STRUCTURAL GENERAL NOTES

1. <u>GENERAL SPECIFICATIONS</u>: Hawaii Standard Specifications for Road and Bridge Construction, 2005, together with Special Provisions prepared for this contract.

2. <u>DESIGN SPECIFICATIONS:</u>

- A. AASHTO LRFD Bridge Design Specifications, 4th Edition, 2007, including subsequent interim revisions.
- B. HDOT HWY-DB Memorandum dated April 15, 2008 with subject Atitle "Design Criteria for Bridges and Structures".

3. <u>LOAD:</u>

- A. Basic Wind Speed: 105 MPH
- B. Seismic Load: Acceleration Coefficient = 0.085 Seismic Performance Zone = 1 Soil Profile Type 1 (S = 1.0)

4. MATERIALS:

- A. Minimum Concrete Compressive Strength (at 28 days)
 - 1. All reinforced concrete = 4,000 psi.
 - 2. All non-reinforced concrete = 3,000 psi
- B. All reinforcing steel shall conform to ASTM A615, Grade 60, unless otherwise noted.

5. REINFORCEMENT:

- A. Unless otherwise noted, the covering measured from the surface of the concrete to the face of any reinforcing bars shall be as follows:

 1. Formed surfaces exposed to earth and weather = 2" Clear.
 - 2. Bottom and sides of footings and where concrete deposited on grade = 3" Clear.
- B. Lap splice length for rebars shall be not less than 48 times bar diameter.

6. <u>STRUCTURAL STEEL:</u>

A. Materials.

NTE

SURVEY PLOTTE
DRAWN BY ____
TRACED BY ___
DESIGNED BY __
QUANTITIES BY __
CHECKED BY __

ORIGINAL PLAN NOTE BOOK

- 1. All structural bolts shall be hot—dip galvanized and conform to the following specifications:
 - a. ASTM A307, Grade A for bolts embedded in concrete.
 - b. ASTM A325, Type 1 for steel to steel connections.
- 2. Nuts shall be hot—dip galvanized and conform to the followings specifications:
 - a. ASTM A563, Grade DH for all galvanized bolts.
- 3. Washers, where required for ASTM A325 bolts, shall conform to ASTM F436, Type 1 and shall be hot—dip galvanized. Washers are required where bolt holes are slotted.
- 4. All rolled shape sections and miscellaneous plates, bars and rods shall conform to ASTM A572, Grade 50, hot—dip galvanized after fabrication.
- 5. Shop welds shall have a minimum ultimate strength of 60,000 psi. or equal to the min. ultimate strength of the material being welded.
- 6. Field welds shall have a minimum ultimate strength of 60,000 psi. or equal to the min. ultimate strength of the material being welded.
- B. All ASTM A307 bolts shall be shear—bearing snug tight type with threads excluded from the shear plane unless otherwise noted.

- C. All ASTM A325 bolts shall be shear—bearing snug tight type with threads not required to be excluded from the shear plane unless otherwise noted.
- D. All steel to steel connections shall be full welded unless noted otherwise.

7. FOUNDATION:

- A. The foundation design is based upon recommendations contained in the Geotechnical Engineering Exploration Report entitled "Geotechnical Engineering Exploration, Kaumualii Highway Widening, Phase 1A Vicinity of Anonui Street to Lihue, Island of Kauai, Hawaii, W.O. 3869–20(A) dated July 17, 2008. The report shall be considered as a part of the construction documents and its recommendations shall be implemented unless otherwise directed by the Engineer. The Contractor may obtain a copy of the report at the State of Hawaii, Dept. of Transportation Highways Division upon written request to the engineer.
 - 1. Design Soil Parameters
 - a. Retaining Wall Foundations
 - 1. Bearing Pressure:

Extreme Event Limit State = 9 KSF
Strength Limit State = 5.4 KSF
Service Limit State = 3 KSF

2. Coefficient Of Sliding Friction: Extreme Event Limit State = 0.46 Strength Limit State = 0.36

3. Passive Pressure Resistance:
Extreme Event Limit State = 350 PCF
Strength Limit State = 175 PCF

b. Static Lateral Earth Pressures at Retaining Structures:
 1. Active Condition, Level Backfill = 40 PCF; (82 PCF below groundwater)

2. At-Rest Condition, Level Backfill = 60 PCF; (92 PCF below groundwater)

c. Dynamic (Seismic) Lateral Earth Pressures at Retaining Structures: 1. Level Backfill = 3H PSF (Rectangular Distribution)

2. Sloping Backfill = 10H PSF (Rectangular Distribution)

- B. Bottom of footings shall be as indicated on the drawings and shall bear on firm on—site soils or property compacted fill.
- C. All fill and backfill material shall be placed in loose lifts not exceeding 8 inches and compacted to at least 95 percent of the maximum dry density as determined by ASTM D1557-91 procedure "C".
- D. Any loose or soft soil encountered during excavation or subgrade compaction operations shall be removed to firm soil and replaced with properly compacted fill.
- E. It is important that all backfill material next to footings and footing keys are properly compacted in order to provide proper lateral earth resistance for structures.
- F. All bottom of footing excavations shall be compacted to 95% prior to placement of reinforcing steel.
- G. The Engineer shall be notified to check all footing excavations prior to placement of reinforcing steel and concrete.

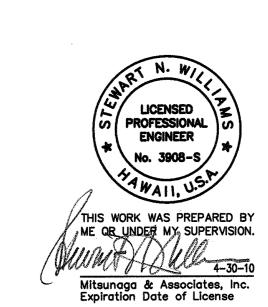
FED. ROAD
DIST. NO.STATEFED. AID
PROJ. NO.FISCAL
YEARSHEET
NO.TOTAL
SHEETSHAWAIIHAW.NH-050-1(31)2009ADD. 378452

8. GENERAL CONSTRUCTION NOTES:

- A. See Standard Specifications and Special Provisions.
- B. All items noted incidental will not be paid for separately.
- C. Standard detail drawings refer to all structures in general, except for modifications as may be required for special condition. For such modifications, refer to the corresponding detailed drawings.
- D. The Contractor shall comply with all construction permits for this project. In addition, the contractor shall comply with all applicable laws of the Federal, State and County governments.
- E. Unless otherwise noted, all vertical dimensions are measured plumb.
- F. The contractor shall verify all site conditions before commencing with the work.
- G. The Contractor shall verify the location of all underground utility lines and notify the respective owners before commencing the work of excavation or drilling shafts.
- H. For concrete finish, refer to Standard Specifications.
- I. Unless otherwise noted, all exposed concrete edges shall be chamfered 3/4" x 3/4".

1 9. CONCRETE BOX CULVERT DRAIN LINES

- A. Concrete Box Culverts at Drain Lines No. 5, 7 and 8 are shown cast—in—place at sheet numbers D4.4/158; D5.5/165 and D6.1/169.
- B. Combined precast and cast—in—place alternates may be submitted for acceptance by the Engineer prior to construction. Combined precast and cast—in place alternates shall comply with the following:
 - 1. Completed in place alternates shall conform to cast—in—place interior configuration and size shown on the drawings referenced at Note 9.A. above.
 - 2. Completed in place alternates shall consist of a cast—in—place bottom slab and bottom fillets with 3 sided precast units above.
 - 3. Precast units shall consists of combined top slab, top fillets and side walls.
 - 4. The Contractor shall design combined precast and cast—in—place alternatives to the most current AASHTO Load and Resistance Factor Design Bridge Design Specifications with Subsequent interims. Refer to Notes 2.A. and 2.B. above. Calculations and shop drawings shall be sealed and signed by a Structural Engineer currently licensed in the State of Hawaii.



STRUCTURAL GENERAL NOTES

KAUMUALII HIGHWAY WIDENING

Vicinity of Anonui Street to Vicinity of Lihue Mill Bridge

FEDERAL—AID PROJECT NO. NH—050—1(31)

Scale: NONE Date: JULY 2009

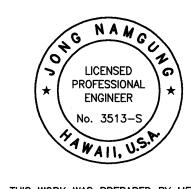
SHEET No. S1 OF S11 SHEETS

CMU RETAINING WALL NOTES:

- (A) See "Structural General Notes" shown on Sheet <u>S1</u> for Structural Construction and Material requirements.
- (B) Construction shall conform to AASHTO Guide Specification for Structural Design of Sound Barriers, 1989, including subsequent interim revisions; Building Code Requirements for Masonry Structures (ACI 530); Specifications for Masonry Structures (ACI 503.1); and Chapters 17 and 21 of the International Building Code, 2003 as amended by the County of Kauai.
- (C) Concrete Masonry
 - 1. Concrete masonry units shall conform to ASTM C90. f'm=3,000 PSI.
 - 2. Mortar shall conform to ASTM C270, Type S, with a minimum 28 day compressive strength of 3,000 PSI.
 - 3. Grout shall conform to ASTM C476, with a minimum 28 day compressive strength of 3,000 PSI.
 - 4. Reinforcing steel in masonry shall be lapped 48 diameters minimum.
 - 5. Horizontal joint reinforcing shall conform to ASTM A82. Provide reinforcing at 24" O.C., lap 8" minimum at all splices.
 - 6. All walls shall be constructed in conventional running bond.
 - 7. All cells shall be grouted solid. Reinforcing shall be secured against displacement prior to grouting by wire positions at intervals not exceeding 150 bar diameters or 6 feet.
 - 8. The clearance between any masonry unit and reinforcing bar shall be 1/2" minimum to 1" maximum. The minimimum clearance between parallel or intersecting reinforcing bars shall be 3/4".
 - 9. When the grouting is stopped for one hour or longer, horizontal construction joints shall be formed by stopping the pour of grout 1-1/2" below the top of the uppermost unit.
 - 10. Prism Test Requirements: The specified compressive strength of masonry at the age of 28-days (f'm) shall be verified on the basis of prisms tested 28-days after fabrication. Unless approved otherwise by the Engineer, and only as specified in Section 2105.2 of the International Building Code, 2003, as amended by the County of Kauai, verification by masonry prism testing shall meet the following:
 - (1) A set of three masonry prisms shall be built and tested prior to the start of construction. Materials used for the construction shall be taken from those specified to be used in the project. Prism shall be constructed under the observation of the Engineer and tested by an Agency approved by the Engineer.
 - (2) Full allowable stresses are used in this design. A set of three prisms shall be built and tested during construction for each 4,000 square feet (555.6 square yards) of wall area.

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR		TOTAL SHEETS
HAWAII	HAW.	NH-050-1(31)	2009	379	452

- (D) Concrete masonry units shall be manufactured with natural aggregates of basalt and limestone. Proportion shall be determined by the manufacturer.
 - Size: 12"x8"x16".
 - 2. Color: Color shall be accepted by the Engineer.
 - 3. Texture: Split-face.
- (E) Unless accepted otherwise by the Engineer, cleanout openings shall be provided at the bottom of walls where the wall is in excess of 4 feet. After cell inspection, the cleanouts shall be sealed before filling with grout.
- (F) Place joint reinforcing in horizontal joints at spacings not exceeding 2 feet so that longitudinal wires are fully embedded in face shell mortar for their entire length. Place reinforcing in horizontal unit bond beams at spacings not exceeding 4 feet. Place and tie vertical reinforcements at top and bottom and interval not exceed 96 diameters of reinforcement or 4 feet.
- (G) Mortar joints shall be straight, clean and in thickness of 3/8 inch plus or minus 1/8 inch. Joints shall be concave.
- (H) After completion of the walls, apply a water-based, water repellent penetrating seal for masonry to all exposed surfaces.
- (I) Minimum concrete compressive strength at 28 days shall be 4,000 PSI.
- (J) All reinforcing bars shall conform to ASTM A615, Grade 60.
- (K) Horizontal joint reinforcing shall be Dur-O-Wal or approved equal, conform to ASTM A82. Provide reinforcing at 24" O.C.
- (L) Allowable tolerances for walls shall be:
 - 1. From Plumb: 1/4 inch for every 10 foot height or proportion thereof.
 - 2. From Level: 1/4 inch for every 20 foot length or proportion thereof.
- (M) Soil Compacting Equipment: No vibratory compaction equipment shall be used in the CMU Retaining Wall construction.



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HITSUNAGA & ASSOCIATES Scale: NONE LICENSE EXPIRES 4-30-10

STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

CMU RETAINING WALL NOTES

KAUMUALII HIGHWAY WIDENING

Vicinity of Anonui Street to Vicinity of Lihue Mill Bridge FEDERAL-AID PROJECT NO. NH-050-1(31)

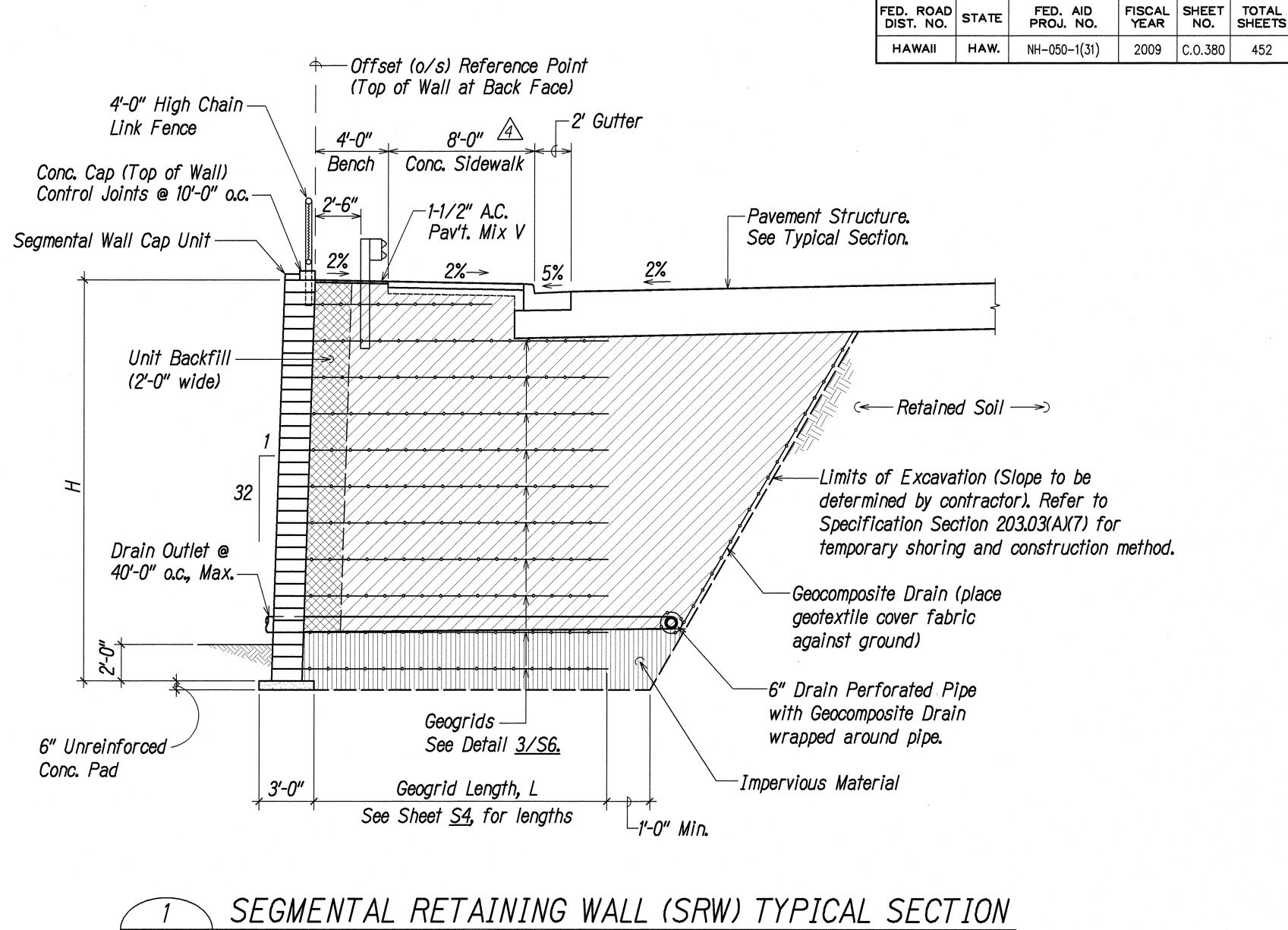
Date: FEB. 2009

SHEET No. S2 OF S11 SHEETS

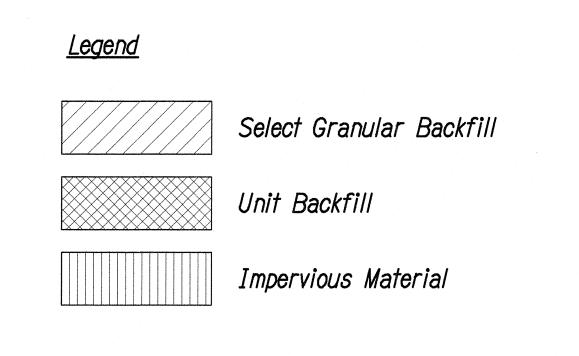


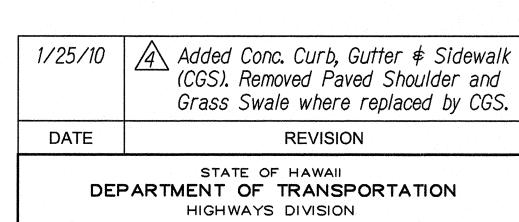
SEGMENTAL RETAINING WALL STRUCTURAL NOTES:

- A. Segmental Concrete Facing Units:
 - 1. Units shall have a 28-day compressive strength of not less than
 - 2. Absorption = 8% maximum for standard weight aggregates.
 - 3. Unit Depth = 21 inches minimum.
 - 4. Unit Width to Height Ratio = 2.25 : 1
 - 5. Unit Weight = 115 lb/unit minimum for standard weight aggregate
 - 6. Inter-Unit Shear Strength = 1000 PLF minimum at 2 PSI normal pressure.
- B. Shear Connectors:
 - Strength of shear connectors between vertical adjacent units shall be applicable over a design temperature of 10 degree F to +100 degree F. Shear connectors shall be 1/2 diameter thermoset isopthalic polyester resin-pultruded fiberglass reinforcement rods. Connectors shall have a minimum flexural strength of 128,000 PSI and short beam shear strength of 6,400 PSI.
 - 2. Shear connectors shall be capable of holding the geogrid in the proper design position during grid pre-tensioning and backfilling.
 - 3. The connection strength between the grogrid and the concrete facing units shall be determined per ASTM D6638.
- C. Geogrid:
 - Geogrid structure shall consist of select high-density polyethylene, polyester or polypropylene resin. Geogrid shall be a geosynthetic reinforcement material having regular and defined open areas, and shall have a long-term design strength of 3,200 lb/ft as determined by ASTM D4395, D5262.
- D. Drainage Collection Pipe (6" Perforated Pipe):
 - Install drainage collection pipe to maintain gravity flow of water from reinforced soil zone.
 - 2. Daylight drainage collection pipe at face of wall, at a maximum spacing of 40 feet, on center. See Retaining Wall No. 1 Profile on Sheet <u>S4</u> for approximate weep hole locations.
 - 3. Minimum slope of perforated drainage pipes shall be 1.0%.







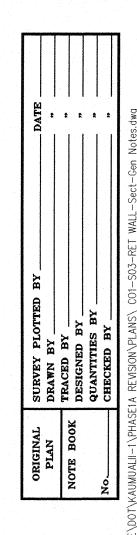


SEGMENTAL WALL NO. 1 STRUCTURAL NOTES & SECTION

KAUMUALII HIGHWAY WIDENING Vicinity of Anonui Street to Vicinity of Lihue Mill Bridge FEDERAL-AID PROJECT NO. NH-050-1(31)

Scale: 1/4" = 1'-0"Date: JAN. 2010 SHEET No. S3 OF S11 SHEETS

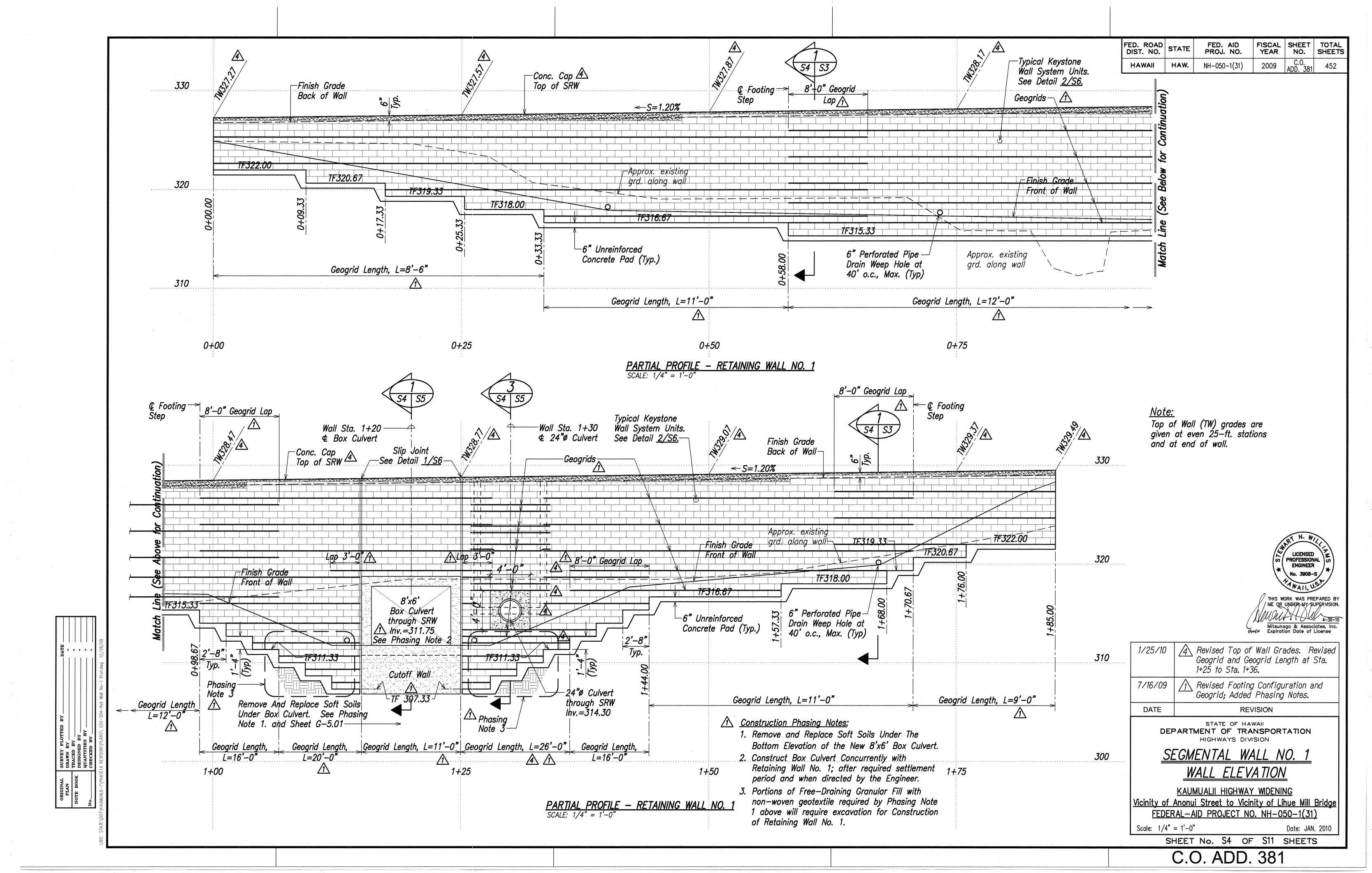
C.O. 380

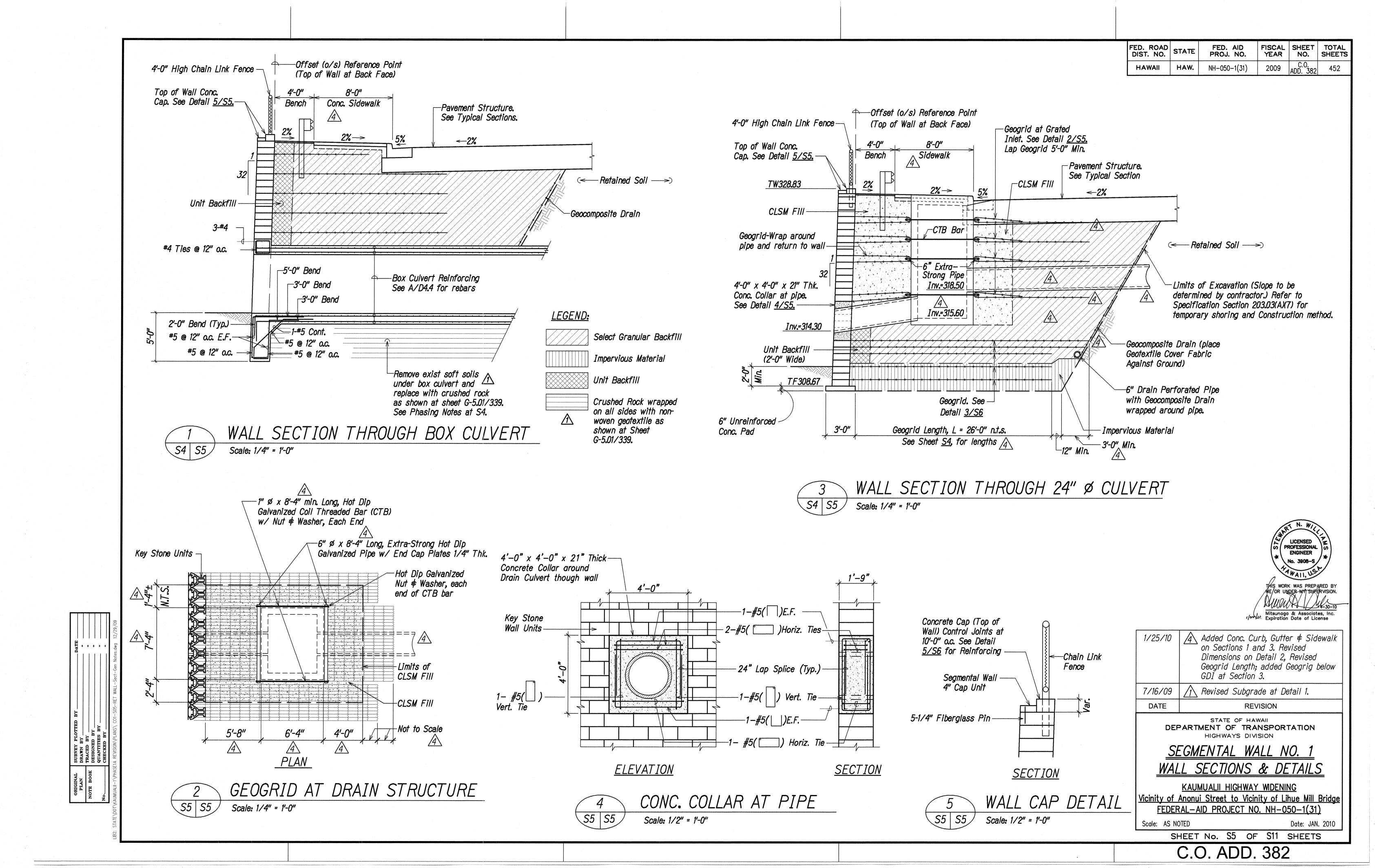


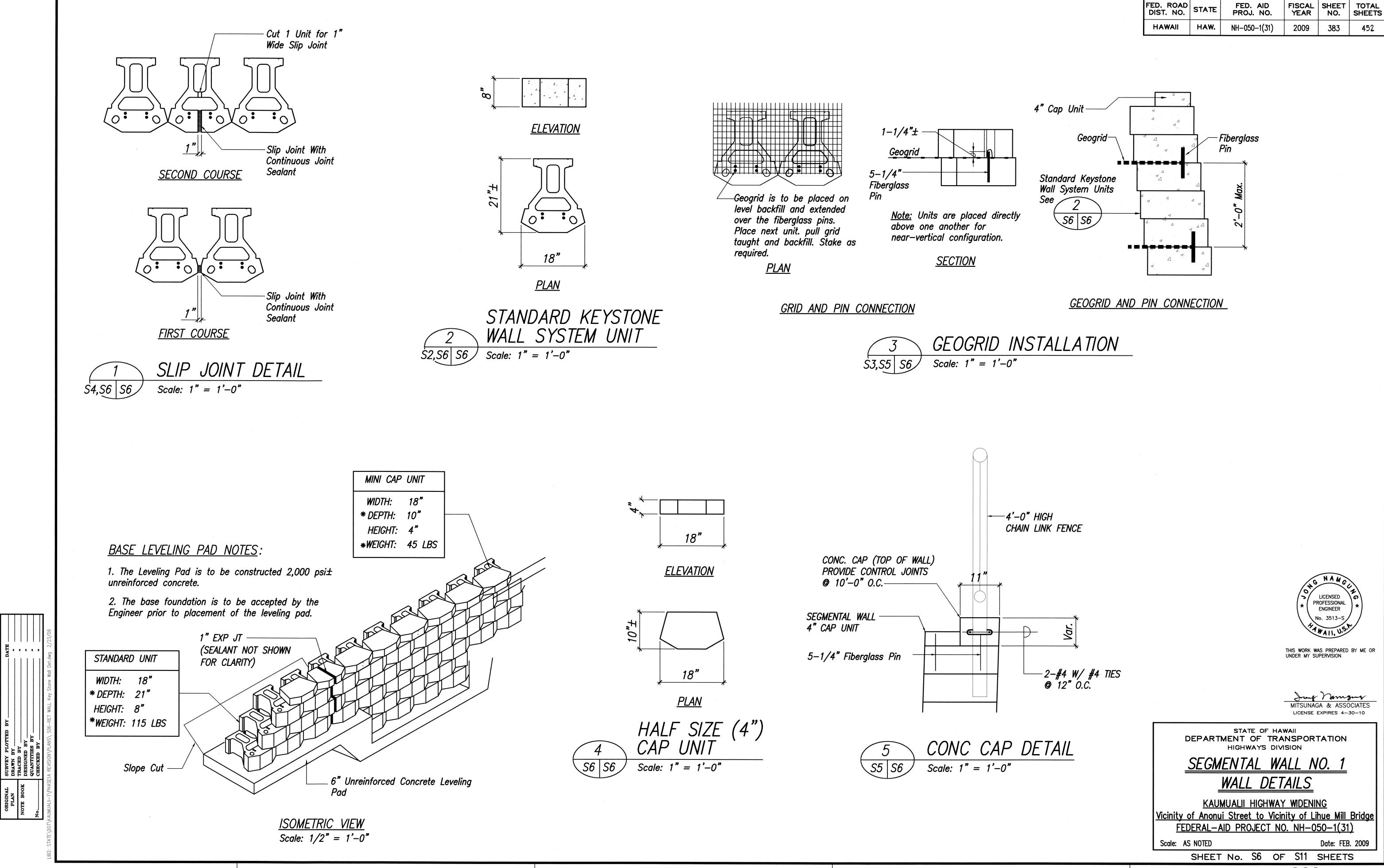
LICENSED PROFESSIONAL

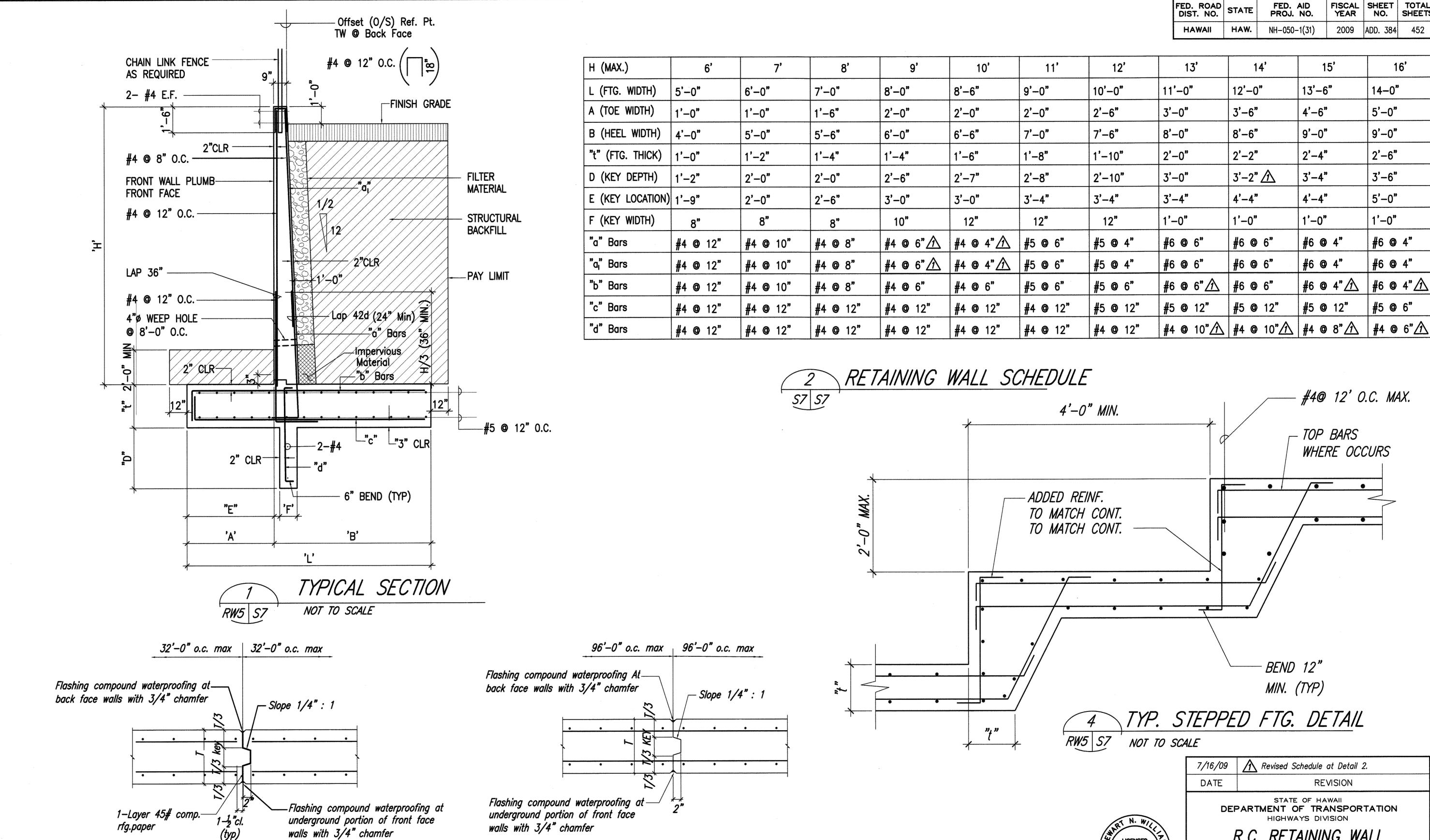
ENGINEER

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AT CONSTRUCTION JOINT

AT CONTROL JOINT

RW5 S7

NOT TO SCALE

TYPICAL RETAINING WALL CONTROL / CONSTRUCTION JOINT DETAIL

R.C. RETAINING WALL LICENSED PROFESSIONAL ENGINEER SECTION AND SCHEDULE KAUMUALII HIGHWAY WIDENING Vicinity of Anonui Street to Vicinity of Lihue Mill Bridge FEDERAL-AID PROJECT NO. NH-050-1(31) Date: JULY 2009 SHEET No. S7 OF S11 SHEETS

- BEND 12"

MIN. (TYP)

REVISION

ADD. 384

STATE OF HAWAII

DEPARTMENT OF TRANSPORTATION

HIGHWAYS DIVISION

FISCAL SHEET TOTAL YEAR NO. SHEETS

2009 ADD. 384 452

14-0"

5'-0"

9'-0"

2'-6"

3'-6"

5'-0"

1'-0"

#6 @ 4"

#6 @ 4"

#5 **@** 6"

#6 @ 4" 🖍 | #6 @ 4" 🔨

#4@ 12' O.C. MAX.

WHERE OCCURS

16'

FED. AID PROJ. NO.

15'

13'-6"

4'-6"

9'-0"

2'-4"

3'-4"

4'-4"

1'-0"

#6 **@** 4"

#6 @ 4"

#5 **@** 12"

— TOP BARS

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NH-050-1(31)

3'-6"

8'-6"

2'-2"

4'-4"

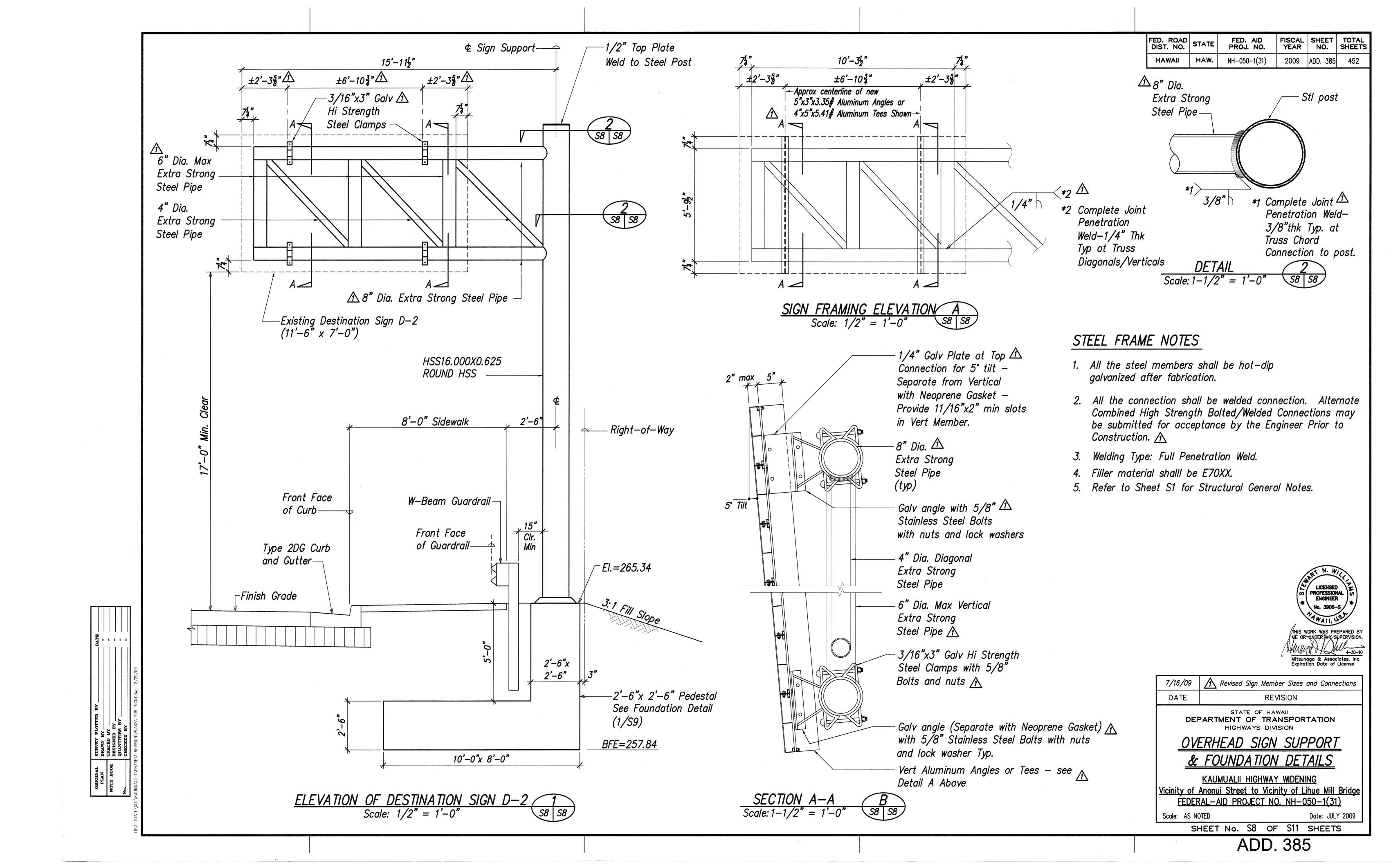
1'-0"

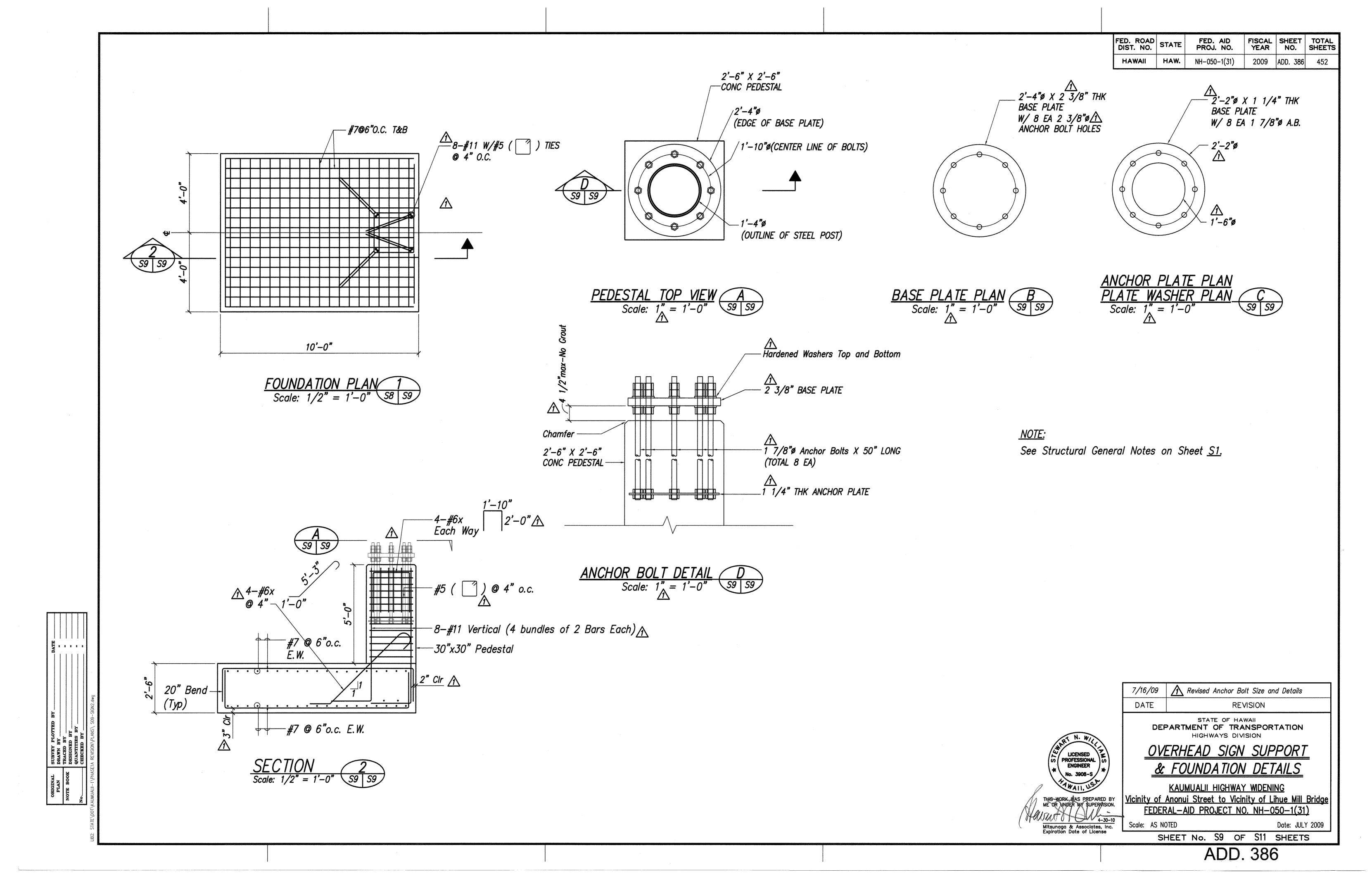
#6 @ 6"

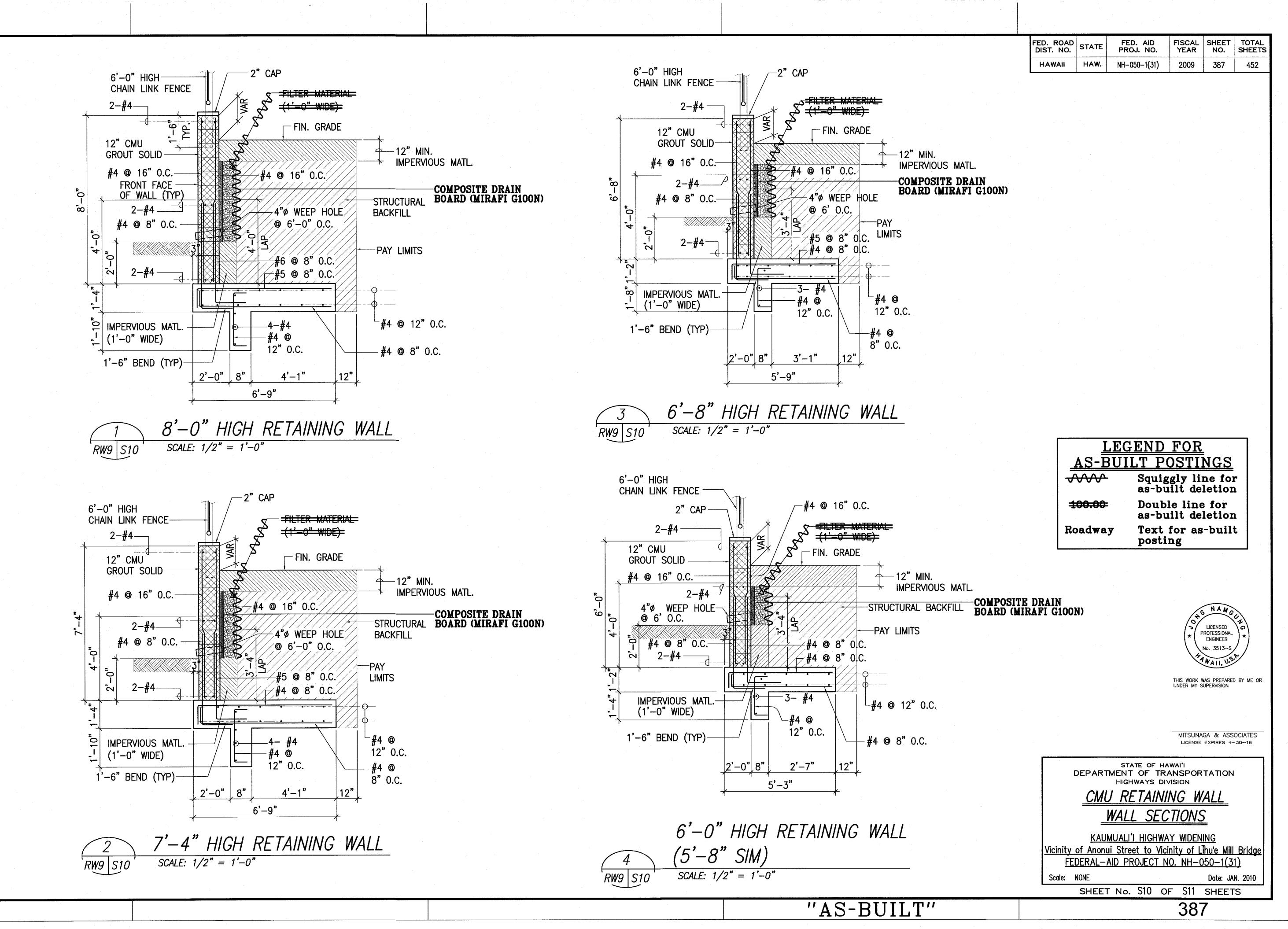
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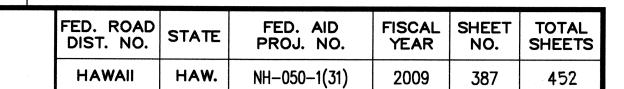
#5 **@** 12"

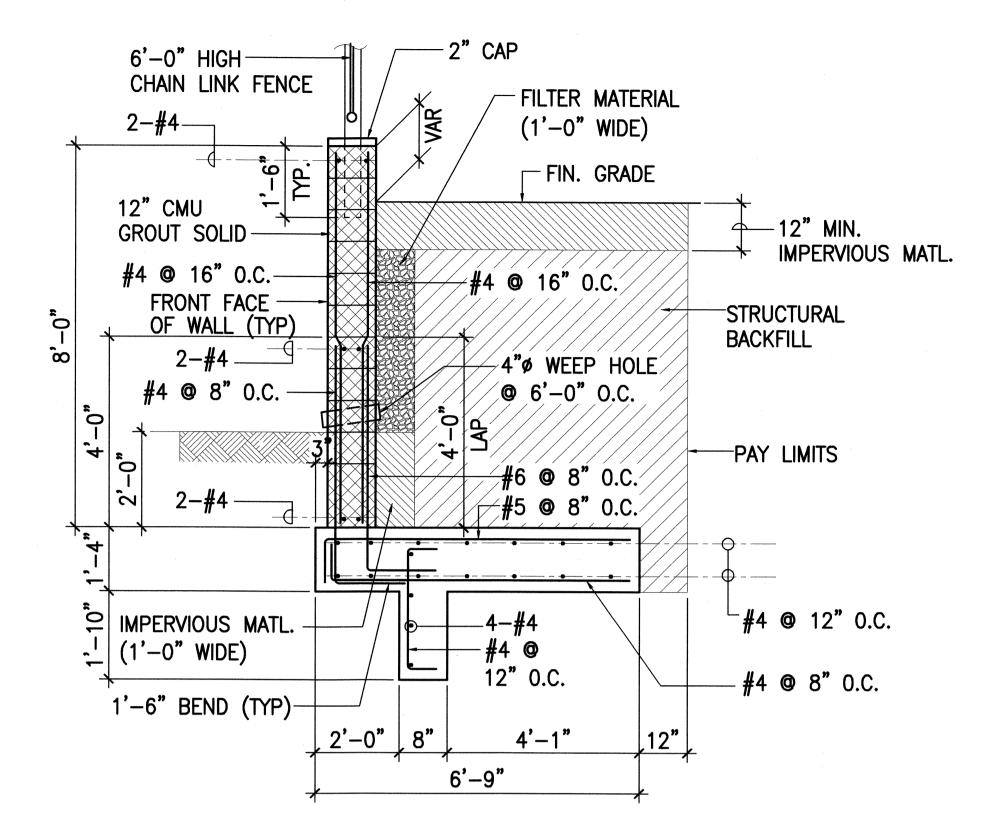
3'-2"



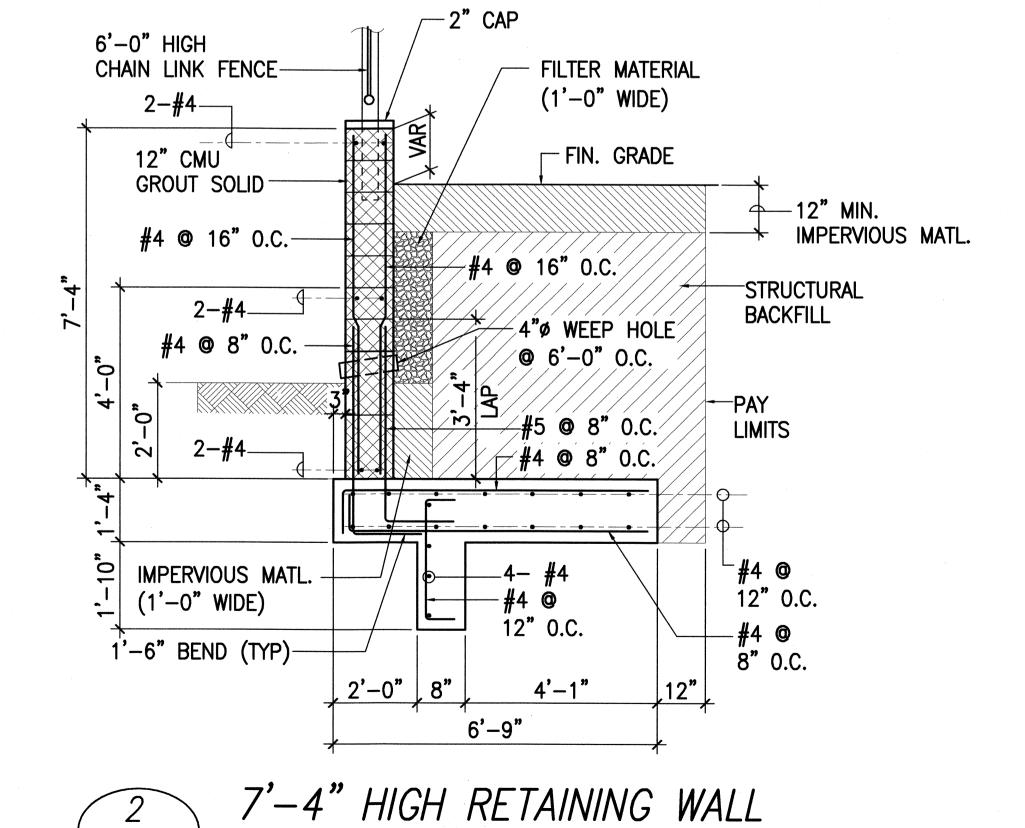








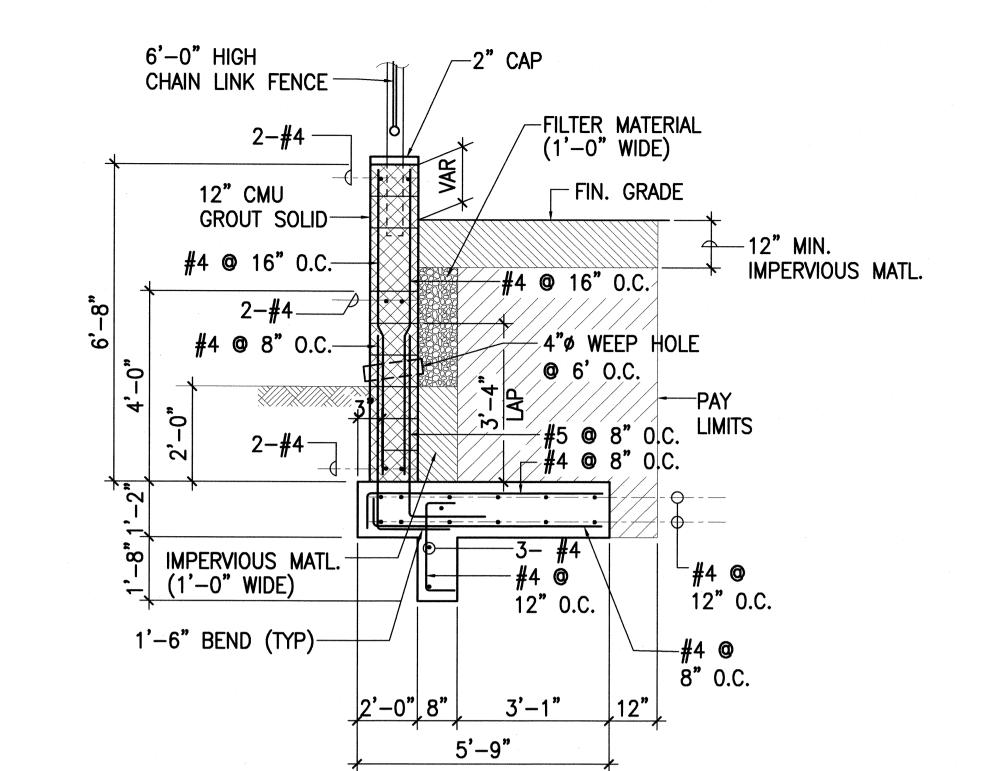
8'-0" HIGH RETAINING WALL SCALE: 1/2" = 1'-0"RW9 S10



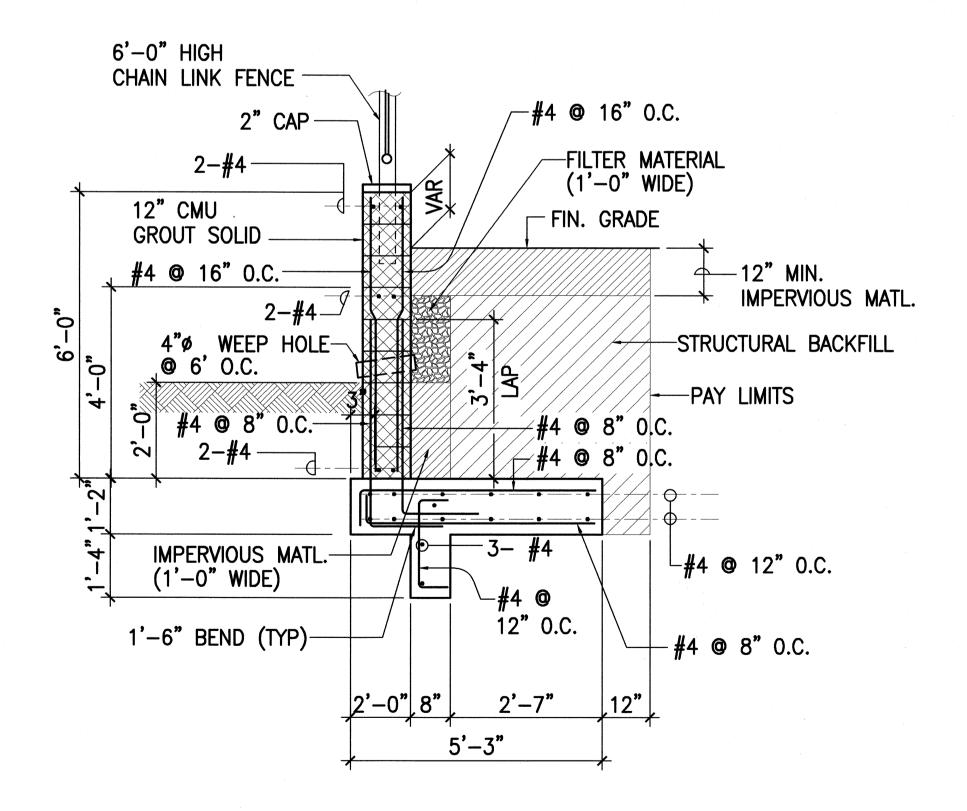
SCALE: 1/2" = 1'-0"

RW9 S10

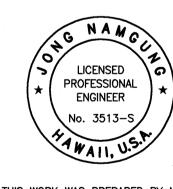
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6'-8" HIGH RETAINING WALL SCALE: 1/2" = 1'-0"RW9 S10



6'-0" HIGH RETAINING WALL (5'-8" SIM)SCALE: 1/2" = 1'-0"RW9 S10



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STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

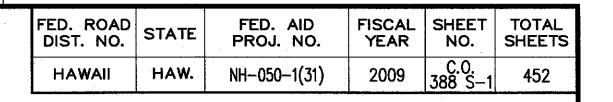
CMU RETAINING WALL WALL SECTIONS

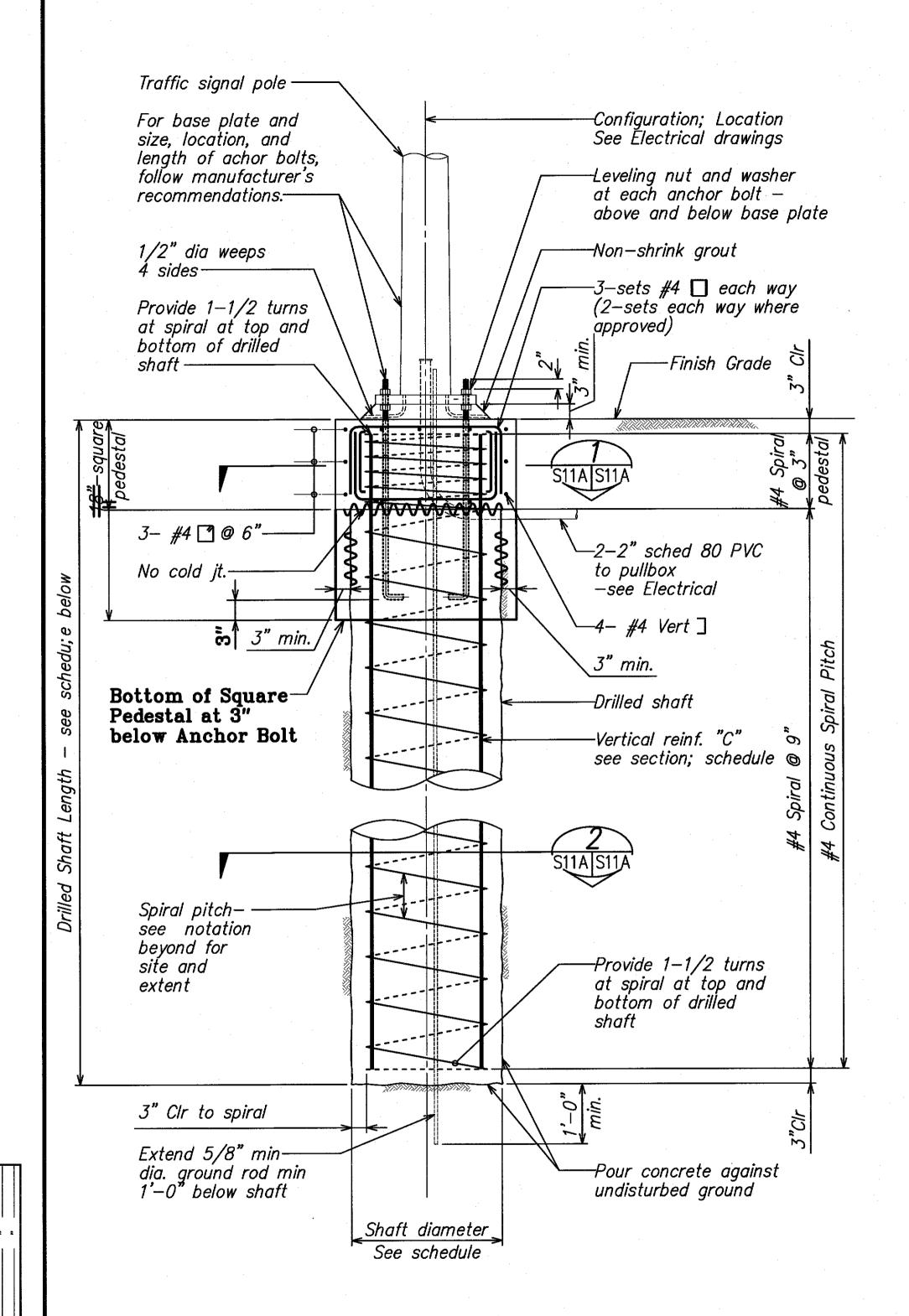
KAUMUALII HIGHWAY WIDENING Vicinity of Anonui Street to Vicinity of Lihue Mill Bridge FEDERAL-AID PROJECT NO. NH-050-1(31)

Date: FEB. 2009 Scale: NONE

SHEET No. S10 OF S11 SHEETS

387

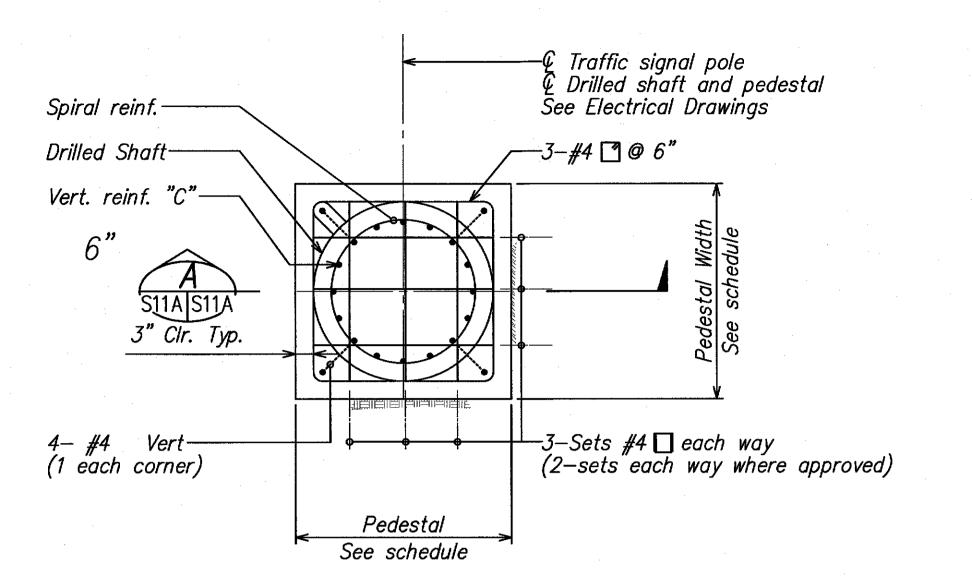






Level or 2H:1V Sloping Ground—Below Water Table							
Min Soil Type	Mast Arm Length	Shaft Dia	Shaft Length	Pedestal Width	Vert Reinf "C"		
Sand &	30'	<i>30"</i>	11'-0"	<i>36</i> "	16-#6		
Gravel	35'	<i>30</i> "	13'-0"	36"	16-#6		
	40'	<i>30"</i>	15'-0"	<i>36</i> "	16-#6		

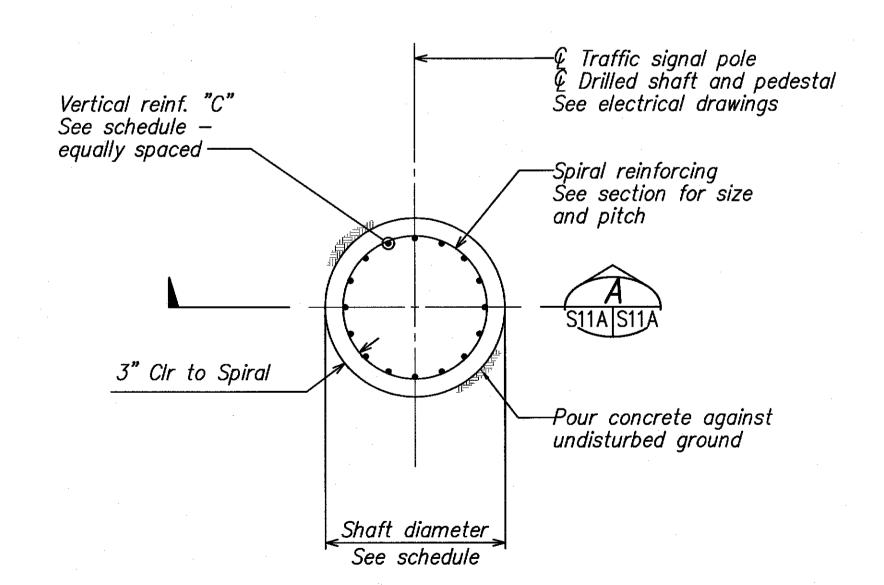
SURVEY PL.
DRAWN BY.
TRACED BY
DESIGNED 1
QUANTITIES



PEDESTAL PLAN DETAIL

Scale: 3/4"= 1'-0"

S11A S11A

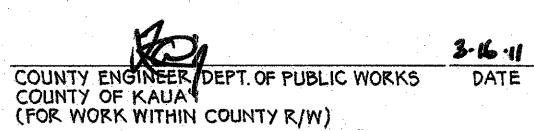


DRILLED SHAFT PLAN DETAIL

Scale: 3/4"= 1'-0"

S11A S

APPROVED:





MITSUNAGA & ASSOCIATES
LICENSE EXPIRES 4-30-16

TYPE II TRAFFIC SIGNAL FOUNDATION

- 1. See Elctrical drawings for Type II Traffic Signal Standard configurations, layouts and Pole Foundation details.
- 2. For Alternate Drilled Shaft Foundations For Type II
 Mast Arm Standard, see details on this sheet.
- 3. The recommendations of the traffic signal pole manufacturer shall be followed. Manufacturer shall select pole, anchor bolts, etc. based on criteria given in the contract documents.
- 4. Materials for details on this sheet are as follows:
 - a. Concrete for drilled shaft and pedestal shall provide compressive strength f'c=4,500 psi (28 day strength) with a maximum water/cement (W/C) ratio of 0.45. See specifications for drilled shaft requirements.
 - b. All connection bolts shall be AASHTO M164 bolts and anchor bolts shall be AASHTO M134-105 bolts.
 - c. Aluminum members and surfaces in contact with structural steel shall be isolated with neoprene material as approved by the Engineer.
- 5. The Contractor shall use templates while installing the anchor bolts. Anchor bolts shall be verified.
- 6. Excavation and backfill shall be considered incidental to the cost of the traffic signal foundation.

LEGEND FOR AS-BUILT POSTINGS

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Double line for as-built deletion
Text for as-built

Roadway Text for posting

TRAFFIC SIGNAL FOUNDATION

KAUMUALI'I HIGHWAY WIDENING

Vicinity of Anonui Street to Vicinity of Lihu'e Mill Bridge

FEDERAL—AID PROJECT NO. NH—050—1(31)

Scale: 3/4" = 1'-0"

SHEET No. S11A OF S11 SHEETS

LT" C.O. 388 S-1



