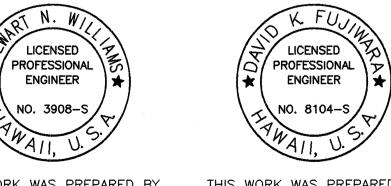
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SHEET	DESCRIPTION DESCRIPTION	SHEET	DESCRIPTION
ASO.1	Index to Drawings	AS5.1	Plan and Secton — Pier 1
AS0.2	Estimated Quantities	AS5.2	Plan and Secton — Pier 2
AS0.3	Bridge General Notes	AS5.3	Plan and Secton — Pier 3
ASO.4	Symbols and Abbreviations	AS5.4	Structural Excavation And Backfill Pay Limits
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AS2.3	Typical Abutment and Pier Sections		
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1071	Top of Dook Claustian Dian	AS8.1	Tonging Pointoroing Plan
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AS4.1	Abutment 1, Wingwall and End Post Layout (Hilo Approach)	730.4	TOPPING TIGHT
AS4.2	Abutment 2, Wingwall and End Post Layout (Pahala Approach)		
AS4.3	Abutment 1, Wingwall Elevations, Details (Hilo Approach)	AS9.1	Railing Section and Details
AS4.4	Abutment 2, Wingwall Elevations, Details (Pahala Approach)	AS9.2	Metal Bikeway and Intermediate Railing on Concrete Railing Details
AS4.5	Foundation Details at Abutment 1	AS9.3	Railing Section and Details
AS4.6	Foundation Details at Abutment 2		
AS4.7	Column Details at Abutment 1		
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AS4.10	Wingwall Sections and Details		
AS4.11	Wingwall Sections and Details		
AS4.12	Wingwall Sections and Details		
AS4.13	End Post Details		
AS4.14	End Post Details		
AS4.15	Approach Slab Details		
AS4.16	Excavation and Backfill Limits		
	· ·		

DATE

 FED. ROAD	STATE	FED. AID	FISCAL	SHEET	TOTAL
DIST. NO.		PROJ. NO.	YEAR	NO.	SHEETS
HAWAII	HAW.	ER-12(2)	2001	31	145



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STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

INDEX TO DRAWINGS

MAMALAHOA HIGHWAY

EMERGENCY REPLACEMENT OF

FORD CROSSING

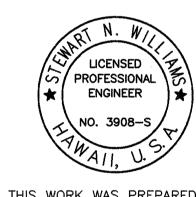
Federal Aid Project No. ER-12(2)

Scale: As Noted

Date: Feb. 2001 SHEET No. ASO.1 OF 49 SHEETS

ITEM NO.	ITEM	QUAN.	UNIT	ITEM NO.	I TEM	QUAN.	UNIT
202.0420	Removal of Existing 4'x10' Concrete Culvert	(1 EACH)	L.S.	504.6100	Concrete for Prestressed Planks	(1160 L.F.)	L. S.
206.6000	Structure Excavation for Abutments and Piers	2680	CY	507.7000	Concrete End Posts	(4 EA)	<i>L.S.</i>
				507.7002	Bridge Railing Including Metal Rails	248	L.F.
206.6001	Structure Excavation for Wingwalls	1675	CY	602.0080	Reinforcing Steel for Abutments	(32,000 LBS)	L. S.
206.7200	Structure Backfill for Abutments	510	CY	002.0000	Their to the Abatiments	(32,000 LD3)	2.0.
				602.0081	Reinforcing Steel for Abutment Footings	(10,000 LBS)	<i>L.S.</i>
206.7201	Structure Backfill for Abutments and Piers (Class D Concrete)	200	CY	602.0082	Reinforcing Steel for Bridge Deck	(65,000 LBS)	L. S.
206.7202	Structure Backfill for Wingwalls	500	CY	602.0083	Reinforcing Steel for Pier Walls	(43,000,195)	L. S.
200.7202	Structure Duckim for Willywalls		67	002.0003	Keimorchig Steel for Fiel Walls	(43,000 LBS)	<i>L.S.</i>
206.7203	Structure Backfill for Wingwalls (Class D Concrete)	100	CY	602.0084	Reinforcing Steel for Pier Wall Footings	(16,000 LBS)	<i>L.S.</i>
313.1000	Controlled Low Strength Material at Wingwalls	280	CY	602.0086	Reinforcing Steel for Wingwalls	(60,000 LBS)	<i>L.S.</i>
503.1090	Concrete for Abutments	(140 CY)	L. S.	602.0087	Reinforcing Steel for Wingwall Footings	(95,000 LBS)	L. S.
503.1091	Concrete for Abutment Footings	(88 CY)	L. S.	602.0088	Reinforcing Steel for Approach Slab	(32,000 LBS)	L. S.
503.1092	Concrete for Bridge Deck	(187 CY)	L. S.	612.0100	Grouted Rubble Paving	130	S. Y.
503.1093	Concrete for Pier Walls	(149 CY)	L.S.				
503.1094	Concrete for Pier Wall Footings	(80 CY)	L. S.				:
503.1096	Concrete for Wingwalls	(220 CY)	L. S.				
503.1097	Concrete for Wingwall Footings	(260 CY)	L. S.				
503.1101	Concrete for Approach Slab	(80 CY)	L. S.	i			

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	ER-12(2)	2001	32	145



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KSF, Inc.

LICENSED PROFESSIONAL ENGINEER

STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

ESTIMATED QUANTITIES

MAMALAHOA HIGHWAY

EMERGENCY REPLACEMENT OF

FORD CROSSING

Federal Aid Project No. ER-12(2)

Scale: As Noted Date: Feb. 2001

Scale: As Noted

SHEET No. AS0.2 OF 49 SHEETS

BRIDGE GENERAL NOTES

- (4) Abutment Walls, Pier Walls, Wing Walls = 3" to ties or outermost reinforcing
- (5) Approach slab top bars = 2" Approach slab bottom bars = 3"
- (6) Concrete cast against and permanently exposed to earth = 3" (7) All others unless otherwise noted = 2".
- (B) Reinforcing bars shall be detailed in accordance with the latest edition of the A.C.I. Detailing Manual unless otherwise noted.
- (C) Minimum clear spacing between parallel bars shall be 1.1/2 times the diameter of bars (for non bundled bars). In no case shall the clear distance between the bars be less than 1 1/2 times the maximum size of the coarse aggregate.
- (D) All dimensions relating to reinforcing bars are to centers of bars unless otherwise noted.
- (E) Reinforcing bars shall be securely tied at all intersections and lap splices except where the spacing of intersections is less than one foot in each direction, in which case alternate intersections shall be tied.
- Vertical wall bars shall be arranged in such a manner as to avoid interference with plank and topping bars above as directed by the Engineer.

6. Plank Bearings:

1. General Specifications: Hawaii Department of Transportation, Standard

2. Design Specifications: AASHTO 1998 LRFD Bridge Design Specifications

asphalt concrete has been provided for in the design.

Soil Profile - Type

Live Load: AASHTO HL-93 Truck Loading

Seismic Loads: Acceleration coefficient - 0.8

All concrete strengths shall be as noted below:

Abutment Wall Footings, Pier Wall

Footings, Wing Wall Footings

Wing Walls, Retaining walls

Abutment Walls, Pier Walls and

Structural

Pre-stressed planks

including foundations

Rails

recommendations.

Reinforcement:

shown:

Bridge Deck

Approach Slab

Except as noted

otherwise all others

fabrication, unless otherwise noted.

prestressed concrete notes.

Deck slabs

details.

A. Top bars

(3) Retaining walls = 2"

Parts

(Second Edition) and its subsequent interim specifications with interim

supplements and modifications by the Highways Division, Department of

(A) Dead Load: An allowance of 25 PSF for future wearing surface of

Seismic Performance Zone - 4

Importance Category — Essential

All concrete with the exception of Class A and Class D concrete shall

have a maximum W/C Ratio of 0.45. The W/C Ratio for Class A and

Tetraguard AS20 shrinkage reducing admixture shall be included in the

concrete mix for the deck topping, in the edge beams on both sides of

the concrete posts and railings on the bridge. The required dosage shall

All reinforcing steel shall be ASTM A 615 Grade 60 unless otherwise noted.

the bridge, in the end beams over the pier and abutment walls, and in

be 96 ounces per cubic yard of concrete and follow all manufacturer's

Reinforcing steel shall be ASTM A 706 where welded connections are

(A) The minimum covering measured from the surface of the concrete to the

face of any reinforcing bars shall be as follows, except as otherwise

(2) For prestressed concrete planks see prestressed concrete plank

B. Bottom bars = $1 \frac{1}{2}$ except as otherwise noted.

Class D Concrete shall follow the standard specifications.

(E) All structural steel shall be ASTM A 36 hot dip galvanized after

(F) All anchor bolts, washers and nuts shall be ASTM A 307 hot dip

galvanized after fabrication, unless otherwise specified.

(G) For materials of prestressed concrete planks see applicable

= 2"

An allowance of 150 PLF on Each Side of the Bridge

for Utility Loads has been provided for in the design.

Classes of Specified Compressive

Concrete Strength, f'c (28 Days)

6000 PSI

5000 PSI

5000 PSI

4000 PSI

5000 PSI

4000 PSI

3000 PSI

Special Provisions prepared for this contract.

Transportation, State of Hawaii.

3. Loads:

(D) Utility Load:

4. Materials:

Specifications for Road and Bridge Construction, 1994, together with

(A) Plank concrete bearing seats shall be poured monolithically with supporting structure. Top of concrete bearing seat shall be finished with a steel trowel to a smooth level surface to the elevation shown on the plans. Grind down high spots as needed to provide an even bearing surface to 1/16"± tolerance.

7. Construction Notes:

- (A) See Standard Specifications and Special Provisions.
- (B) In general, top of concrete deck slab shall be constructed to follow the roadway vertical and horizontal curves and superelevations.
- Except as otherwise noted, all vertical dimensions are measured plumb.
- (D) The Contractor shall verify all site conditions and not rely upon these plans for stream location, etc. Conditions may differ from those shown.
- (E) The Contractor shall verify the location of all utility lines and notify the respective owners before commencing with excavation, and any temporary piling or sheeting.
- For concrete finish see Standard Specifications and Special Provisions.
- (G) Construction joints may be relocated or additional ones added subject to the approval of the Engineer.
- (H) Unless otherwise noted, all exposed concrete edges shall be chamfered $3/4" \times 3/4"$
- (J) Contractor shall verify footing elevations before fabricating footing and wall reinforcing.

General:

- All items noted incidental will not be paid for separately.
- Standard detail drawings refer to all structures in general, except for modifications as may be required for special conditions. For such modifications refer to the corresponding detailed drawings.

Foundation:

- (A) For boring logs and other geotechnical information, see foundation report by Geolabs, Inc. and Sheets AG1 through AG9.
- (B) General:
 - Footings are located in or near the Ordinary High Water Mark (OHWM), see BMP sheets. Construction at these locations may be complicated by the presence of water and cobbles or boulders. Additionally, the stream is subject to flash flooding. Shoring of the excavation may be required for the construction of these footings.

HAWAII	HAW.	ER-12(2)	2001	ADD. 33	145

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(2) The pay limits for excavation for the Ford Crossing Structures, shown on sheets no. AS4.16, AS5.4 and AS5.5 should not be considered as indicative of actual excavation requirements.

FED. ROAD

DIST. NO.

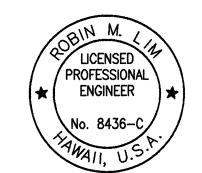
- (C) Design Soil / Rock Parameters:
 - (1) Bearing pressure on basalt rock
 - A. Extreme event limit state = 60 ksf
 - B. Strength limit state = 36 ksf
 - C. Service limit state = 20 ksf
 - (2)Passive resistance of intact rock
 - A. Extreme event limit state = 35 ksf (rectangular distribution)
 - B. Strength limit state = 17.5 ksf (rectangular distribution)
 - (3)Coefficient of friction of basalt rock A. Extreme event limit state = 0.75
 - B. Strength limit state = 0.64
 - (4)Side shear between concrete and intact basalt rock
 - A. Extreme event limit state = 2 ksf
 - (5)Passive spring resistance for Type A structure backfill at abutments
 - A. Extreme event limit state = 30 ksf/inch with 1 inch maximum deflection

(6)Static lateral earth pressure

- A. Active condition, level backfill = 35 pcf
- B. Active condition, 2H: 1V backfill = 50 pcf
- C. At-rest condition, level backfill = 50 pcf
- D. At-rest condition, 2H: 1V backfill = 65 pcf

(7)Dynamic lateral earth pressure

- A. Level backfill = $120H^2$ plf (rectangular distribution) (for abutment walls)
- B. Level backfill = $36H^2$ plf (rectangular distribution)
- (for wingwalls and site retaining walls) (D) Blasting will not be allowed on this project.
- 10. Reference Drawings:
- (A) For construction sequence, see Sheet AS10.1.



Di auxobs.

Revise note

DESCRIPTION

7-31-01

DATE

PROFESSIONAL **ENGINEER**

LICENSED

PROFESSIONAL

ENGINEER

Savid K. Fujiwara

STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION

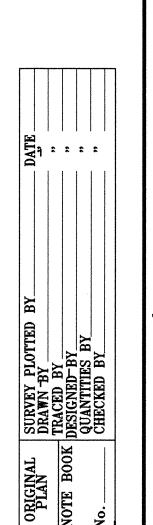
HIGHWAYS DIVISION

BRIDGE GENERAL NOTES

MAMALAHOA HIGHWAY EMERGENCY REPLACEMENT OF <u>FORD CROSSING</u>

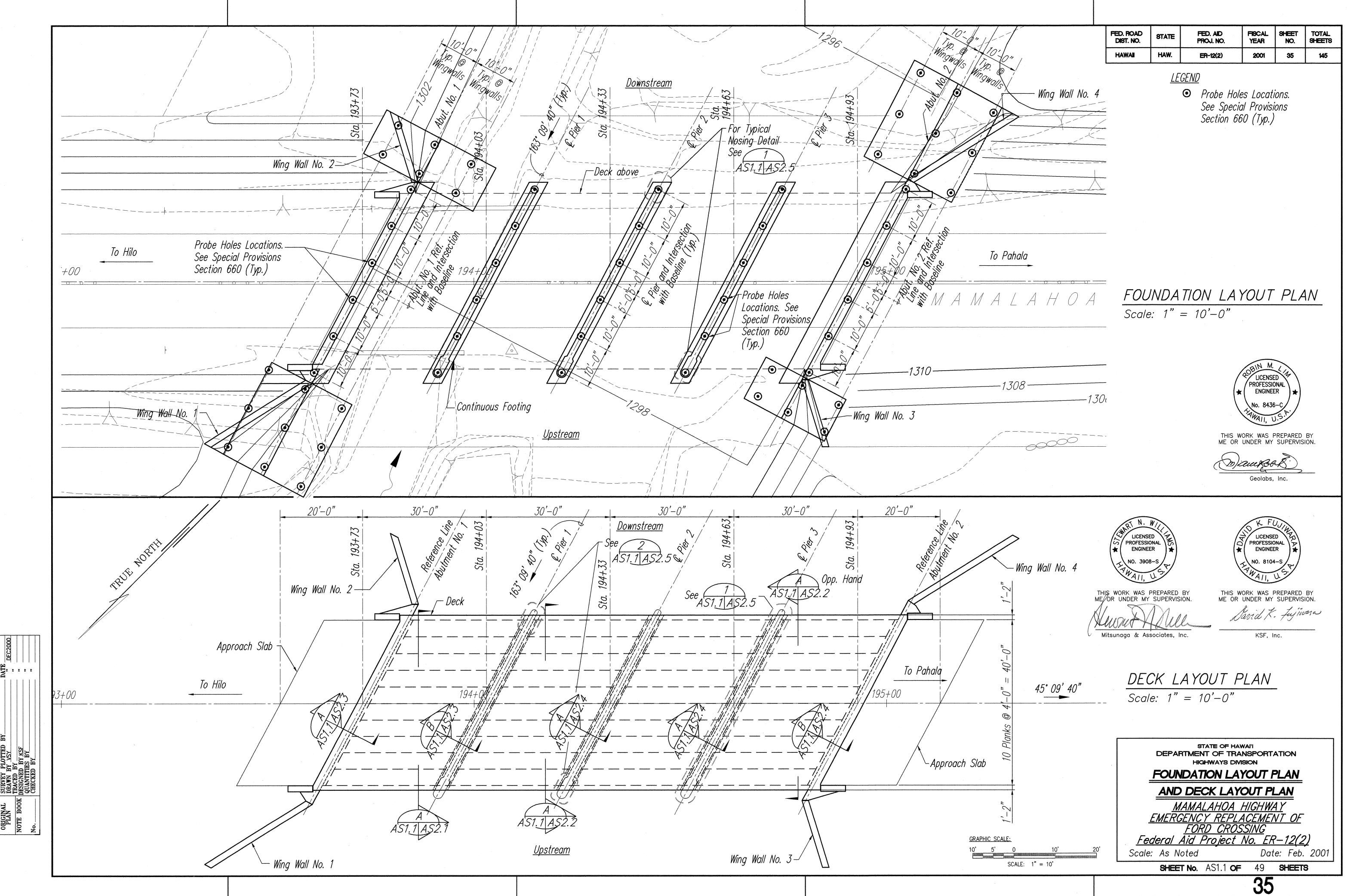
Federal Aid Project No. ER-12(2) Scale: As Noted

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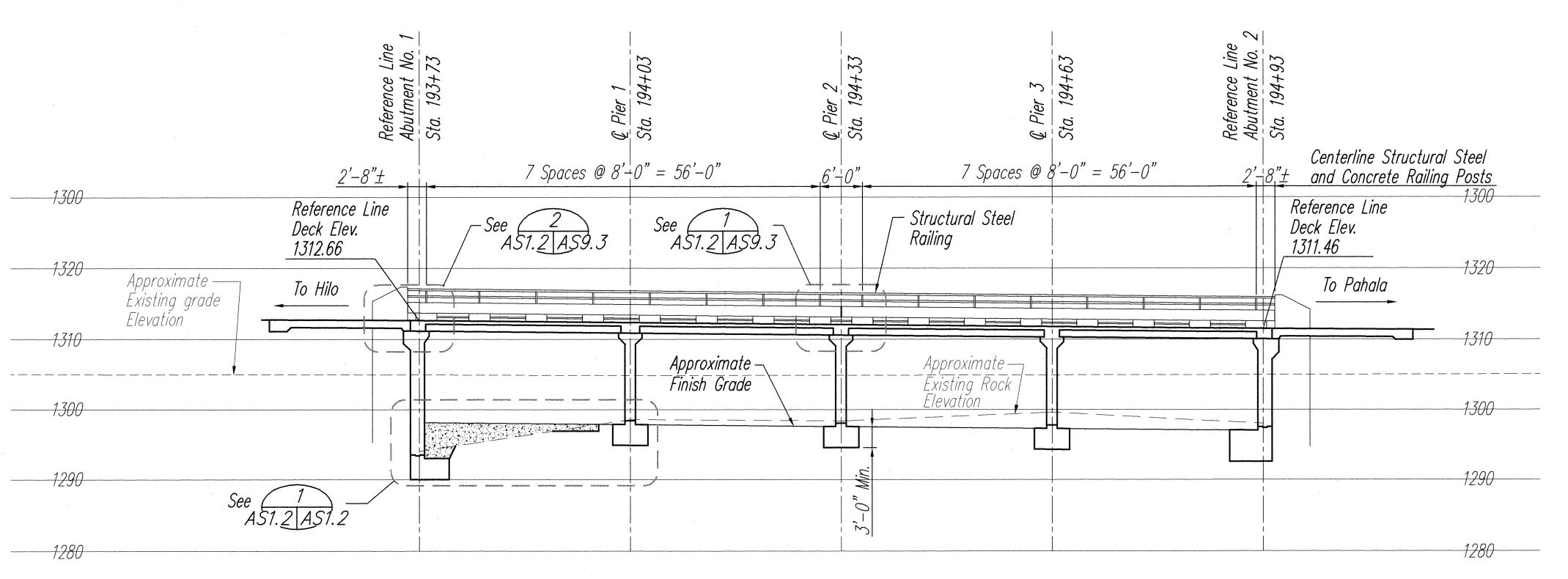


		SYMBOLS A	ND ABBREVIATIONS					FED. ROAD STATE		FISCA YEAF	-
		<u> </u>						HAWAII HAW.	ER-12(2)	2001	
<i>@</i>	At	f'c	Specified compressive	ОС, ос	On center						
>	Greater than		strength (28 days)								
		FA	Force account								
		FF	Front face								
		Fin.	Finish								
		Ft.	Foot, feet	P(e)	Effective Prestress Force						
<i>Abut.</i>	Abutment	Ftg.	Footing	Psf.	Pounds per square foot						
4/t.	Alternate	3		Psi.	Pounds per square inch						
Арргох.	<i>Approximate</i>				, ,						
4 <i>z</i> .	Azimuth										
		Galv.	Galvanize								
		Gr., grd.	Ground, grade	R	Radius						
		, ,		Rdwy.	Roadway						
Bal.	Balance			Ref.	Reference						
Beg.	Begin			Reinf.	Reinforcement						
Bet.	Between			Req., Req'd.	Required						
<i>Bm.</i>	Beam	H, h	Height	Ret.	Retaining						
Bot.	Bottom	Horiz.	Horizontal		J	j.		•			
Brg.	Bearing	HS	High strength								
_	-	Hwy.	Highway								
		,									
				Sect.	Section				× .		
				SF	Square foot						
	Center line			Sht.	Sheet						
ÖIP	Cast in place	Int.	Interior	Spcg.	Spacing						
71., clr.	Clear	Inv.	Invert	Spcs.	Spaces						
LSM	Controlled Low Strength Material			Sta.	Station						
Ponc.	Concrete			Std.	Standard						
Col.	Column			Stirr.	Stirrup						
Cont.	Continue			Str.	Stream	. •					
Corresp.	Corresponding	Jt.	Joint	Struct.	Structure						
CRM	Cement Rubble Masonry			Symm.	Symmetrical						
CY	Cubic yards			- Oymmi	Syrrir our						
Pet.	Detail	/	Length	Transv.	Transverse						
Dim.	Dimension	Lbs.	Pounds	Тур.	Typical						
		LF	Linear foot	196.	1,701041						
		Longit.	Longitudinal								
		LS	Lump sum						-		
			Lanny Jann								
	Each			Vert.	Vertical						
F	Each face										
7., elev.	Elevation									\A/ A ***	
- q.	Equal, equally	Мах.	Maximum			WART N. WILL	X FUUIN	DEPAR	STATE OF HAW TMENT OF TRAI	ANSPORT	₽.
W	Each way	Min.	Minimum			LICENSED PROFESSIONAL SALES	LICENSED PROFESSIONAL ENGINEER		HIGHWAYS DIVIS	ISION	
XC.	Excavation			W/C	Water/Cement Ratio	★ ENGINEER NO. 3908-S	1 1	SYMBO	LS AND ABE	BREVI	/[
xist.	Existing			WW	Wingwall	NO. 3908-S	NO. 8104-S				***************************************
хр.	Expansion					AII, U.	AII, U.		AMALAHOA I	HIGHM	<u> </u>
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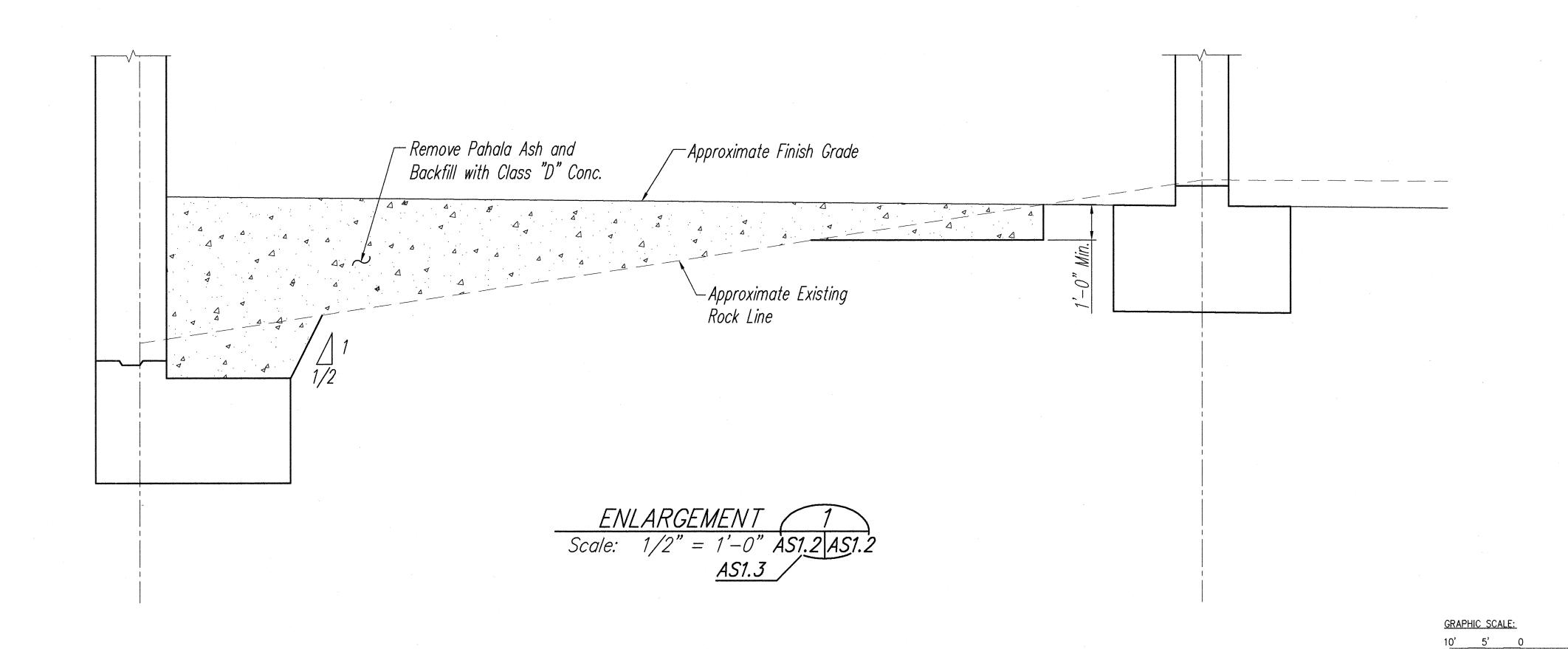
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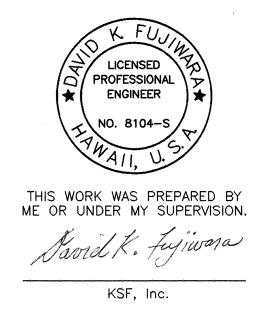


~	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
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LONGITUDINAL SECTION ALONG CENTERLINE STRUCTURE Scale: 1" = 10'-0"





STATE OF HAWAI'I
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

LONGITUDINAL SECTION ALONG

CENTERLINE STRUCTURE

MAMALAHOA HIGHWAY

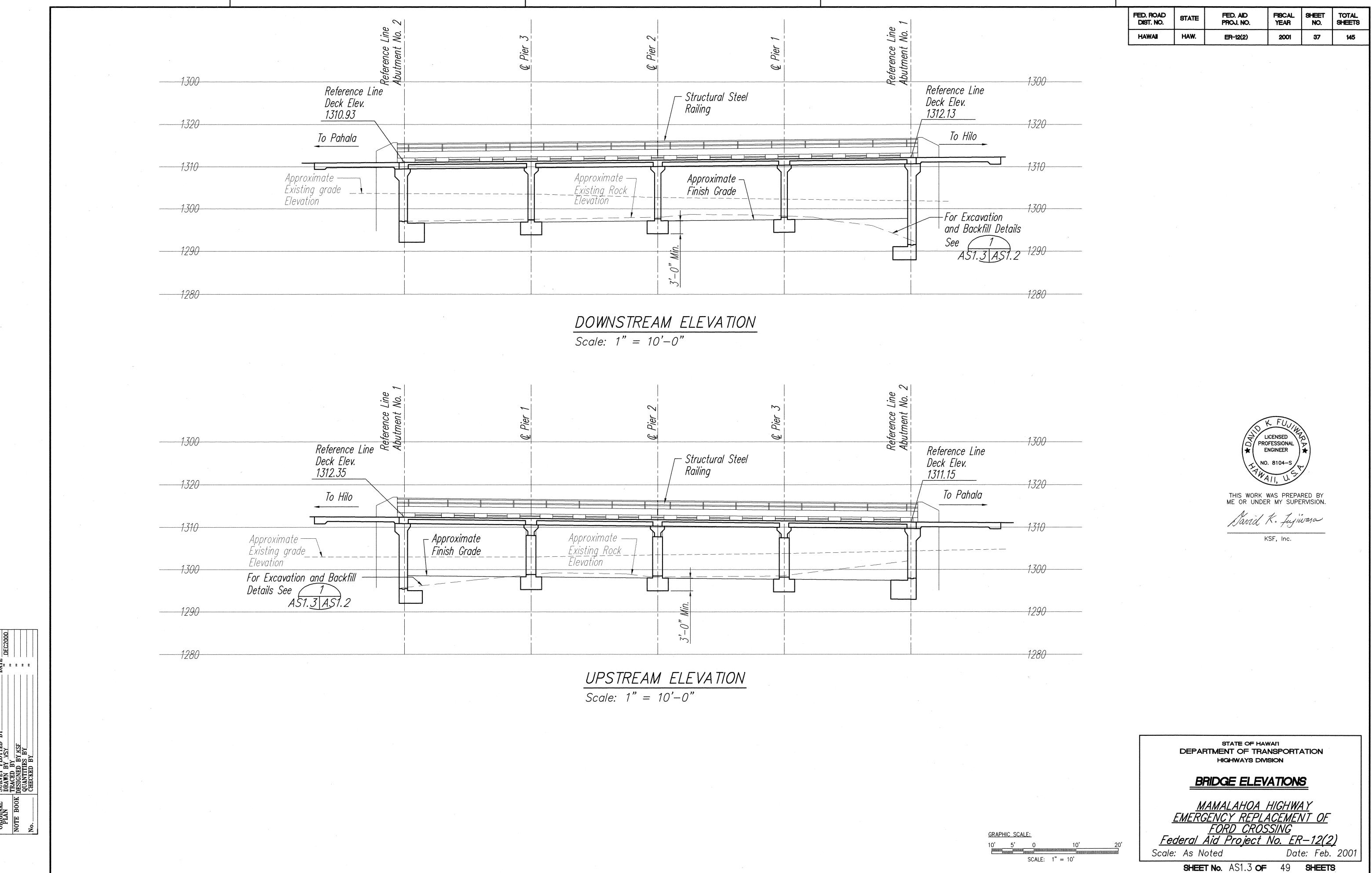
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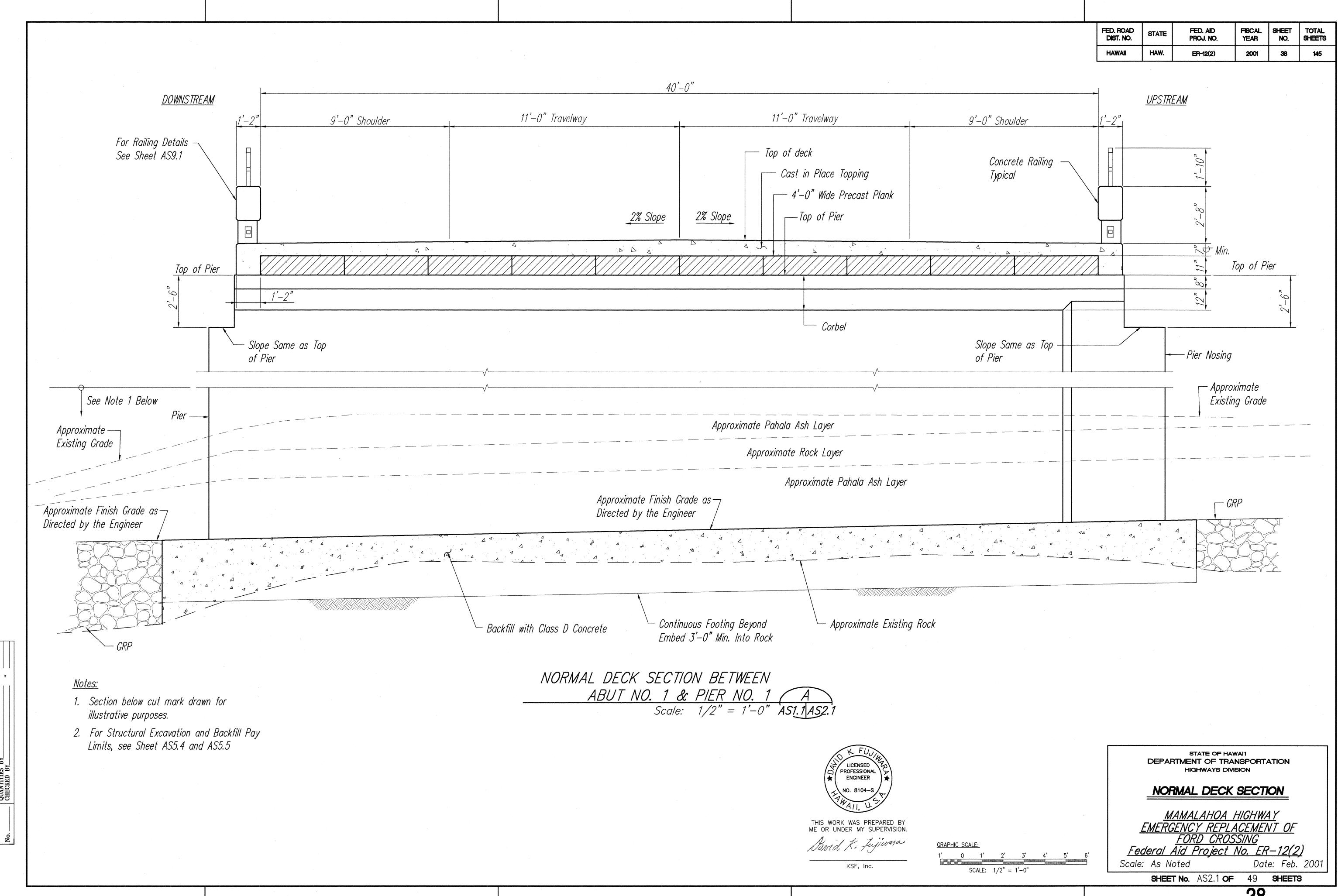
FORD CROSSING

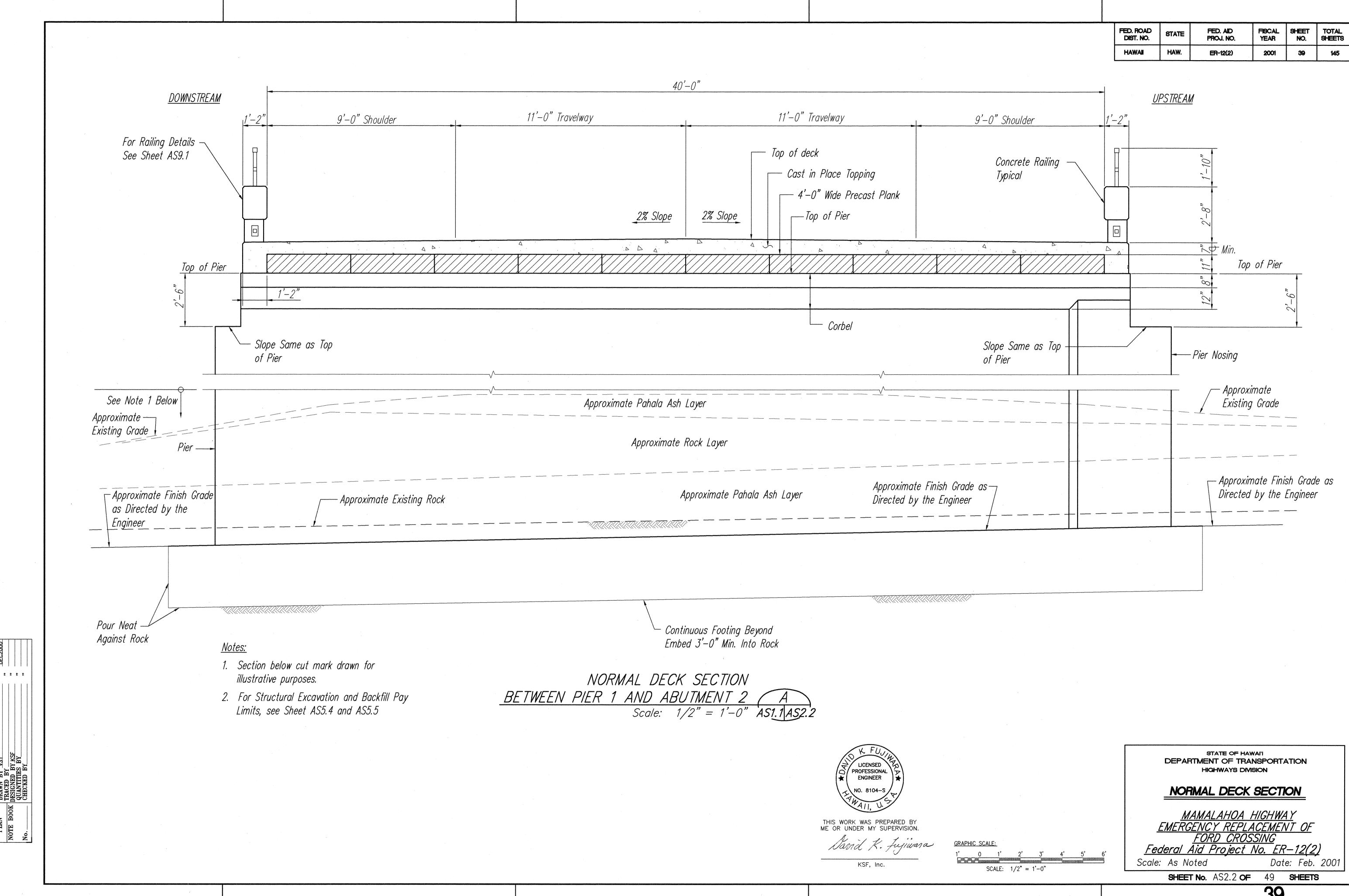
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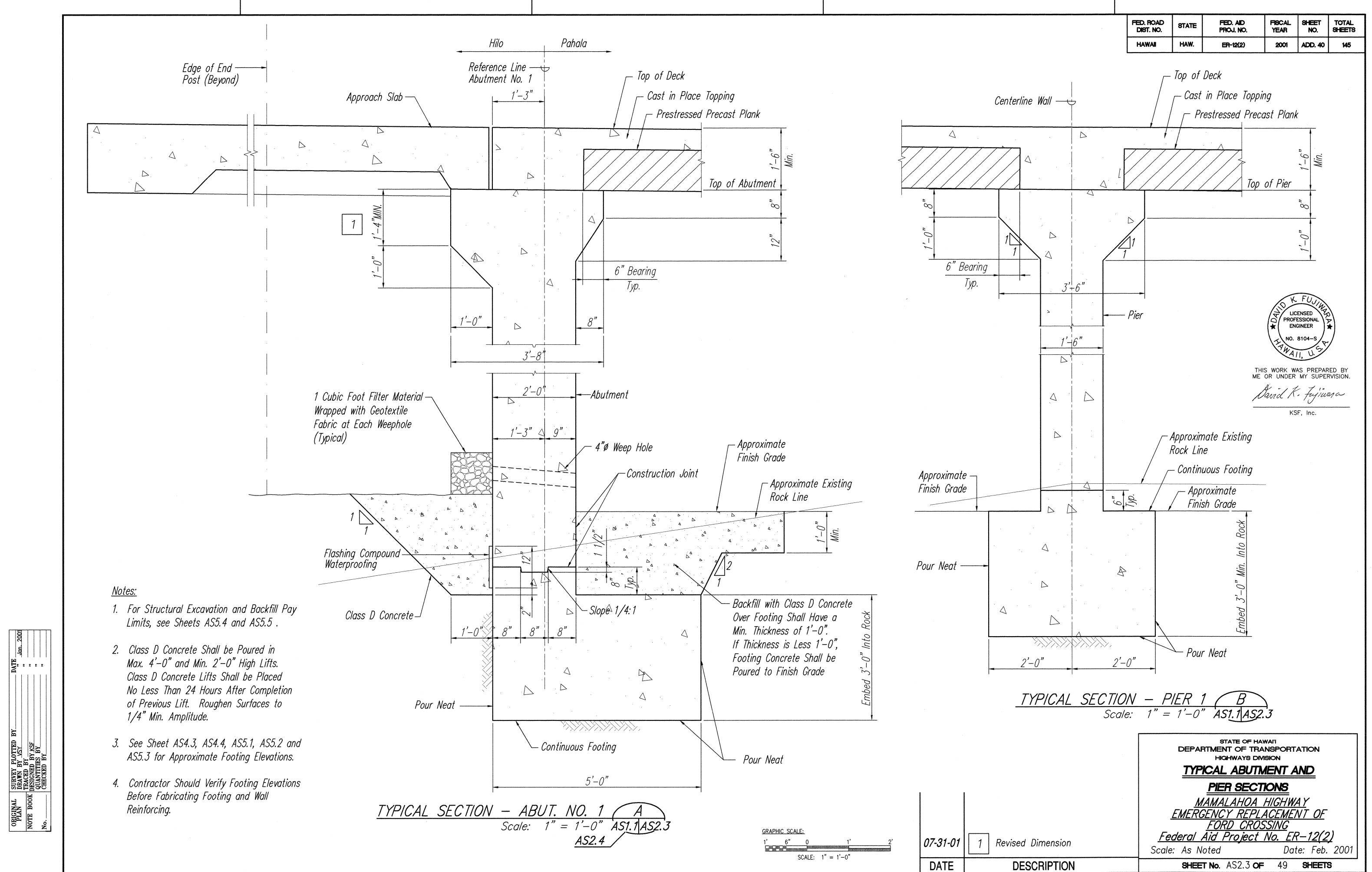
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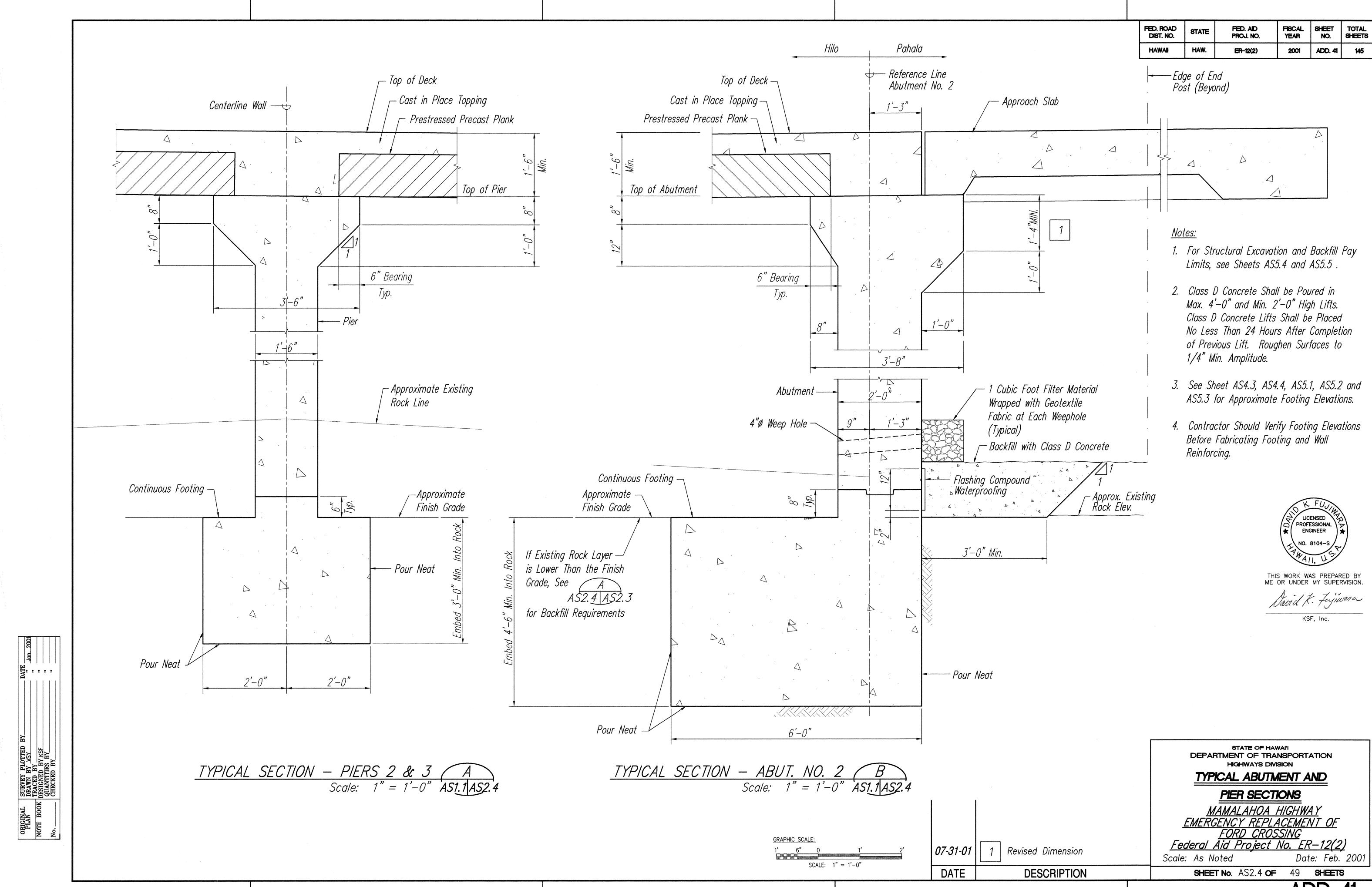


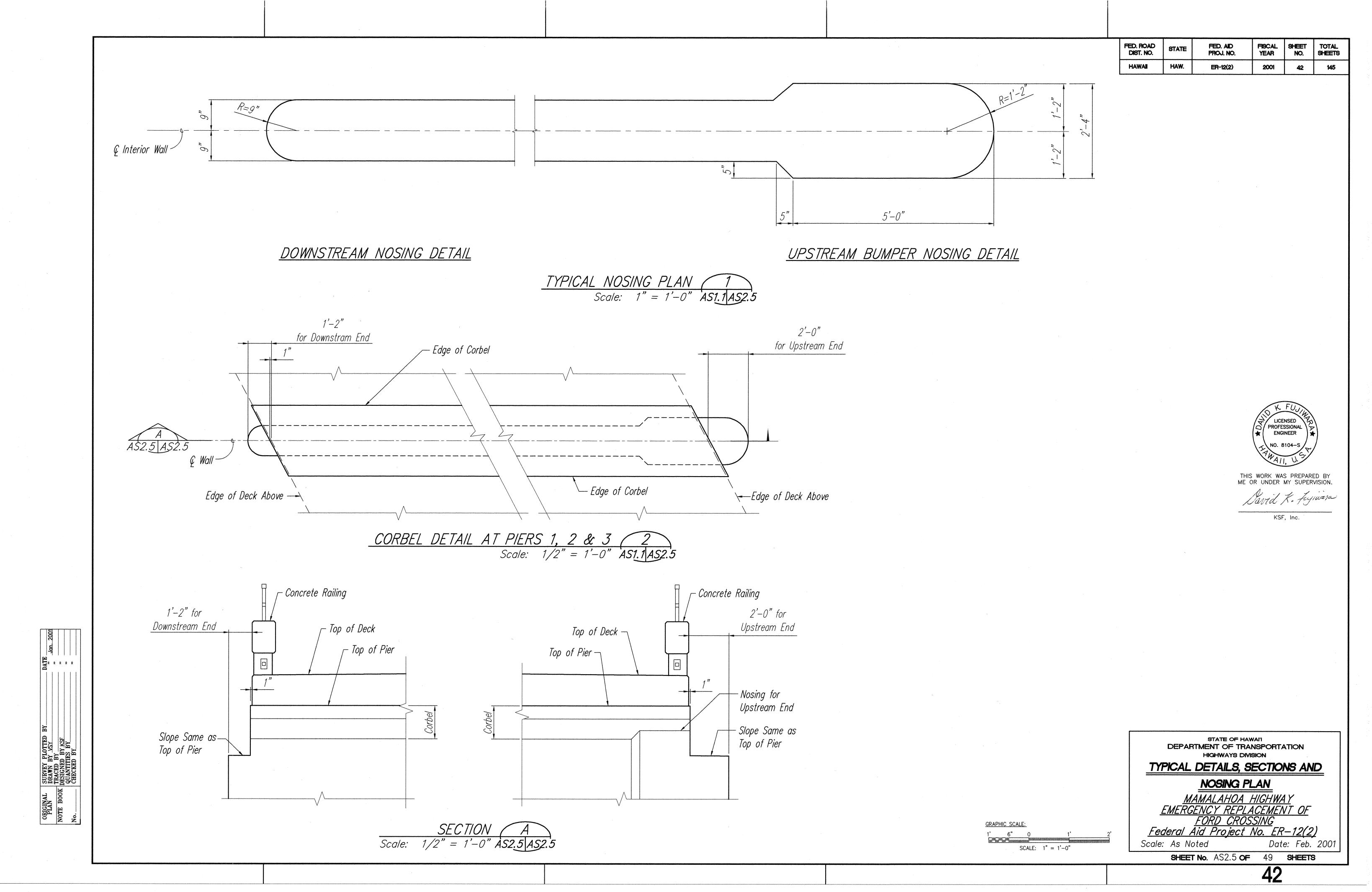


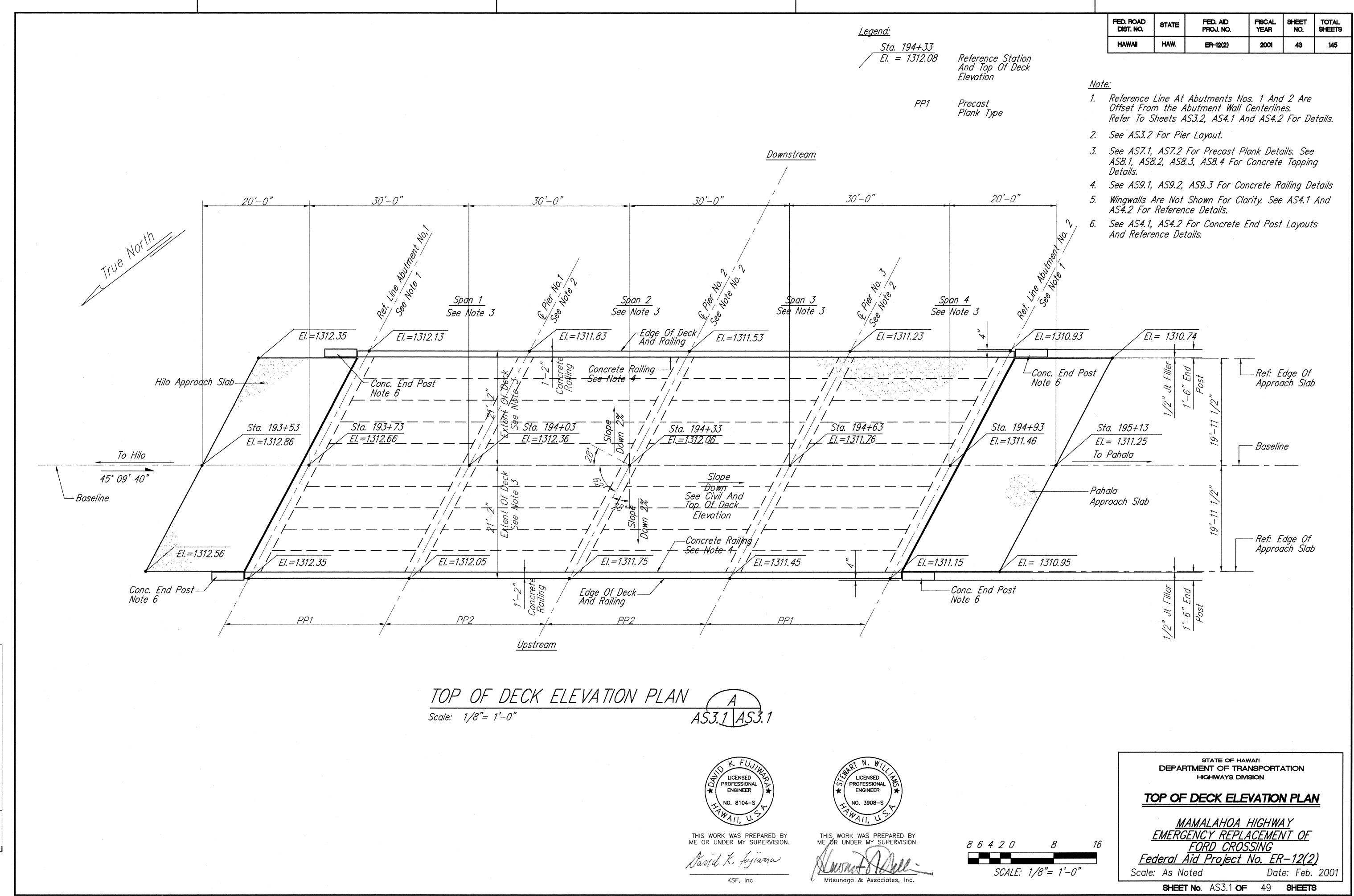




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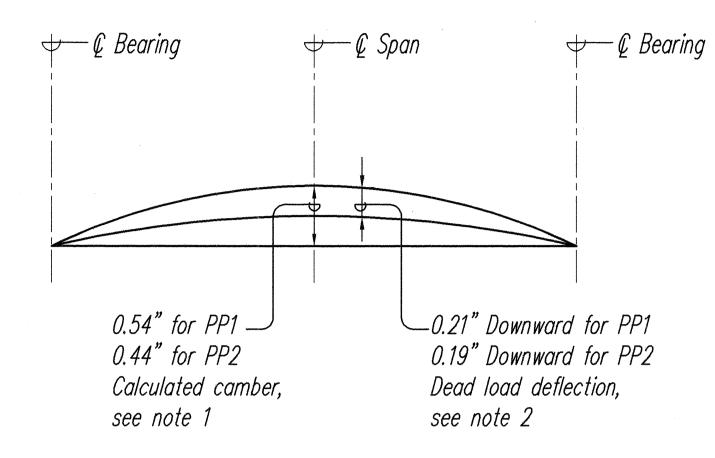




FED. ROAD DIST. NO. STATE FED. AID PROJ. NO. FISCAL YEAR NO. SHEETS

HAWAII HAW. ER-12(2) 2001 44 145

<u>UPSTREAM</u>



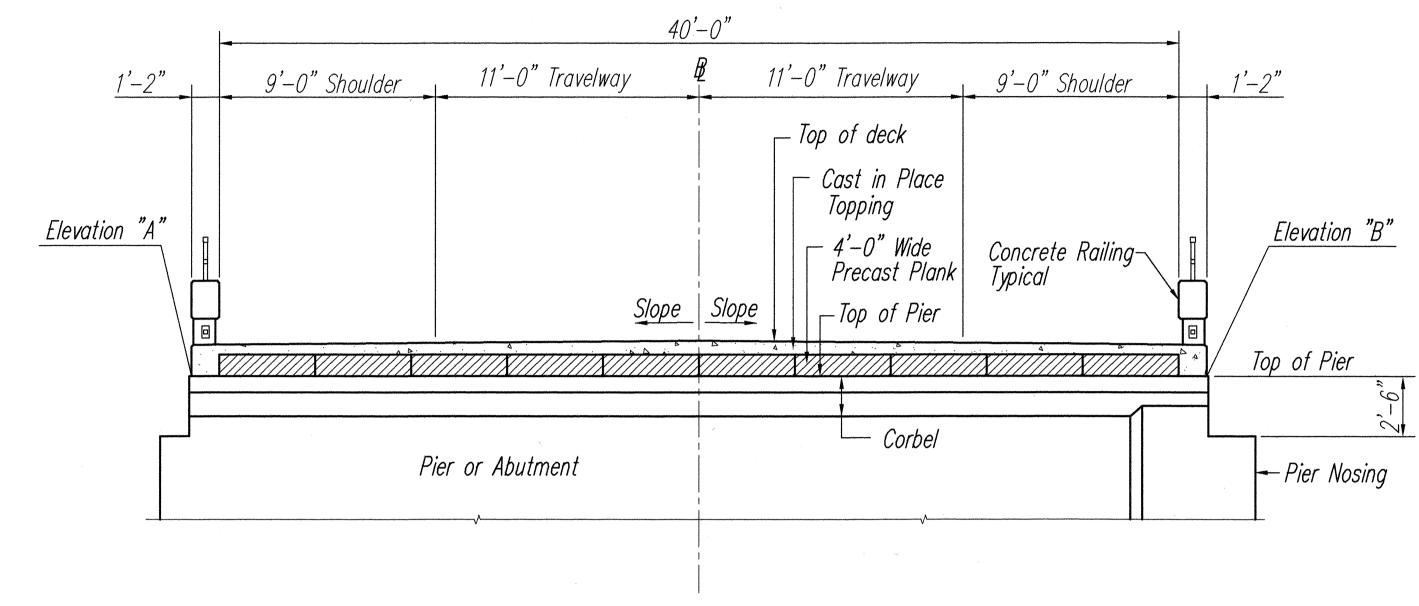
CAMBER AND DEFLECTION NOTES:

- 1. The calculated camber includes the effect of the prestress force and the weight of the girder at time of erection. Positive values shown for calculated camber indicate a net upward deflection. The calculated camber value has been multiplied by creep factors to approximate the effect of camber growth and concrete creep. The actual camber shall not exceed the calculated camber by more than 1/2".
- 2. The dead load deflection is for the weight of the topping.
- 3. All cambers and deflections are in inches.



LOCA TION	ELEVATION "A"	ELEVATION "B"
Abut. 1	1310.56	1310.78
Pier 1	1310.26	1310.48
Pier 2	1309.96	1310.18
Pier 3	1309.66	1309.88
Abut. 2	1309.36	1309.58



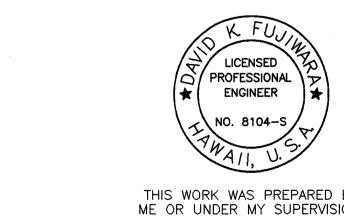


NOTES:

- 1. Top of Pier and Abutment slopes shall be constant from Elevation "A" to Elevation "B".
- 2. Top of Pier and Abutment also slope parallel with the bridge and shall have a constant slope of 1% in that direction. The higher elevation is on the Hilo side.
- 3. Elevations "A" and "B" accounts for camber.
- 4. Elevations are at centerline Pier and Abutment Reference Line.

NORMAL DECK SECTION AT PIER A

Scale: 1/4" = 1'-0" S3.2 S3.2



Sand K. Fujiwasa

GRAPHIC SCALE:

1' 0 1' 2' 3'

KSF, Inc.

SCALE: 1/2" - 1'-0'

HIC SCALE:

0 1' 2' 3' 4' 5' 6'

SCALE: 1/2" = 1'-0"

DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIMISION

TOP OF PIER ELEVATIONS

AND CAMBER DIAGRAM

MAMALAHOA HIGHWAY

EMERGENCY REPLACEMENT OF

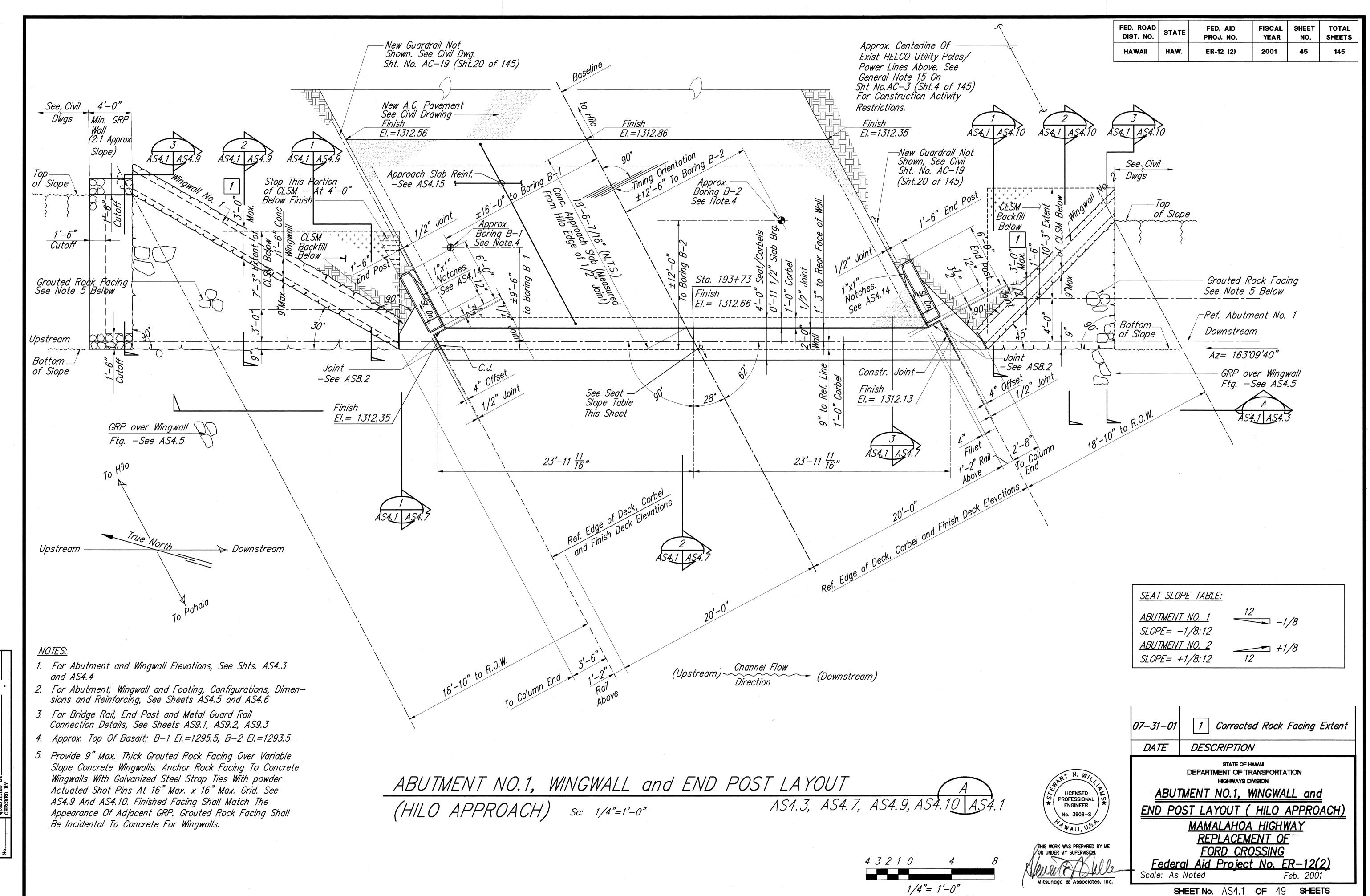
FORD CROSSING

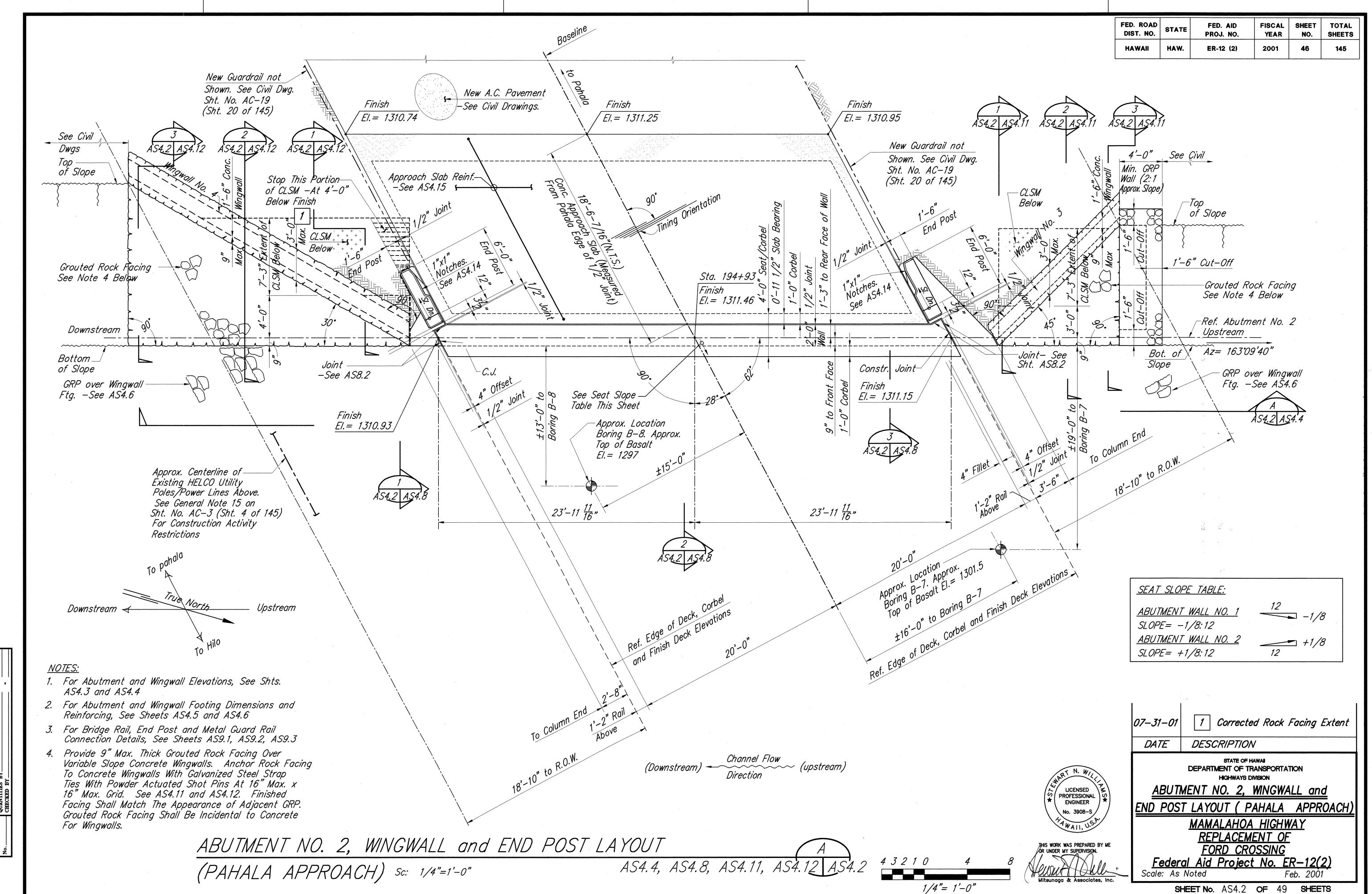
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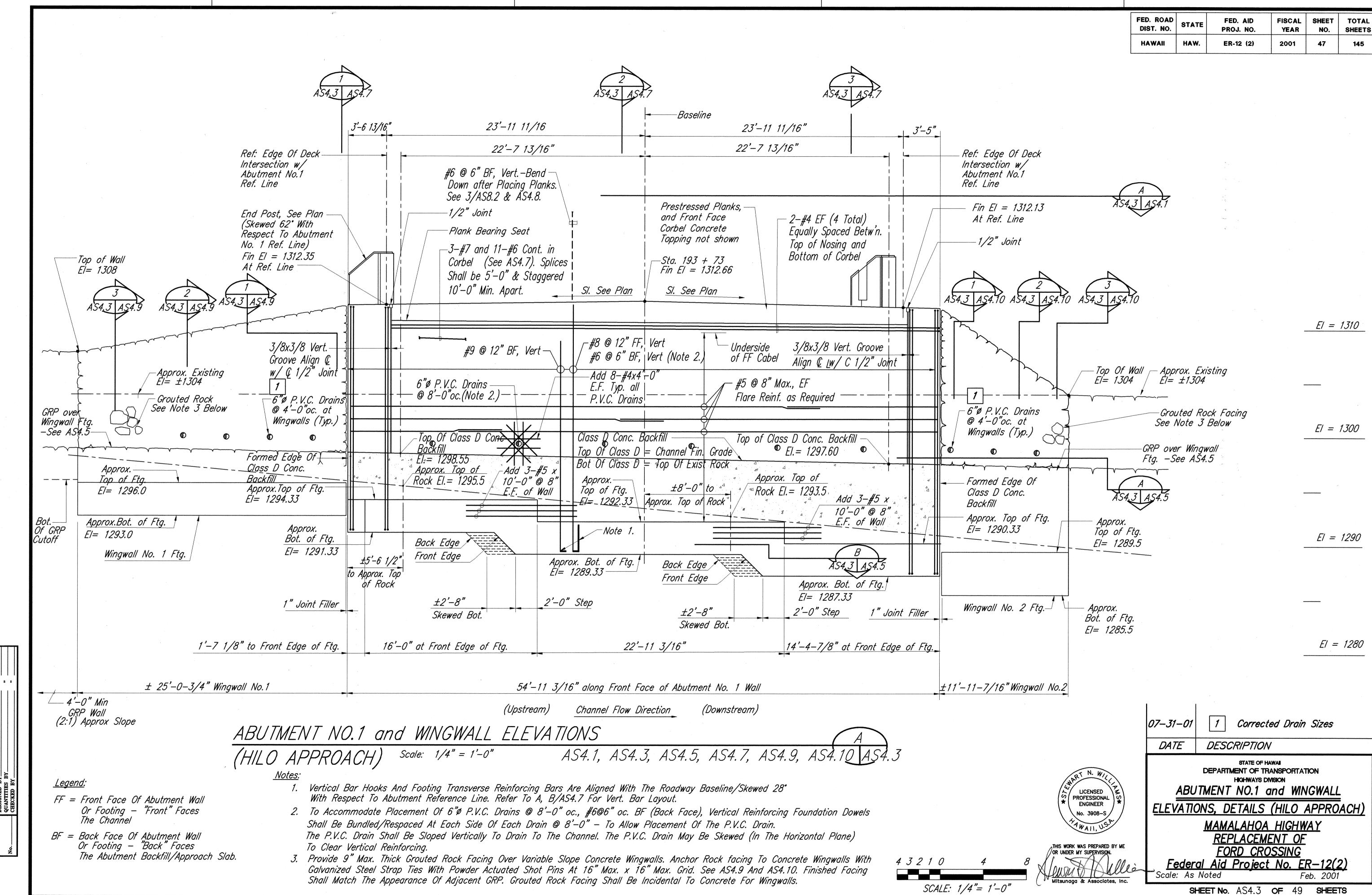
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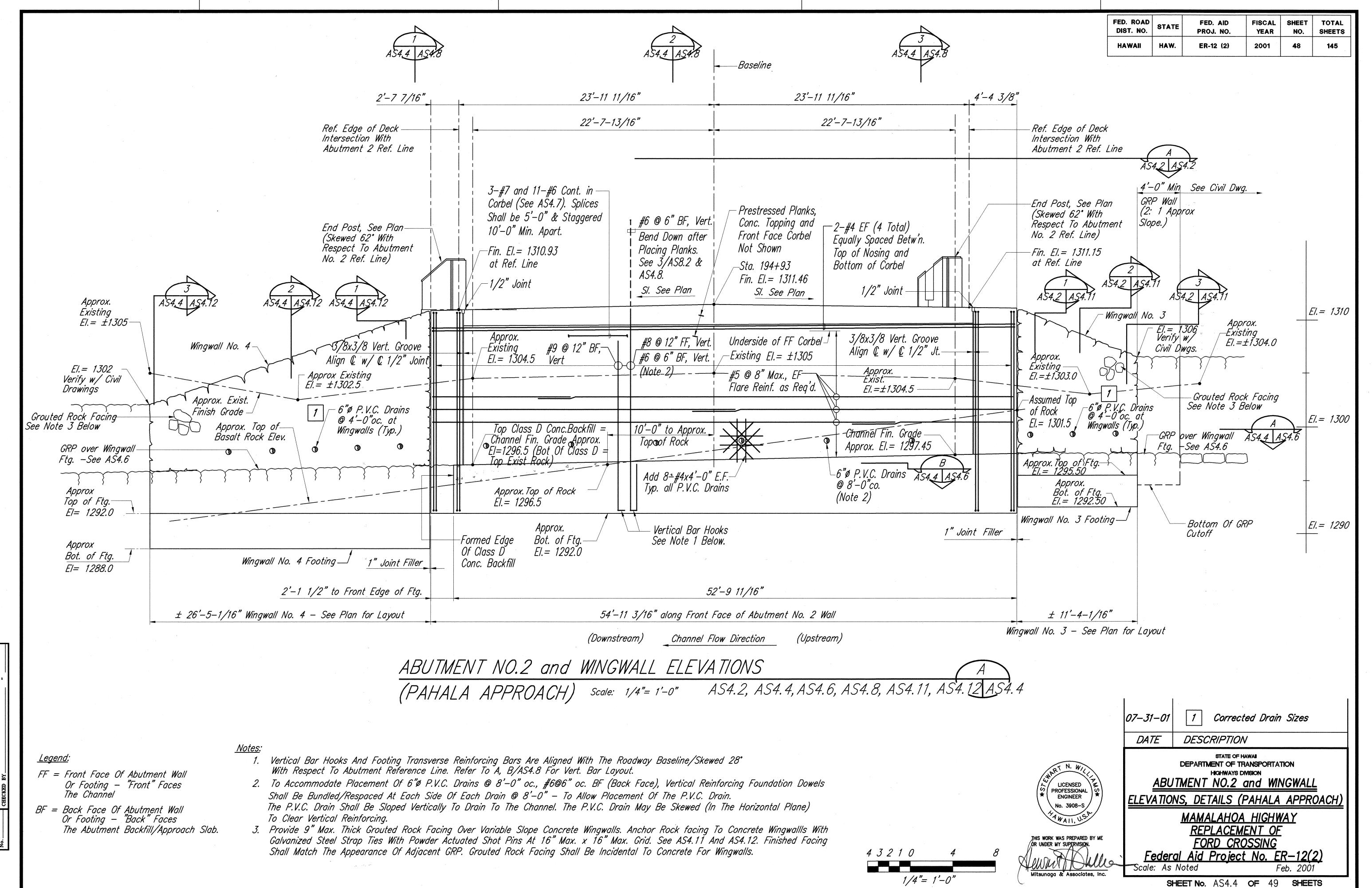
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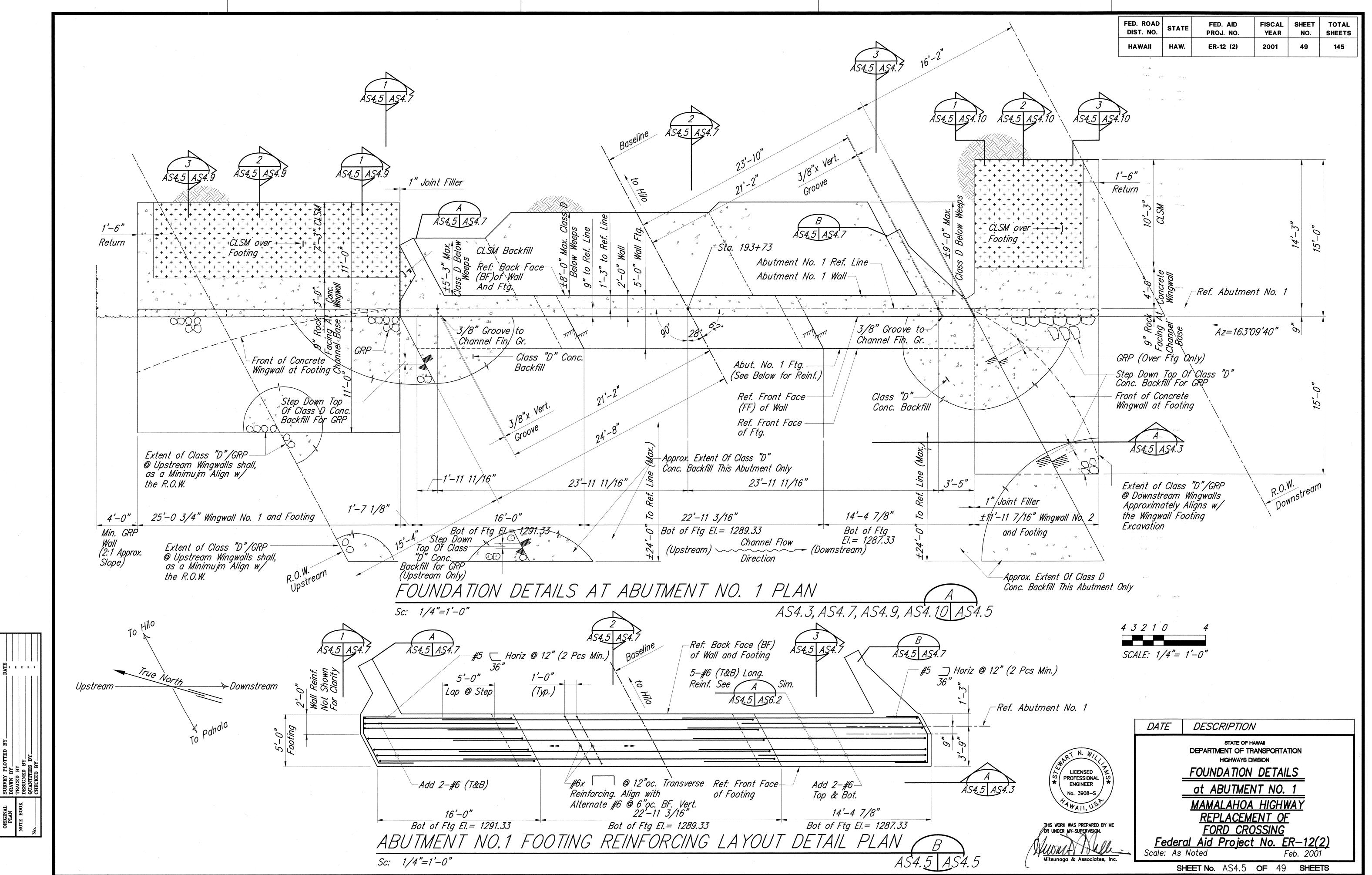
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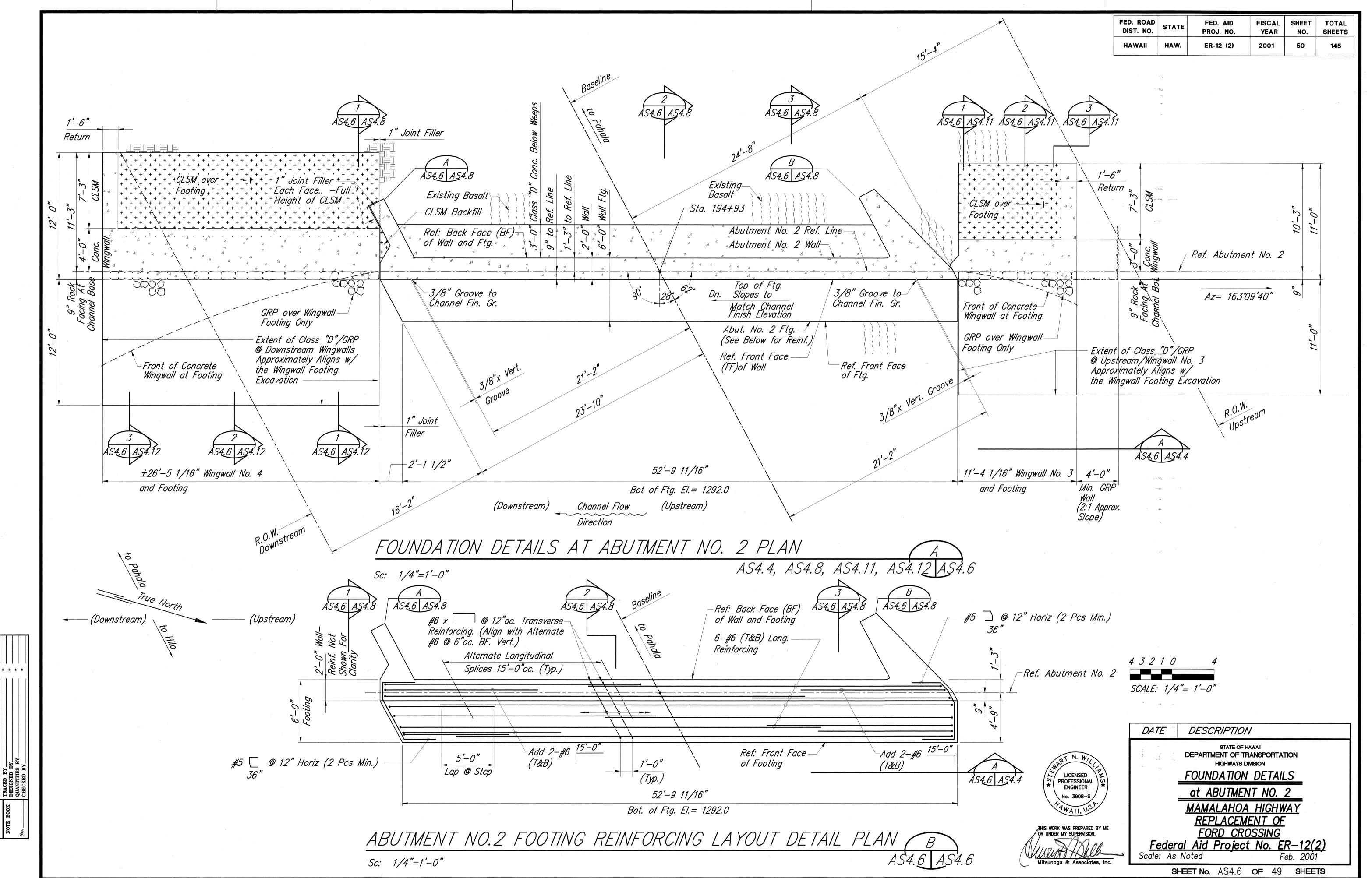


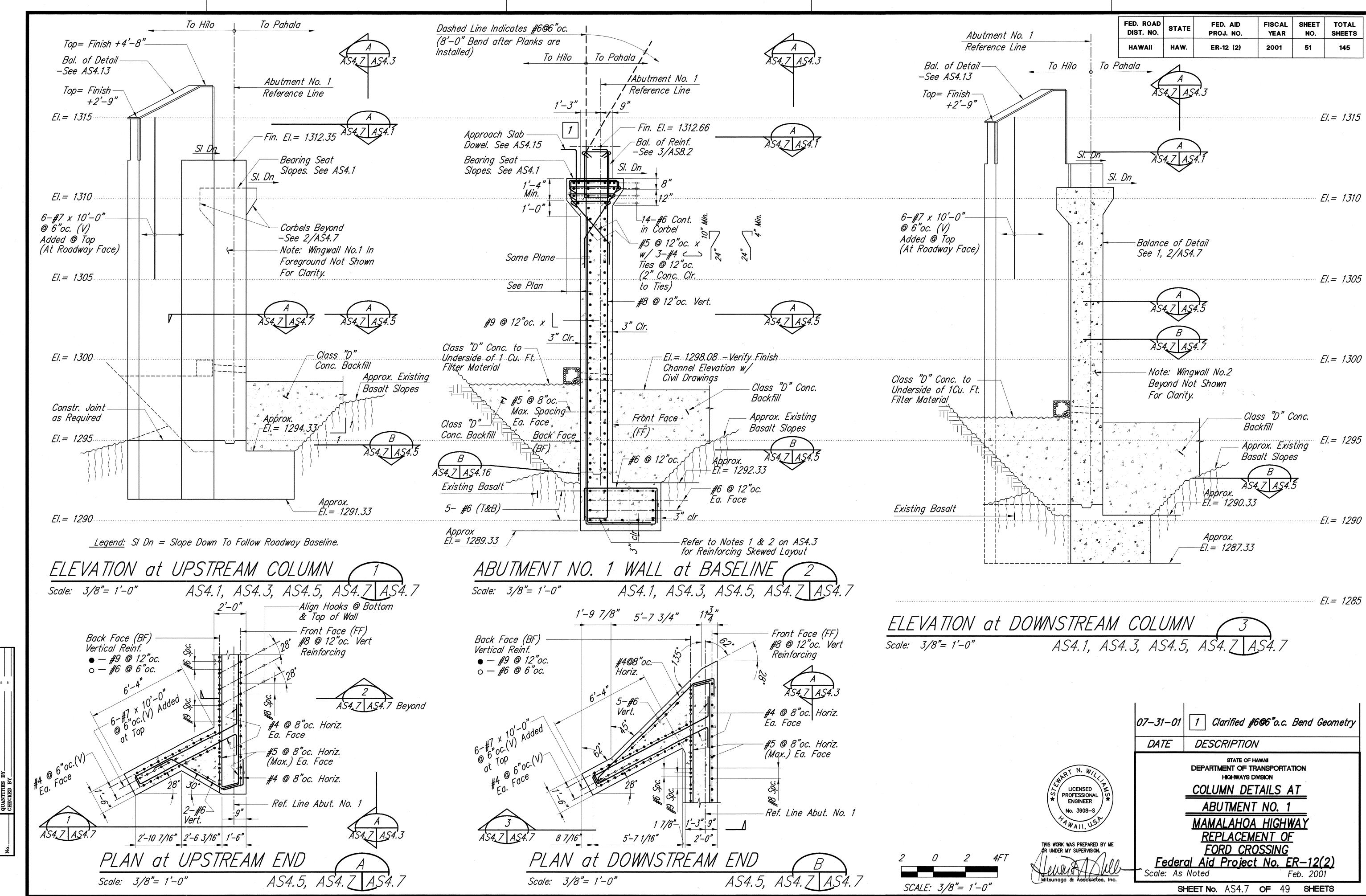


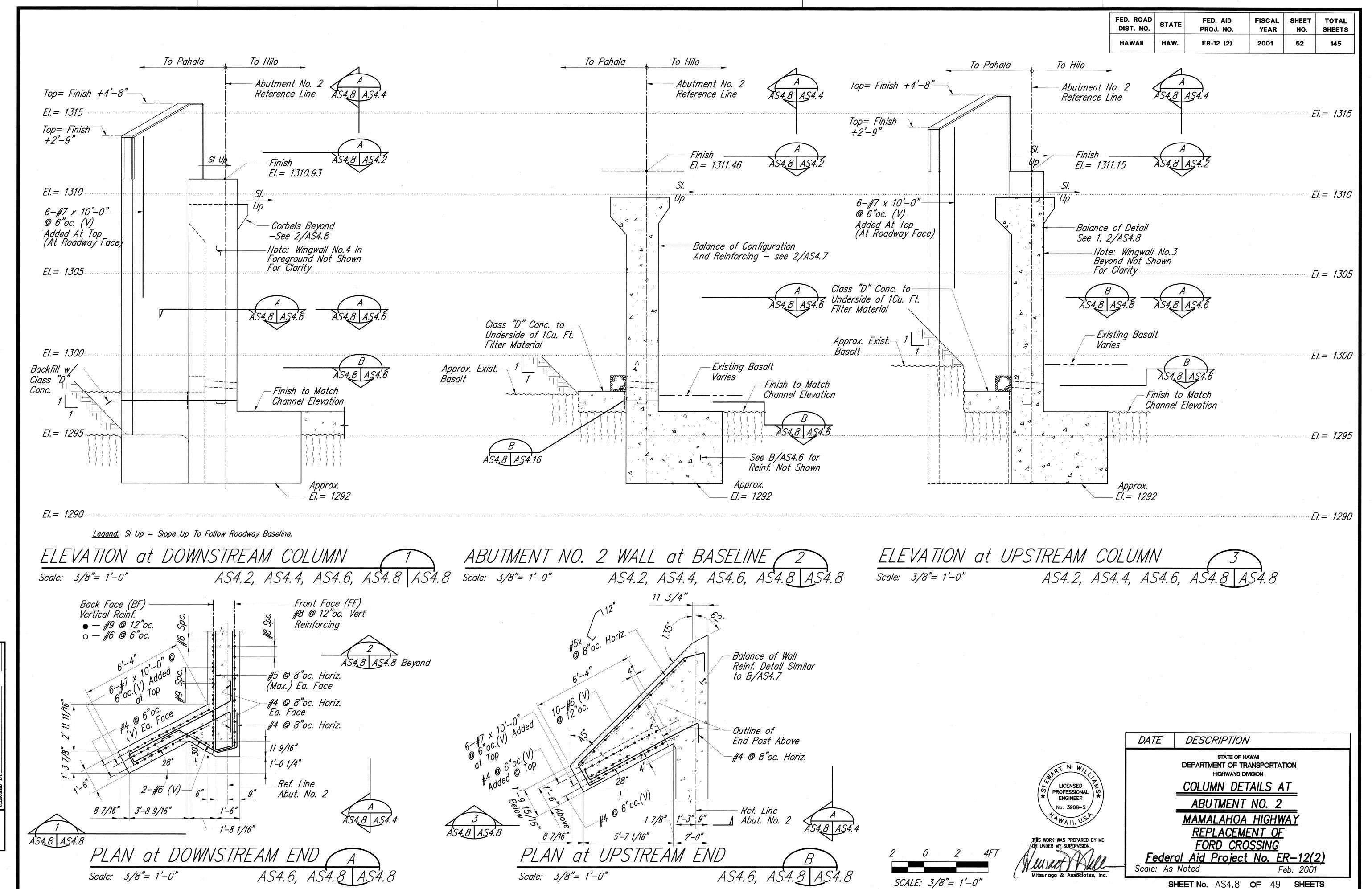


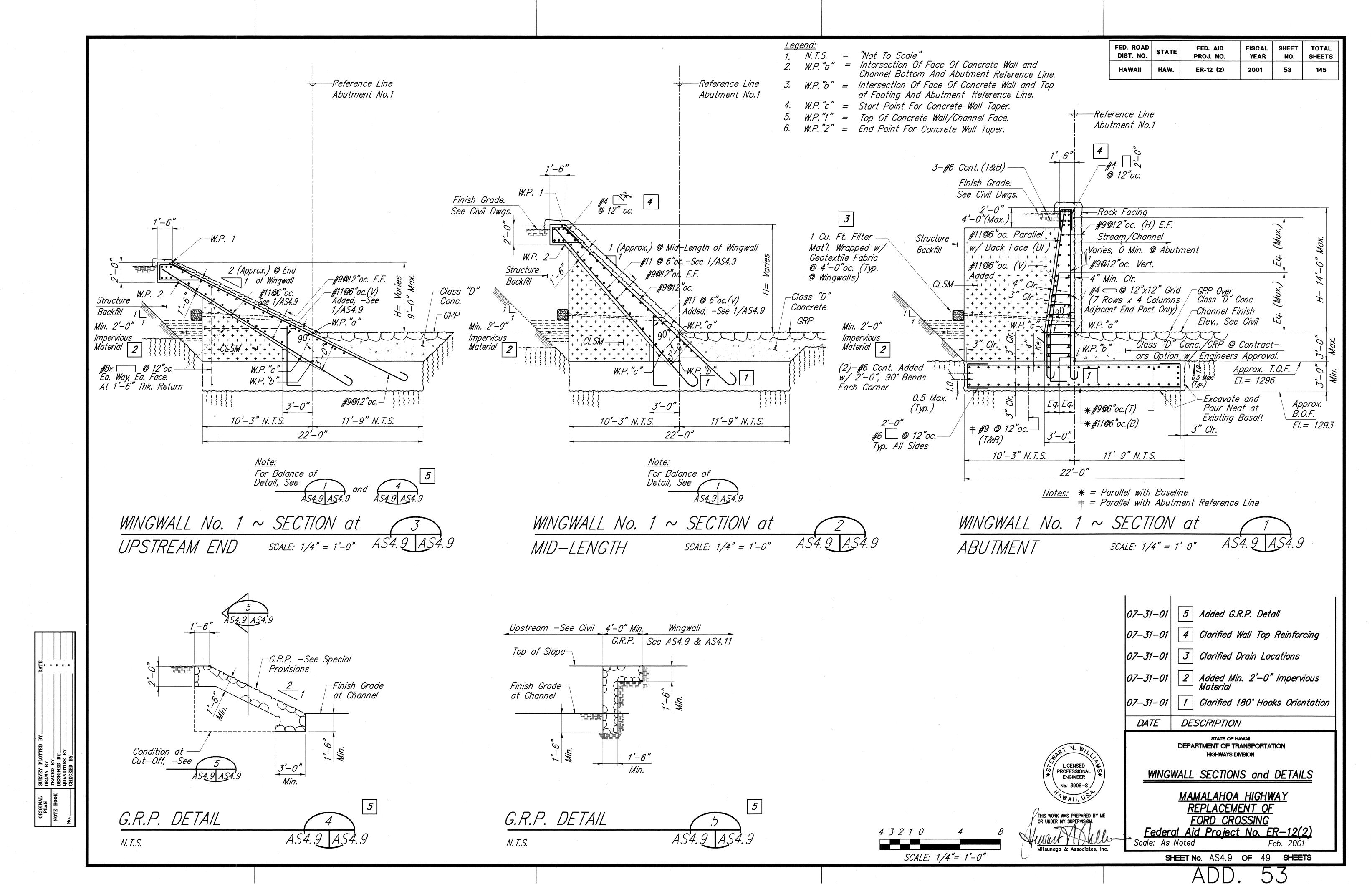


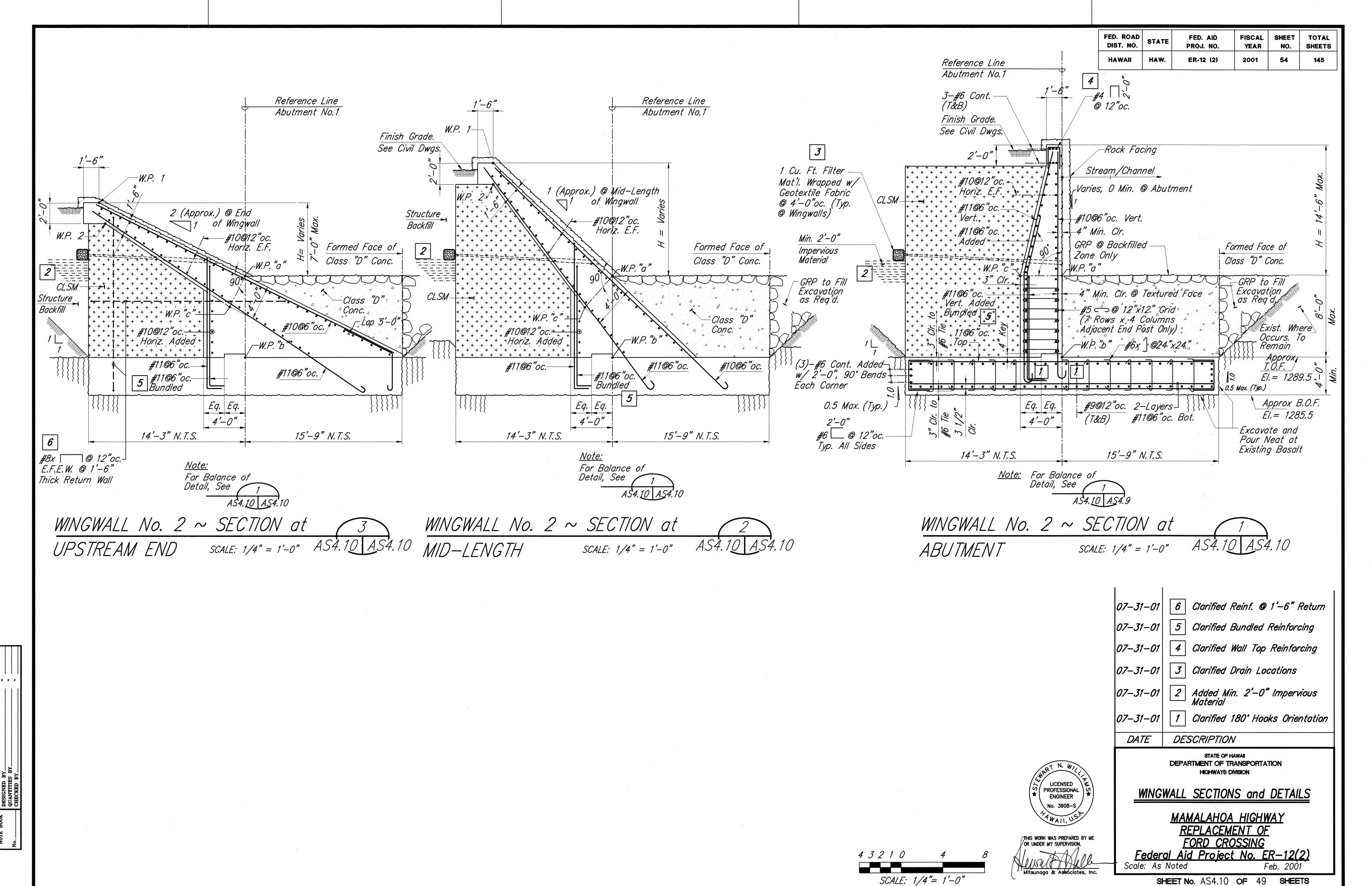




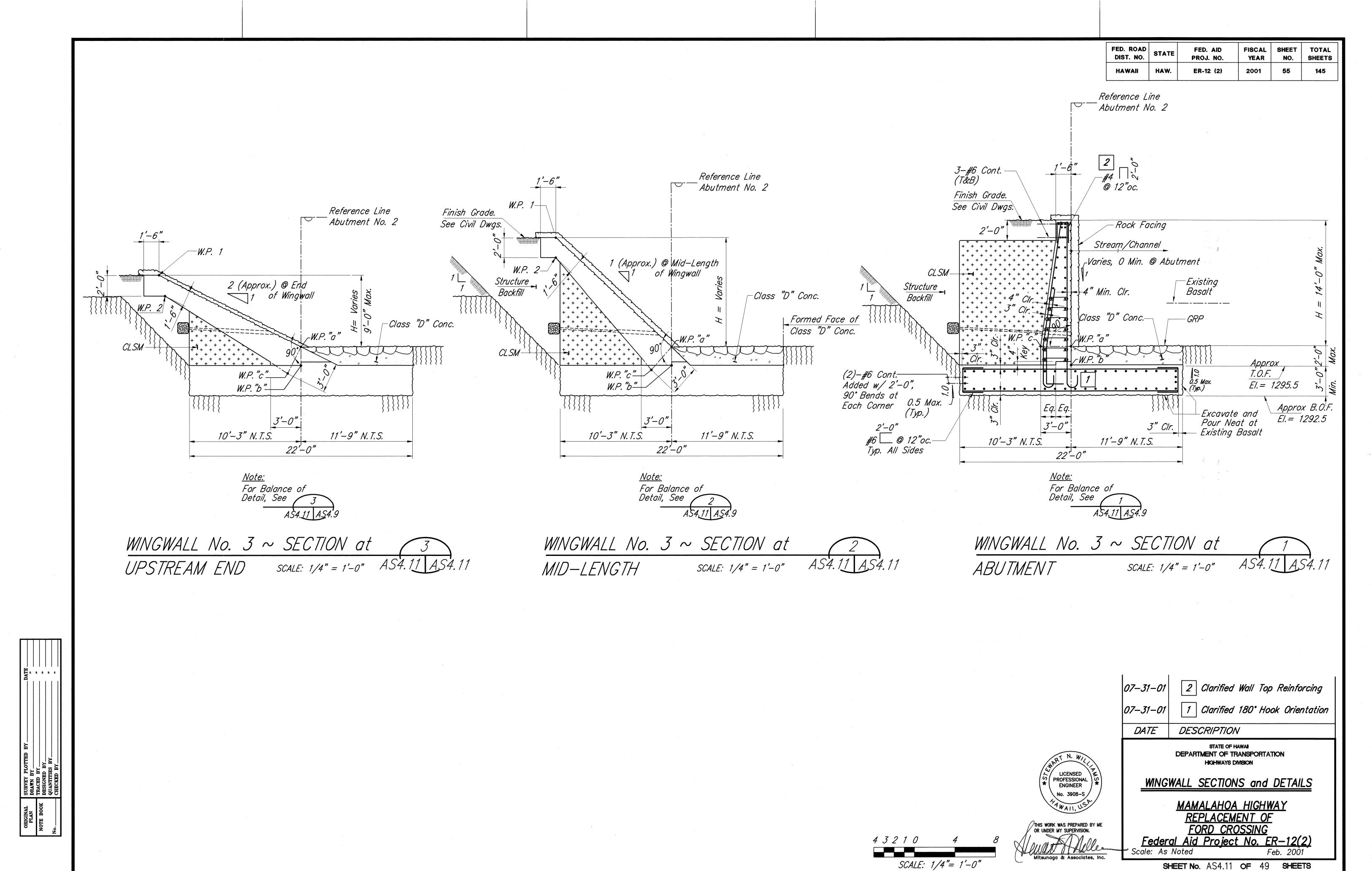




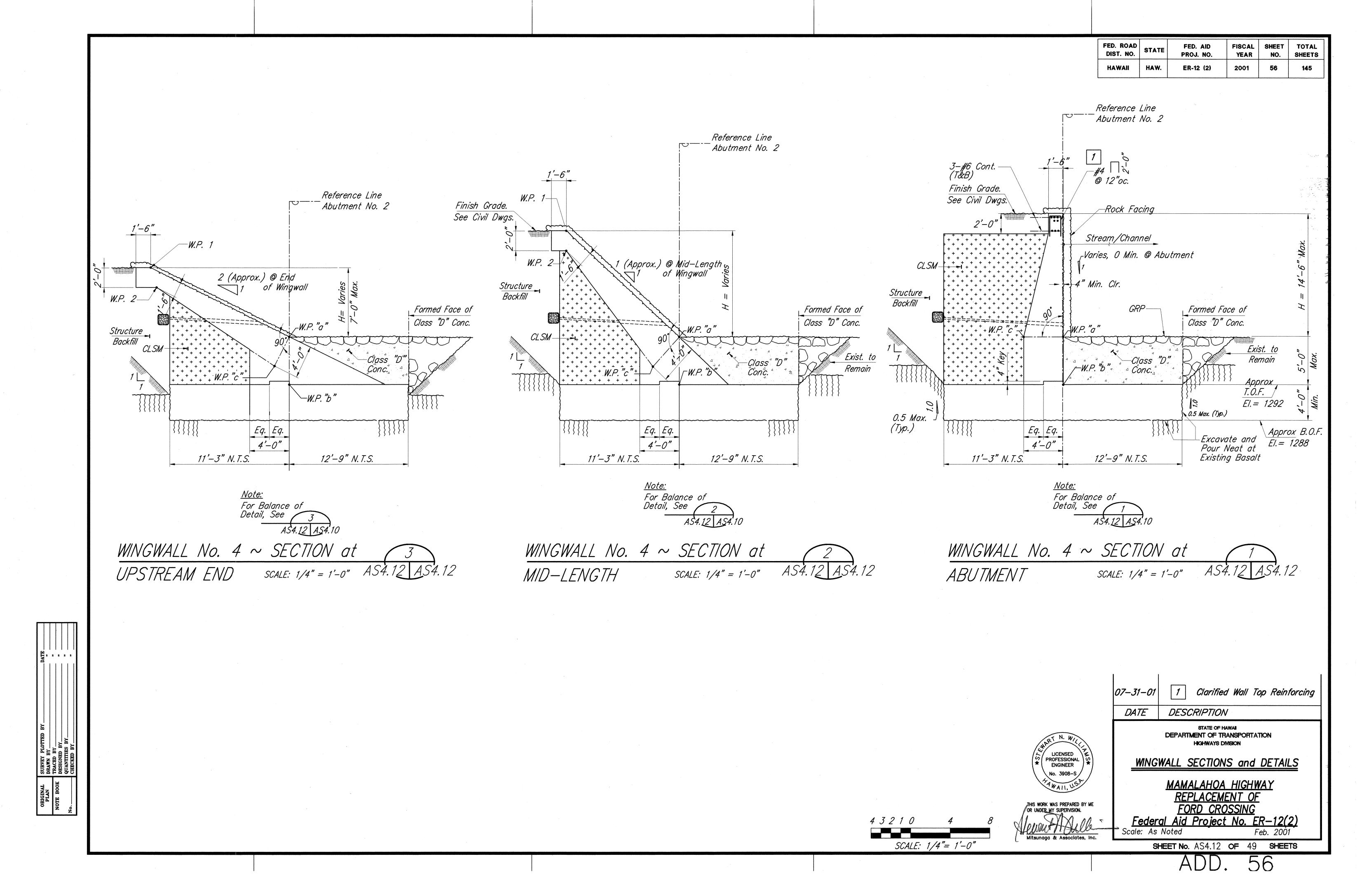




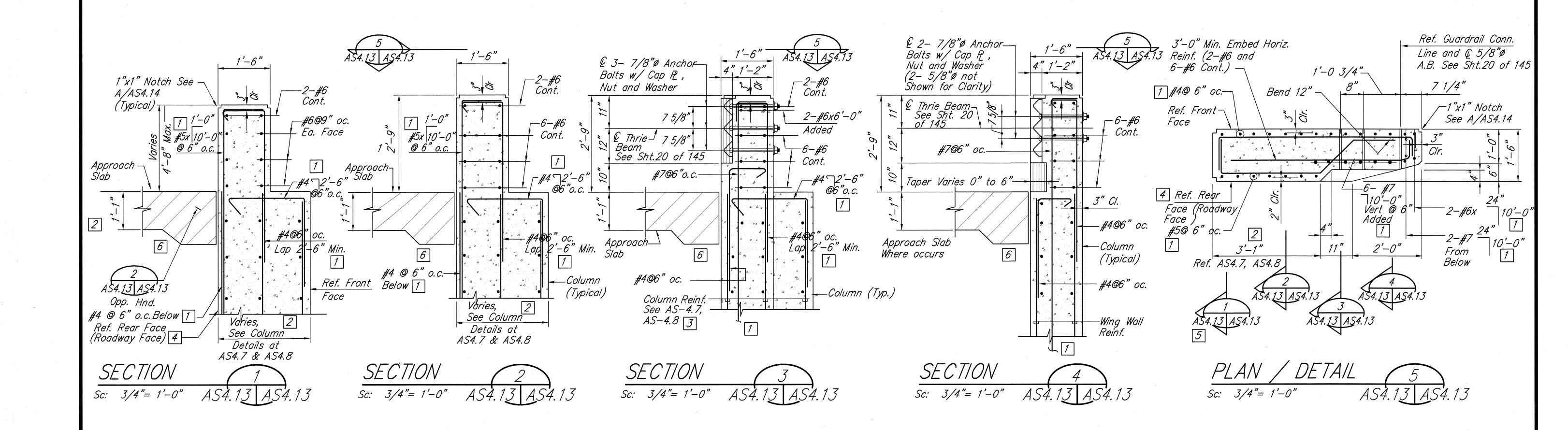
ADD 54

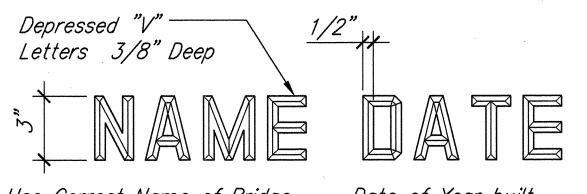


SHEET No. AS4.11 OF 49 SHEETS 5



FED. ROAD	STATE	FED. AID	FISCAL	SHEET	TOTAL
DIST. NO.		PROJ. NO.	YEAR	NO.	SHEETS
HAWAII	HAW.	ER-12 (2)	2001	57	145





<u>Use Correct Name of Bridge</u> NOTES: (See Note 3 Below)

Date of Year built (See Note 3 Below)

- 1. Unless otherwise Directed by The Engineer, The Bridge Name and Date built shall be placed at the "Trailing" End Post on each side of the roadway.
- 2. Exact details and spacing of letter and figures and location shall be as directed by the Engineer. Gothic letters and figures approximating dimensions shown will be acceptable if approved by the Engineer.
- 3. Unless Otherwise Directed By The Engineer. The Bridge Name Shall Be MAKAKUPU BRIDGE. The Date Shall Be 2001.

TYPICAL DETAIL OF LETTERS AND FIGURES AT CONCRETE END POST

BRIDGE IDENTIFICATION DETAIL Not to Scale

SC: 3/4"= 1'-0"

LICENSED PROFESSIONAL

ENGINEER

|07-31-01 6 Clarified Approach Slab Edge 07-31-01 5 Clarified Reference Detail 07-31-01 4 Clarified Roadway Face 07-31-01 3 Corrected Reference Note 07-31-01 2 Clarified End Post Size 07-31-01 1 Clarified Reinforcing

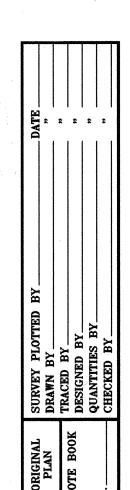
DESCRIPTION

STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

END POST DETAILS

MAMALAHOA HIGHWAY REPLACEMENT OF FORD CROSSING
Federal Aid Project No. ER-12(2)

SHEET No. AS4.13 **OF** 49 SHEETS



Grind To
1/8" RADIUS

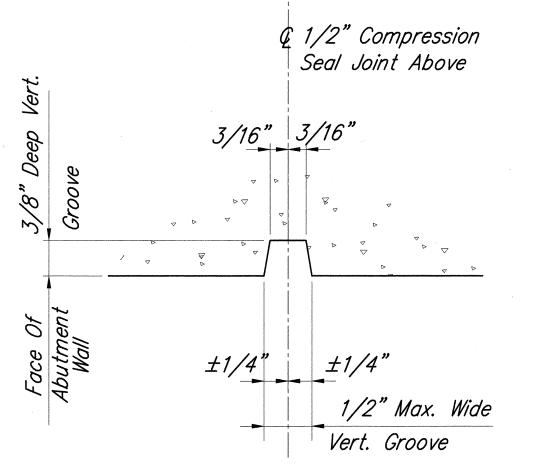
Rail Posts, End Posts

Notch Notes:

- 1. 1"x1" Corner Notches Shall Be Provided At All Corners Of Concrete Bridge Rails, Bridge Rail Posts And Including Top And Vertical Faces Of Concrete End Posts (Except At Vertical Face Of End Post Where Metal Thrie Beam Connection Occurs).
- 2. All Outer Corners Of Notches Shall Be eased By Grinding to A Uniform, Continuous 1/8" Radius
- 3. Color And Texture Of Concrete Shall Match Adjacent Surface Color And Texture.

1"x1" NOTCH DETAIL AT RAILS, RAIL POSTS AND END POSTS
Sc: 3/4" Full Size

AS4.14 AS4.14



<u>Note:</u>

Vertical Groove Shall Not Be Greater

Than 3/8", so That Concete Cover

= 2 5/8" Min. At Abutment Wall

Reinforcina.

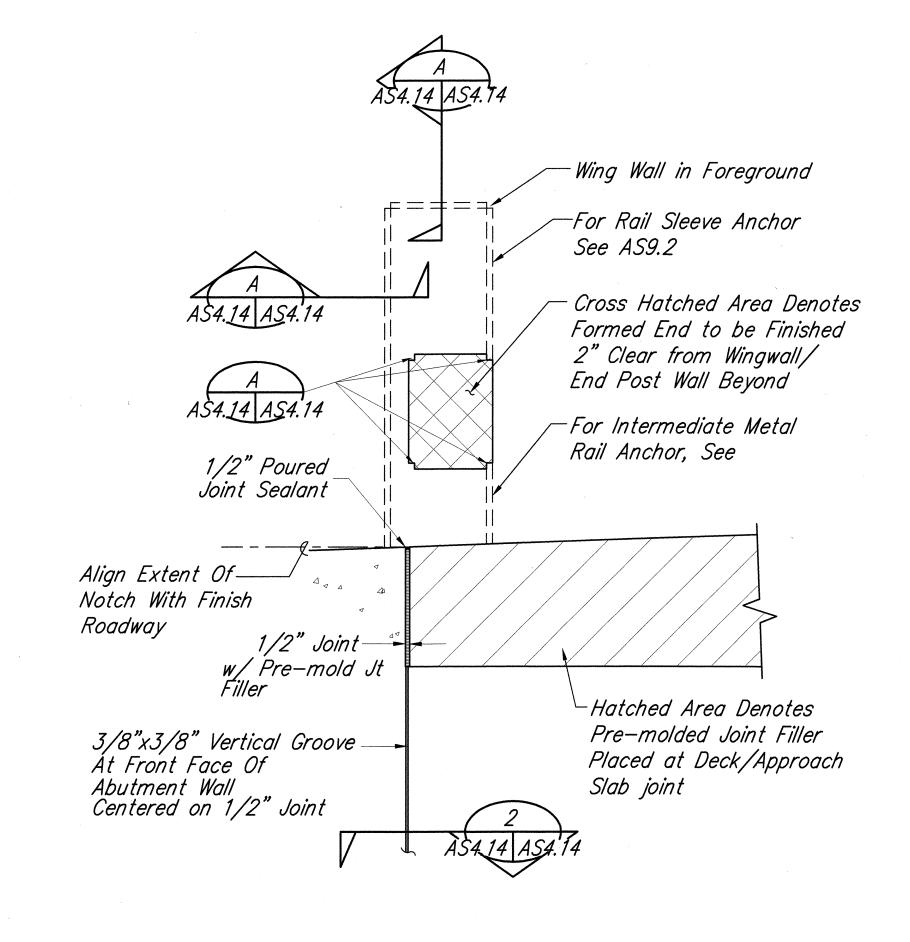
3/8" x 3/8" VERTICAL GROOVE 2

Sc: Full Size

AS4.14 AS4.14

FED. ROAD DIST. NO. STATE FED. AID PROJ. NO. FISCAL SHEET TOTAL SHEETS

HAWAII HAW. ER-12 (2) 2001 58 145

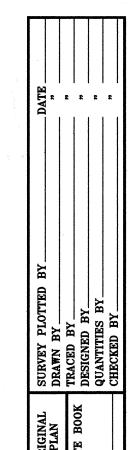


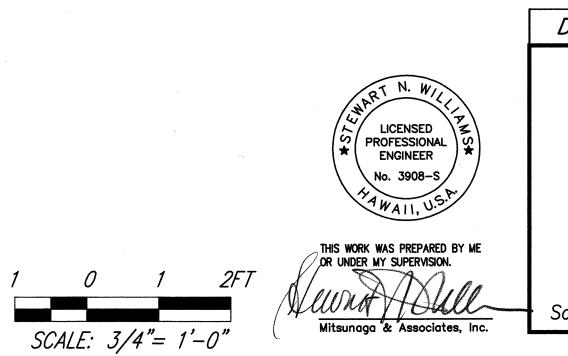
TYPICAL JOINT

DETAIL AT RAILS

sc: 3/4"=1'-0"







ATE DESCRIPTION

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

END POST DETAILS

MAMALAHOA HIGHWAY

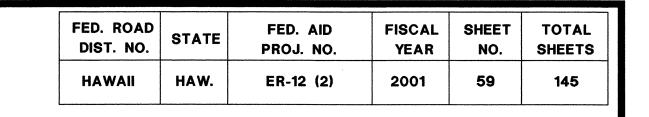
REPLACEMENT OF

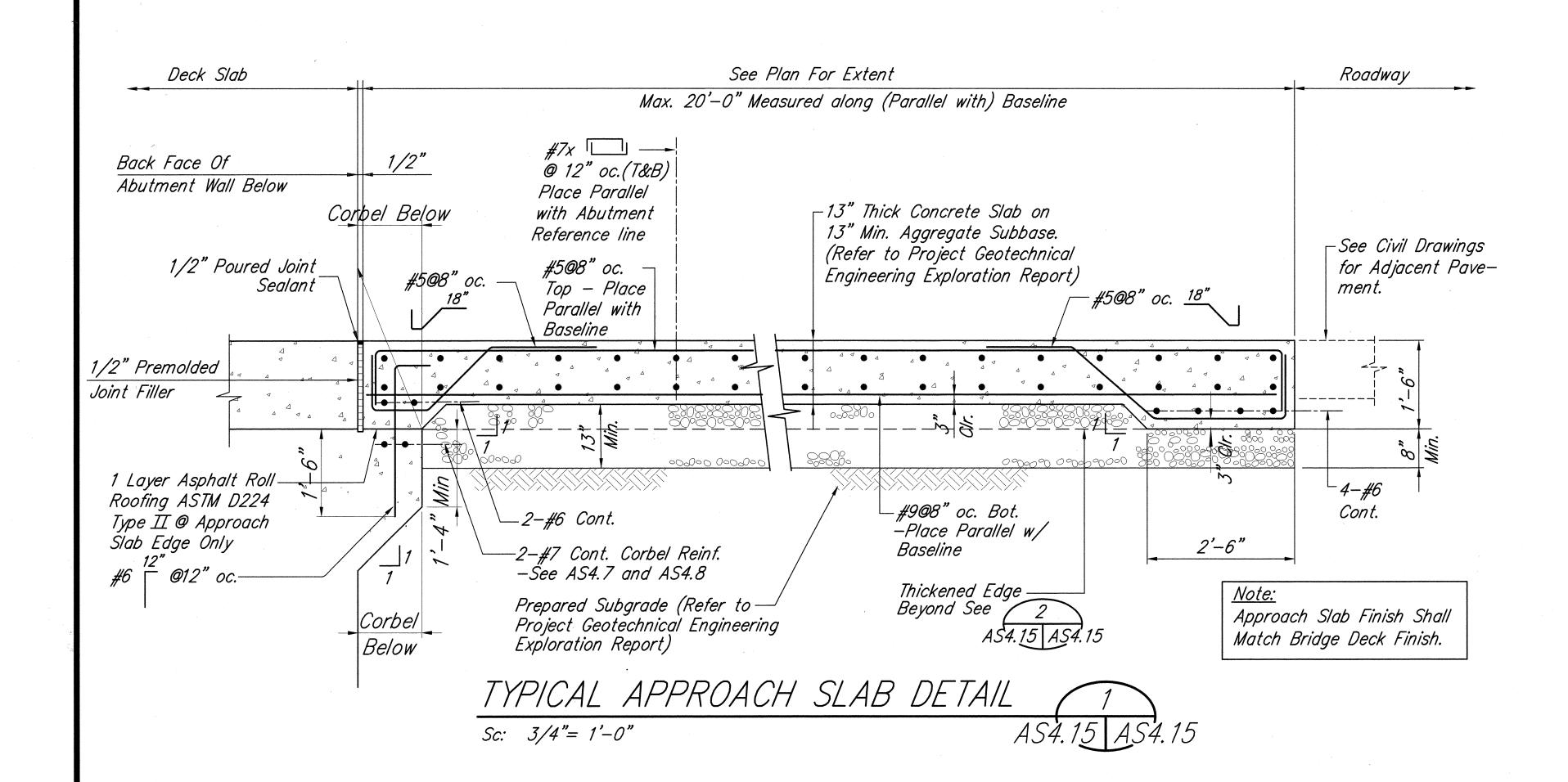
FORD CROSSING

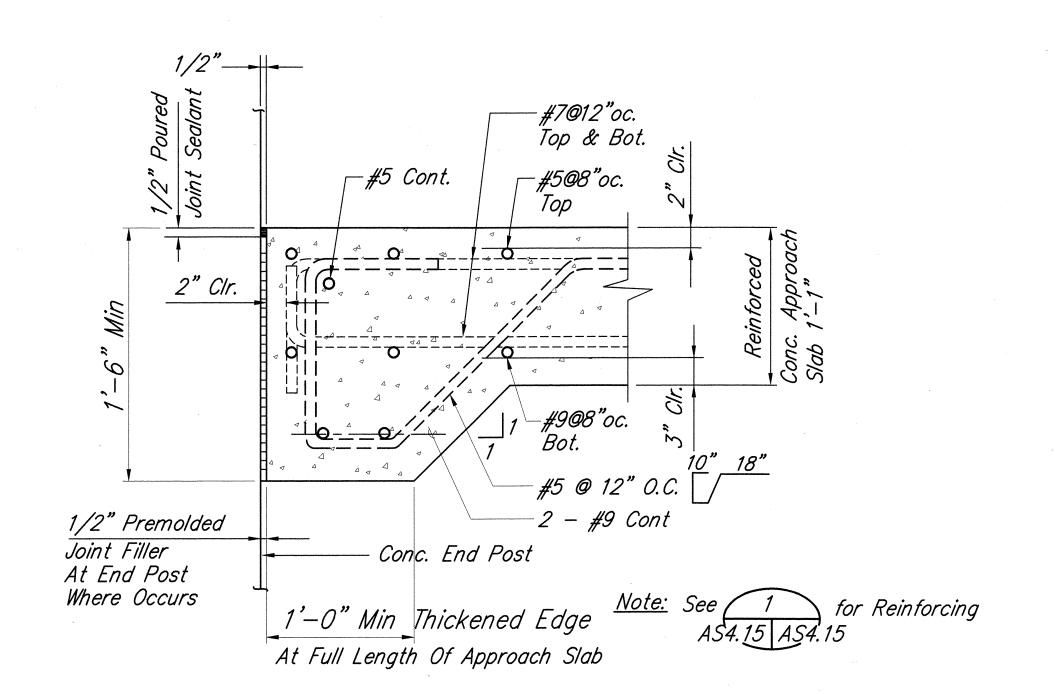
Federal Aid Project No. ER-12(2)

As Noted Feb. 2001

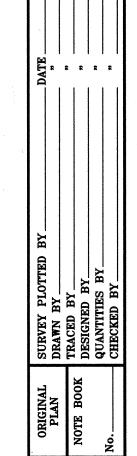
SHEET No. AS4.14 OF 49 SHEETS

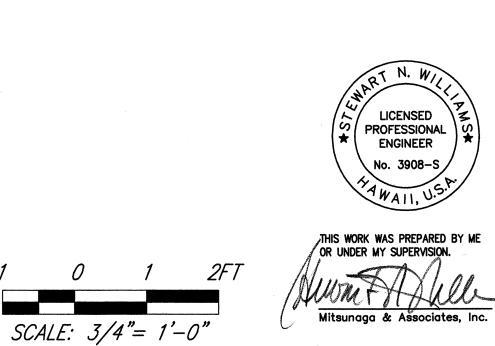












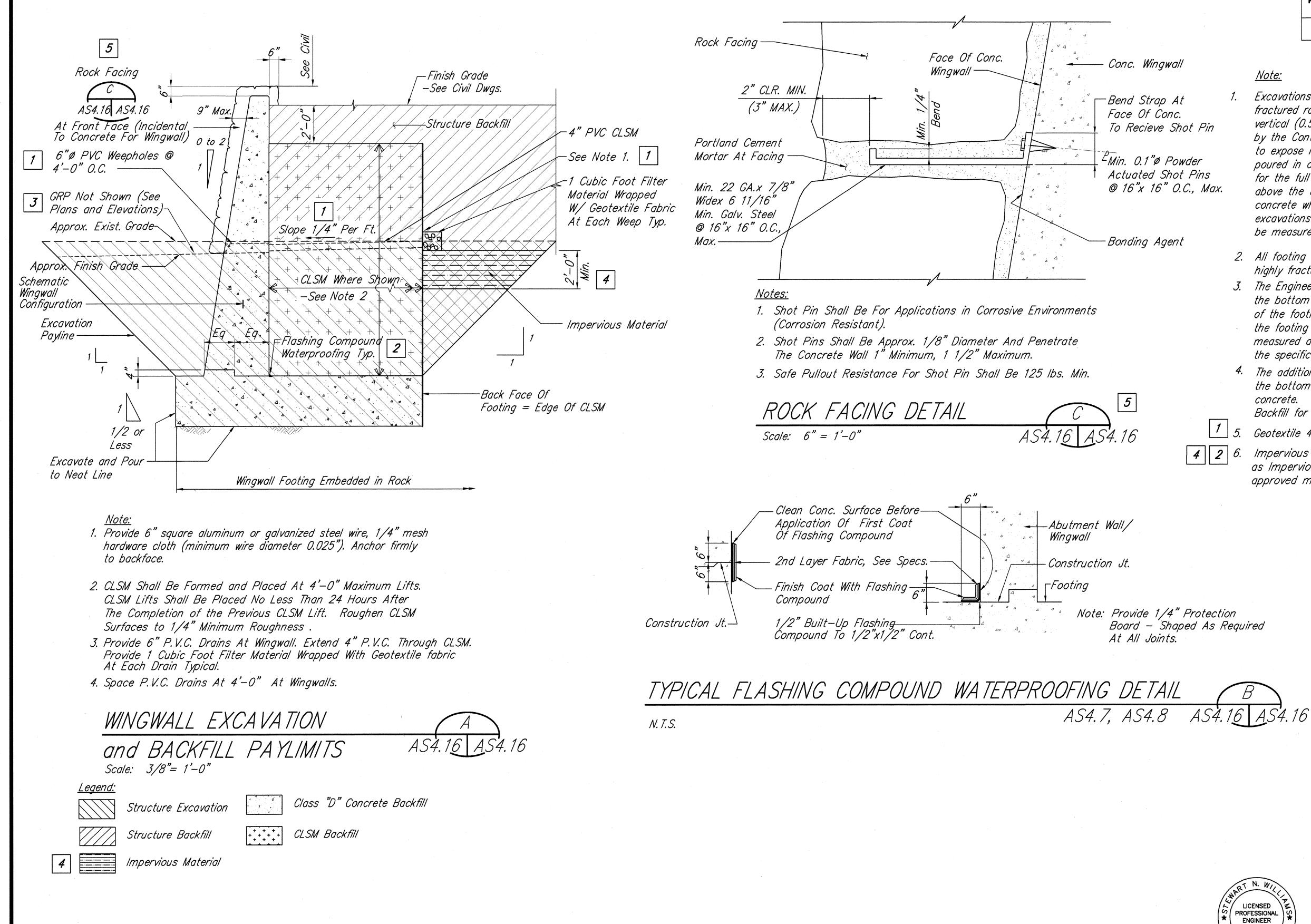
DESCRIPTION

STATE OF HAWAII
DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

APPROACH SLAB DETAILS

MAMALAHOA HIGHWAY REPLACEMENT OF
FORD CROSSING
Federal Aid Project No. ER-12(2)

SHEET No. AS4.15 OF 49 SHEETS



FED. ROAD FISCAL SHEET TOTAL FED. AID DIST. NO. PROJ. NO. YEAR NO. SHEETS HAW. ER-12 (2) 2001 60 HAWAII

- Excavations for footings in Basalt Rock formation (including fractured rock and clinker materials) shall be kept to nearvertical (0.5H:1V) or steeper. Inadvertent over-excavations by the Contractor shall be cleaned of loose and fractured rock to expose intact rock. Concrete for the final footing shall be poured in contact with the sides of the excavation for the full for the full designed footing thickness. The resulting void space above the top of the footing shall be backfilled with Class D concrete where indicated . Backfilling the inadvertent overexcavations with footing concrete or Class D concrete will not be measured and paid for separately.
- 2. All footing excavations shall be cleaned of loose materials and highly fractured rock to the satisfaction of the Engineer.
- 3. The Engineer may direct the Contractor to over-excavate below the bottom of footing elevation and to the sides of the edges of the footing excavations. The additional over-excavation of the footing excavations, as directed by the Engineer, will be measured and paid for as structure excavation as indicated in the specifications.
- 4. The additional over-excavation as directed by the Engineer, below the bottom of footing elevation shall be backfilled with Class D concrete. This item will be measured and paid for as Structure Backfill for Abutments and Wingwalls (Class D concrete).
- Geotextile 4" and 6" PVC shall be incidental to Structural backfill.
- 4 2 6. Impervious material as selected by the Engineer shall be made as Impervious as possible by Pneumatic Tamping or other approved methods and shall be incidental to Structure Backfill.

THIS WORK WAS PREPARED BY ME

SCALE: 3/8"= 1'-0"

5 Added Rock Facing Detail 07-31-01 4 Added Impervious Material 07-31-01 3 Corrected Notes 07-31-01 2 Added Notes 07-31-01 1 | Corrected Rock Facing Extent DESCRIPTION

> STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

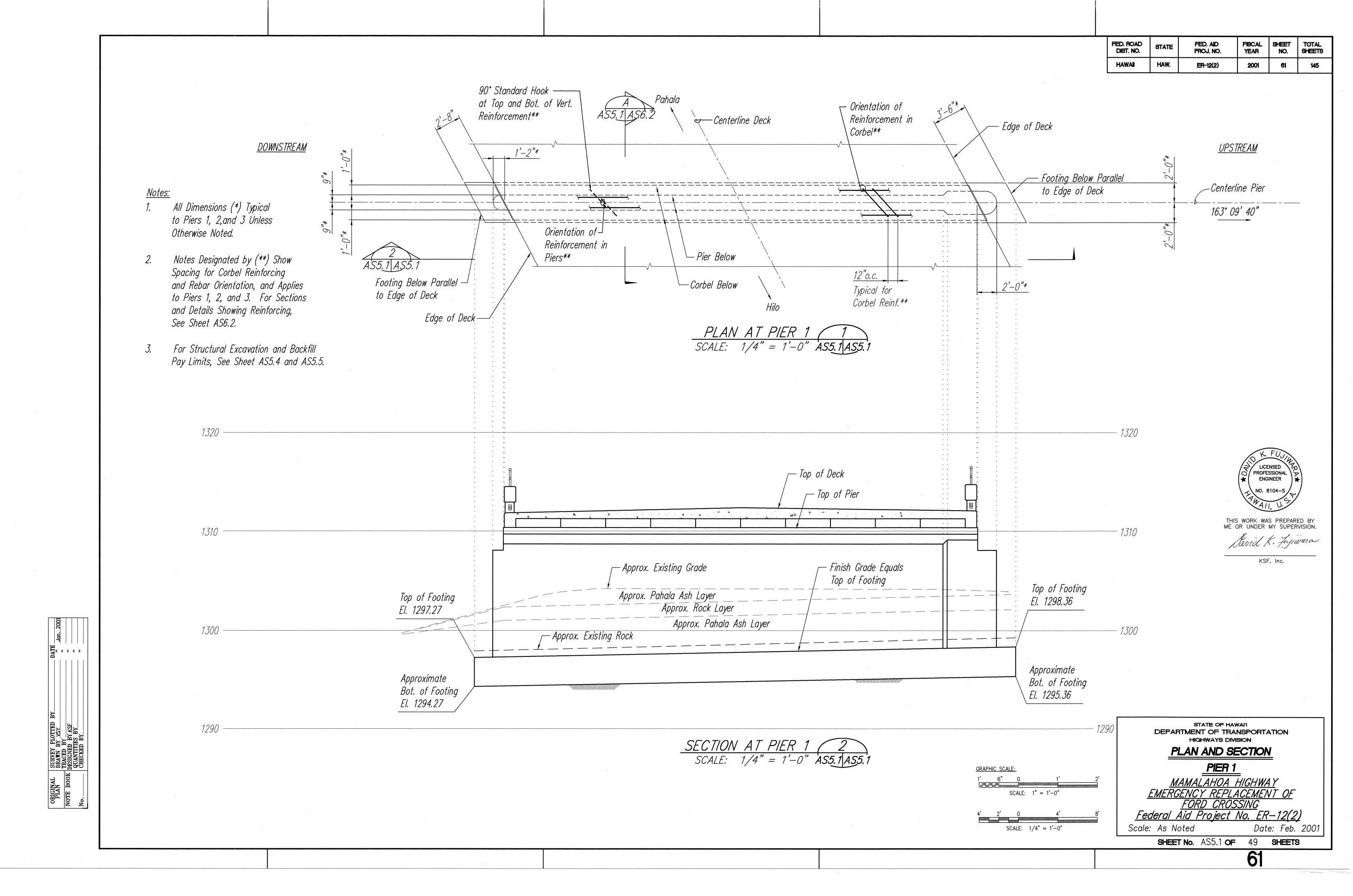
EXCAVATION and BACKFILL LIMITS

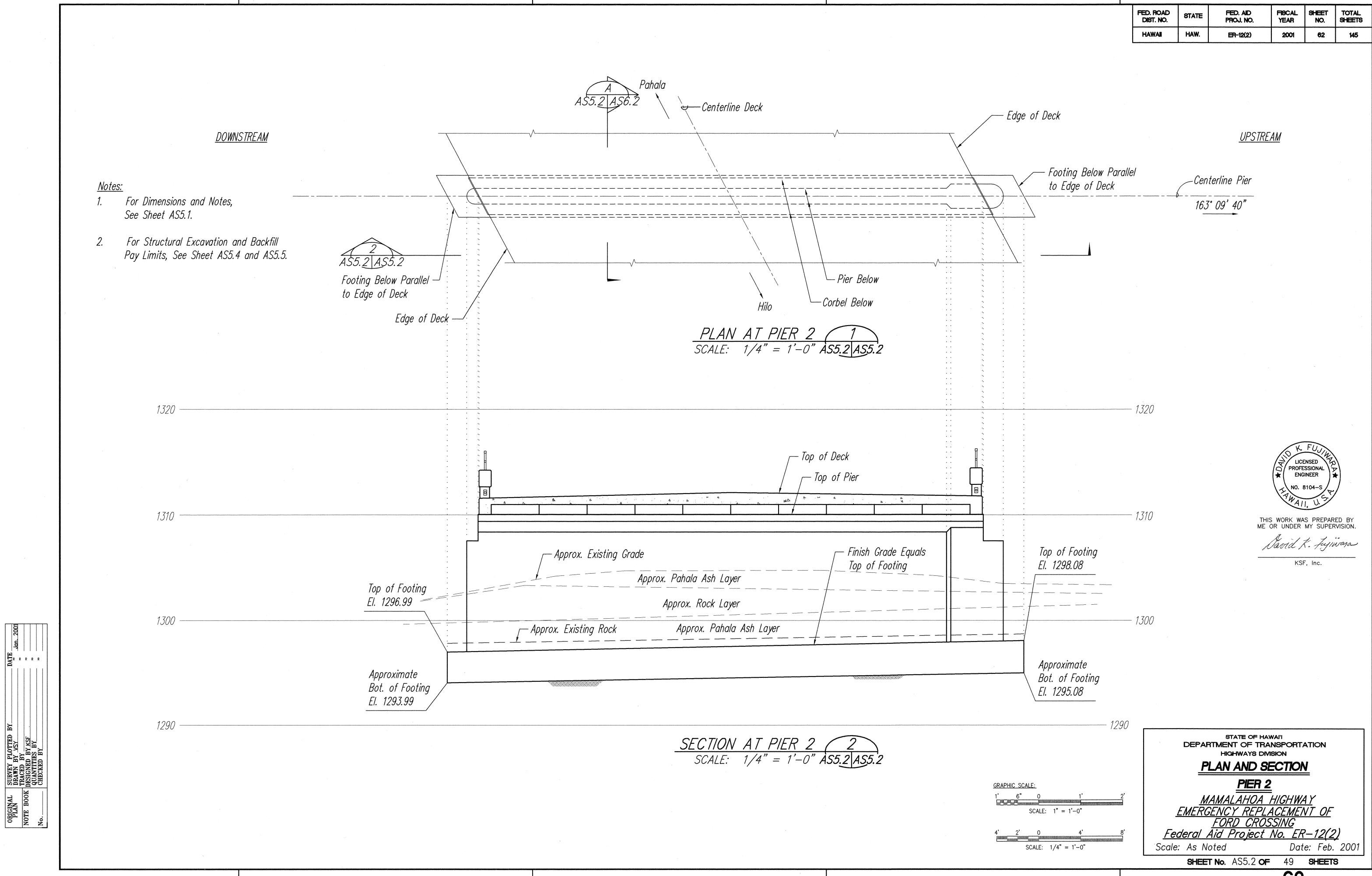
MAMALAHOA HIGHWAY REPLACEMENT OF FORD CROSSING

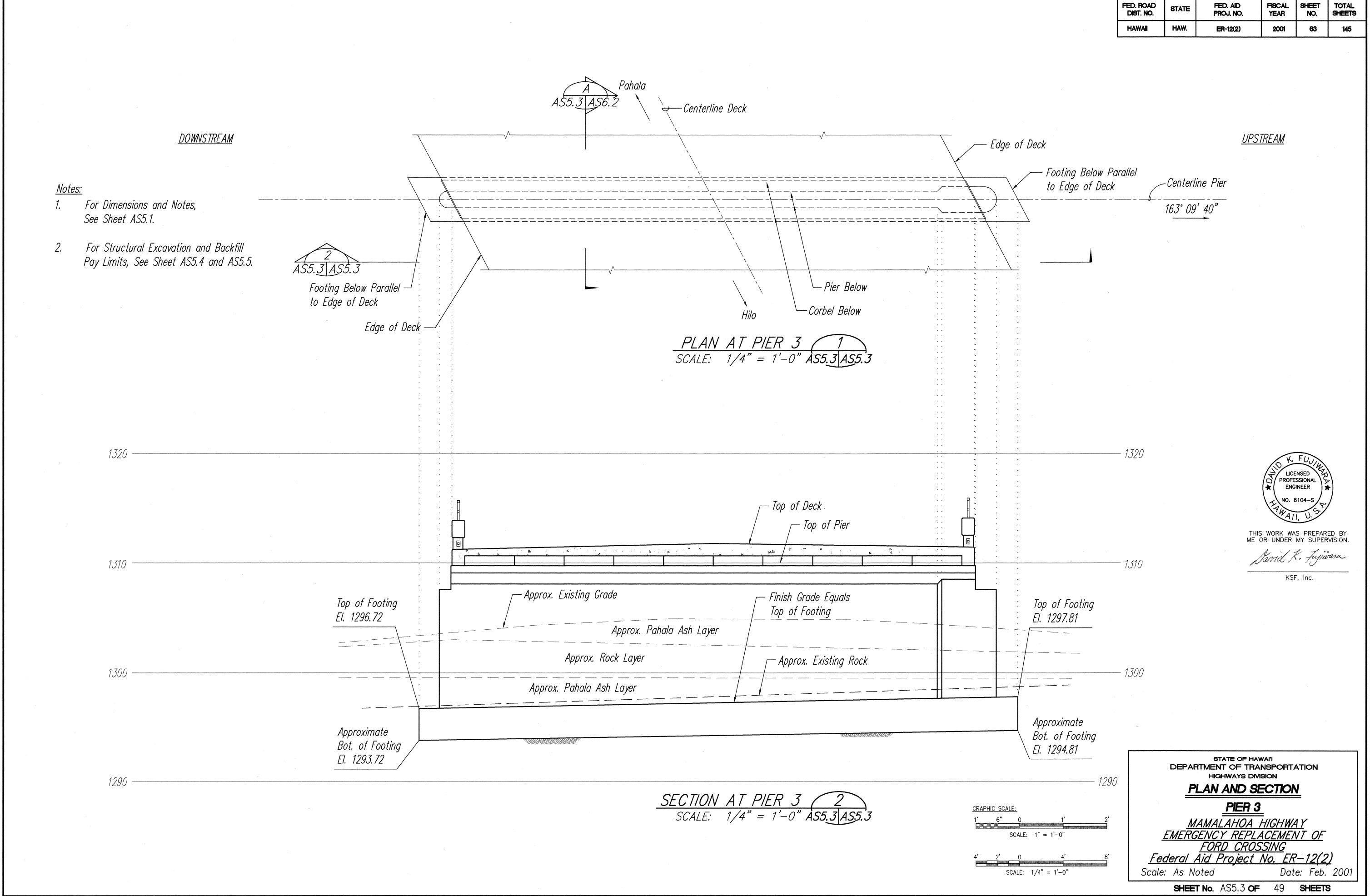
Federal Aid Project No. ER-12(2)

SHEET No. AS4.16 OF 49 SHEETS

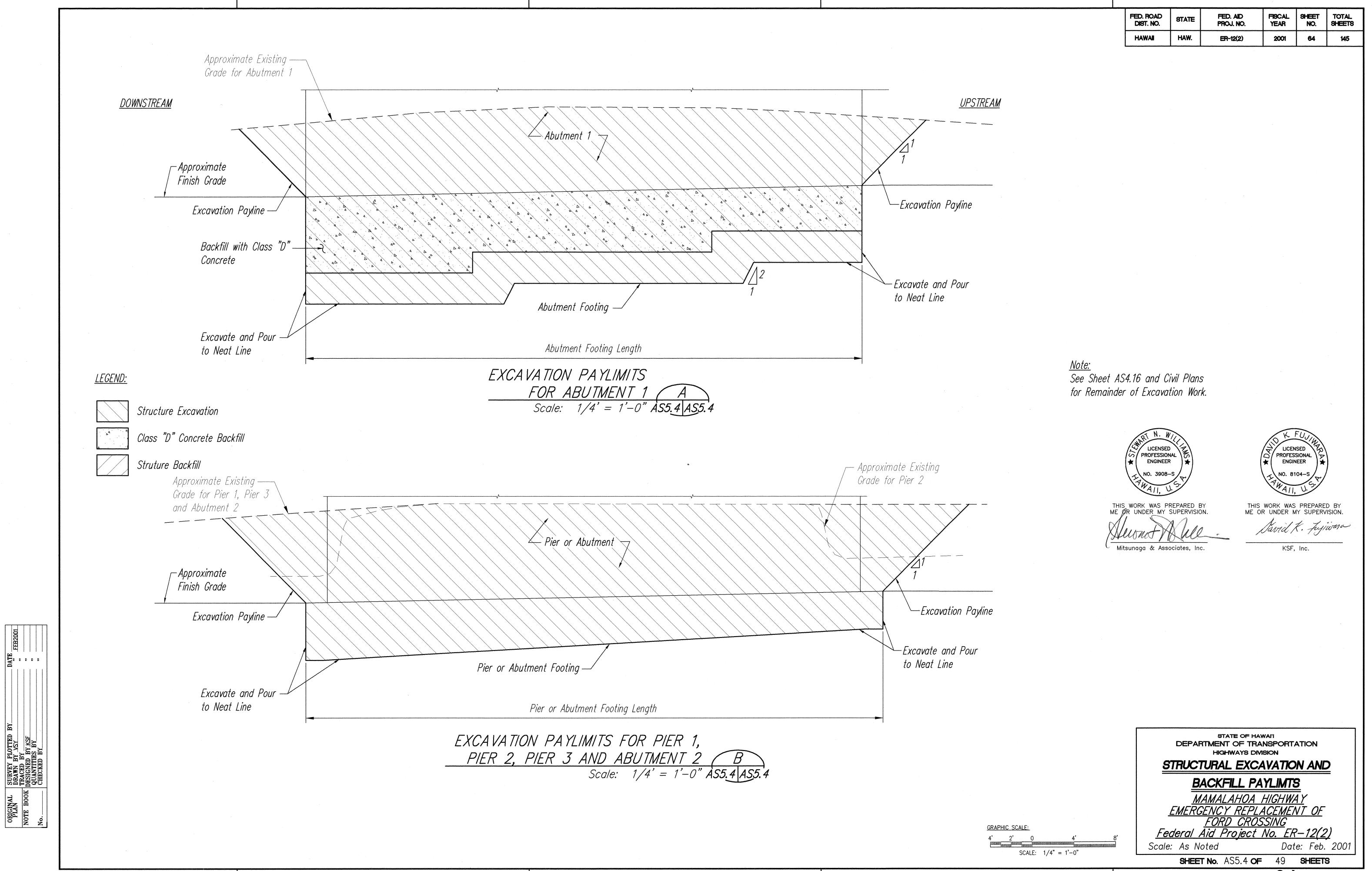
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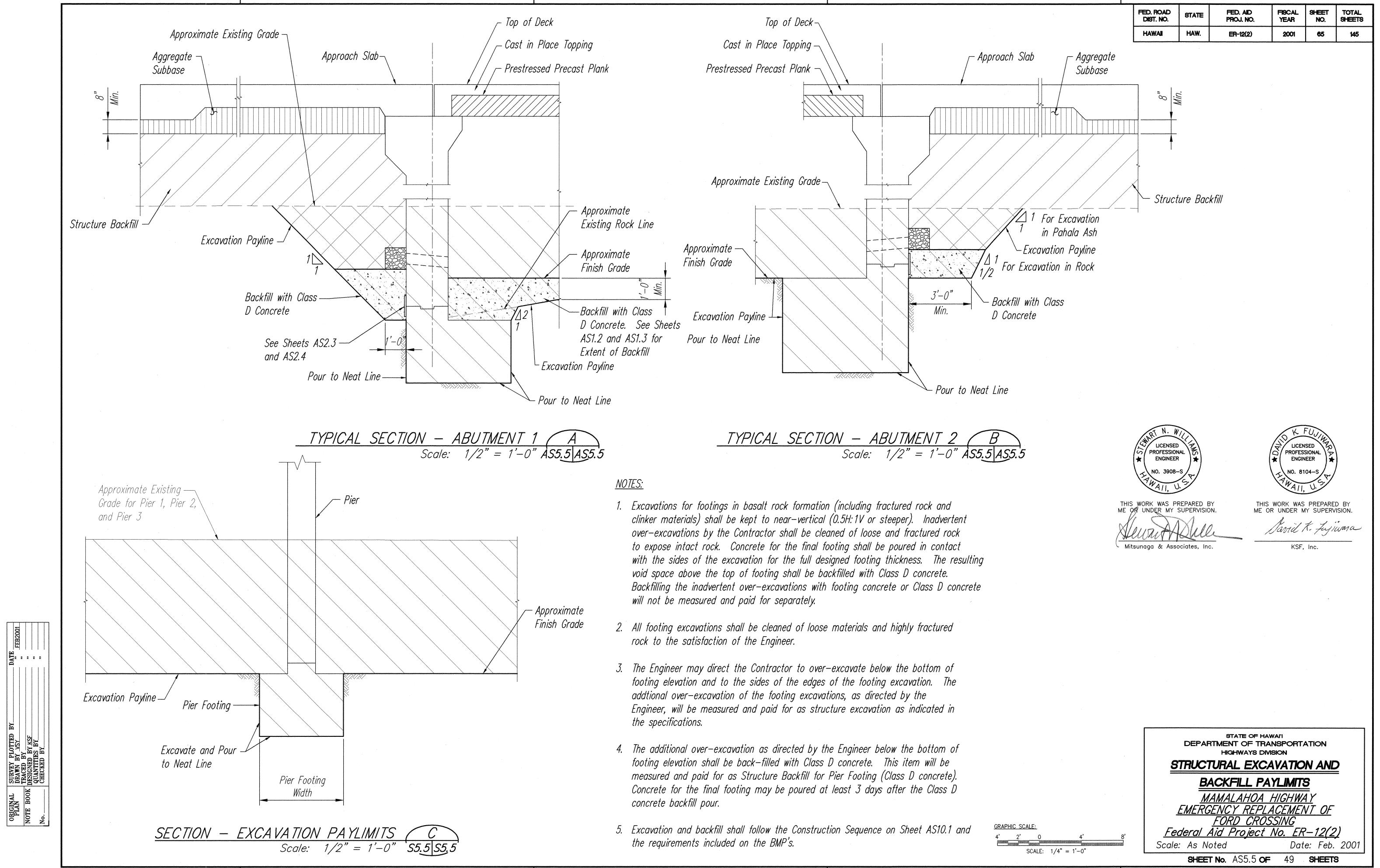


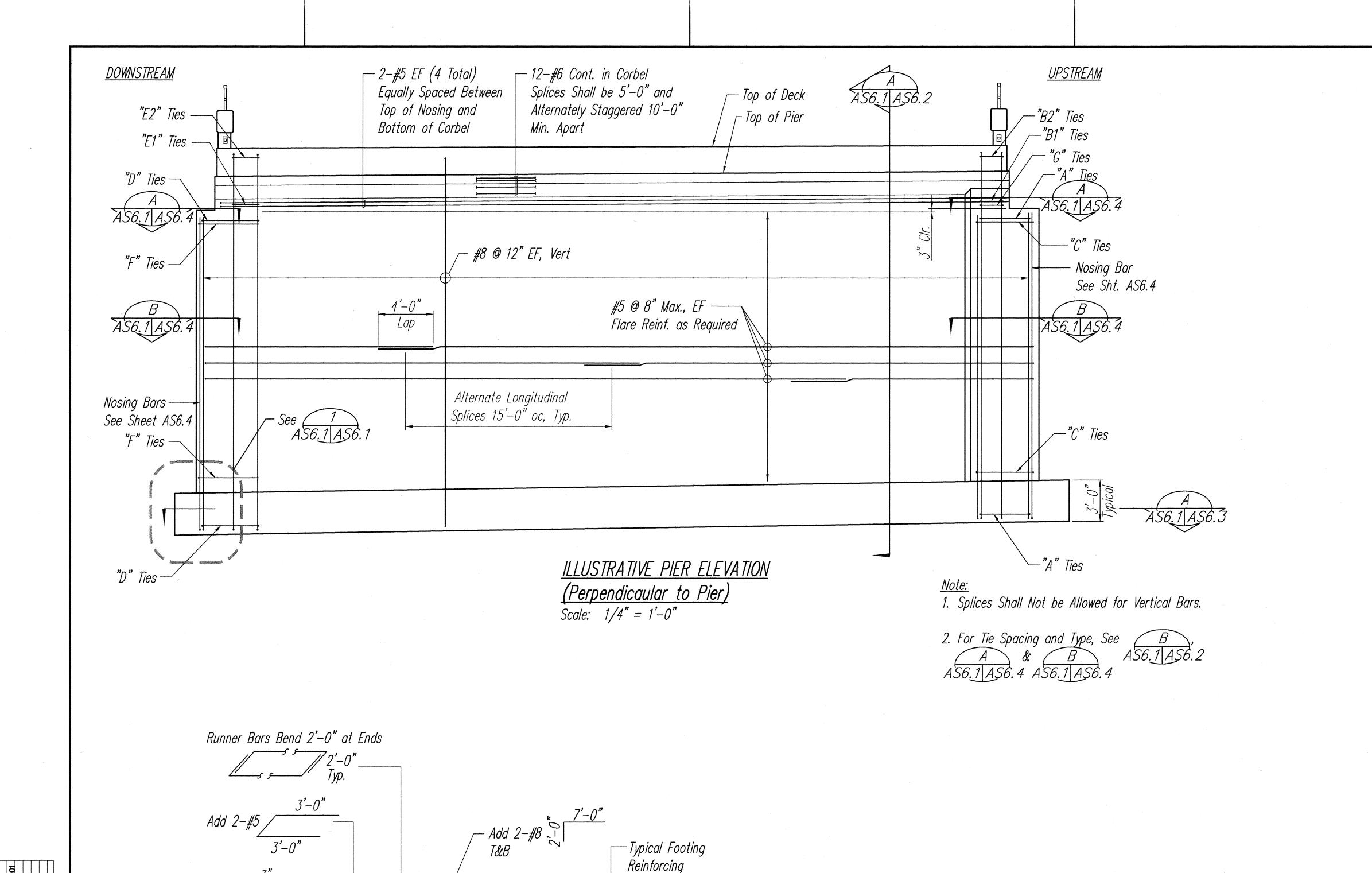




ORIGINAL PLOTTED BY
DRAWN BY VSY
TRACED BY
TRACED BY
DESIGNED BY
QUANTITIES BY
CHECKED BY

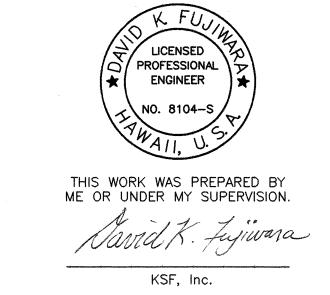


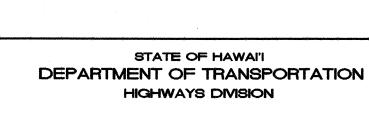




FED. ROAD DIST. NO. STATE FED. AID PROJ. NO. FISCAL YEAR NO. SHEETS

HAWAII HAW. ER-12(2) 2001 66 145





TYPICAL PIER ELEVATION AND DETAILS

MAMALAHOA HIGHWAY

EMERGENCY REPLACEMENT OF

FORD CROSSING

Federal Aid Project No. ER-12(2)

Scale: As Noted Date: Feb. 2001

SHEET No. AS6.1 OF 49 SHEETS

2'-0" Bend -

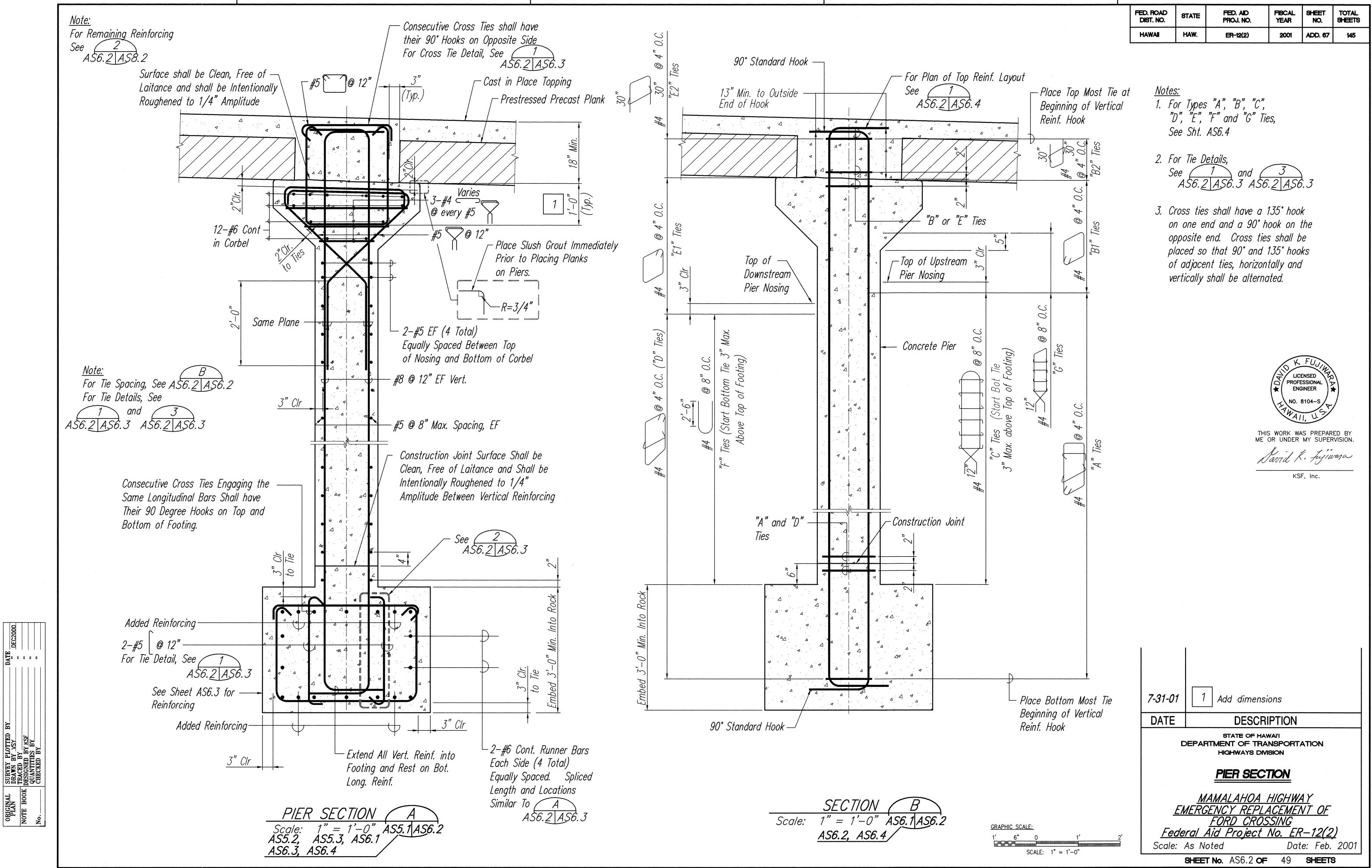
- Same Plane

Typical

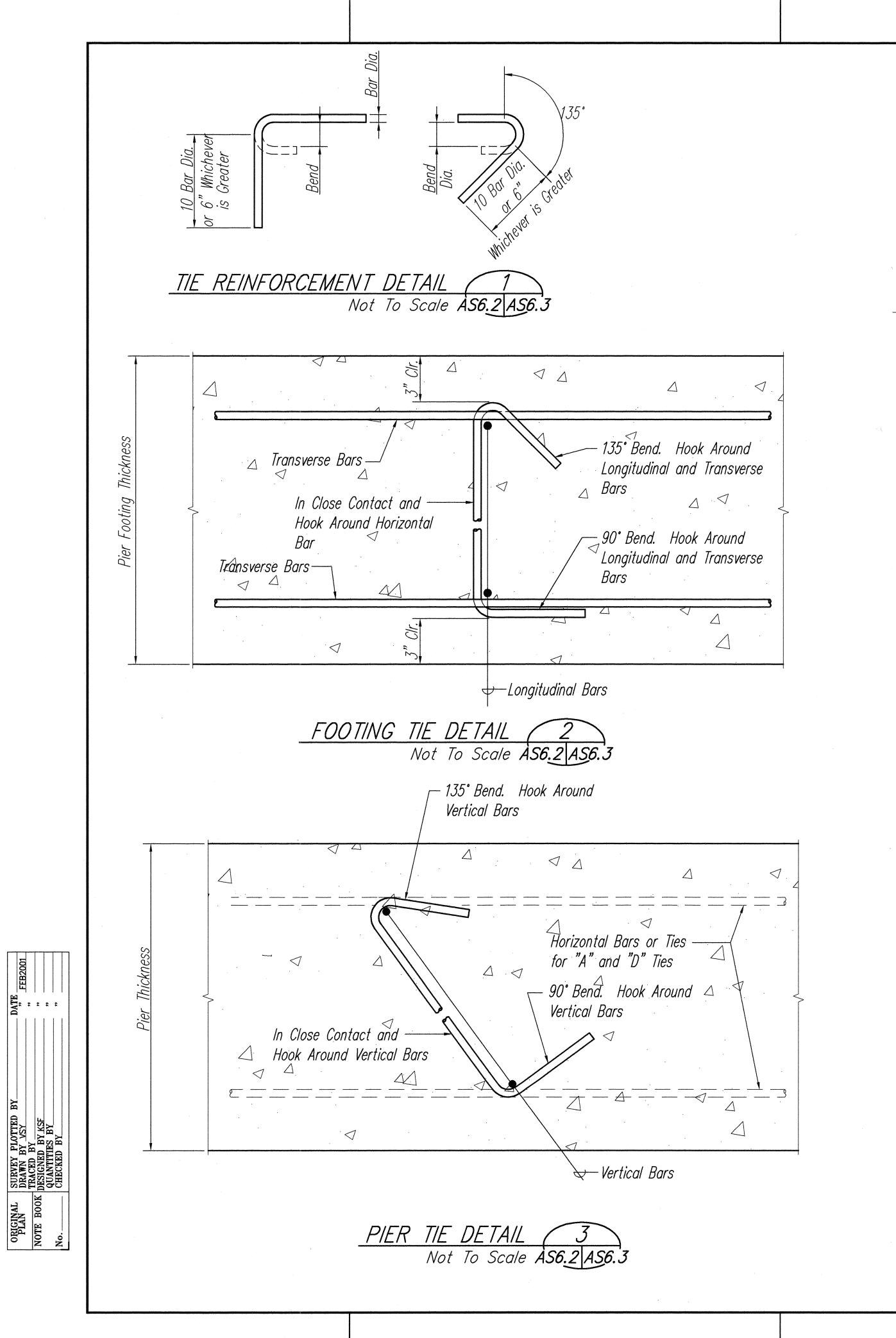
Alternate -

135° Bend

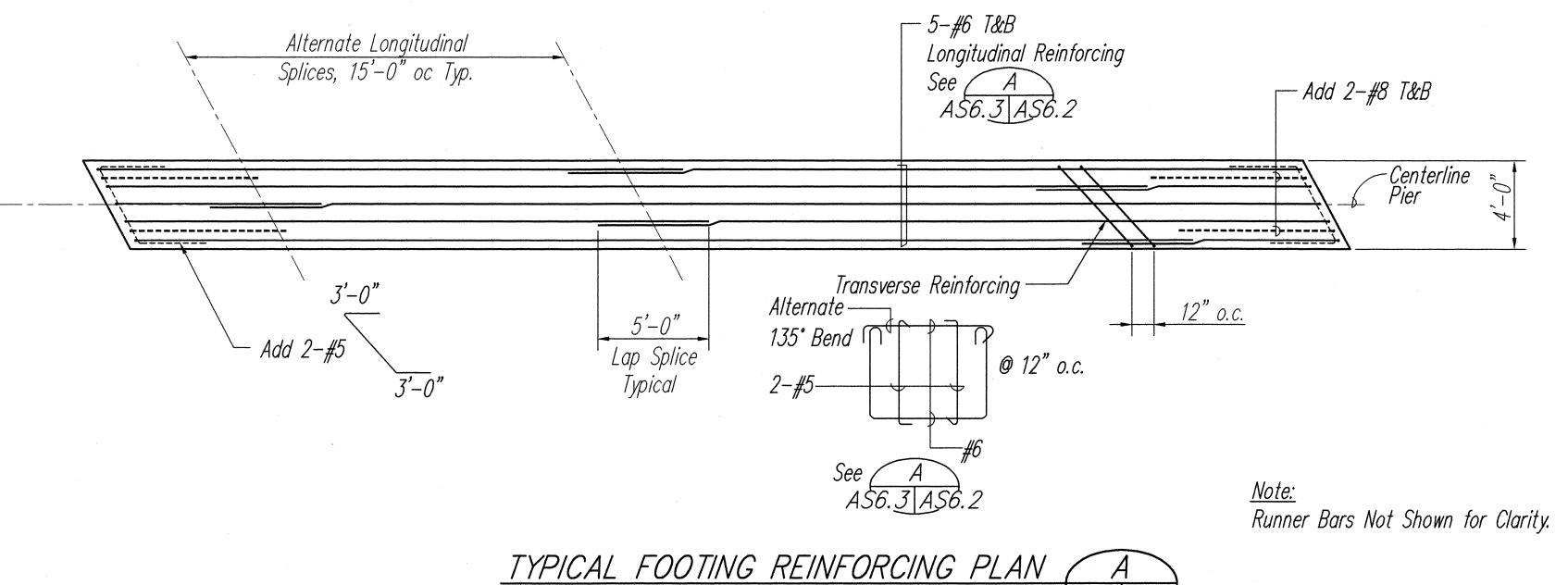
 $\frac{TYPICAL END CONDITION}{Scale: 1/2" = 1'-0" AS6.1 AS6.1}$ $\frac{Fed}{Scale: 1/2" = 1'-0" AS6.1 AS6.1}$

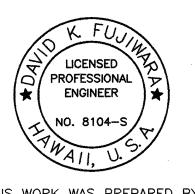


ADD. 67



FED. ROAD DIST. NO. FISCAL SHEET TOTAL YEAR NO. SHEETS FED. AID PROJ. NO. STATE 2001 ER-12(2)





THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

KSF, Inc.

GRAPHIC SCALE: SCALE: 1/4" = 1'-0"

Scale: 1/4" = 1'-0" AS6.1 AS6.3 AS6.2

STATE OF HAWAI'I
DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

TYPICAL PIER FOOTING

REINFORCING PLAN

MAMALAHOA HIGHWAY

EMERGENCY REPLACEMENT OF

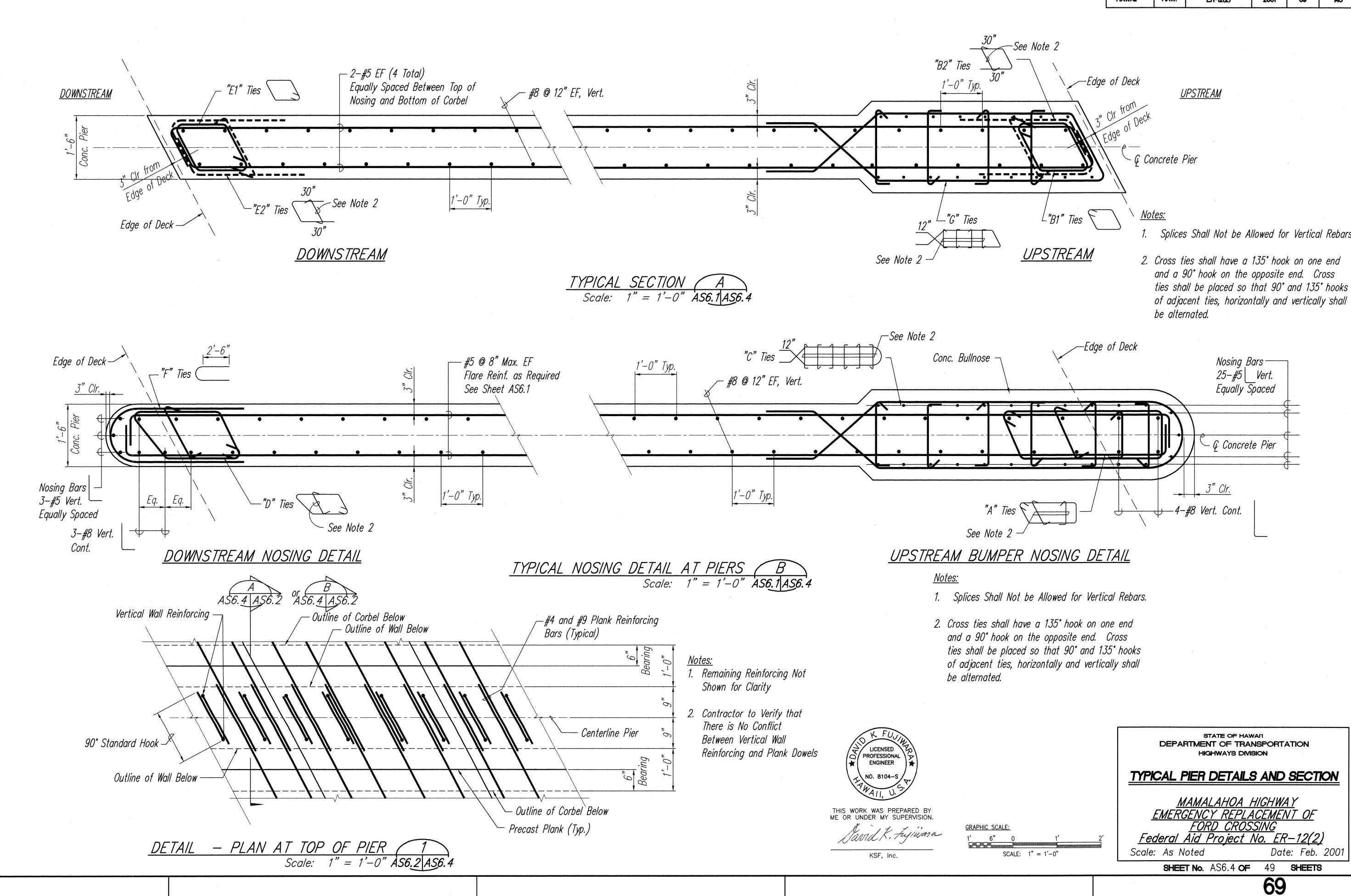
FORD CROSSING

Federal Aid Project No. ER-12(2)

Scale: As Noted Date: Feb. 2001

SHEET No. AS6.3 **OF** 49 SHEETS

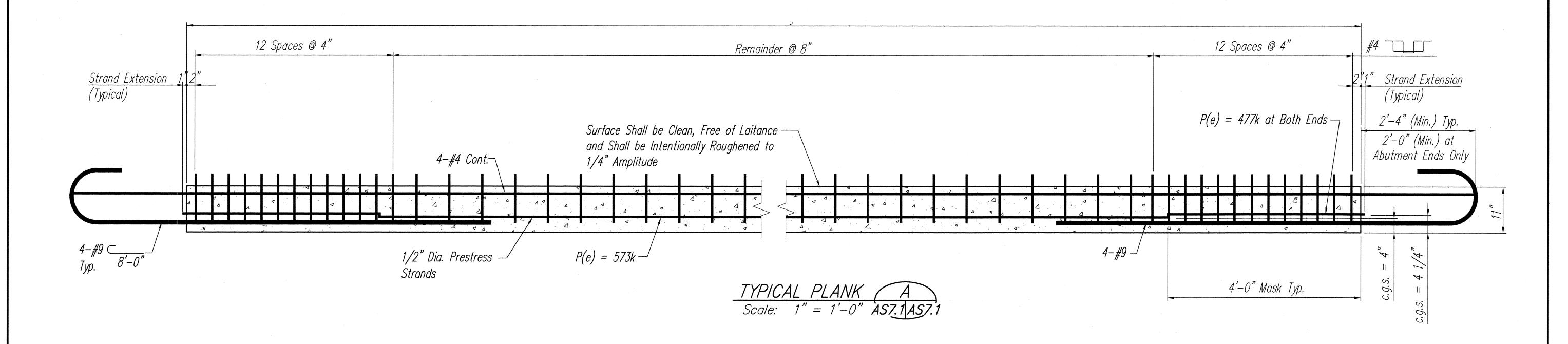
FED. ROAD DIST. NO. FISCAL YEAR SHEET NO. TOTAL SHEETS FED. AID PROJ. NO. **STATE** HAWAII 2001 —Edge of Deck <u>UPSTREAM</u> © Concrete Pier L"B1" Ties Splices Shall Not be Allowed for Vertical Rebars. 2. Cross ties shall have a 135° hook on one end and a 90° hook on the opposite end. Cross ties shall be placed so that 90° and 135° hooks of adjacent ties, horizontally and vertically shall be alternated. —Edge of Deck Nosing Bars— 25-#5 __Vert. Equally Spaced - @ Concrete Pier STATE OF HAWAI'I
DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION



ORIGINAL DRAWN BY VSY TRACED BY TRACED BY DESIGNED BY KSE QUANTITIES BY CHECKED BY

FED. ROAD DIST. NO. STATE FED. AID PROJ. NO. FISCAL YEAR NO. SHEETS

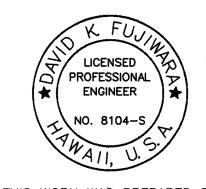
HAWAII HAW. ER-12(2) 2001 70 145



PRESTRESSED PLANK NOTES

- 1. Prestressed concrete 28 day strength f'c = 6,000 PSI. Prestressed concrete strength at time of release fci = 4,500 PSI.
- 2. Pretensioned strands shall be 7 wire 1/2" Ø low relaxation steel strands (Area = 0.153 in) with an ultimate tensile strength of 270 KSI. For properties, see "State Standard Specifications."
- 3. Non-prestressed reinforcing steel shall be grade 60. For properties, see "State Standard Specifications."
- 4. Strand pattern shall be symmetrical about the longitudinal & of the plank.
- 5. Strand release sequence shall not induce any lateral deflection of the plank.
- 6. Contractor shall submit shop drawings indicating proposed strand pattern, releasing sequence, reinforcing details & hold down device details to the Engineer prior to fabrication.

- 7. During curing, care shall be taken to avoid any lateral deflection of the plank due to improper orientation. Steam curing may be used to accelerate strength gain.
- 8. Lifting devices shall be placed as close as possible to the centerline of bearings of the plank. Details and locations of lifting devices shall be submitted to the Engineer for approval. Such approval does not relieve the Contractor of his responsibilities if beam is damaged due to failure of the lifting device.
- 9. See Sheet AS8.1 for location of plank mark numbers.



THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

Savid K. Fujiwara

KSF, Inc.

RAPHIC SCALE:
' 6" 0 1'

SCALE: 1" = 1'-0"

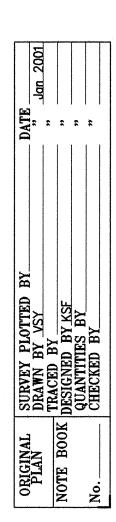
STATE OF HAWAI'I
DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

PRECAST PLANK SECTION AND DETAILS

<u>MAMALAHOA HIGHWAY</u> <u>EMERGENCY REPLACEMENT OF</u> <u>FORD CROSSING</u> <u>Federal Aid Project No. ER-12(2)</u>

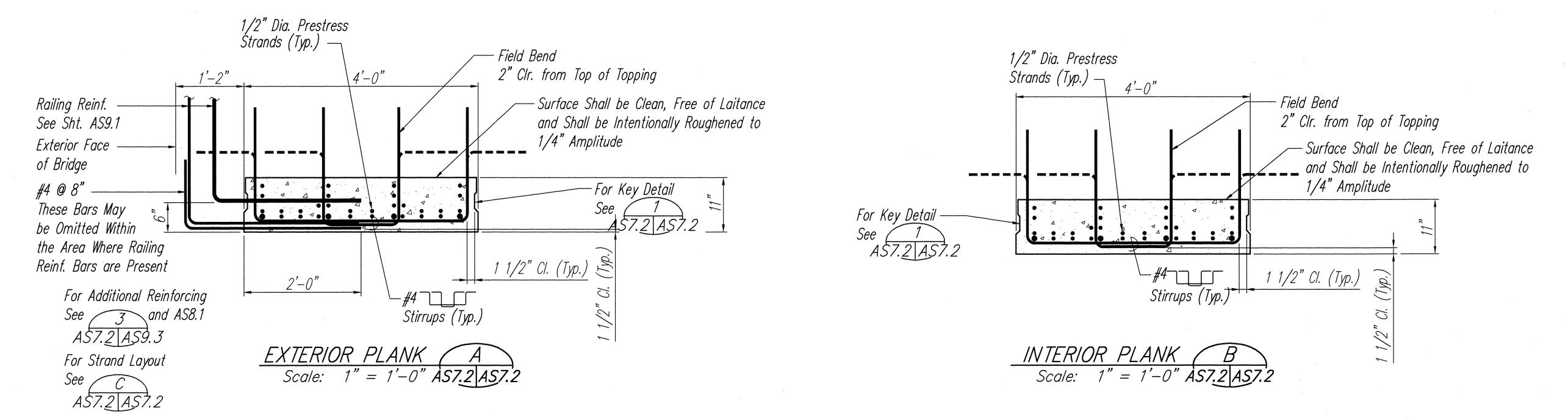
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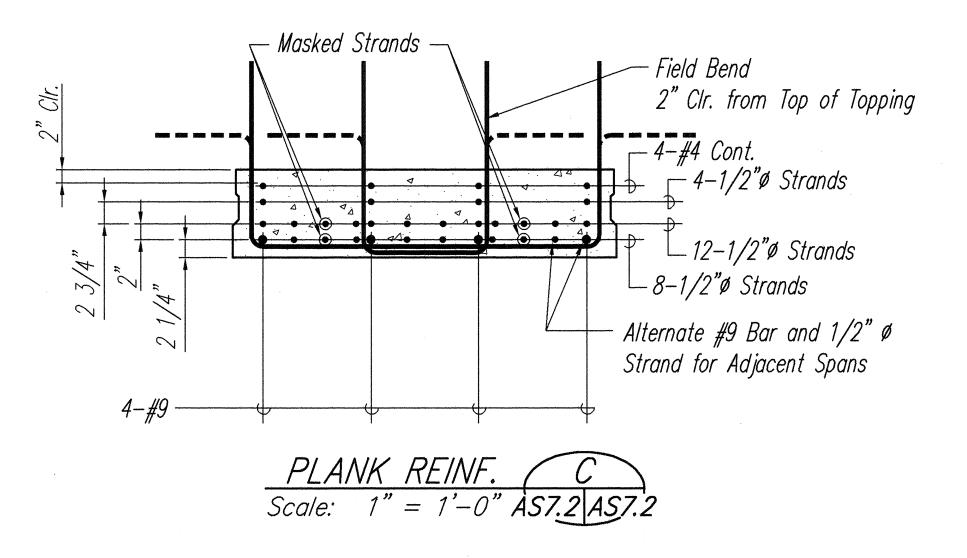
SHEET No. AS7.1 OF 49 SHEETS

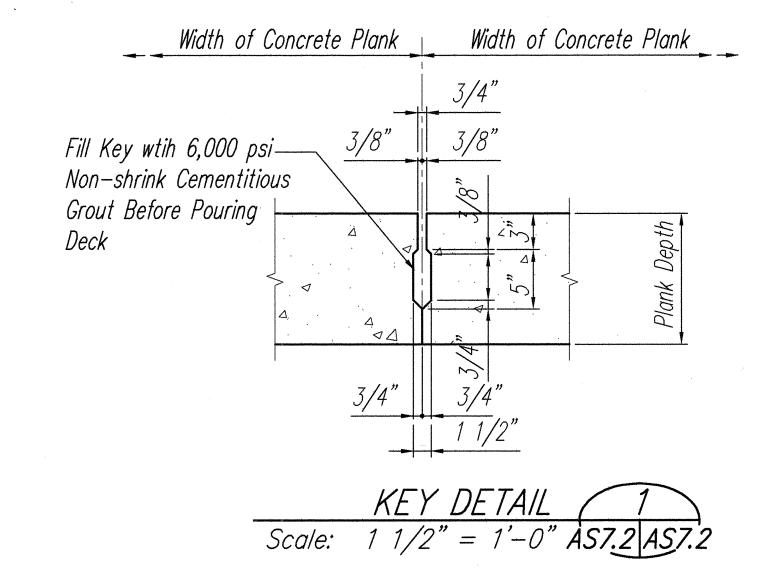


Date: Feb. 2001

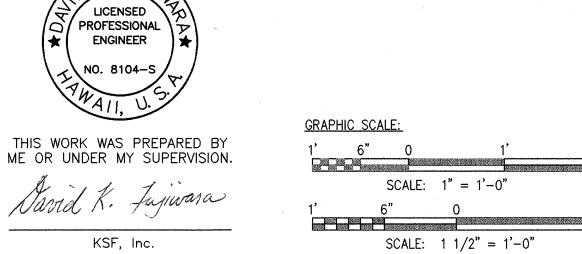
NA P		FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS	
HAWAII	HAW.	ER-12(2)	2001	71	145	







PRECAST PLANK PP1 AND PP2



STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DMSION

PRECAST PLANK SECTION AND DETAILS

MAMALAHOA HIGHWAY

EMERGENCY REPLACEMENT OF

FORD CROSSING

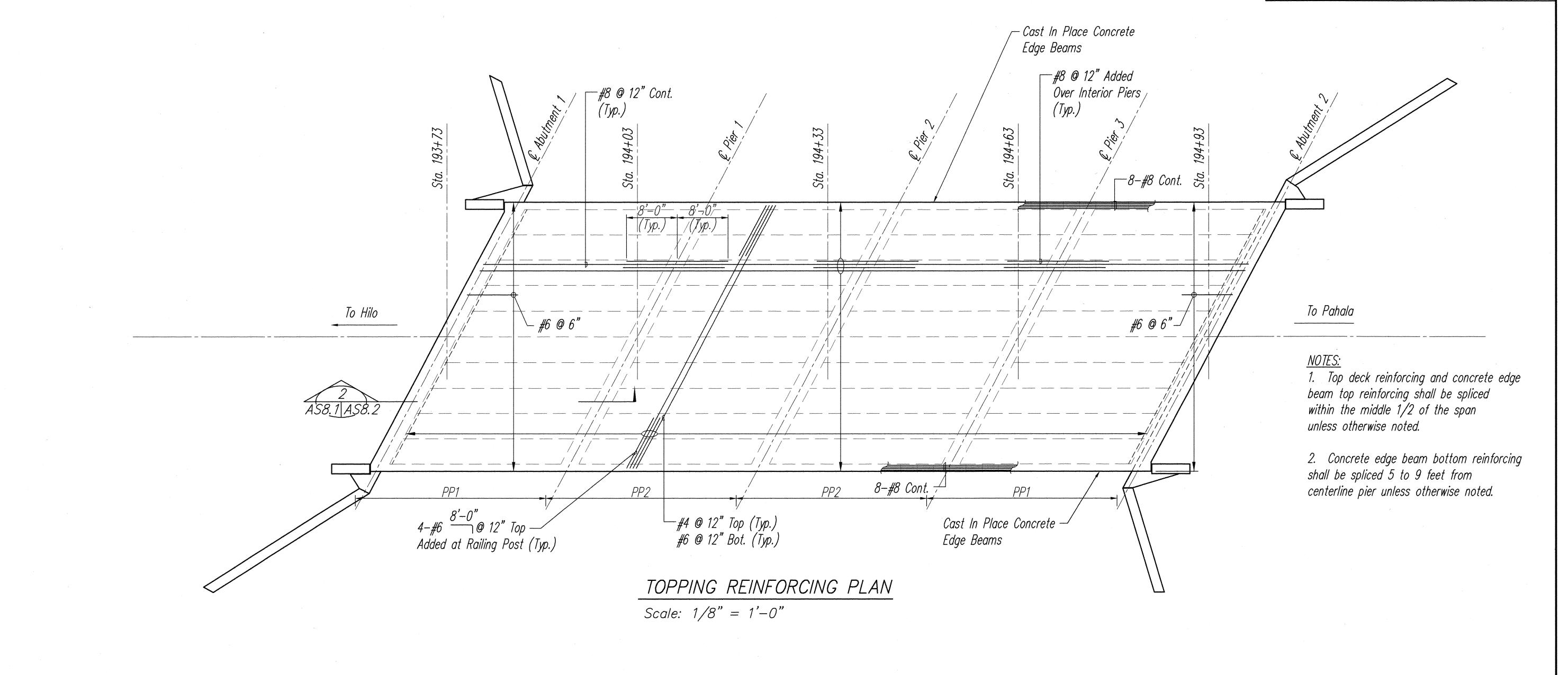
Federal Aid Project No. ER-12(2)

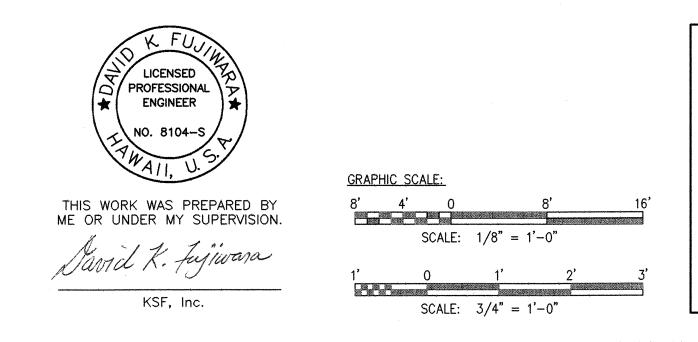
Scale: As Noted Date: Feb. 2001

SHEET No. AS7.2 OF 49 SHEETS

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FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	ER-12(2)	2001	72	145





STATE OF HAWAI'I
DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

TOPPING REINFORCING PLAN

MAMALAHOA HIGHWAY

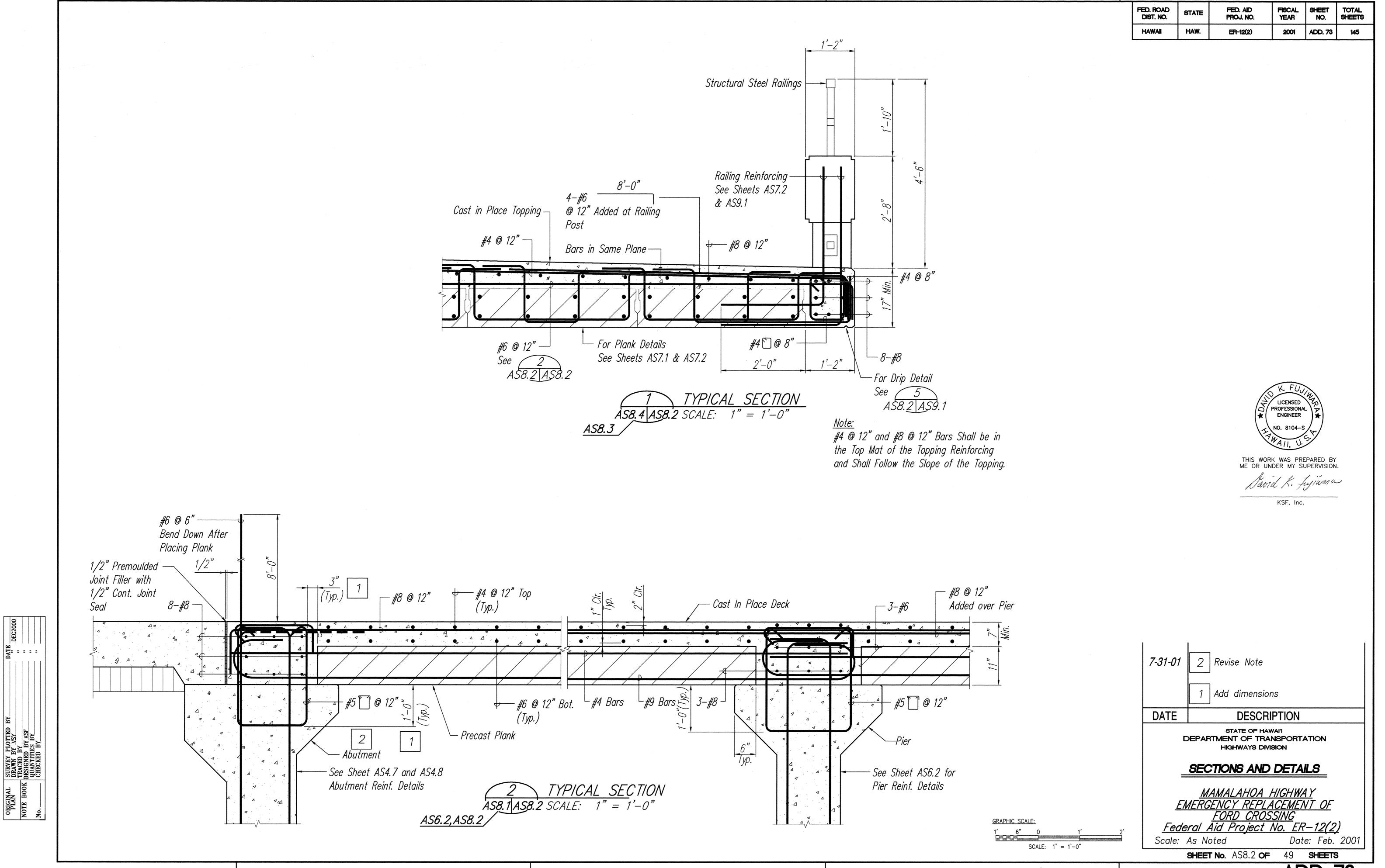
EMERGENCY REPLACEMENT OF

FORD CROSSING

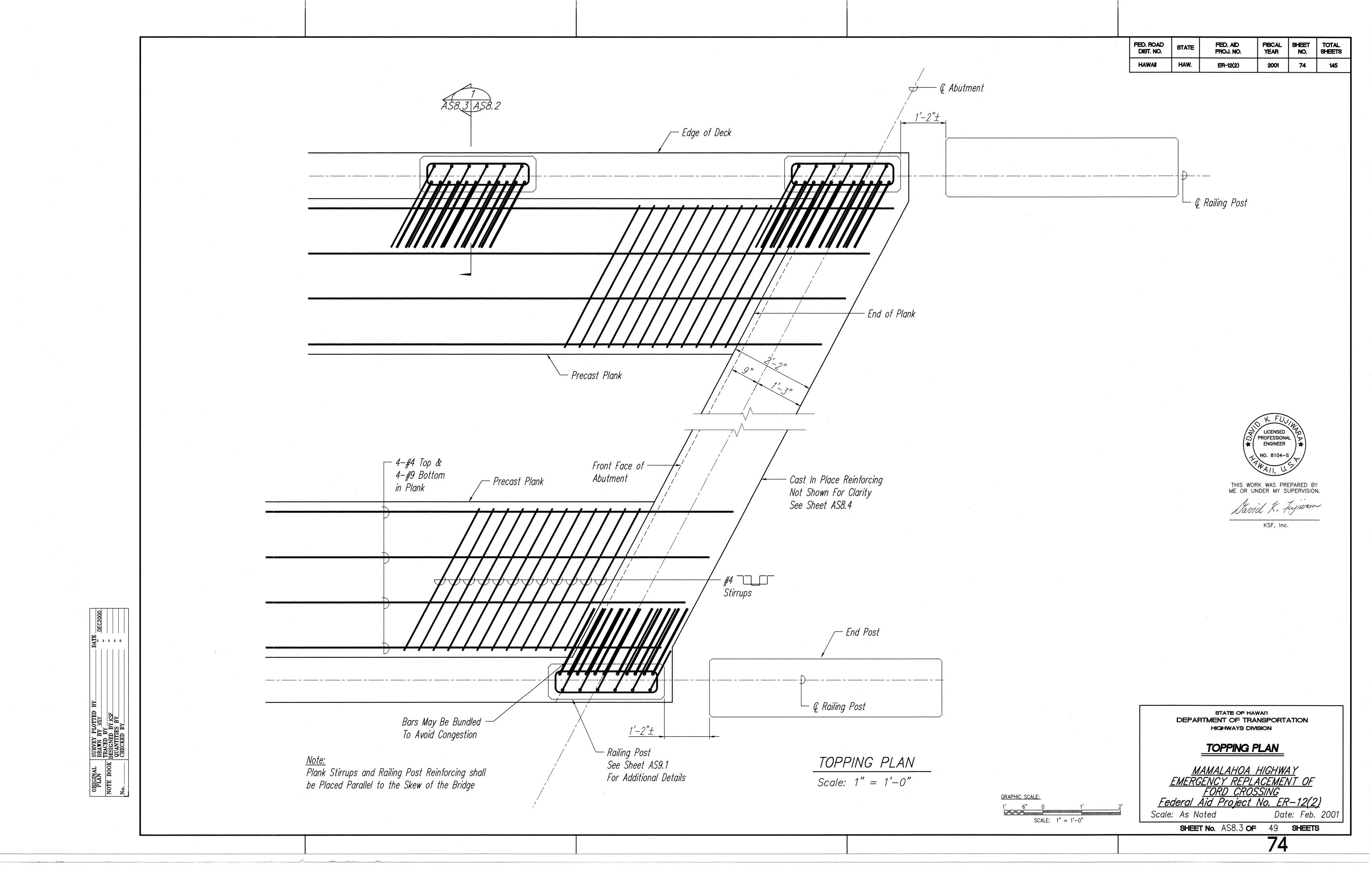
Federal Aid Project No. ER-12(2)

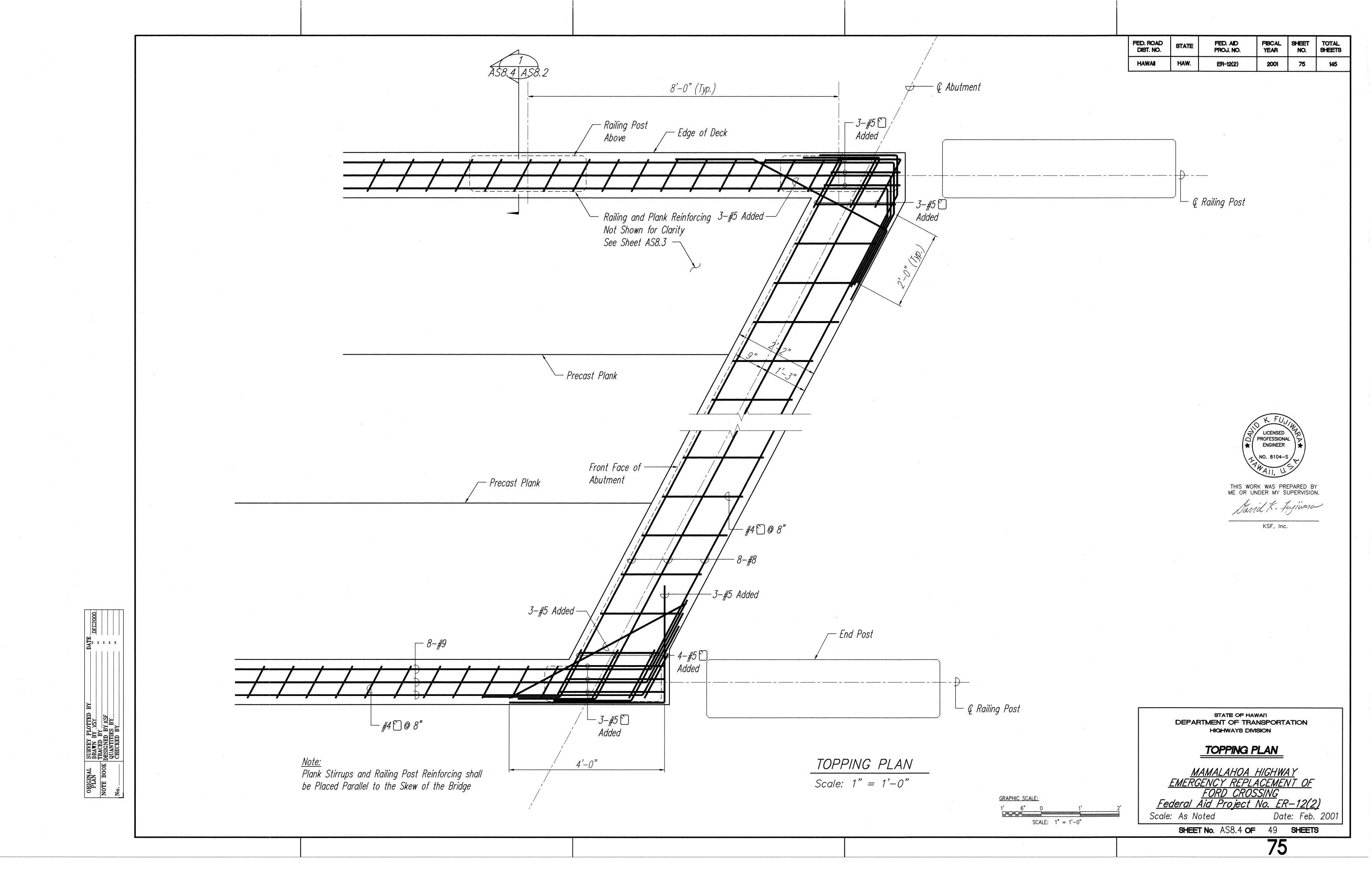
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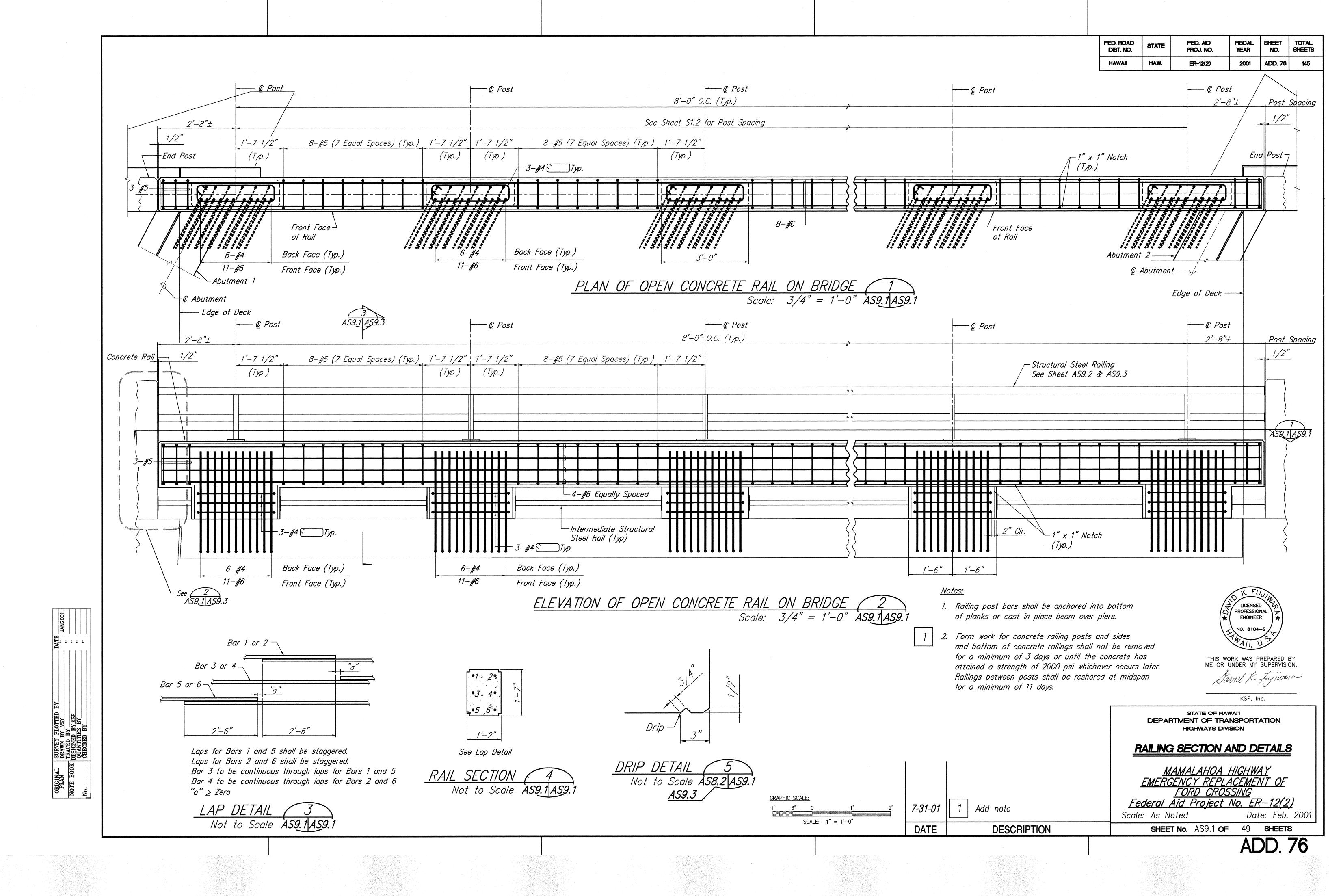
Date: Feb. 2001 SHEET No. AS8.1 OF 49 SHEETS

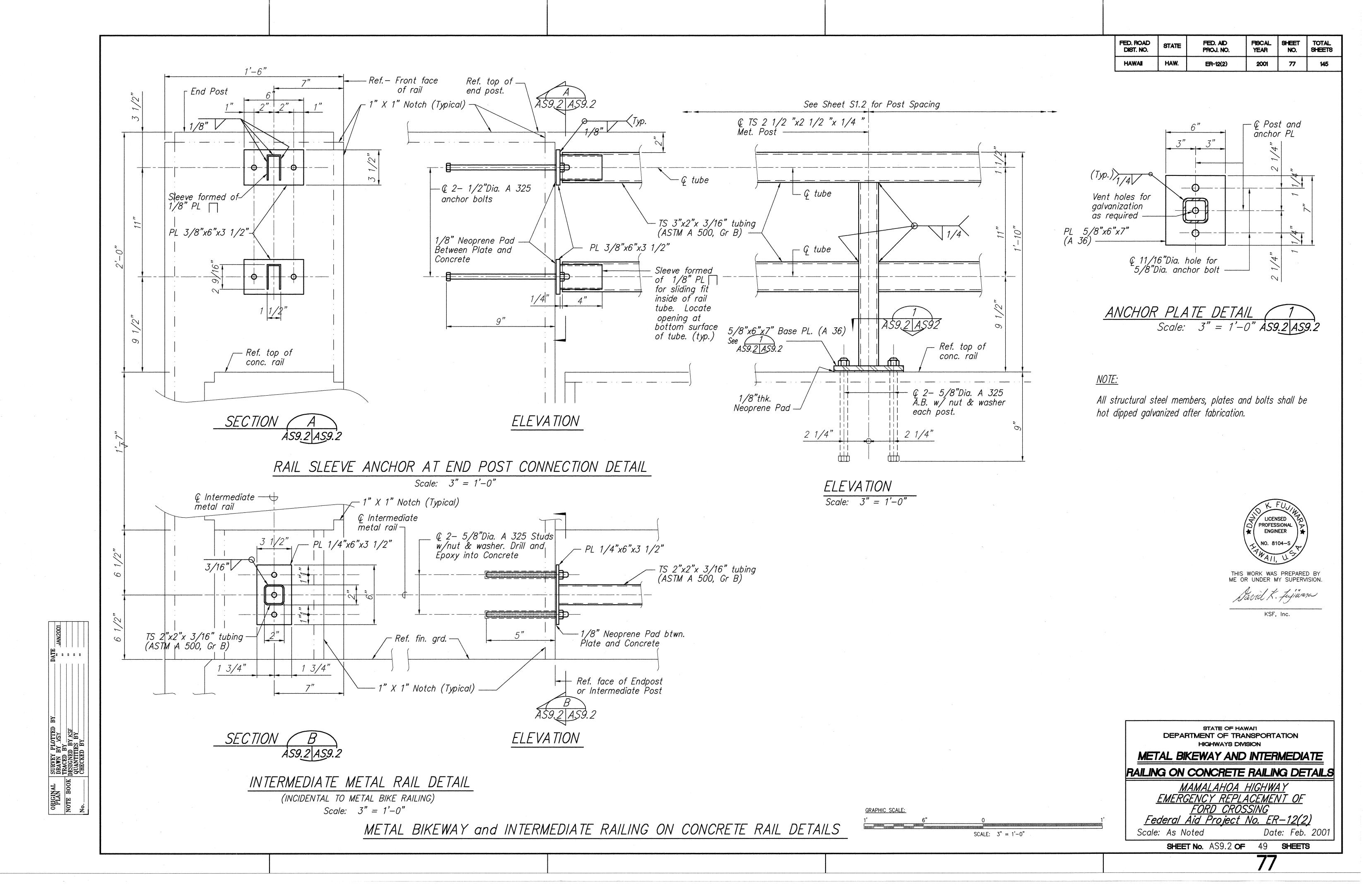


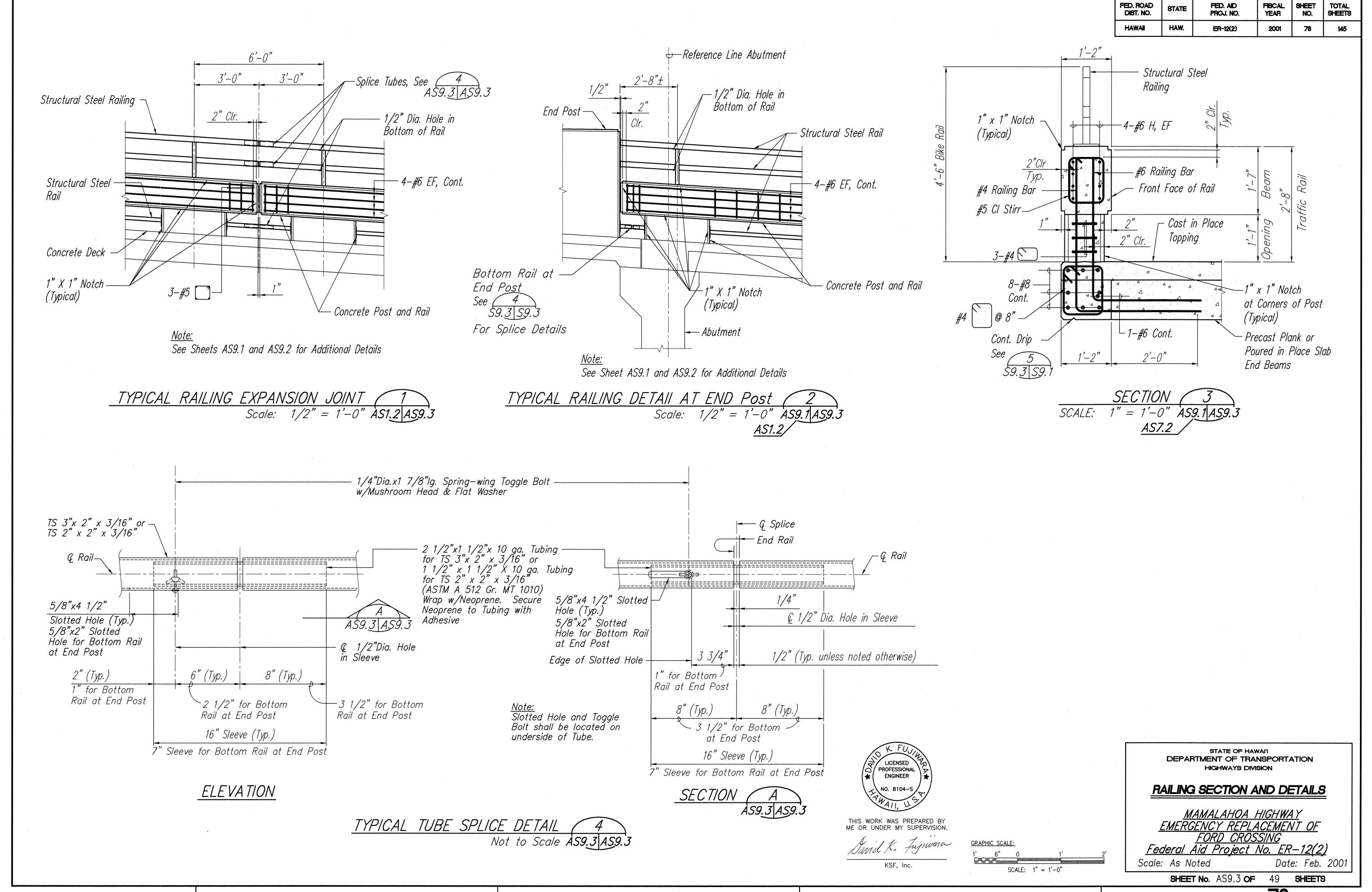
ADD. 73











ORIGINAL BRAWN BY VSY TRACED BY DESIGNED BY BESIGNED BY CHECKED BY CHECKED BY CHECKED BY

78

FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	ER-12(2)	2001	79	145

CONSTRUCTION SEQUENCE

STAGE 1: Construct the following in accordance with the Bridge and Roadway Best Management Practices (BMP's):

> (1) Abutments No. 1 and No. 2 Footings, Piers No. 2 and No. 3 Footings, Wingwalls No. 3 and No. 4 Footings

Backfill with Class "D" concrete between Pier 3 and Abutment 2

(3) Backfill with Class "D" concrete and place GRP in Channel at Wingwalls 3 and 4

(4) Pier No. 1, Wingwalls No. 1 and No. 2 Footings

Abutment No. 1 Wall Stem to Invert El.+0.5 ft. or greater

Backfill with Class "D" concrete between Abutment 1 and Pier 1

Backfill with Class "D" concrete and place GRP at Wingwalls 1 and 2

(8) Abutment Walls, Pier Walls and Wingwalls

<u>NOTES</u>

(A) Probing and Grouting are included in Stage 1.

(B) Abutment Walls, Pier Walls and Wingwalls may be poured at any time after their footings are poured providing they meet the requirements of the BMP's and specifications.

(C) Backfilling behind the Abutment Walls and Wingwalls shall not occur until the Class "D" Concrete in front of the Abutment Walls and Wingwalls shown on Sections 1/AS1.2 and A/AS2.1 has been poured and has attained a strength of 1000 psi or 7 days after the Class "D" concrete has been poured, whichever occurs later.

Note: Stages 2 through 4 will not require containment of work unless the Stream is flowing. Spilled Concrete on the Stream/Channel shall be removed in accordance with the Bridge and Roadway Best Management Practices (BMP's).

STAGE 2: Erect Precast Planks at least Seven (7) days after Pier and Abutment pour or until Pier and Abutment concrete has attained a compressive strength of 3,500 psi, whichever occurs later.

STAGE 3: Pour Deck Topping

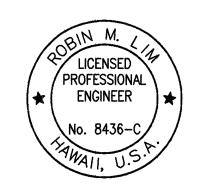
STAGE 4: Construct Railings

STAGE 5: Bridge may be opened for Traffic after the concrete in the Topping and Railing have attained a compressive strength of 5,000 psi and 4,000 psi., respectively, or at least Seven (7) days after the latest pour, whichever occurs later.

STAGE 6: Remove Detour Road and construct embankments in accordance with the Detour Road and Embankments BMP's.

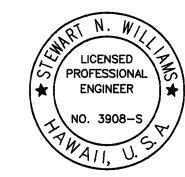
<u>NOTE:</u>

No more than one vehicle at one time shall be located on the fill behind the abutment until Seven (7) days after entire deck topping is poured or the topping has attained its Twenty-eight (28) day compressive strength whichever occurs later. This area shall be across the whole width of the abutment extending Twenty (20) feet back from the Abutment Wall. This applies to all vehicles over Ten (10) tons.



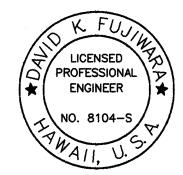
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.





THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION.

Mitsunaga & Associates, Inc.



Savid K. Fujiwana

STATE OF HAWAI'I DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION

CONSTRUCTION SEQUENCE

<u>MAMALAHOA HIGHWAY</u> <u>EMERGENCY REPLACEMENT OF</u> <u>FORD CROSSING</u> <u>Federal Aid Project No. ER-12(2)</u>

Scale: As Noted

Date: Feb. 2001

SHEET No. AS10.1 OF 49 SHEETS

