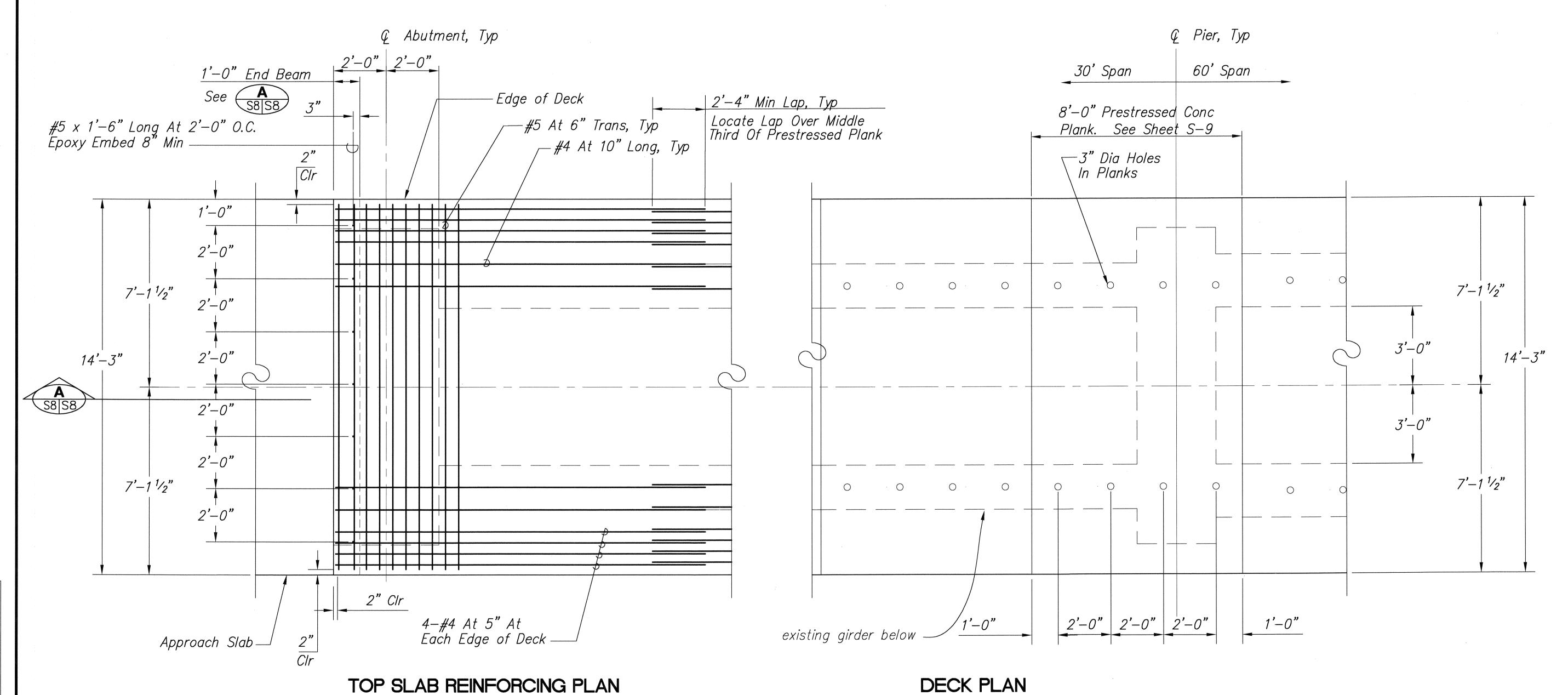
As Noted Date: May 2001

SHEET No. S-6 OF 13 SHEETS

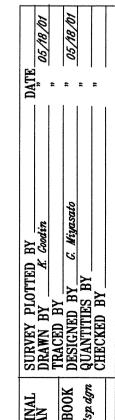
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FED. ROAD DIST. NO. STATE PROJ. NO. FISCAL YEAR NO. SHEET NO. SHEETS

HAWAII HAW. 56A-02-00M 2002 24 30



Scale: 1/2" = 1'-0"



Scale: 1/2" = 1'-0"



DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION
TOP SLAB REINFORCING

DECK PLAN

KUHIO HIGHWAY

DECK PLAN

KUHIO HIGHWAY

Wailua River Plantation Bridge
Replace and Reconstruct Pavement

and Structural Supports

Project No. 56A-02-00M

ale: As Noted

Date: May 2001

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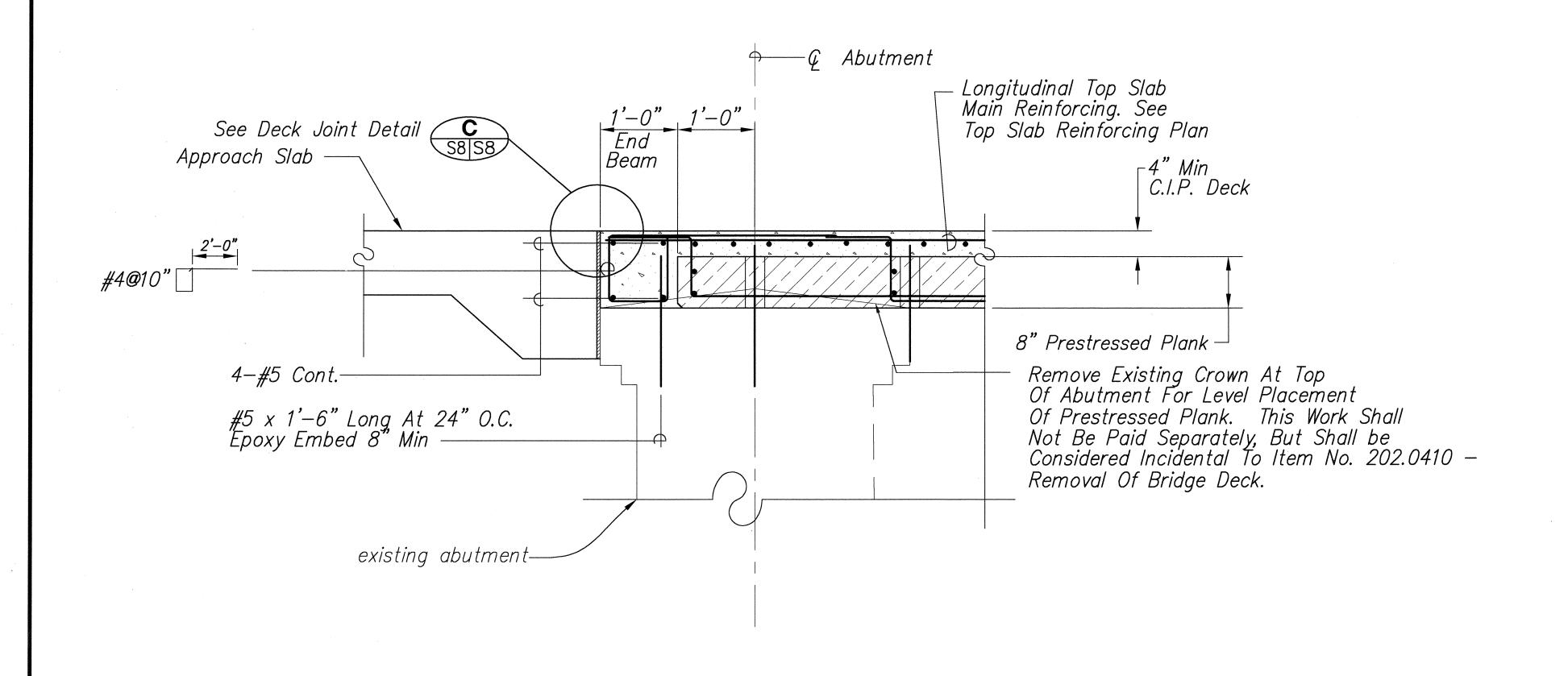
Scale: As Noted

Date: May 2001

SHEET No. S-7 OF 13 SHEETS

FED. ROAD DIST. NO. STATE PROJ. NO. FISCAL YEAR NO. SHEETS

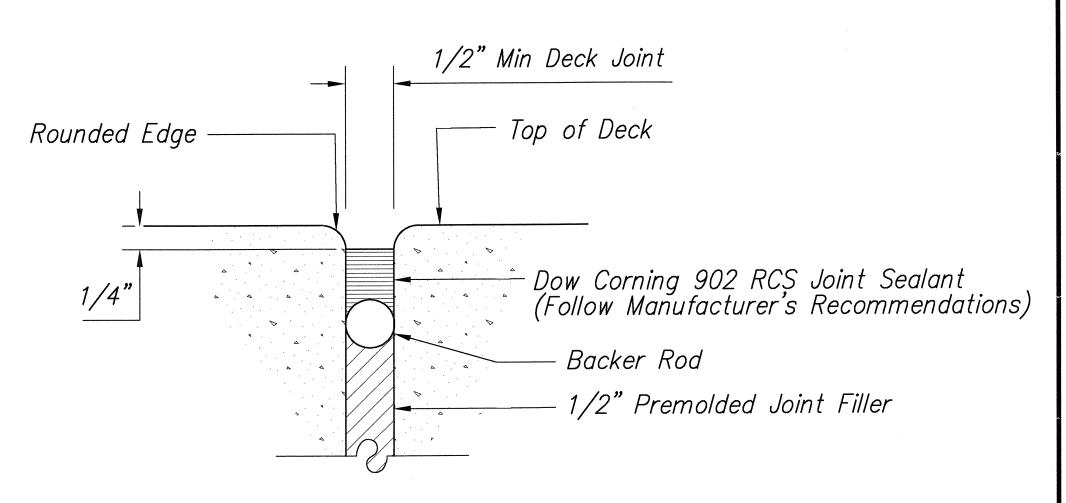
HAWAII HAW. 56A-02-00M 2002 25 30



END BEAM SECTION

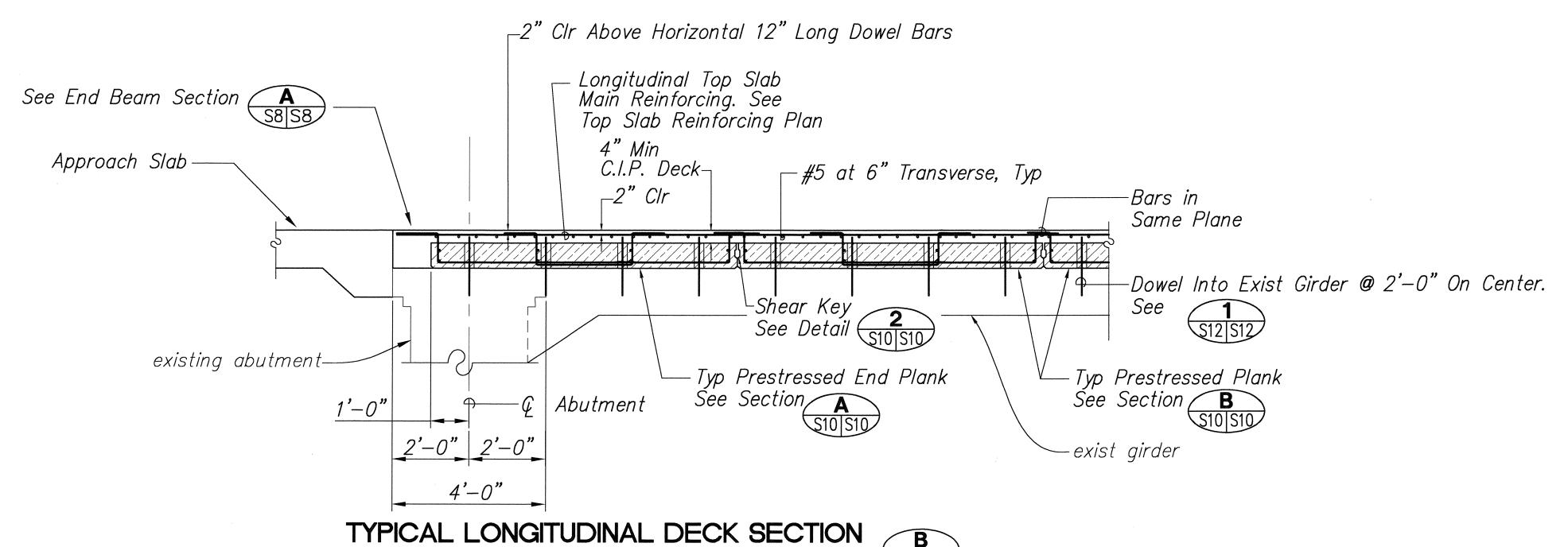
Scale: 1" = 1'-0"

Scale: 1/2" = 1'-0"



DECK JOINT DETAIL C
Scale: NTS

Note: New Deck Joints (2 each) And All Associated Labor And Materials Shall Not Be Paid For Separately, But Shall Be Considered Incidental To Item No. 411.1117 — 12—Inch Concrete Pavement For Approach Slab



A \$8|\$8



DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

LONGITUDINAL DECK SECTION

END DEAM AND LONT DETAIL

END BEAM AND JOINT DETAIL

KUHIO HIGHWAY

Wailua River Plantation Bridge

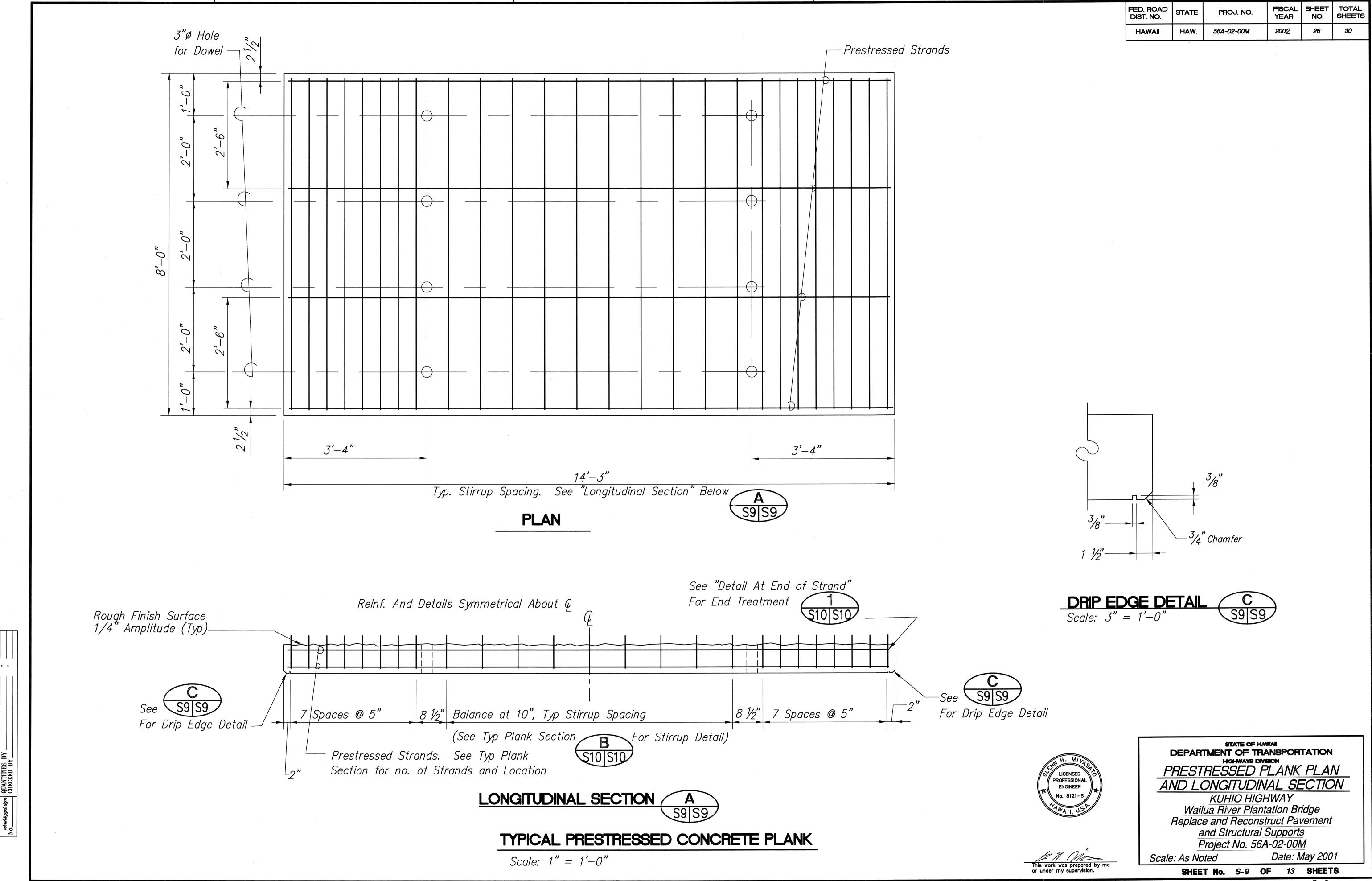
Wailua River Plantation Bridge
Replace and Reconstruct Pavement
and Structural Supports
Project No. 56A-02-00M
le: As Noted Date: May 2001

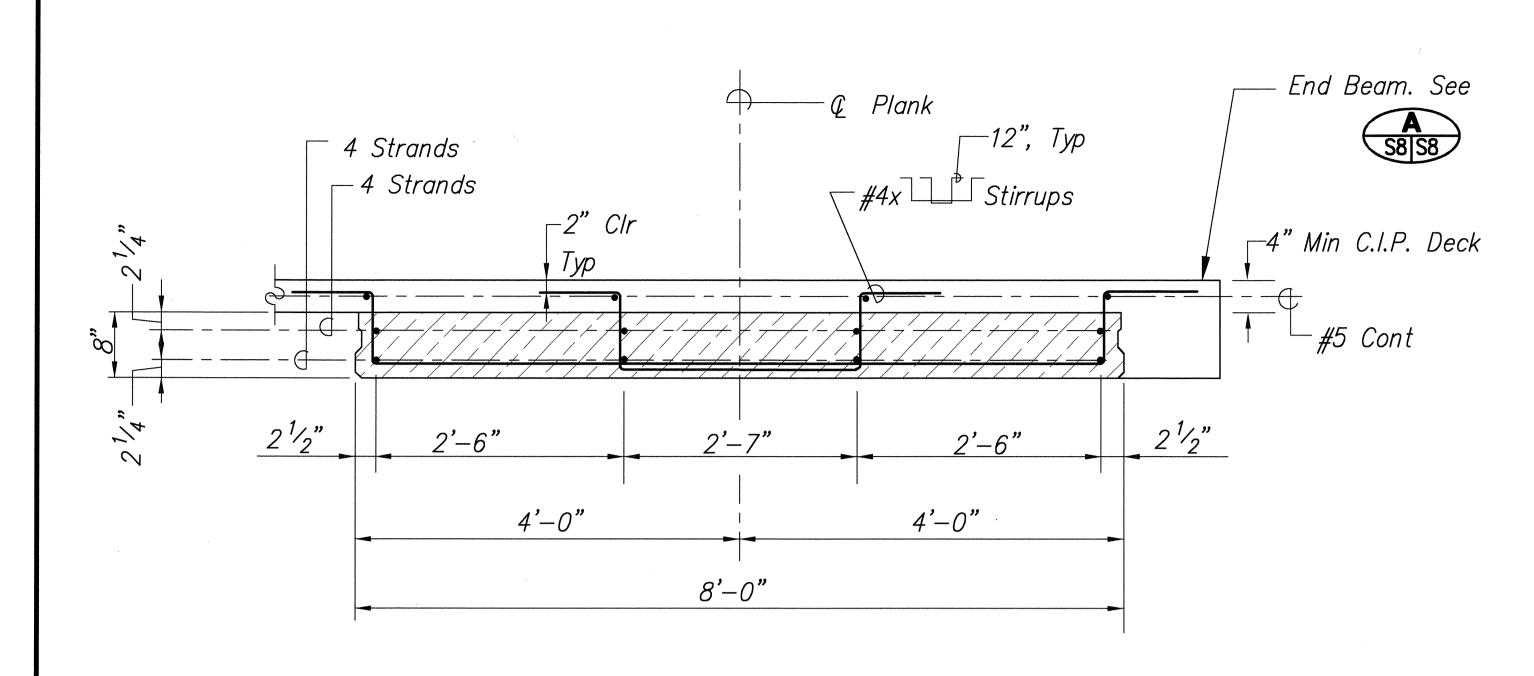
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SHEET No. S-8 OF 13 SHEETS

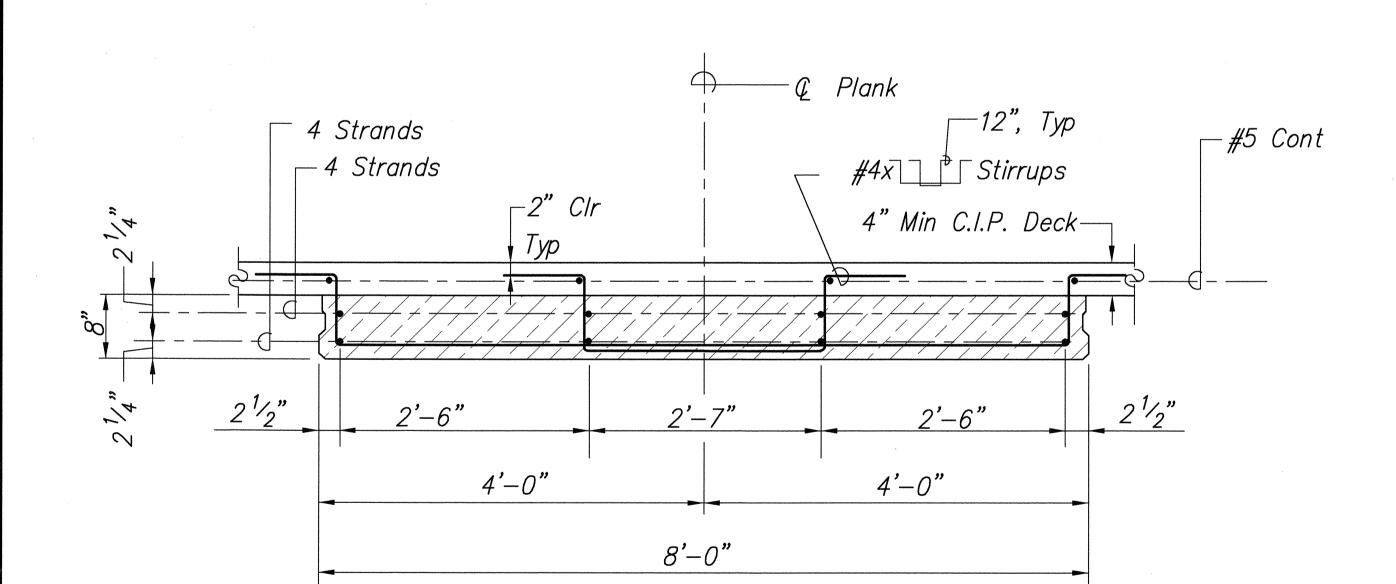
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TYPICAL END PLANK TRANSVERSE SECTION

Scale: 1" = 1'-0"



TYPICAL INTERIOR PLANK TRANSVERSE SECTION

Scale: 1" = 1'-0"



NOTES:

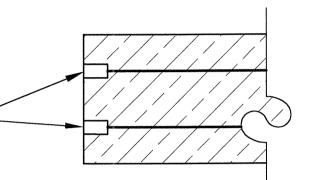
- 1. All reinforcing steel in concrete planks shall be incidential to prestressed concrete planks and will not be paid for separately. Reinforcing Steel: Grade 60, Stainless Steel Clad
- 2. Prestressed Concrete Planks:

f'c = 6,000 psi at 28 days

f'c = 4,000 psi at transfer

- 3. Prestressed Strands: Uncoated 1/2"\$\sigma 270 ksi, 7 Wire Low relaxation strand
- 4. Prestress at transfer shall be 0.70 f's
- 5. Final design stress is based on loss of 45,000 psi.

Cut Back Strand For Min 1½" Clr Cover Fill Void With Shrinkage Compensating Grout Approved By The Engineer

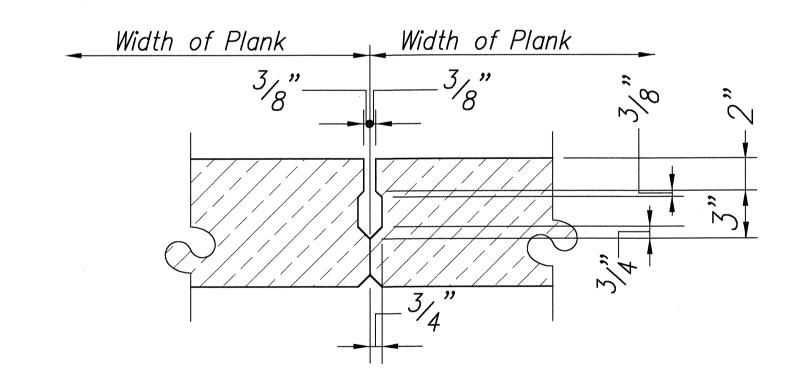


FED. ROAD DIST. NO.

DETAIL AT END OF STRAND

Scale: NTS





Continuous Space Between Planks Shall Be Filled With Grout After All Planks Are In Place And Connected, And Before Placing Ultra Thin White Topping Concrete Deck Slab. The Grout Shall Be Of Shrinkage Compensating Type And Shall Be Approved By The Engineer.

DETAIL OF KEY

Scale: NTS





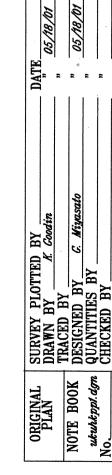
STATE OF HAWA!!
DEPARTMENT OF TRANSPORTATION PRESTRESSED PLANK TRANSVERSE SECTIONS AND DETAILS

KUHIO HIGHWAY Wailua River Plantation Bridge Replace and Reconstruct Pavement and Structural Supports Project No. 56A-02-00M

Scale: As Noted

Date: May 2001 SHEET No. S-10 OF 13 SHEETS

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FISCAL SHEET TOTAL YEAR NO. SHEETS

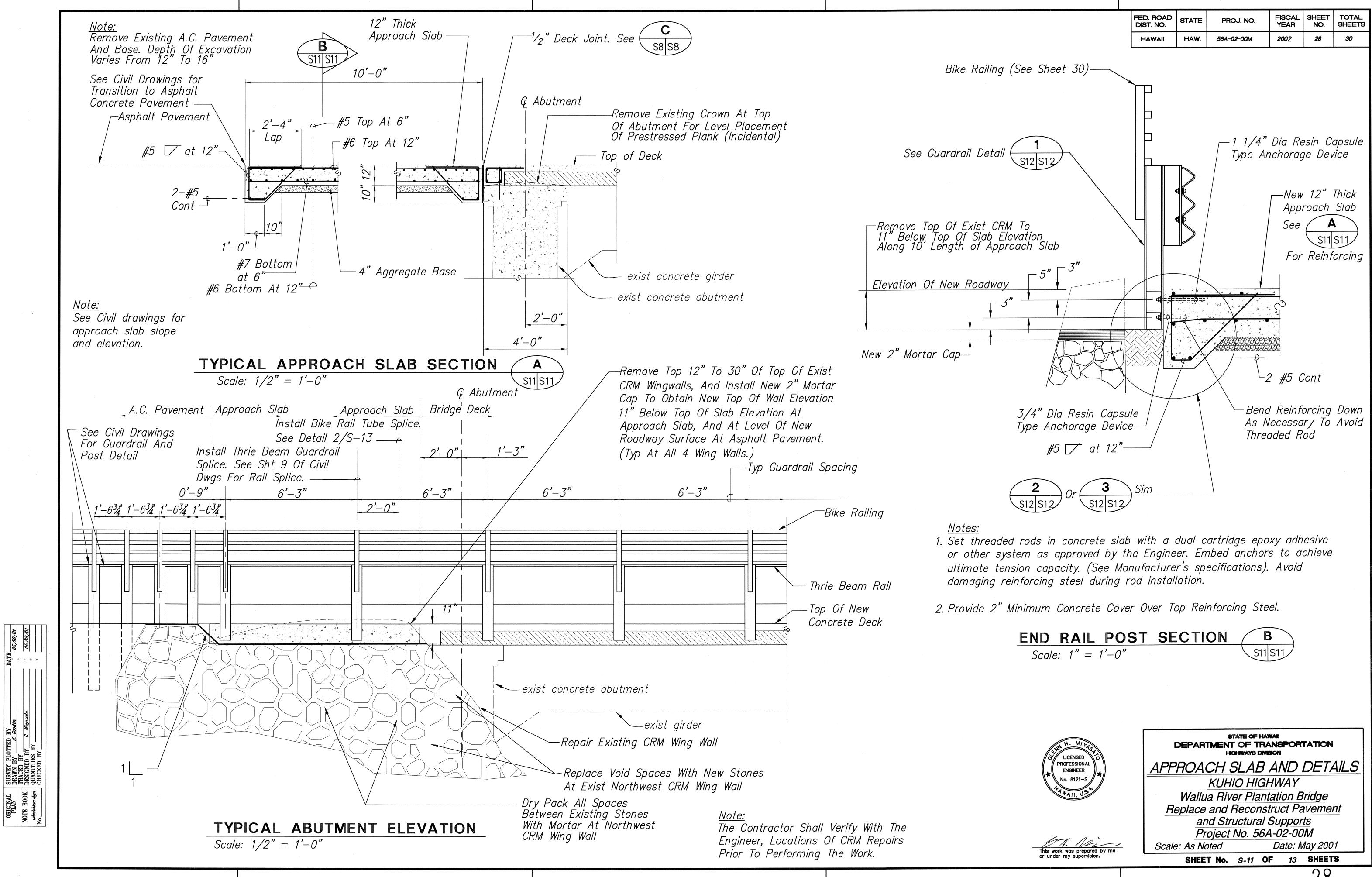
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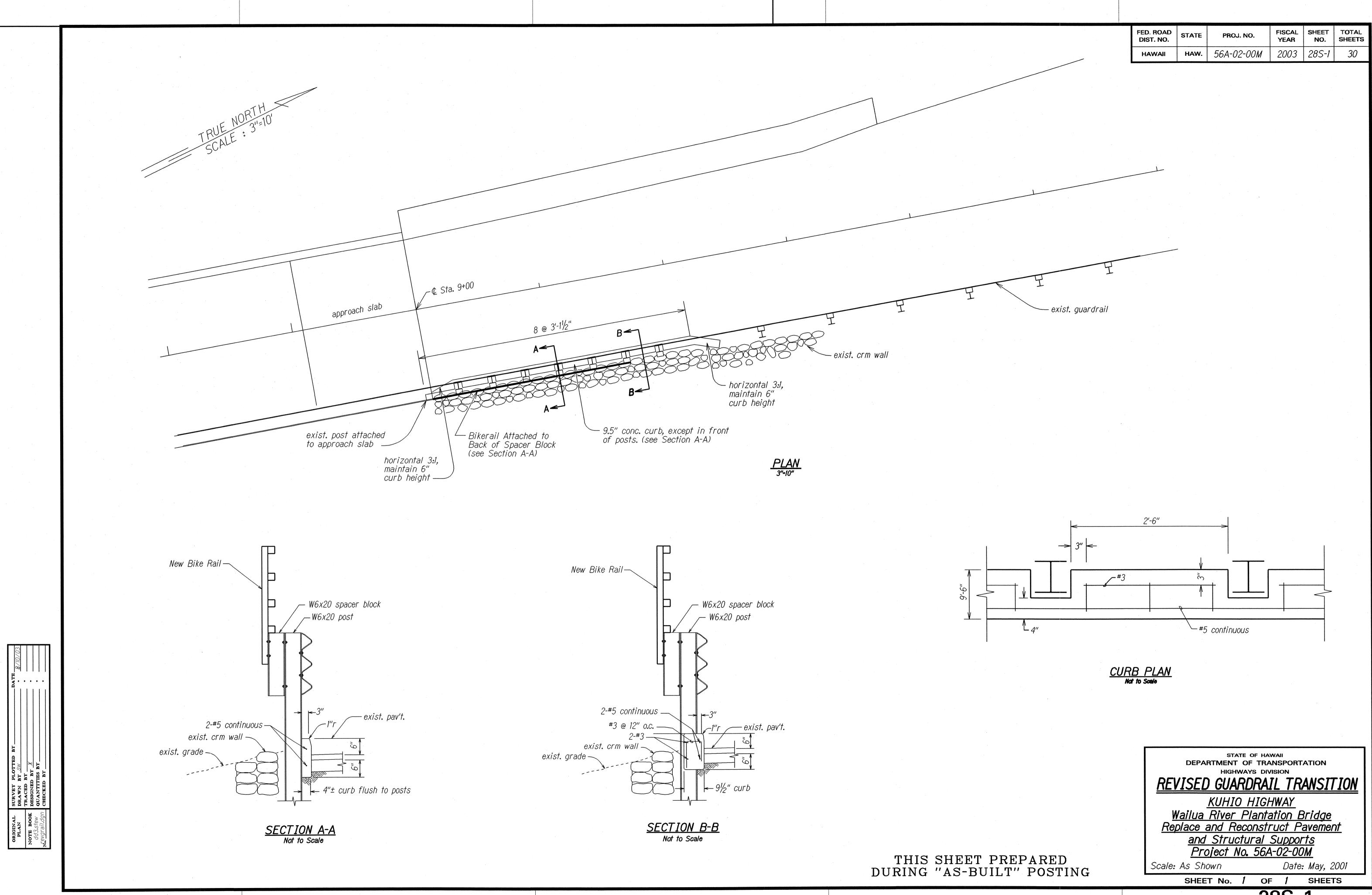
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